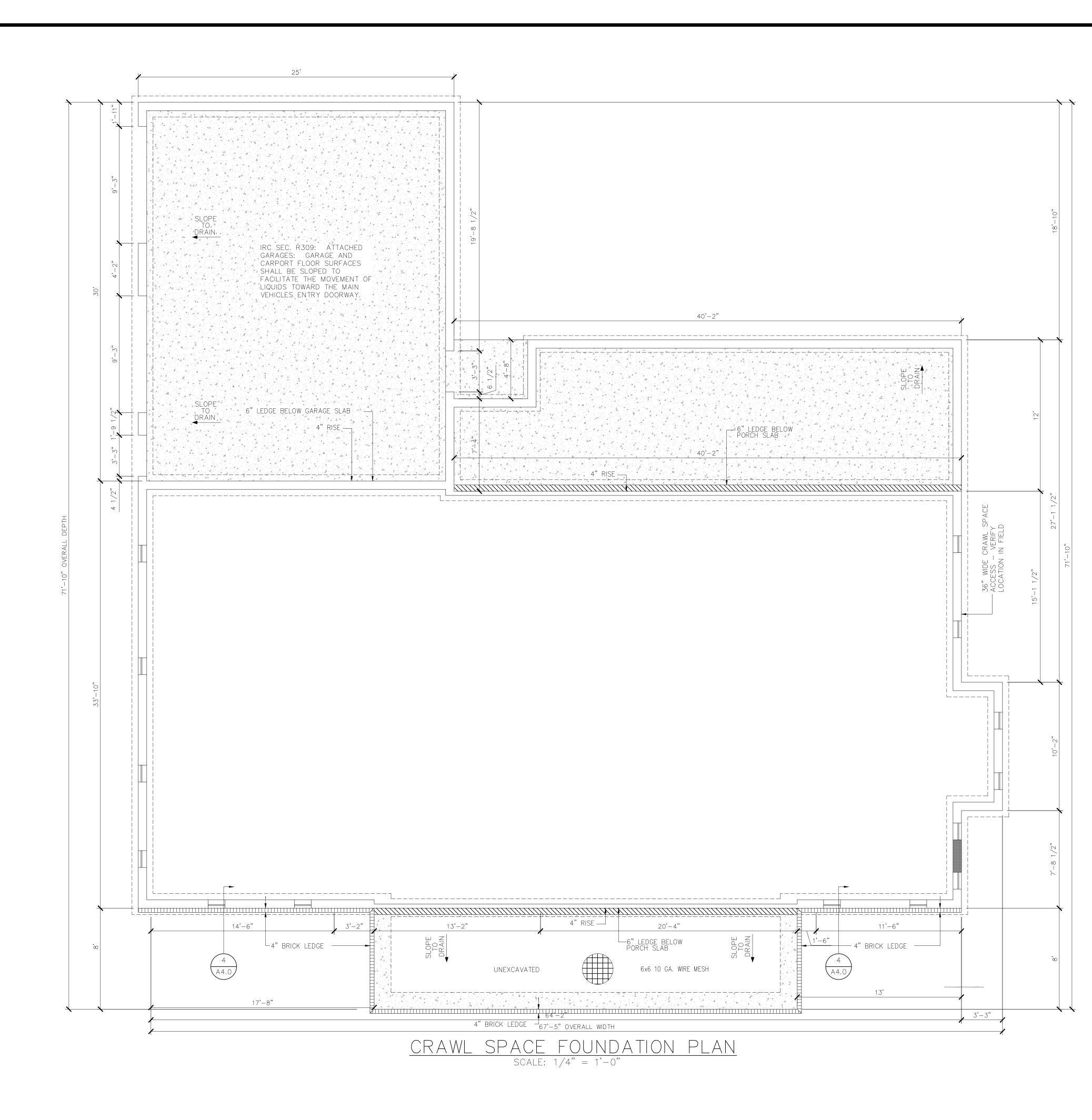


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5' KNEE WALL	<section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header>
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	Project No.: SPRINGBLUFF-MIRROR DATE: JUNE 10, 2022 DRAWN BY: Steven Madden DESIGNED BY: Steven Madden COPYRIGHT NOTE: O These Plans Are Subject To Federal Copyright Laws And Are To Be Used For The Lot Number And Subdivision Indicated In This Title Block Only. Use On Any Other Site is Prohibited. OCOPYRIGHT 2022
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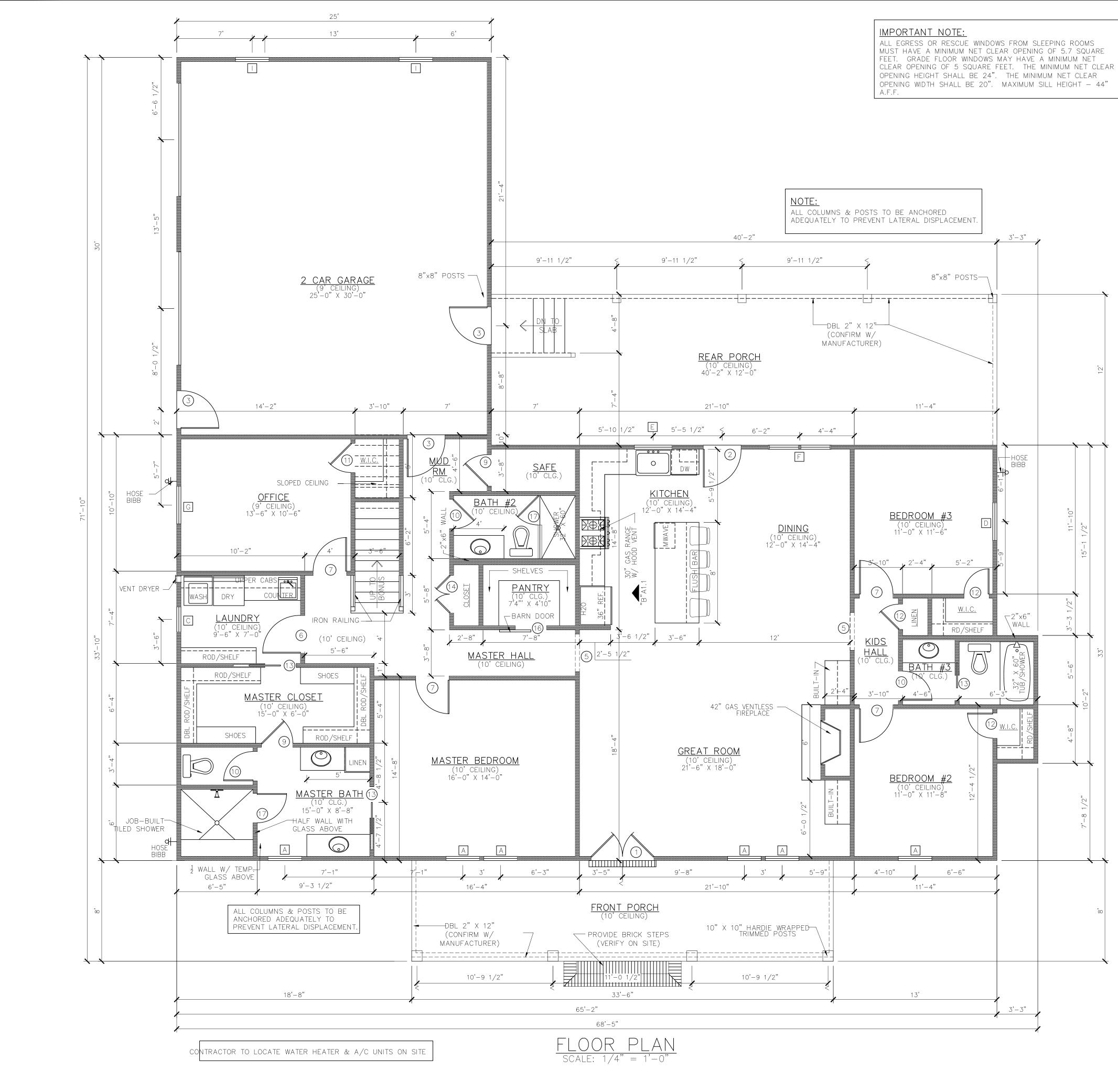
FOUNDATION GENERAL NOTES

THIS GENERIC FOUNDATION PLAN IS DESIGNED FOR NON EXPANSIVE SOILS WITH A BEARING CAPACITY OF AT LEAST 2500 PSF AND AN EFFECTIVE FRICTION ANGLE OF NO LESS THAN 30°. THIS PLAN IS NOT CERTIFIED FOR A SPECIFIC LOCATION, RECOMMENDED SITE GEOTECHNICAL INVESTIGATION AND COORDINATION OF THE FOUNDATION PLAN WITH SITE CONDITIONS BY A LOCAL ENGINEERING FIRM.

- CONCRETE SHOULD HAVE MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS CONCRETE DESIGN MIX SHOULD BE IN ACCORDANCE WITH ACI-318 (LATEST VERSION).
- 2. `ALL CONVENTIONAL REINFORCING STEEL SHALL MEET ASTM-A615 (GRADE 60). REINFORCING STEEL SHALL BE DETAILED AND ACCESSORIES PROVIDED IN ACCORDANCE WITH THE LATEST "ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES"
- 3. REINFORCEMENT SHALL HAVE 3" COVER IN THE GRADE BEAM BOTTOMS, 3" COVER IN THE BEAM SIDES AND TOP. 1-1/2" COVER IN THE SLAB TOPS AND THE BOTTOMS, UNLESS NOTED OTHERWISE OF BARRIER.
- CONCRETE SHALL BE WELL CONSOLIDATED. 6. THE CONTRACTOR SHALL VERIFY ALL DROPS, OFF-SET, BRICK LEDGES, AND BLOCK OUTS AN ARCHITECTURAL PLANS AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES THAT MAY EXIST.
- 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF THE STRUCTURAL DRAWINGS WITH ALL OTHER DRAWINGS. 8. ALTERATION TO OR DEVIATION FROM THE INFORMATION
- SHOWN ON THIS SHEET WITHOUT THE WRITTEN ADVANCED APPROVAL FROM THE ENGINEER WILL VOID DESIGNERS RESPONSIBILITY. 9. THIS PLAN IS FOR GRADE BEAM LOCATION AND REBAR
- LAYOUT ONLY. 10. ALL SUBGRADE FILL SHALL BE SELECT GRANULAR MATERIAL COMPACTED TO 95%%% MODIFIED PROCTOR
- DENSITY IN A MAXIMUM OF 6" LIFTS. 11. A MINIMUM OF 4" OF CONCRETE SHALL BE MAINTAINED
- THROUGHOUT THE ENTIRE SLAB. 12. ALL RUNOFF WATER SHALL BE CARRIED AWAY FROM THE
- SLAB TO PREVENT SATURATION OF THE SUBBASE. 13. ALL TREES WITHIN CLOSE PROXIMITY SHALL BE MOVED TO PREVENT THE ROOTS FROM EXTENDING UNDER THE SLAB.
- 14. REMOVE A MINIMUM OF 6" OF EXISTING SOIL PRIOR TO
- PLACING ANY FILL.
- 15. A MAXIMUM OF 2.0 FEET TO FILL MAY BE PLACED ON THE SITE.
- 16. FOLLOW REQUIREMENTS OF LOCAL JURISDICTIONS FOR REQUIRED DEPTH TO FROST LINE. CONTACT ENGINEER SHOULD REQUIREMENTS EXCEED THE LIMITS OF THIS DESIGN
- 17. NO FIELD SUPERVISION PROVIDED UNDER THIS SEAL UNLESS OTHERWISE NOTED. *ASSUMED 0.5 SF OF NET FREE AREA PER VENT -FIELD VERIFY

**MINIMUM ONE VENT WITHIN 3'-0" OF EACH CORNER AND ONE VENT EACH SIDE OF STRUCTURE

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Project No.: SPRINGBLUFF-MIRROF DATE: JUNE 10, 2022 DRAWN BY: Steven Madden
DESIGNED BY: Steven Madden
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Sheet: 3 OF 5 □ Preliminary Dwg.
□ Bidding Doc.



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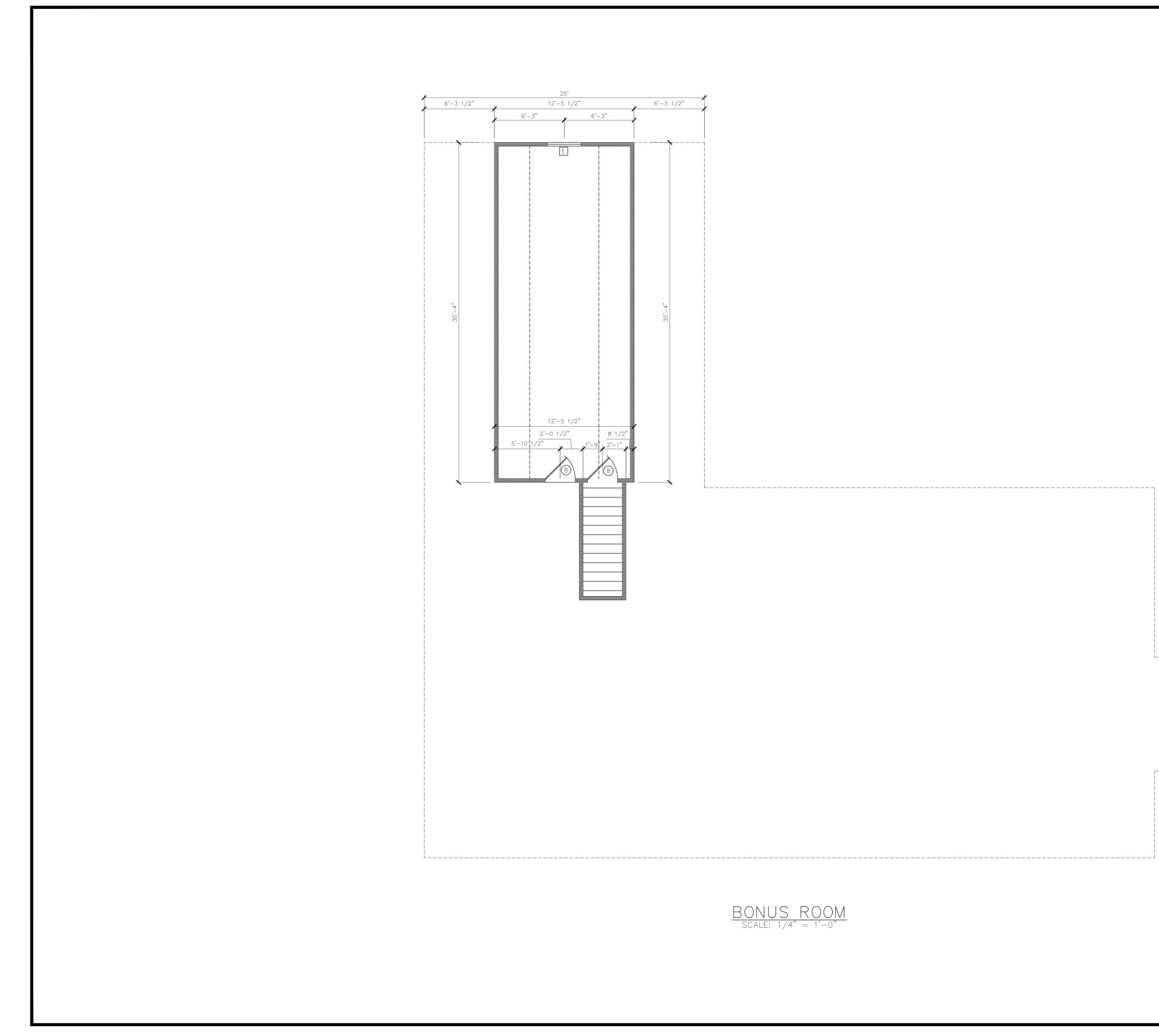
	DOOR SCHEDULE	
SIZE	DESCRIPTION	QTY.
DBL 2'6" X 8'0"	EXTERIOR 4 LITE 3/4 FRENCH SOLID WOOD DOORS	1 PAIR
3'0" X 8'0"	EXTERIOR 4 LITE FULL FRENCH WOOD DOOR	1
3'0" X 7'0"	EXTERIOR 4 LITE 1/2 FRENCH METAL DOOR	3
2'8" X 8'0"	EXTERIOR 6 PANEL METAL DOOR	1
3'0" X 8'0"	CASED OPENING	2
3'0" X 8'0"	INTERIOR HORIZONTAL 6 PANEL H.C. MASONITE DOOR	1
2'8" X 8'0"	INTERIOR HORIZONTAL 6 PANEL H.C. MASONITE DOOR	4
2'8" X 6'8"	INTERIOR HORIZONTAL 6 PANEL H.C. MASONITE DOOR	2
2'6" X 8'0"	INTERIOR HORIZONTAL 6 PANEL H.C. MASONITE DOOR	2
2'4" X 8'0"	INTERIOR HORIZONTAL 6 PANEL H.C. MASONITE DOOR	3
2'4" X 6'8"	INTERIOR HORIZONTAL 6 PANEL H.C. MASONITE DOOR	1
2'0" X 8'0"	INTERIOR HORIZONTAL 6 PANEL H.C. MASONITE DOOR	4
2'6" X 8'0"	INTERIOR HORIZONTAL 6 PANEL H.C. MASONITE POCKET DOOR	3
DBL 1'6" X 6'8"	INTERIOR HORIZONTAL PANEL H.C. MASONITE DOORS	1 PAIR
2'8" X 6'8"	6 PANEL SOLID CORE MASONITE ATTIC ACCESS DOOR	1
2'4" X 8'0"	SLIDING BARN DOOR - OWNER SELECT	1
2'4" X 6'0"	TEMPERED FRAMELESS GLASS SHOWER DOOR	2

WINDOW SCHEDULE

OPENING SIZE	DESCRIPTION	QTY.
2'8" X 6'0"	2/2 LITE VINYL SINGLE HUNG WINDOW INSULATED	6
4'0" X 4'0"	DBL 4 LITE VINYL CASEMENT WINDOW INSULATED	1
2'0" X 3'0"	2/2 LITE VINYL SINGLE HUNG WINDOW INSULATED	1
3'0" X 6'0"	2/2 LITE VINYL SINGLE HUNG WINDOW INSULATED	0
3'0" X 5'0"	4 LITE VINYL FIXED WINDOW INSULATED	2
DBL 2'6" X 7'0"	2/2 lite vinyl single hung windows insulated	1 PAIR
3'0" X 5'0"	2/2 LITE VINYL SINGLE HUNG WINDOWS INSULATED	2
2'6" X 4'0"	2/2 lite vinyl fixed window insulated (see elevs)	2
2'8" X 5'0"	2/2 LITE VINYL SINGLE HUNG WINDOWS INSULATED	2

SQUARE FOOTAGE
1ST FLOOR LIVING2147
BONUS ROOM 339
FRONT PORCH268
REAR PORCH506
GARAGE750
TOTAL LIVING2486
TOTAL SQ. FT3836

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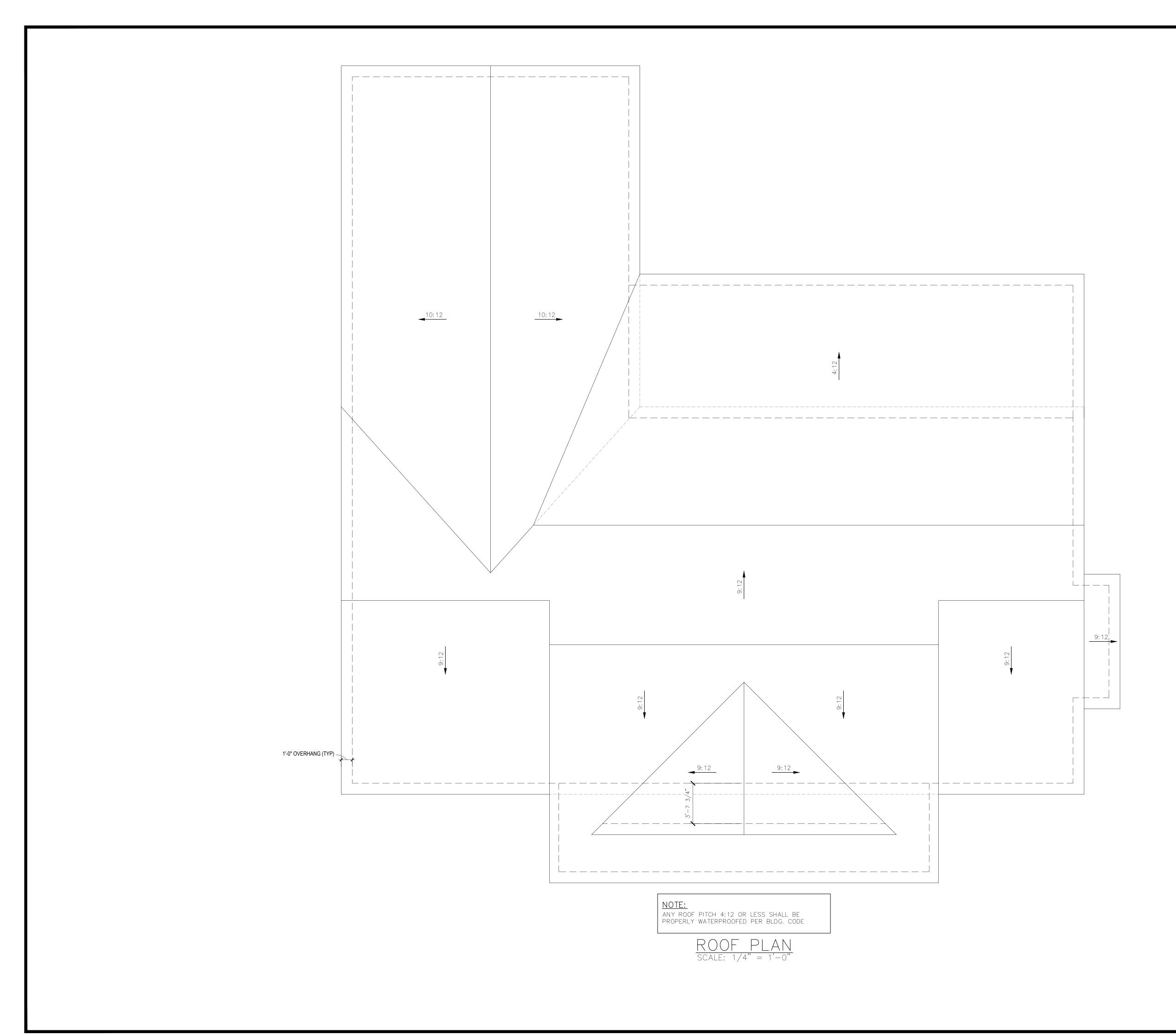




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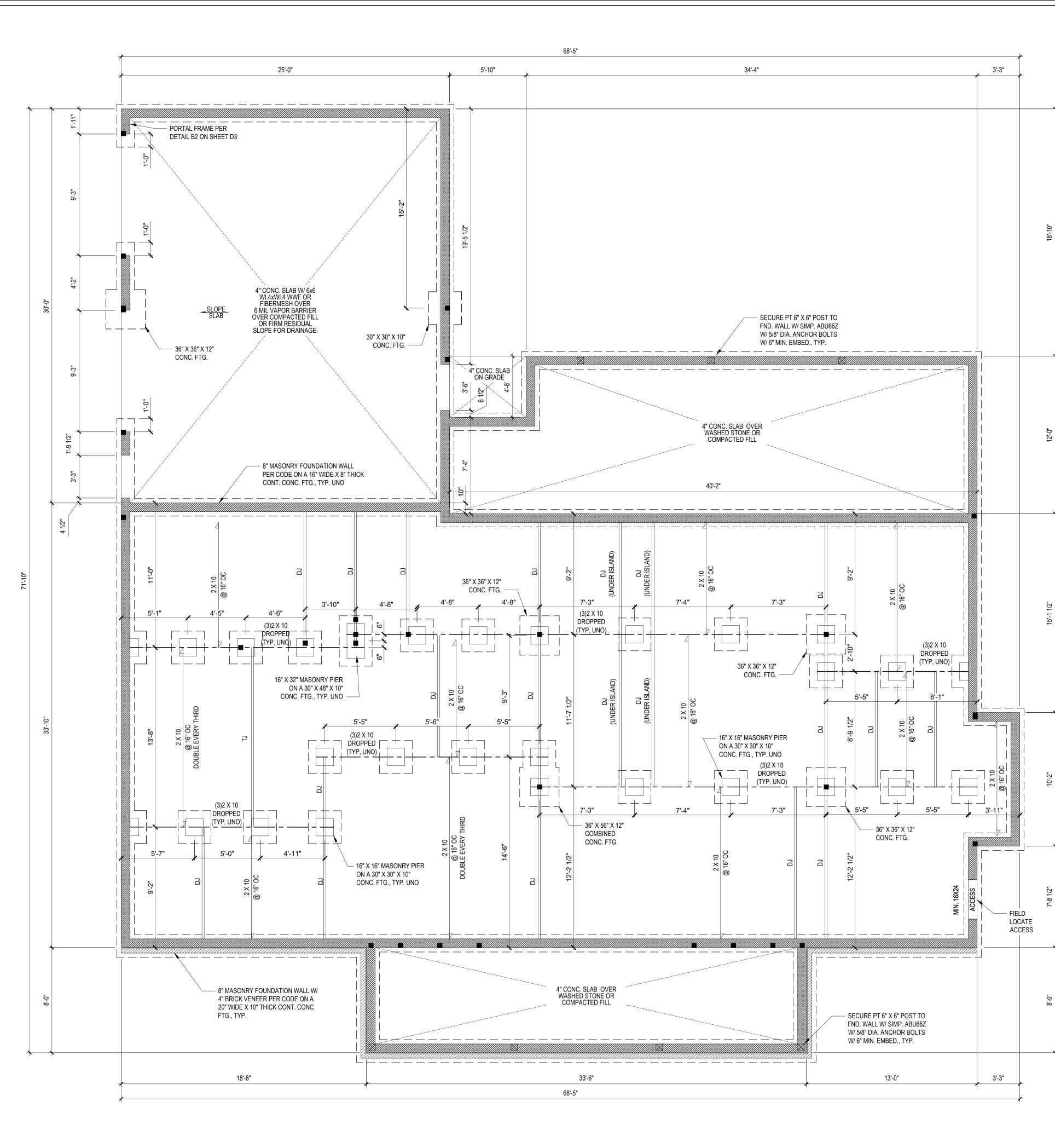
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 Sheet: 6 OF 6 Preliminary Dwg. Bidding Doc. Construction Doc.

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FOUNDATION PLAN

1/4" = 1'-0"

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
	(* • • •)	(* 5.)	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

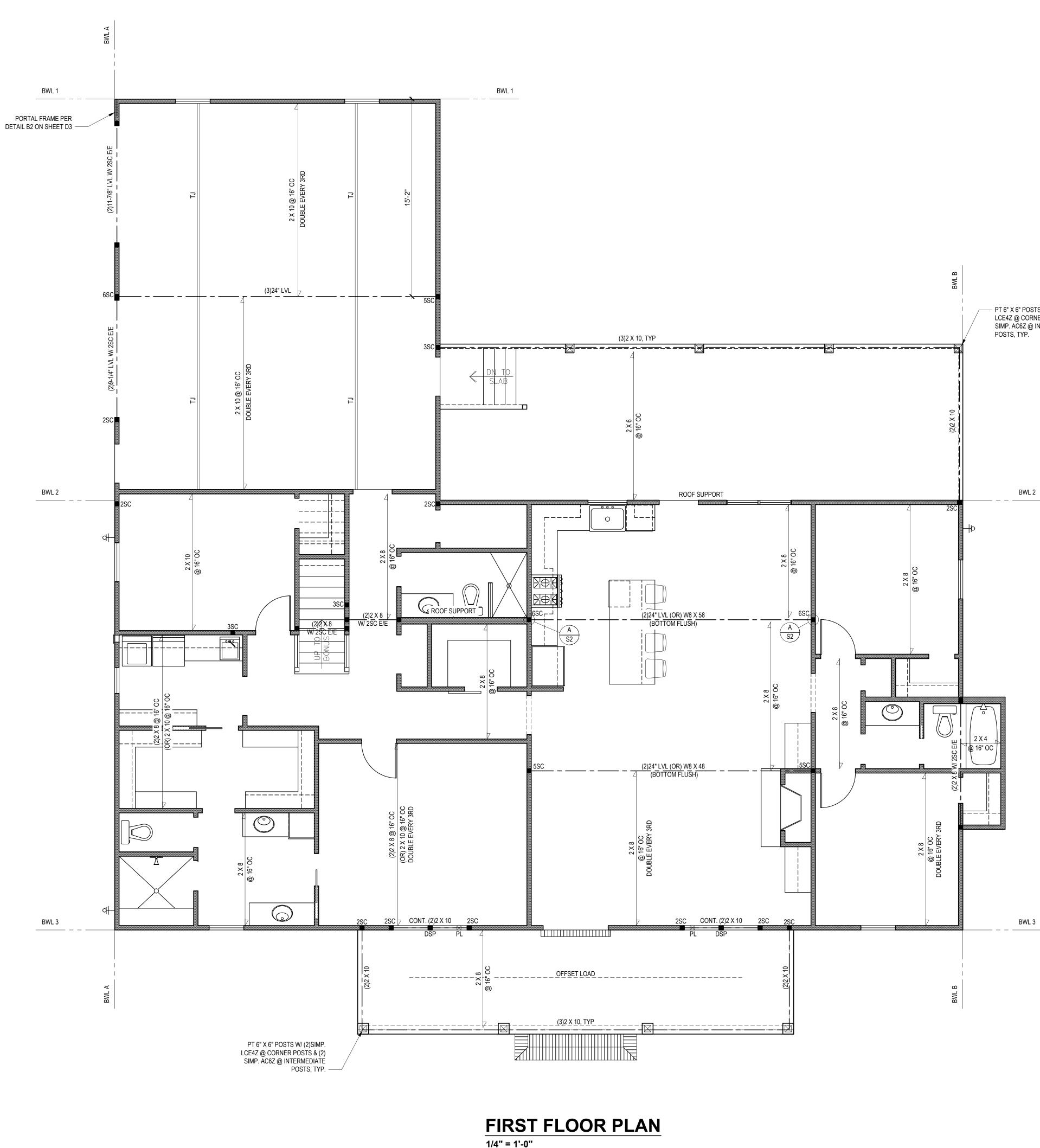
STRUCTURAL NOTES:

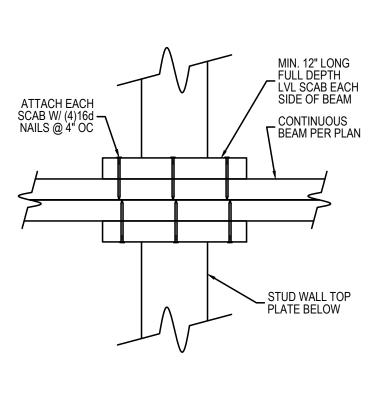
- 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS
- AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
 ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- 7) ALL STRUCTURAL STEEL SHALL BE A Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 9) ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
 PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION. 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

2080 SQ. FT. OF CRAWL SPACE / 150 = 13.87 SQ. FT. OF REQ'D VENTILATION WITHOUT CROSS VENTILATION 13.87 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 16 VENTS REQ'D (BASED ON 8" X 16" VENTS)1 -OR-2080 SQ. FT. OF CRAWL SPACE / 1500 = 1.39 SQ. FT. OF REQ'D VENTILATION WITH CROSS VENTILATION 1.39 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 2 VENTS REQ'D (BASED ON 8" X 16" VENTS)2 1) VENT LOCATIONS MAY VARY FROM THOSE SHOWN ON PLAN, HOWEVER VENTS SHALL BE PLACED TO PROVIDE ADEQUATE VENTILATION AT ALL POINTS AND TO PREVENT DEAD AIR POCKETS. THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 1/1500 OF THE CRAWL SPACE GROUND AREA WHERE THE REQUIRED OPENINGS ARE PLACED SO AS TO PROVIDE CROSS VENTILATION OF THE CRAWL SPACE. THE INSTALLATION OF OPERABLE LOUVERS SHALL NOT BE PROHIBITED. ONE FOUNDATION VENT SHALL BE WITHIN 3 FEET OF EACH CORNER OF THE BUILDING. TO PREVENT RAINWATER ENTRY WHEN THE CRAWL SPACE IS BUILT ON A SLOPED SITE, THE UPHILL FOUNDATION WALLS MAY BE CONSTRUCTED WITHOUT WALL VENT OPENINGS, VENT DAMS SHALL BE PROVIDED WHEN THE BOTTOM OF THE FOUNDATION VENT OPENING IS LESS THAN 4 INCHES ABOVE THE FINISHED EXTERIOR GRADE. WALL VENTED CRAWL SPACES REQUIRE FULL COVERAGE GROUND VAPOR RETARDERS. CRAWL SPACE VENTILATION CALCULATION

NO SCALE

do so will void Tyndal P.A. liability. *Please review these do Tyndall Engineering & interpret that all dime recommendations, etc. presented in these	hiques, sequences, orceaution. repancies on plans are nmediate attention of & Design, P.A. Failure to I Engineering & Design, cuments carefully. & Design, P.A. will nsions, documents were ce construction begins.
TYNDALL ENGINEERING & DESIGN, P.A.	+ 919 773-1200 = # 919 773-9688 250 Shipwesh Orive = Garner = North Caroline = 27529 www.tyndellengineering.com
client: STEVE NORDAN	Plan: MCLAMB RESIDENCE
FOUNDATION PLAN	1ST FLOOR FRAMING
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A BEAM BEARING ENHANCEMENT SCALE: 1-1/2" = 1'-0"

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
	(-)		LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS
- AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS. 3) ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
- 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT. ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT 8)
- ALL CONCRETE, fc = 3000 PSI MIN.
- 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.) 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

BRACING PANEL LENGTHS REQUIRED: BWL A = 10.0 FT BWL B = 10.0 FT BWL 1 = 4.9 FT BWL 2 = 10.2 FT BWL 3 = 5.3 FT
BRACING PANEL LENGTHS PROVIDED: BWL A = 37.83 FT CS-WSP BWL B = 30.0 FT CS-WSP BWL 1 = 19.67 FT CS-WSP BWL 2 = 39.83 FT CS-WSP / GB BWL 3 = 43.5 FT CS-WSP

STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- LESS. 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.

 $\langle 1 \rangle$ REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.

- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- $\overline{(3)}$ 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON [/] NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS. 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
- BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN
 - 67% OF WALL HEIGHT
 - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT.
 - 48" FOR OPENINGS GREATER THAN 85% OF
 - WALL HEIGHT
- $\langle 4 \rangle$ SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.

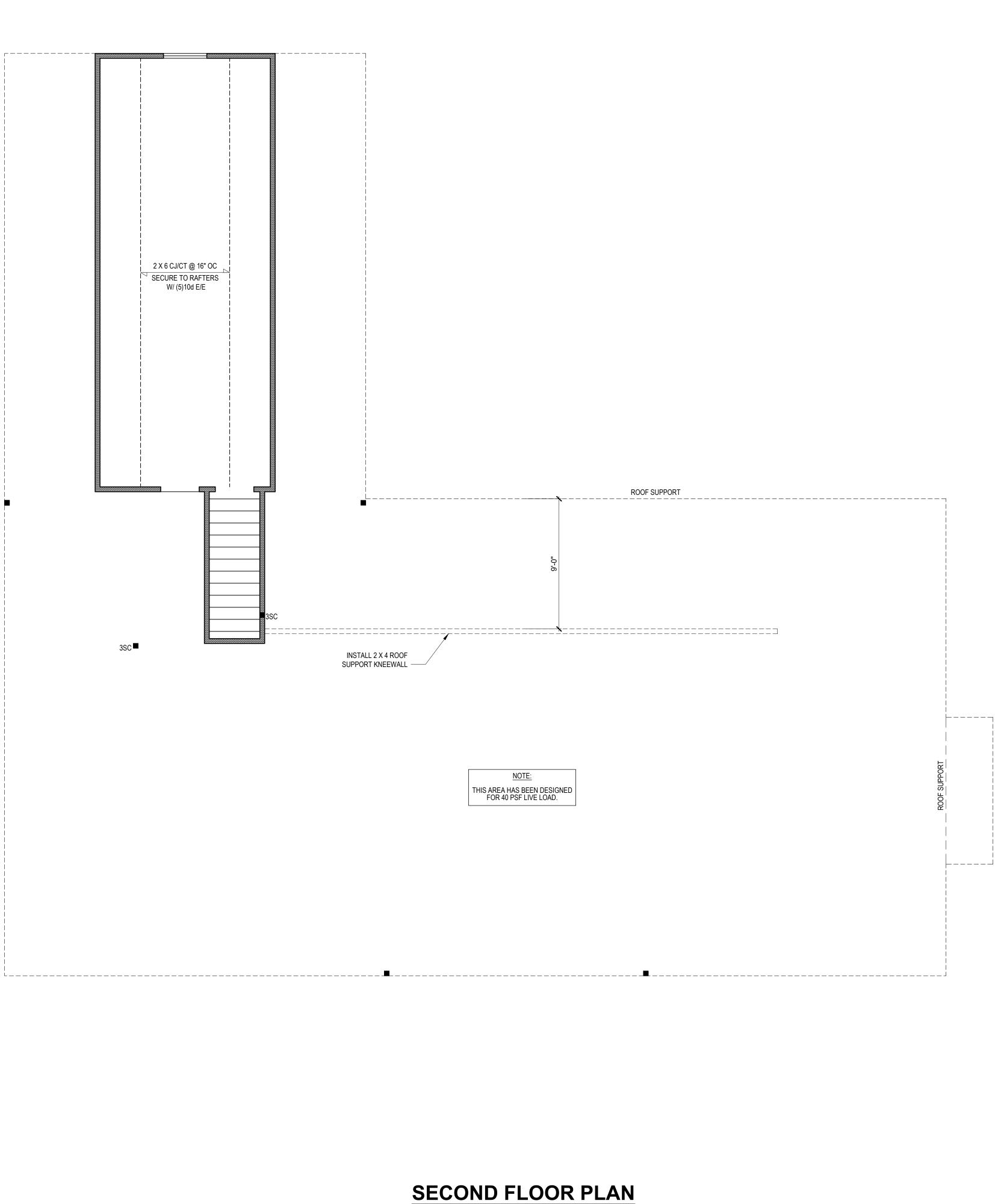
 $\langle 5 \rangle$ MINIMUM 800# HOLD-DOWN DEVICE

zineers seal does not include constru *Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.
*Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. P.A. liability. Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were eemed acceptable once construction ٦Ż NDAL 5 H STEVE HEADER FRAMING -00R -00R ᆔᆔ 1ST 2ND Project #: DRB2201-0186 Date: 10/17/22 Engineered By: AM DWG. Checked By: AWL Scale: SEE PLAN REVISIONS Date: Remarks Sheet Number **S**2 2 of 7

— PT 6" X 6" POSTS W/ (2)SIMP. LCE4Z @ CORNER POSTS & (2) SIMP. AC6Z @ INTERMEDIATE

BWL 2

BWL 3



1/4" = 1'-0"

DESIGN LOADS

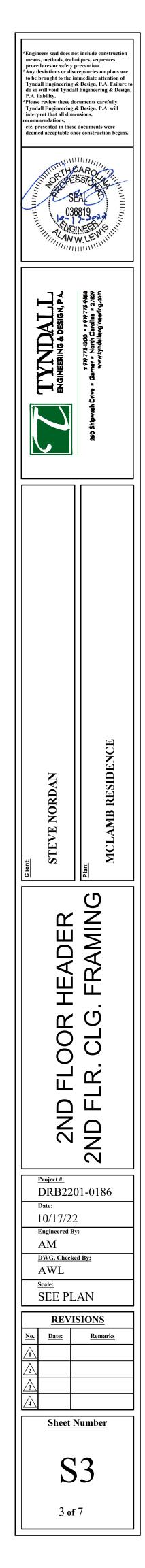
	LIVE LOAD DEAD LOAD (PSF) (PSF)		DEFLECTION		
		(- /	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

STRUCTURAL NOTES:

1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS
- AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
 ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI (I.E. iLEVEL MICROLAM)

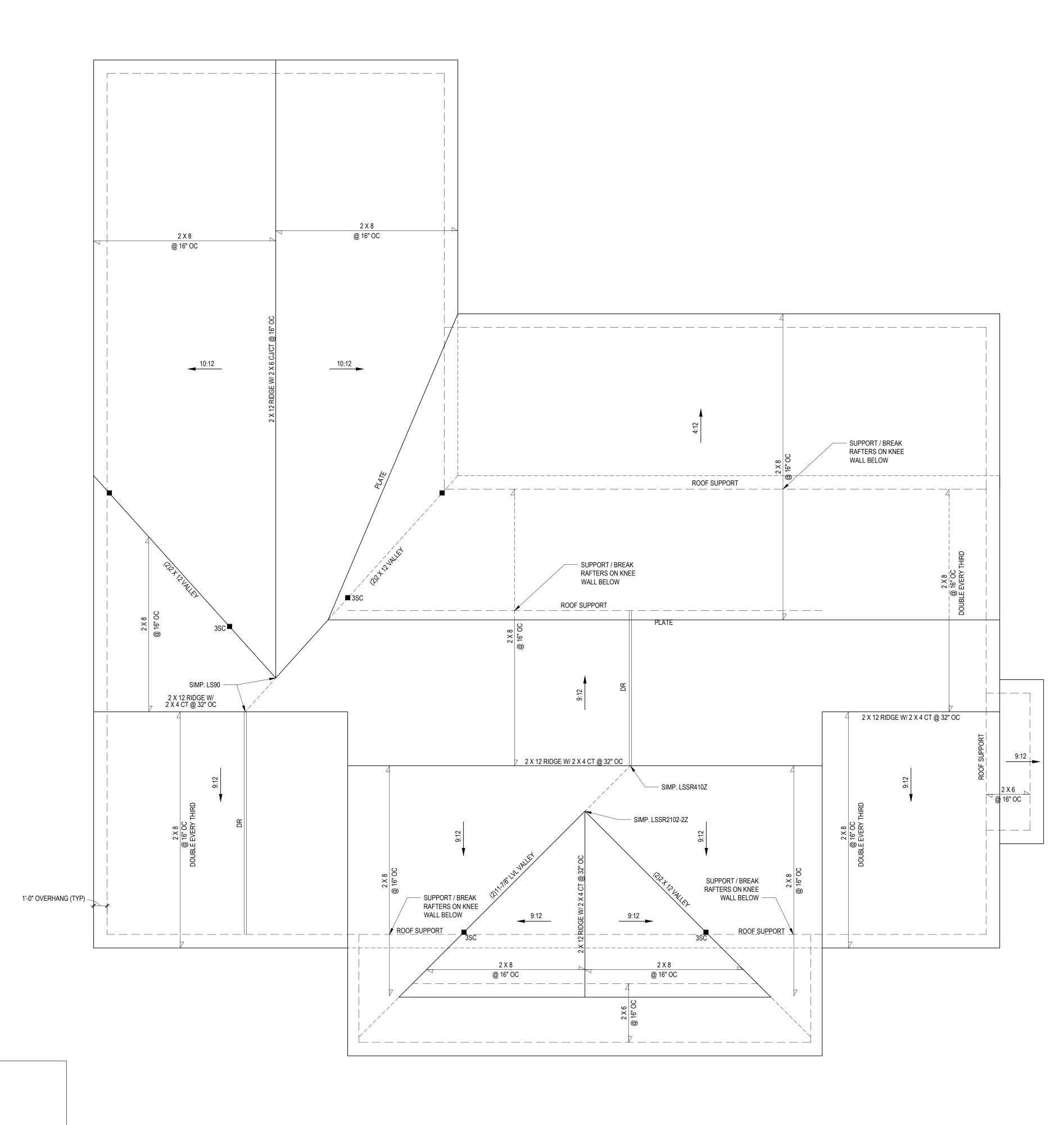
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
- 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 9) ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
- MASONRY. 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- IRC. 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
- LEAST HORIZONTAL DIMENSION.UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



* ATTIC VENTILATION CALCULATION NO SCALE

2) CATHEDRAL CEILINGS SHALL HAVE A 1" MINIMUM CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.

- CALCULATION BASED ON VENTILATORS USED AT LEAST 3'-0" ABOVE THE COMICE VENTS WITH THE BALANCE OF VENTILATION PROVIDED BY EAVE VENTS.
- 3023 SQ. FT. OF ATTIC / 300 = 10.08 SQ. FT. INLETS/OUTLETS REQUIRED





*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution. *Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. *Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.					
Image: Shipwash Drive - Gamer - North Carolina - 2784 250 Shipwash Drive - Gamer - North Carolina - 27829					
client: STEVE NORDAN	Plan: MCLAMB RESIDENCE				
ROOF PLAN	PL/				
Date: 10/17/22 Engineered By AM DWG. Checke AWL Scale: SEE PL REVI No. Date: 1 2 3 4 Sheet N	DRB2201-0186 Date: 10/17/22 Engineered By: AM DWG. Checked By: AWL Scale: SEE PLAN REVISIONS No. Date: Remarks A A Date: Remarks A A Sheet Number Sheet Number				

				STRU	JCTURAL NOTES	8				
1)	ALL CONSTRUCTION	N SHALL CONFORM TO	O THE LATEST REQUI	REMENTS OF "I	NORTH CAROLINA ST	ATE 2018 RESIDEN	ITIAL BUILDING			
	CODE", IN ADDITION	TO ALL LOCAL CODE	ES AND REGULATIONS	S.						
2)	DESIGN LOADS:									
					LOAD DEAD	-	DEFLEC	TION		
				(P	SF) (PS	SF)	LL	TL		
		ALL	FLOORS	2	10 1	0	L/360	L/240		
		· · · · ·	// walk up stairs)			0	L/360	L/240	_	
			ull down access) C (no access)			5	L/240	L/180 L/180	_	
			VAL BALCONY		10 1		L/240	L/240		
			ROOF		20 1		L/240	L/180		
		ROC	OF TRUSS	2		0	L/240	L/180	_	
		11W	ND LOAD		BASE	ON 120 MPH (EXF	POSURE B)			
		S	EISMIC		S	EISMIC ZONES A,	B & C			
3)	MINIMUM ALLOWAB	LE SOIL BEARING PRE	ESSURE = 2000 PSF							
4)			AY COMPRESSIVE ST	RENGTH OF 30	00 PSI AND A MAXIMU	IM SLUMP OF FIVE	INCHES			
	UNLESS NOTED OTH	IERWISE. (U.N.O.)								
5)			AGAINST FOUNDATIO 018 NC BUILDING COI				-			
			ED BACKFILL HEIGHT.				,, , , , ,			
6)			(Fb = 800 PSI, BASED							
			E ELEMENTS SHALL BI NAL EACH SINGLE ME			SL(LLNIO)				
	ALL LSL LUMBER TO	BE 3.5" WIDE NOMIN	IAL EACH SINGLE ME	MBER AND Fb =	2325 PSI, E = 1.6M PS	I (Ù.N.O.)				
7)			SHALL BE AT (2) 2x10							
,			R INTERIOR AND EXT							
8)	ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.									
9)	STEEL BEAMS SHAL	I BE SUPPORTED AT	EACH END WITH A M	INIMUM BEARIN	IG I FNGTH OF 3-1/2" /	AND FULL FLANGE	WIDTH.			
0)	PROVIDE SOLID BEA	RING FROM BEAM SU	UPPORT TO FOUNDAT	FION. BEAMS SH	HALL BE ATTACHED T	DEACH SUPPORT	WITH TWO (2)			
			. SUPPORT IS CONSIE RE NAILED OR BOLTE				ILED TO THE			
					C					
10)	PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.									
11)	FOUNDATION DRAIN	IAGE-DAMP PROOFIN	IG OR WATERPROOFI	NG PER SECTIO	ON 405 AND 406 OF NO	BUILDING CODE.				
12)	ROOF VALUES BOTH	ALL BE DESIGNED FC	DR 28.0 POUNDS PER ATIVE SHALL BE AS F TO 1.5/12		(LBS/SQFT) OR GREA	TER POSITIVE AN	D NEGATIVE PR	ESSURE.		
		ROOF PITCHES 1.5/12 ROOF PITCHES 6/12 T HT 30'-0" OR LESS								
13)	FOR ROOF SLOPES	FROM 2/12 THROUGH	1 4/12, BUILDER TO IN	STALL 2 LAYER	S OF 15# FELT PAPER					
14)	REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.									
15)	PROVIDE CONTINUC	OUS SHEATHING PER	SECTION 602.10.3 OF	THE 2018 NCR	С.					
, 16)	PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.									
,	UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.									
17)	REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.									
18)	PSL COLUMNS DESI	GNED WITH MAXIMUN	M HEIGHT OF 9'-0" (U.1	N.O.)						
19)	PROVIDE A MINIMUM	/I OF 500# UPLIFT & LA	ATERAL CONNECTION	NAT TOP AND B	OTTOM OF PORCH C	DLUMNS. (U.N.O.)				
	MAXIMUM MASONR'	(PEIR HEIGHT SHALL	NOT EXCEED FOUR	TIMES ITS I FAS	ST HORIZONTAL DIME	NSION.				
20)										
20)			Y TO VERIFY ALL DIM NOT RESPONSIBLE F					N BEGINS.		
20) 21)										
,										
,										
,					W/OOD	MVCC			CIVD d	
21)		DŅ . SKYLIGHT ^b	GLAZED FENESTRATION	CEILING m	WOOD FRAMED WALL	MASS WALL	FLOOR	BASEMENT ^{C,O} WALL	SLAB ^d R-VALUE	CRAWL SPACE ^C WALL
21)	TYNDALL ENGINEEF	DN SKYLIGHT ^b U-FACTOR	-	CEILING ^m R-VALUE	FRAMED WALL R-VALUE	WALL R-VALUE	FLOOR R-VALUE	WALL R-VALUE		WALL R-VALUE
21) CLIMAT		DN SKYLIGHT ^b U-FACTOR 0.55	FENESTRATION	R-VALUE 38 or 30	FRAMED WALL R-VALUE	WALL R-VALUE		WALL	R-VALUE	WALL
21) CLIMAT ZONE	TYNDALL ENGINEEF	^{b,j} U-FACTOR 0.55	FENESTRATION SHGC ^{b,<u>k</u> 0.30}	R-VALUE 38 or 30 cont 38 or 30	FRAMED WALL R-VALUE <u>15</u> or 13 + <u>2.5</u> ^h 15 or	WALL R-VALUE 5/13 or 5/10 cont 5/13 or	R-VALUE 19	WALL R-VALUE <u>5/13</u> ^f	R-VALUE AND DEPTH 0	WALL R-VALUE 5/13
21) CLIMAT ZONE	TYNDALL ENGINEEF	^{b,j} U-FACTOR	FENESTRATION SHGC ^{b,<u>k</u>}	R-VALUE <u>38 or 30</u> <u>cont</u>	FRAMED WALL R-VALUE <u>15</u> or 13 + <u>2.5</u> ^h 15 or	WALL R-VALUE 5/13 or 5/10 cont	R-VALUE	WALL R-VALUE	R-VALUE AND DEPTH	WALL R-VALUE

* TABLE N1102.1 CLIMATE ZONES 3-5 NO SCALE a. R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE. b. THE FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIENT (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION. c. "10/15" MEANS R-10 CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME OR R-15 CAVITY INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR CRAWL SPACE WALL. d. FOR MONOLITHIC SLABS, INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNWARD TO THE BOTTOM OF THE FOOTING OR A MAXIMUM OF 24" BELOW GRADE WHICHEVER IS LESS. FOR FLOATING SLABS, INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 24", WHICHEVER IS LESS. R-5 SHALL BE ADDED TO THE REQUIRED SLAB EDGE R-VALUES FOR HEATED SLABS. e. DELETED f. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY FIGURE N1101.7 AND TABLE N1101.7. g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM.

h. THE FIRST VALUE IS CAVITY INSULATION, THE SECOND VALUE IS CONTINUOUS INSULATION, SO "13+5" MEANS R-13 CAVITY INSULATION PLUS R-5 INSULATED SHEATHING. "15+3" MEANS R-15 CAVITY INSULATION. PLUS R-3 INSULATED SHEATHING. IF STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR, INSULATING SHEATHING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25 PERCENT OF THE EXTERIOR, SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2. "13 + 2.5" MEANS R-13 CAVITY INSULATION PLUS R-2.5 SHEATHING. i. FOR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WALL. j. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A U-FACTOR NO GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY. k. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER THAN 0.70 SHALL BE_____

PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.

 I.
 R-30 SHALL BE DEEMED TO SATISFY THE CEILING INSULATION REQUIREMENT WHEREVER THE FULL HEIGHT OF UNCOMPRESSED R-30 INSULATION EXTENDS OVER THE WALL TOP PLATE

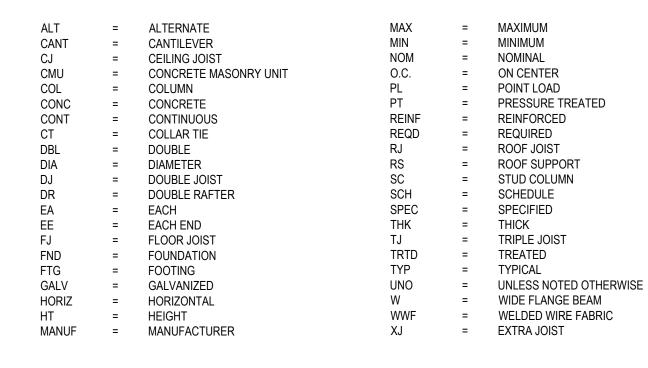
 AT THE EAVES. OTHERWISE R-38 INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR WITHIN 1 INCH OF THE ATTIC ROOF DECK.
 m. TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF; THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE.

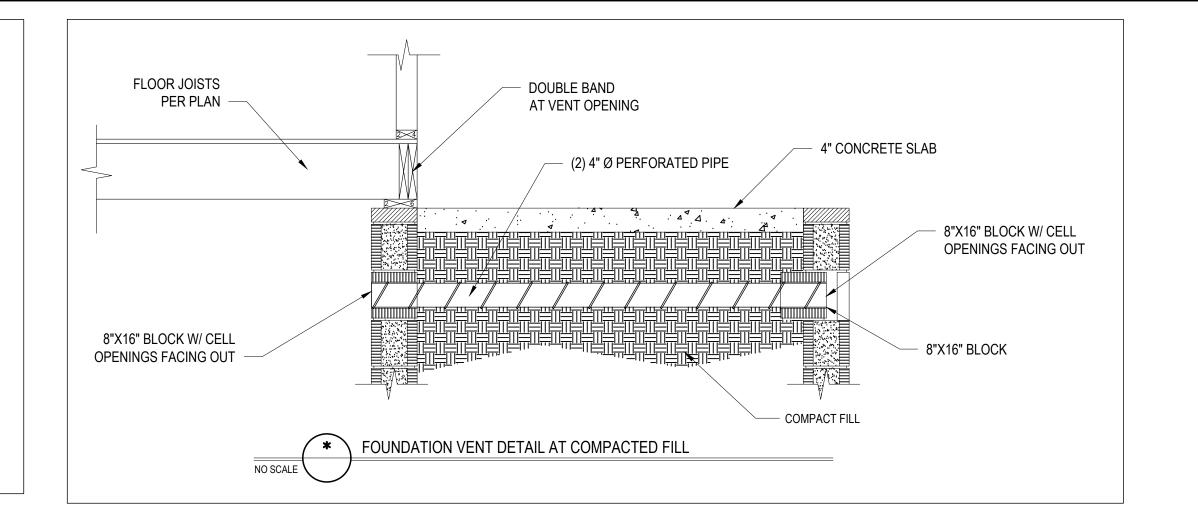
n. R -19 FIBERGLASS BATTS COMPRESSED AND INSTALLED IN A NOMINAL 2 × 6 FRAMING CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED AND INSTALLED IN A 2X4 WALL IS NOT DEEMED TO COMPLY.

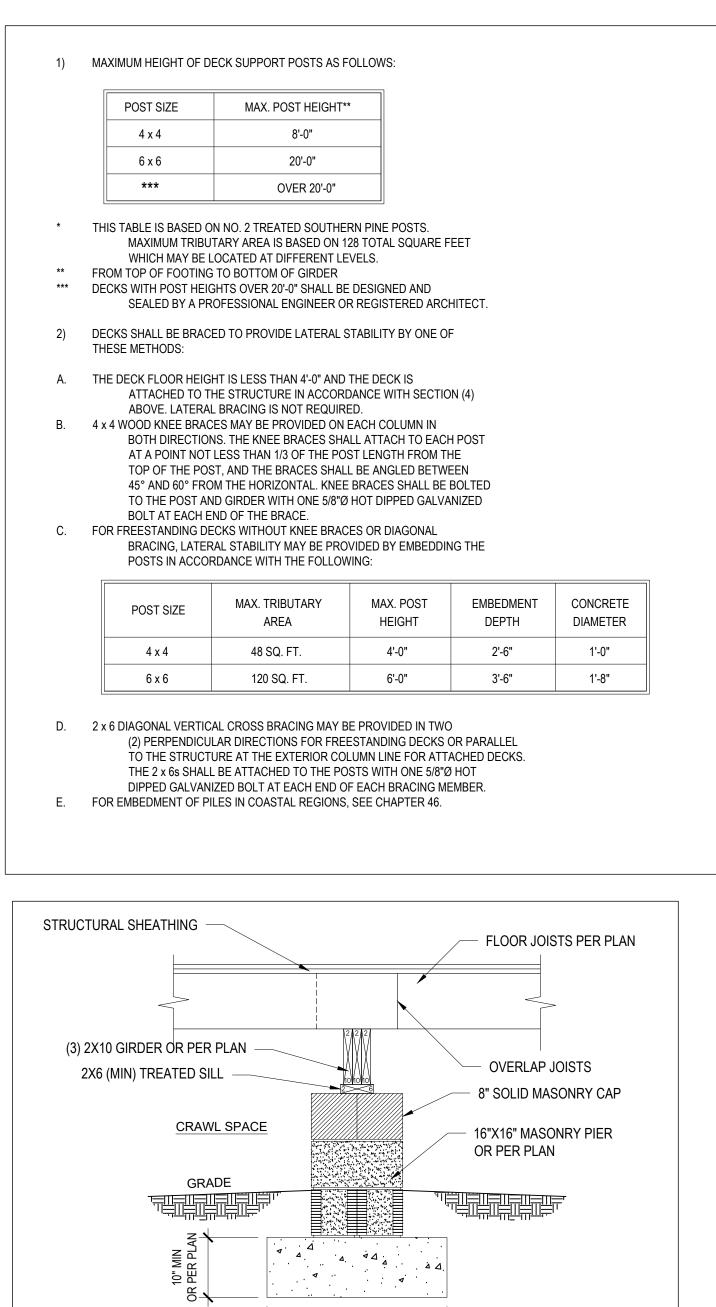
0. BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT.

ä







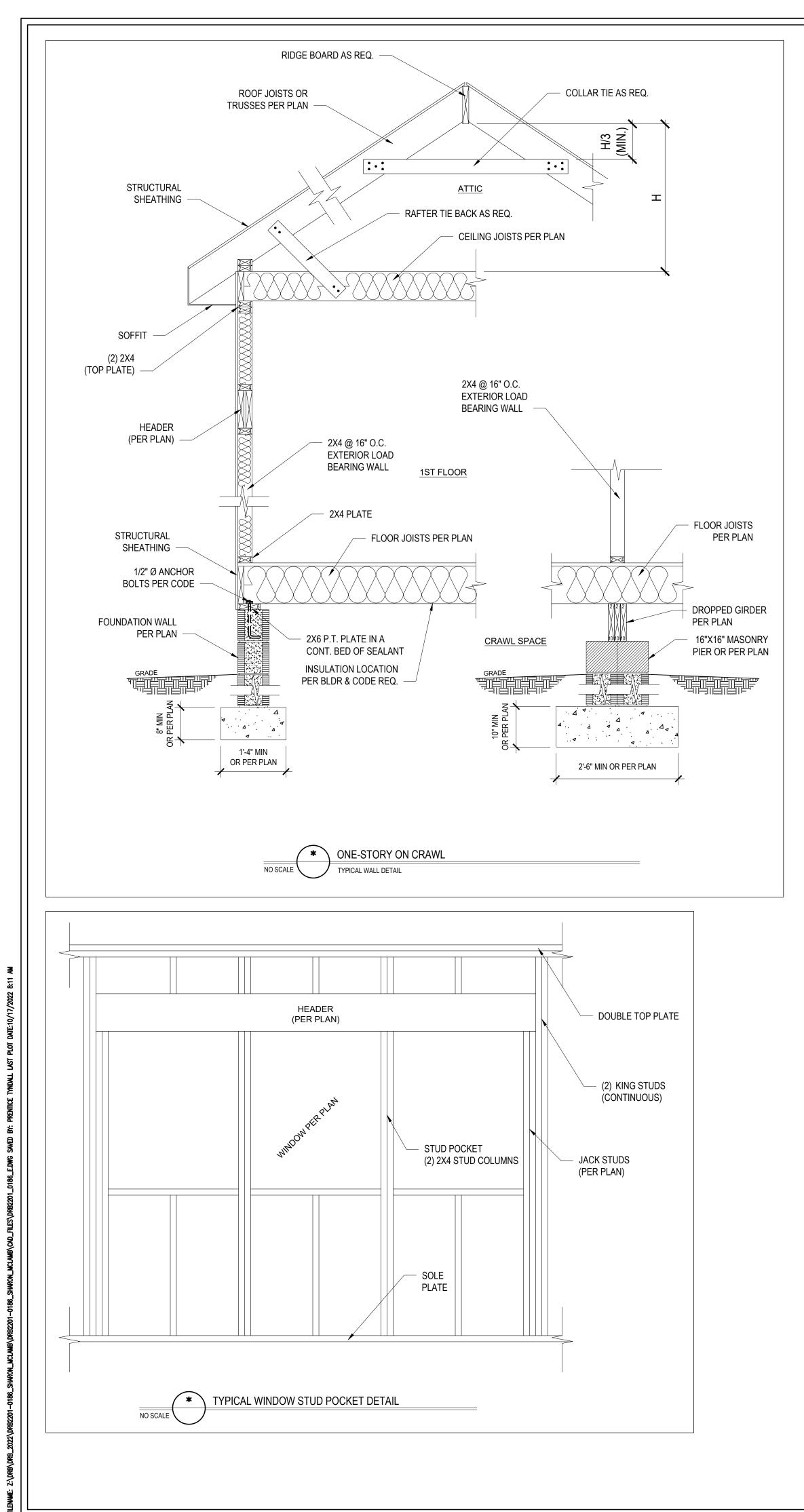


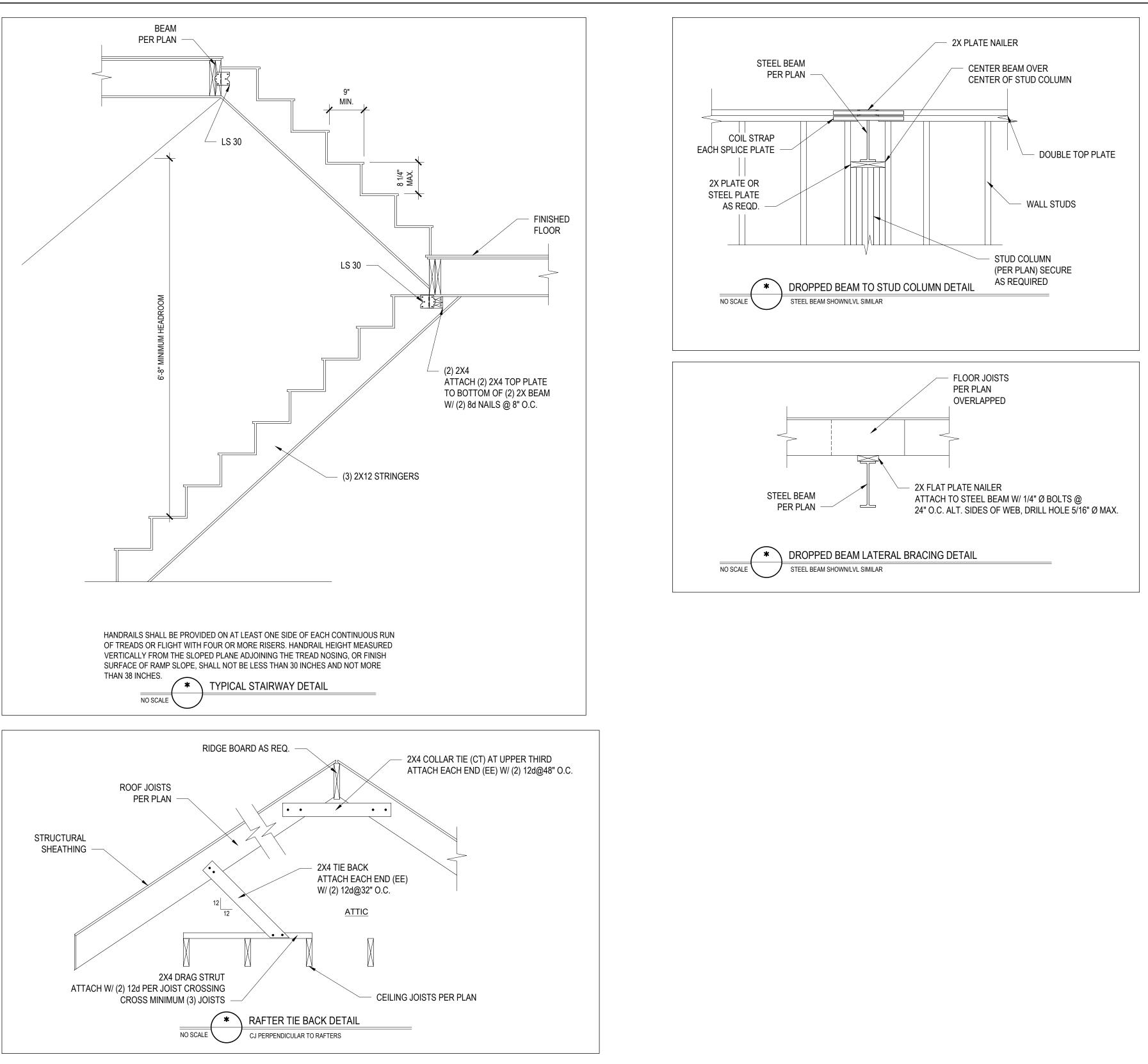
2'-6" MIN OR PER PLAN

DROPPED GIRDER DETAIL

NO SCALE

means, methods, tech procedures or safety p *Any deviations or disc to be brought to the in Tyndall Engineering 4 do so will void Tyndal P.A. liability. *Please review these do Tyndall Engineering 4 interpret that all dime recommendations, etc. presented in these deemed acceptable on	*Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions,				
TYNDALL ENGINEERING & DESIGN, P.A.	z50 Shipwash Drive = Garner = North Carolina = 27529 www.tyndallangineering.com				
Client: STEVE NORDAN	Plan: MCLAMB RESIDENCE				
STANDARD	STANDARD DETAILS				
Project #: DRB2201-0186 Date: 10/17/22 Engineered By: AM DWG. Checked By: AWL Scale: SEE PLAN <u>REVISIONS</u> No. Date: Remarks AWL Sheet Number D1					





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TYNDALL ENGINEERING & DESIGN, P.A.	250 Shipwash Drive = Garner = North Carolina = 27529 Www.tyndallangineering.com				
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D2 6 of 7					

