PROFILE	PLY	ID	SPAN	SPAN	LEFT F	HGHT	SIZE LOCATION	REACTIO	NS			
	1	01-02-00 ET1	22-03-00	22-03-00		ュ		Joint 20 23.3 lbs.	Joint 21 127.3 lbs.	Joint 22 151.1 lbs.	Joint 23 145.5 (bs.	Joint 24 147.0 lbs.
CANOL PROMISE	3	01-02-00 F1	22-03-00	22-03-00				Joint 17 83.2 lbs. -411.2 lbs.	Joint 20 1783.7 lbs. 645.1 lbs.	Joint 28 896.6 lbs. 243.4 lbs.		
1 232121 244144	1	01-02-00 F1A	22-03-00	22-03-00			i 	Joint 19 -96.3 lbs. -541.3 lbs.	Joint 22 2193,1 lbs. 1182.0 lbs.	Joint 32 963.6 lbs. 322.1 lbs.		
	1	01-02-00 F2	13-04-08	13-04-08]		Joint 9 707.0 lbs. 372.7 lbs.	Joint 16 707.0 lbs. 375.7 lbs.	- 		i
	4	01-02-00 F3	18-04-08	18-04-08		<u></u>		Joint 14 990.6 lbs. 505.3 lbs.	Joint 22 996.9 lbs. 506.6 lbs.			
	1	01-02-00 F3A	18-04-08	18-04-08		<u></u>		Joint 16 1296.4 lbs. 859.6 lbs.	Joint 26 1114.0 lbs. 623.7 lbs.			<u> </u>

00-11-00

ITEM TYPE

Hangers, USP

ITEMS

13

QTY

03-06-08

LENGTH

03-06-08

SIZE

HUS 410

Joint 5

PART NUMBER

522.9 lbs.

452.8 lbs.

Joint 8

446.1 lbs.

374.9 lbs.

SIMPSON (HUS410)

NOTES

Reaction Summary of Order

ROOF & FLOOR
TRUSSES & BEAMS
Reilly Road Industrial Park P.O. Box 40408

Fayetteville, N.C. 28309 (910) 864-TRUS

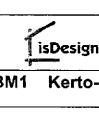
ORDER# J0223-0539 11 REQ. QUOTE DATE QUOTE # ORDER DATE 02/00/20 CUSTOMER ACCT# 000127 **DELIVERY DATE** II7.7 CUSTOMER PO# DATE OF INVOICE NONE INVOICE # ORDERED BY TO BE PRE-PAID **TERMS** COUNTY Harnett NONE SALES REP Dwayne Naylor SUPERINTENDANT Jonathan Landry **JOBSITE PHONE #** () -SALES AREA

	Cash	JOB NAME: Moore Res	idence	LOT# - SUBDIV	':-	
SOL	CASH OR CHECKS ONLY	MODEL:Floor	TAG: Custom	JOB CATEGORY: B &	S - Build and Ship	
D FO	CASH OR CHECKS ONLY NO CREDIT CARDS, () -	DELIVERY INSTRUCTION	5:			
HE	Charles Moore	SPECIAL INSTRUCTIONS:	:			
ا ہا	- Harnett Co., NC	Charles Moore (910)-333-7336 seymour1661@gmail.com		Р	LAN SEAL DATE: BY	N/A

BUILDING DEPARTMENT	OVERHA	NG INFO	HEEL HEIGHT	00-04-05	REQ. LAYOUTS	REQ. ENGINEERING	QUOTE	JL	02/06/23
Floor Order	END CUT	RETURN			NONE	NONE	LAYOUT	JL	02/06/23
1 1001 01001	PLUMB		GABLE STUDS	24 IN. OC	NONE	NONE	CUTTING	JL	02/06/23

ITEMS

11 141			···		<u> </u>
QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
2	LVL Beams (Sized)	LVL, 1-3/4" x 11-7/8" (S) ;	19-00-00		BM1
5	LVL Beams (Sized)	LVL, 1-3/4" x 11-7/8" (S)	13-00-00		BM2 & BM3
2	LVL Beams (Sized)	LVL, 1-3/4" x 18" (S)	19-00-00		BM4
3	LVL Beams (Sized)	LVL, 1-3/4" x 18" (S)	23-00-00		GDH
2	Hangers, USP	MSH422			SIMPSON (THA422)



Member Information

Client: Project:

Address:

Cash

Custom

Date: Input by: 2/6/2023

Jonathan Landry

Page 1 of 13

Job Name:

Moore Residence Project #: J0223-0539

Reactions UNPATTERNED Ib (Uplift)

1.750" X 11.875" 2-Ply - PASSED BM₁ Kerto-S LVL

.evel: Level

	 	<i>,,</i> ,,,,,,,,,,,,,,,,				
		1				
	 • • •	40 × 10 × 10 × 10 × 10 × 10 × 10 × 10 ×	The state of the s	The state of the s		W 11.7
SPF End Grain					2 SPF End Grain	

Туре:	Girder		Application:	Floor	Brg
Plies:	2		Design Method:	ASD	1
Moisture Con	dition: Dry		Building Code:	IBC/IRC 2015	2
Deflection LL	480		Load Sharing:	No	
Deflection TL	: 360		Deck:	Not Checked	
Importance:	Normal	- 11	Ceiling:	Gypsum 1/2"	l
Temperature:	Temp <	= 100°F			
					Beari
					Bear
					1-8
					End
Analysis Re	esults				Grai
Analysis	Actual	Location	Allowed Capa	city Comb.	Case 2 - S End

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	758	675	0	0
2	Vertical	0	758	675	0	0

Analysis Re	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6125 ft-lb	9'	22897 ft-lb	0.267 (27%)	D+S	L
Unbraced	6125 ft-lb	9'	6135 ft-lb	0.998 (100%)	D+S	L
Shear	1238 lb	1'3 3/8"	101 9 7 lb	0.121 (12%)	D+\$	L
LL Defl inch	0.172 (L/1227)	9' 1/16"	0.439 (L/480)	0.391 (39%)	S	L
TL Defl inch	0.364 (L/578)	9' 1/16"	0.585 (L/360)	0.623 (62%)	D+S	L

ings ring Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. SPF 3.500" Vert 14% 758 / 675 1433 L D+S 14% 758 / 675 1433 L D+S SPF 3.500" Vert Grain

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 16'2 7/8" o.c.
- 7 Lateral slenderness ratio based on single ply width.

, Lan	Ciai bichideiricas latio casca on ar	igic pry middi.			I						
ID	Load Type	Location	Trib Width	Şide	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	75 PLF	0 PLF	75 PLF	0 PLF	0 PLF	ZB1	
l					A 54 5						

9 PLF Self Weight

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design ortheria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitablely of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals Handling & Installation

- LVI beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply feetuning details, beam strength values, and code
- featuring details, beam strength values, and code approxisis.

 Darriaged Beams must not be used. Design assumes top adge is laterally restrained. Provide lateral support at bearing points to avoid lateral displacement and retation.

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer info

Comtech Reilly Road Industrial Park P.O. Box 40408, NC USA 28309 910-864-878?



Client: 2/6/2023 Page 2 of 13 Cash Date: Project: Input by: Jonathan Landry Custom isDesign Job Name: Moore Residence Address: Project #: J0223-0539 1.750" X 11.875" Level: Level 2-Ply - PASSED BM₁ Kerto-S LVL 1 SPF End Grain 2 SPF End Grain 18 **Multi-Ply Analysis** Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6". Capacity 0.0 PLF 163.7 PLF Yield Limit per Foot Yield Limit per Fastener 81.9 lb. Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination Duration Factor 1.00

Celebilated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ansure the component subability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire relardant or corrol

Handling & Installation

and ling & installation.

LVI, beans must not be cut or drilled.
Refer to manufacturer's product information regarding invalidation requirements, multi-ply textening idetails, beam strongth values, and code approval's Damaged Beams must not be used.
Design assumss lop adge is laterally realizated. Provide laters: support at beeing points to avoid lateral displacement and rotation.

6. For flat roofs provide proper drainage to prevent

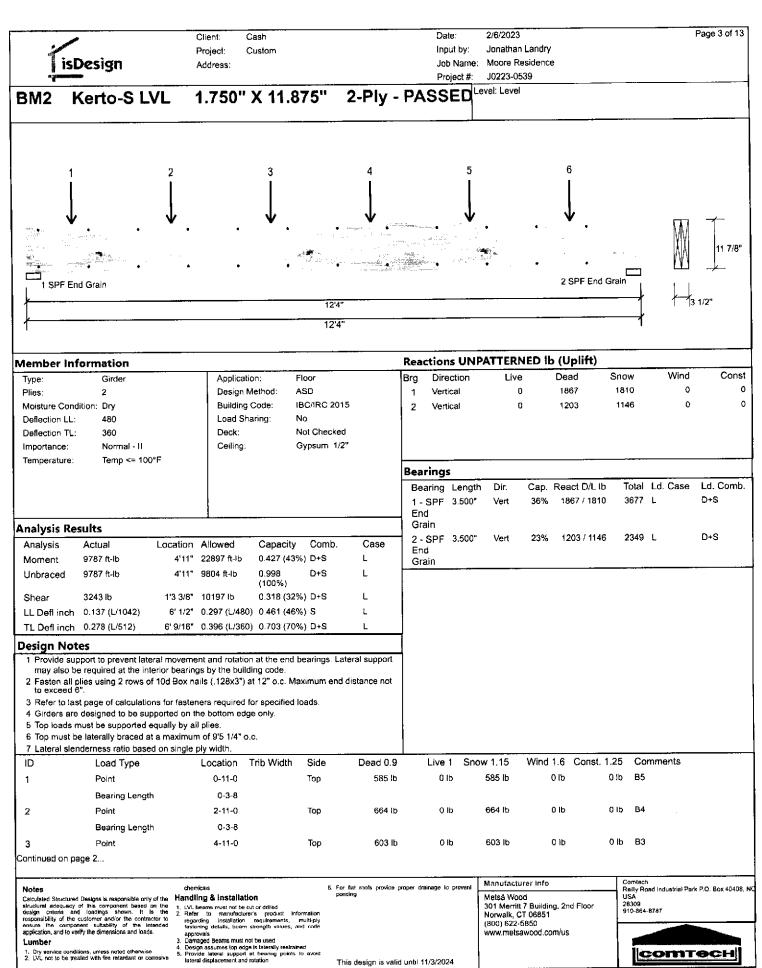
This design is valid until 11/3/2024

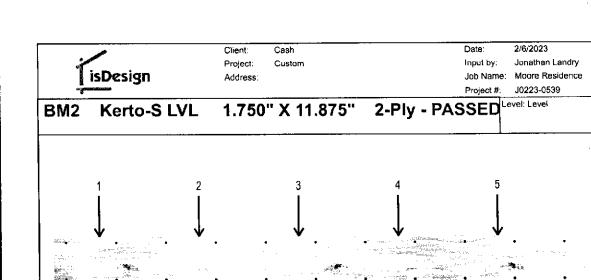
Manufacturer Info Metsä Wood

Metsa vocod 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Comtach Reilly Road Industrial Park PO Box 40408, NO USA 28308 910-864-6787







Continued	from page 1						•			
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
	Bearing Length	0-3-8								
4	Point	6-11-0		Тор	572 lb	dI 0	572 lb	0 lb	Q lb	B2
	Bearing Length	0-3-8								
5	Point	8-11-0		Тор	457 lb	0 lb	457 lb	0 lb	0 lb	B1-GR
	Bearing Length	0-3-8								
6	Point	10-11-0		Тор	75 lb	0 16	75 lb	0 lb	0 lb	ZB1
	Bearing Length	0-3-8								
	Self Weight				9 PLF					

12'4' 12'4"

Notes	
Calculated Structured Designs is responsible only of structural adequacy of this component based on design criteria and loadings shown. It is	ţh
responsibility of the customer and/or the contractor ensure the component suitability of the intend application, and to verify the dimensions and loads.	ŀ

Lumber

1 SPF End Grain

Handling & Installation

Handling & Installation

1. IVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply lastening details, buses sherigin values, and code approvals

2. Damagad Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

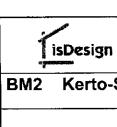
Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Connecti Reilly Road Industrial Park P.O. Box 40408, No USA 28309 910-864-8787

Page 4 of 13

2 SPF End Grain





Client: Project.

Address:

Cash

Custom

Date:

2/6/2023

Input by: Jonathan Landry Job Name: Moore Residence

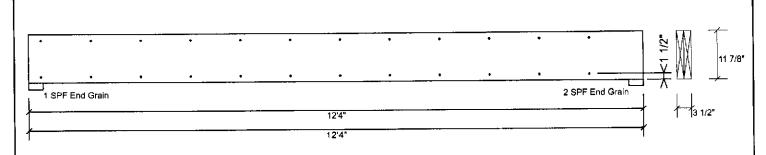
J0223-0539 Project #:

Kerto-S LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	163.7 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	1V	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on his design ortheria and loadings shown. It is the responsibility of the customer and/or the contractor to neutric the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

Handling & Installation

1. IVU beams must not be cut of drilled

2. Reter to manufacturer's product information regarding installation requirements, multi-ply featuring installation requirements, multi-ply featuring installation requirements, multi-ply featuring installation and code approvals

3. Damagged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide factorial support at bearing points to avoid lateral obspiacement and rotation

This design is valid until 11/3/2024

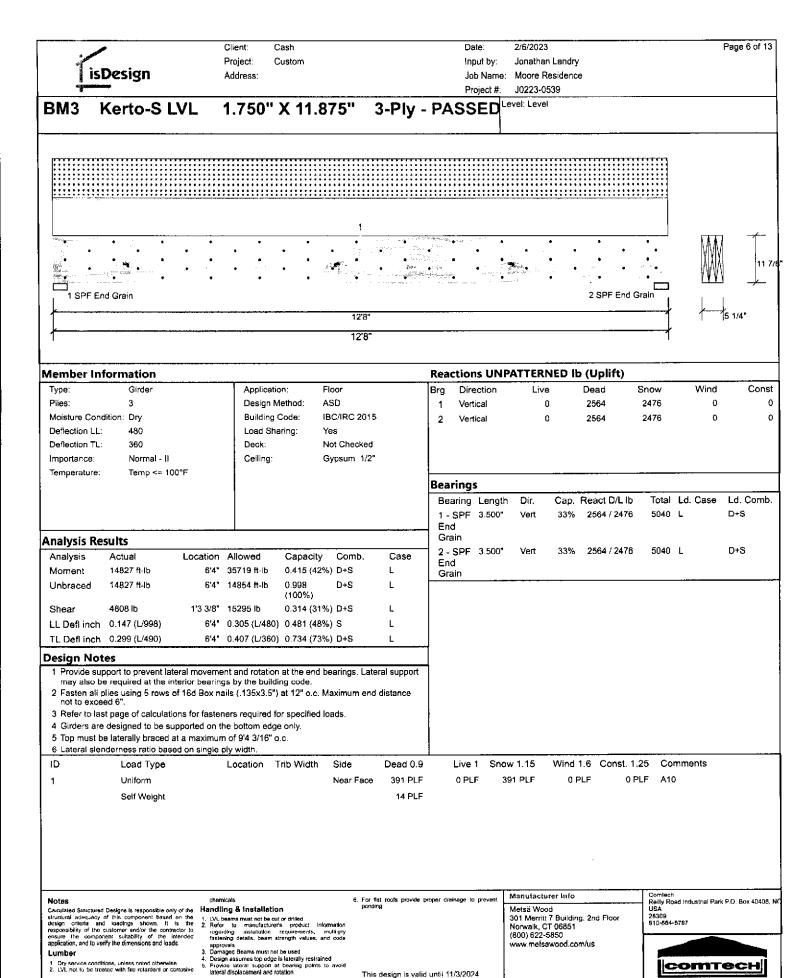
Manufacturer info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Comlech Reilly Road Industrial Park P.O. Box 40408, No USA 28309 910-864-8787

Page 5 of 13





This design is valid until 11/3/2024

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Lumber

COMTECH

Page 7 of 13 Client: Cash Date: 2/6/2023 Project: Custom Input by: Jonathan Landry isDesign Job Name: Moore Residence Address: Project #: J0223-0539 1.750" X 11.875" Level: Level 3-Ply - PASSED **BM3** Kerto-S LVL 2 SPF End Grain 1 SPF End Grain 12'8 12'8" **Multi-Ply Analysis** Fasten all plies using 5 rows of 16d Box nails (.135x3.5") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed Capacity 521.3 PLF oad Yield Limit per Foot 536.1 PLF 107.2 lb. Yield Limit per Fastener IV Yield Mode Edge Distance 1 1/2" Min. End Distance 3" Load Combination D+S Duration Factor 1.15

Calculated Structured Designs is responsible only of the structural adoquacy of this component based on the design ortherid and loadings shown. If is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Hancilling & Installation

1. LVL beams must not be out or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply featening details, beam strength values, and code approvals

3. Dameged Boarns must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

(800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech Reilly Road Industrial Park P.O. Box 40408, NO USA 28309 910-884-8787





Client:

Cash

Project: Address: Custom

Date: Input by: 2/6/2023

Jonathan Landry

Job Name: Moore Residence Project #: J0223-0539

Kerto-S LVL BM4

1.750" X 18.000"

2-Ply - PASSED

_evel: Level

Reactions UNPATTERNED Ib (Uplift)

Vert

,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
		444444444444444444444444444444444444444
La Marian		
	4	
	The second secon	MAY 1
Land of the second of the seco		
SPF End Grain		2 SPF End Grain
	18'5"	13 1/2*
	100	1 10 1/2

ſ	Туре:	Girder	Application:	Floor	Brg	Direction	Live	Dead	Snow
I	Plies:	2	Design Method:	ASD	1	Vertical	368	3398	3131
١	Moisture Condition:	Dry	Building Code:	IBC/IRC 2015	2	Vertical	368	3398	3131
Ì	Deflection LL:	480	Load Sharing:	No					
I	Deflection TL:	360	Deck:	Not Checked					
I	Importance:	Normal - II	Ceiling:	Gypsum 1/2"					
I	Temperature:	Temp <= 100°F							

Analysis Res	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	28648 ft-lb	9'2 1/2"	49428 ft-lb	0.580 (58%)	D+S	L
Unbraced	28648 ft-lb	9'2 1/2"	28811 ft-lb	0.994 (99%)	D+S	L
Shear	5302 lb	1'9 1/2"	15456 lb	0.343 (34%)	D+\$	L
LL Defl inch	0.260 (L/830)	9'2 9/16"	0.449 (L/480)	0.579 (58%)	8	L

Bearing	S						
Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	63%	3398 / 3131	6529	L	D+\$

63% 3398 / 3131

Fnd Grain 2-\$PF 3.500*

End Grain

D+S 6529 L

Wind

0

0

Const

Q

0

Page 8 of 13

Design Notes

TL Defl inch 0.542 (L/398)

Member Information

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.

9'2 9/16" 0.599 (L/360) 0.905 (90%) D+S

- 2 Faster all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 4'5 15/16" o.c.
- 7 Lateral slenderness ratio based on single ply width.

םו	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	340 PLF	0 PLF	340 PLF	0 PLF	0 PLF	C2	
2	Tie-In Far	0-0-0 to 18-5-0	1-0-0	Far Face	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	Floor Loads	
2	Tie-In Near	0-0-0 to 18-5-0	0-0-0	Τορ	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	Floor Loads	
	Self Weight				14 PLF			-			

Cafculated Structured Designs is responsible only of the structural adequacy of this component based on line design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to resure the component authority of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive.

Handling & Installation

- I. VVL beams must not be cut or drilled
 Refer to manufacturer's product information requiring installation requirements, multi-ply featening details, beam strength values, and code approvals.
- featening details, beem strength values, and code approvals. Damaged Beams must not be used. Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation.

Manufacturer info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

This design is valid until 11/3/2024

Reilly Road industrial Park P.O. Box 40408, N USA 28309 910-864-8787



Page 9 of 13 Date: 2/6/2023 Cash Client: Project: Custom Input by: Jonathan Landry isDesign Job Name: Moore Residence Address: J0223-0539 Project #: Level: Level 1.750" X 18.000" 2-Ply - PASSED Kerto-S LVL 2 SPF End Grain 1 SPF End Grain 18'5" 18'5" Multi-Ply Analysis Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c., Maximum end distance not to exceed 6". 11.2 % 27.5 PLF Load 245.6 PLF Yield Limit per Foot Yield Limit per Fastener 81.9 lb. IV Yield Mode 1 1/2" Edge Distance Min. End Distance 3" D+L Load Combination 1.00 Duration Factor

Notice:
Carculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and leadings shown. It is the responsibility of the customer end/or the contractor to ensure the component suitability of the interood application, and to verify the dimensions and loads. Lumber

Notes

Ory service conditions, unless noted otherwise
 LVL, not to be treated with the retardant or corrosive.

Handling & Installation

LVL beams must not be out or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-pty fastening details, heam strength values, and code

approvals

Demaged Beams must not be used

Demaged assumes top odgo is laterally restrained

Provide lateral support at bearing points to avoid lateral displacement and rotation

For Rat roofs provide proper drainage to prevent ponding

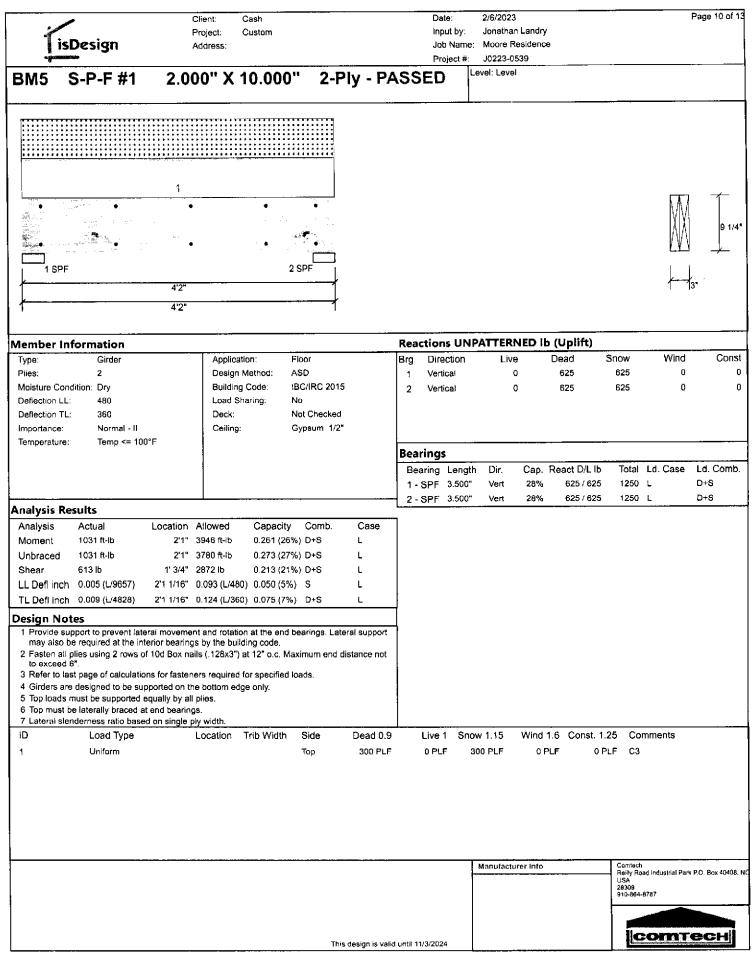
This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech Reilty Road Industrial Park P.O. Box 40408, NI USA 28309 910-864-8787





	sDesign	Client: Project: Address:	Cash Custom		Date: Input by: Job Name Project #:	2/6/2023 Jonathan Landry Moore Residence J0223-0539	Page 11 of 13
ВМ5	S-P-F #1	2.000" X	10.000"	2-Ply - PASSE	ED	evel: Level	
•	•	•	•	1 1/2"			9 1/4"
	•	•	•	• + +			
1 SP	F	4'2"	2 SF	PF			3*
│ ├ ──		4'2"		 			
Multi-Ply							
Capacity	olies using 2 row	0.0 %	s (.128x3") at 1.	2" o.c Maximum end di	stance no	ot to exceed 6".	
Load Yield Limit per Yield Limit per		0.0 PLF 157.4 PLF 78.7 lb.					
Yield Mode Edge Distance	÷	IV 1 1/2"					
Min. End Dista Load Combina		3*					
Duration Factor		1.00					
						Manufacturer info	Comlech Rellly Road Industria: Park P.O. Box 40408, N
							Relity Road industria: Fark P.O. Box 40408, N USA 28309 910-864-8787
				This design is valid until 11/3	3/2024		соттесн



Client: Project:

Address:

Cash

Custom

Date: input by: 2/6/2023

Jonathan Landry

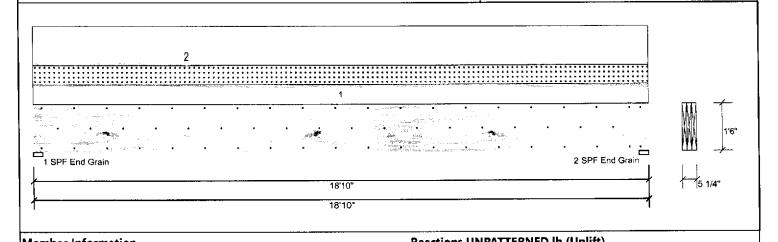
Page 12 of 13

Job Name: Moore Residence J0223-0539 Project #:

GDH Kerto-S LVL 1.750" X 18.000"

3-Ply - PASSED

Level: Level



End Grain 2 - SPF 3.500"

End Grain Vert

53% 6130 / 1978

Member Into	rmation			Keac	TIONS UNP	ALIEK	MEDI	o (opinic)		
Type:	Girder	Application:	Floor	Brg	Direction	Liv	ê	Dead	Snow	1
Plies:	3	Design Method:	ASD	1	Vertical		0	6130	1978	
Moisture Conditi	ion: Dry	Building Code:	IBC/IRC 2015	2	Vertical		0	6130	1978	
Deflection LL:	480	Load Sharing:	Yes							
Deflection TL:	360	Deck:	Not Checked							
Importance:	Normal - II	Ceiling:	Gypsum 1/2*							
Temperature:	Temp <= 100°F									
				Bear	ings					
				Bea	ring Length	Dir.	Cap.	React D/L lb	Total	Ld. (
				1 1 - 9	SPE 3.500"	Vert	53%	6130 / 1978	8108	L

Analysis Res	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	36421 ft-lb	9'5"	77108 ft-lb	0.472 (47%)	D+S	L
Unbraced	36421 ft-lb	9'5"	36512 ft-lb	0.998 (100%)	D+S	L
Shear	6597 lb	1'9 1/2"	23184 lb	0.285 (28%)	D+S	L
LL Defl inch	0.117 (L/1889)	9'5 1/16"	0.460 (L/480)	0.254 (25%)	\$	L
TL Defl inch	0.479 (L/461)	9'5 1/16"	0.613 (L/360)	0.781 (78%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 5'5 1/8" o.c.

/ Lateral signice	erness ratio based on single	piy wiuth.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	210 PLF	0 PLF	210 PLF	0 PLF	0 PLF	C1GE	
2	Uniform			Тор	420 PLF	0 PLF	0 PLF	0 PLF	0 PL F	Brick	
	Self Weight				21 PLF						

Notes	
Calculated Structured Designs is responsible on attructurar adequacy of this component based design oriteria and loadings shown. It responsibility of the customer and/or the contrensure the component suitability of the application, and to verify the dimensions and loadings.	on the is like actor to intended

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Pranciumg & Installation

 1. IV. beams must not be out or drilled

 2. Refor to manufacturer's product information regarding installation requirements, multi-pty featening obtains, hearn strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at begaing points to avoid tateral dispitalcement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech Reilly Road Industrial Park P.O. Box 40408, N USA 28309 910-864-8787

Wind

Case

8108 L

0

0

Const

Ld. Comb. D+\$

D+S

0

0



	isDesign	Client: Project: Address:	Cash Custom	, _W - 4(Date: Input by: Job Name: Project #:	2/6/2023 Jonathan Landry Moore Residence J0223-0539	Page 13 of 1
GDH	Kerto-S L\	/L 1.750"	' X 18.000"	3-Ply - PASS	ED	evel: Level	
	· · · ·						116"
1 SPF	End Grain		• •	 	<u> </u>	2 SPF End Grain	
ļ				18'10" 18'10"		· · · · · · · · · · · · · · · · · · ·	5 1/4*
•				10 10		l	
Fasten all	l alies usina 3 row	s of 10d Box nail	s (128x3") at 12"	' a.c. Nail from both si	das Mari	mum end distance not to exceed	
6". Capacity Load Yield Limit p Yield Limit p	er Foot	0.0 % 0.0 PLF 245.6 PLF 81.9 lb.		G.C., Nail Holl Both Si	ues. Maxi	maniferia distance not to exceed	
6". Capacity Load Yield Limit p	er Foot er Fastener ice stance nation	0.0 % 0.0 PLF 245.6 PLF		G.C., Nail Holli Both Si	des. Maxi	maniferia distance not to exceed	
6", Capacity Load Yield Limit p Yield Limit p Yield Mode Edge Distan Min. End Dis Load Combi	er Foot er Fastener ice stance nation	0.0 % 0.0 PLF 245.6 PLF 81.9 lb. IV 1 1/2" 3"		G.C., IVall Holl Both Si	des. Maxi	main end distance not to exceed	
6", Capacity Load Yield Limit p Yield Limit p Yield Mode Edge Distan Min. End Dis Load Combi	er Foot er Fastener ice stance nation	0.0 % 0.0 PLF 245.6 PLF 81.9 lb. IV 1 1/2" 3"		G.C., IVall Holl Both Si	des. Maxi	main end distance not to exceed	
6", Capacity Load Yield Limit p Yield Limit p Yield Mode Edge Distan Min. End Dis Load Combi	er Foot er Fastener ice stance nation	0.0 % 0.0 PLF 245.6 PLF 81.9 lb. IV 1 1/2" 3" 1.00				indifficial distance not to exceed	

Notes
Calculated Structured Designs is responsible only of the structural adequacy of this component bused on the design orderie and feedings shown. It is the design orderies and feedings shown. It is the design orderies and feedings shown. It is the design assumes that the understanding of the design assumes to be design assumes top dept is traveally restrained.

1. Dry service conditions, unless noted otherwise.
2. LVL not to be treated with fire mixedant or corrosive.

chemicals

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Comtech Reilly Road Industrial Park P.C. 80x 40408, NO USA 28309 910-864-8787





Trenco

818 Soundside Rd Edenton, NC 27932

Re: J0223-0539 Moore Residence

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: 156496098 thru 156496104

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844



February 7,2023

Liu, Xuegang

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Pły	Moore Residence	15040000
J0223-0539	ET1	GABLE	1	1	 	156496098

Comtech, Inc.

Fayetteville, NC - 28314,

| Job Reference (optional)

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Feb 6 10:56:43 2023 Page 1

ID:H?WdV7YlehtS?ynQPhAFS1zqDSo-?hx8mixzi_r79r8rOaJ758jUCW29QK8NiLcXeczntio

0-1-8

Scale = 1:37.1

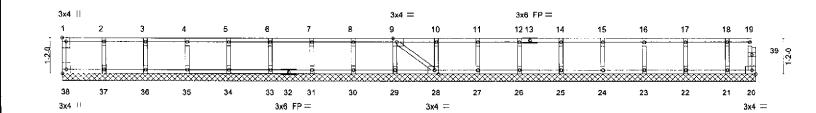


Plate Offs	1-4-0 + 2- 1-4-0 1- sets (X.Y)	-0-0 4-0-0 5-4-0 -4-0 1-4-0 1-4-0 [1:Edge,0-1-8], [9:0-1-8,	1-4-0 Carlon Carl	1-0-0 1-4-0 1-4-0 1-4-0 9 Edge) 138:E	1-4-0		1-0	1-4-0	1-4-0	1-4-0	18-8-0	1-4-0	1-4-0 22-3-0 1-4-0 0-11-0
			Logoj, [Lo.o-1-	O,Edgoj, joo.E	.uga,0-1-0]								
LOADING	G (psf)	SPACING-	2-0-0	ÇSI.		DEFL.	in	(loc)	l/defl	L/d	PL	ATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT	20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.01	Vert(CT)	n/a	-	n/a	999			
BCLL	0.0	Rep Stress Incr	YEŞ	WB	0.03	Horz(CŤ)	0.00	20	п/а	n/a			
BCDL	5.0	Code IRC2015/T	PI2014	Matrix-	-s	(- /					· We	eight: 95 lb	FT = 20%F, 11%

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)

OTHERS 2x4 SP No.3(flat)

.x4 GF 140.3(ilat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

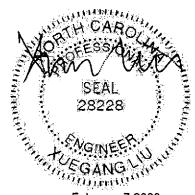
REACTIONS. All bearings 22-3-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 38, 20, 37, 36, 35, 34, 33, 31, 30, 29, 28, 27, 26, 25, 24, 23, 22, 21

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown,

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.



February 7,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 5/19/2020 REFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a fruss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and 8CSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Moore Residence	
J0223-0539	F1	Floor	3	1	156	496099
			-	,	Job Reference (optional)	i i

Comtech, Inc.

1-3-0

Fayetteville, NC - 28314,

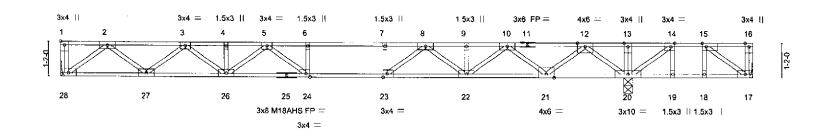
8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Feb 6 10:56:45 2023 Page 1 ID:H?WdV7YlehtS?ynQPhAFS1zqDSo-x32uBOyDEb6r08IEV?MbAZocLJUZu5Yg9f5eiVzntim

4-4

Scale = 1:37.2

0-10-12





		18-2-12 18-2-12				22-3					
18-2-12 4-0-4 Plate Offsets (X,Y) [1:Edge,0-1-8], [14:0-1-8,Edge], [15:0-1-8,Edge], [23:0-1-8,Edge], [24:0-1-8,Edge]											
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.92 BC 0.97 WB 0.61 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.30 24-26 -0.41 24-26 0.05 20	l/defl >732 >532 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 115 lb	GRIP 244/190 186/179 FT = 20%F, 11%E			

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) **BRACING-**

TOP CHORD

Structural wood sheathing directly applied or 2-2-0 oc purlins,

except end verticals.

BOT CHORD

Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS.

(size) 28=Mechanical, 17=Mechanical, 20=0-3-8

Max Uplift 17=-411(LC 3)

Max Grav 28=897(LC 10), 17=83(LC 4), 20=1784(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1856/0, 3-4=-3049/0, 4-5=-3049/0, 5-6=-3412/0, 6-7=-3412/0, 7-8

TOP CHORD 2-3=-1856/0, 3-4=-3049/0, 4-5=-3049/0, 5-6=-3412/0, 6-7=-3412/0, 7-8=-3412/0, 8-9=-2301/0, 9-10=-2301/0, 10-12=-648/0, 12-13=0/1813, 13-14=0/1813, 14-15=0/816

BOT CHORD 27-28=0/1114, 26-27=0/2574, 24-26=0/3353, 23-24=0/3412, 22-23=0/2869, 21-22=0/1582.

20-21=-438/0, 19-20=-816/0, 18-19=-816/0, 17-18=-816/0
WEBS 2-28=-1397/0, 2-27=0/966, 3-27=-934/0, 3-26=0/607, 12-20=-1766/0, 12

2-28=-1397/0, 2-27=0/966, 3-27=-934/0, 3-28=0/607, 12-20=-1766/0, 12-21=0/1290, 10-21=-1225/0, 10-22=0/924, 5-26=-288/0, 8-22=-723/0, 8-22=-0/924, 5-24=-0/924, 5

10-21=-1225/0, 10-22=0/924, 5-26=-388/0, 8-22=-733/0, 8-23=0/894, 5-24=-196/443,

7-23=-410/0, 15-17=0/1007, 14-20=-1360/0, 14-19=0/352, 15-18=-317/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x6 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 411 lb uplift at joint 17.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nalls. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 8) CAUTION. Do not erect truss backwards.



February 7,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 5/19/20/20 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guildance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSUTPIC Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2570 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road

Job Truss Truss Type Qty PΙγ Moore Residence 156496100 J0223-0539 F1A Floor Job Reference (optional)

Comtech, Inc.

1-3-0

Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Feb 6 10:56:47 2023 Page 1 ID:H?WdV7YiehtS?ynQPhAFS1zqDSo-tSAec4_TmDMZeSScdQO3F_uzO7CfMwSzdyalnNzntik

1-2-8 1-2-8 1-2-8 1-2-8 0-11-12 1-2-8

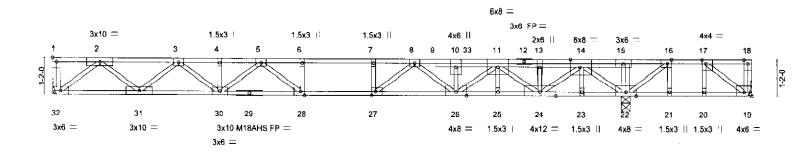
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

Scale = 1:36.8





<u> </u>										
Plate Offse	ets (X,Y)	[1:Edge,0-1-8], [14:0-3-8	,Edge]. [16:0-1	18-2-12 -8,Edge], [17:0-1-8,Edg	e], [19:Edge,0-1-8],	[27:0-1-8,Edge], [28:0-1	I-8,Edge]		
LOADING TOLL TODL	40.0 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.00 1.00	CSI. TC 0.88 BC 0.87	DEFL. Vert(LL) Vert(CT)	in (loc) -0.26 28 -0.36 27-28	I/defl >836 >607	L/d 480 360	PLATES MT20 M18AHS	GRIP 244/190 186/179
BCLL BCDL	0.0 5.0	Rep Stress Incr Code IRC2015/Ti	NO Pl2014	WB 0.91 Matrix-S	Horz(CT)	0.06 22	n/a	n/a	Weight: 126 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP 2400F 2.0E(flat)

2x4 SP No.1(flat) BOT CHORD

2x4 SP No.3(flat) WEBS

> (size) 32=Mechanical, 22=0-3-8, 19≃Mechanical

Max Uplift 19=-541(LC 3)

Max Grav 32=964(LC 10), 22=2193(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown,

TOP CHORD 2-3=-2026/0, 3-4=-3376/0, 4-5=-3376/0, 5-6=-4002/0, 6-7=-4002/0, 7-8=-4002/0,

8-10=-3383/0, 10-11=-3378/0, 11-13=-1049/0, 13-14=-1049/0, 14-15=0/2470,

15-16=0/2464, 16-17=0/1059

BOT CHORD 31-32=0/1203, 30-31=0/2816, 28-30=0/3769, 27-28=0/4002, 26-27=0/3648, 25-26=0/2407,

24-25=0/2407, 23-24=-565/0, 22-23=-565/0, 21-22=-1059/0, 20-21=-1059/0,

19-20=-1059/0

2-32=-1509/0, 2-31=0/1071, 3-31=-1029/0, 3-30=0/715, 5-30=-502/0, 5-28=0/659,

6-28=-317/0, 14-22=-2355/0, 14-24=0/1909, 11-24=-1693/0, 11-26=0/1213,

10-26=-479/0, 8-26=-355/70, 16-22=-1861/0, 16-21=0/413, 17-19=0/1321, 17-20=-369/0,

8-27=-44/632, 7-27=-311/21

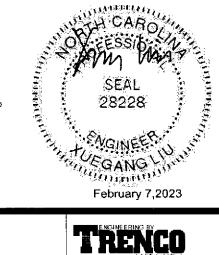
WEB\$

- 1) Unbalanced floor live loads have been considered for this design.
- All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x4 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 541 lb uplift at joint 19.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 8) CAUTION, Do not erect truss backwards.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 426 lb down at 13-2-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 19-32=-10, 1-18=-100



February 7,2023

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTE&® connectors. This design is based only upon parameters shown, and is for an individual building component, not a loss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801



Job	Truss	Truss Type	Qty	Ply	Moore Residence	
·				'	156	3496100
J0223-0539	F1A	Floor	11	i 1		
					Job Reference (optional)	
	avetleville NC - 28314	·		8 430 s Jai	n 6 2022 MiTek Industries, Inc., Mon Feb 6 10 56:47 2023, Pa	ae 2

ID:H?WdV7YlehtS?ynQPhAFS1zqDSo-tSAec4_TmDMZeSScdQO3F_uzO7CfMwSzdyalnNzntik

LOAD CASE(\$) Standard Concentrated Loads (lb) Vert: 33=-346(F)

WARN NG - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-88 and BCSI Building Component Safety Information available from Truss Plate institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20501

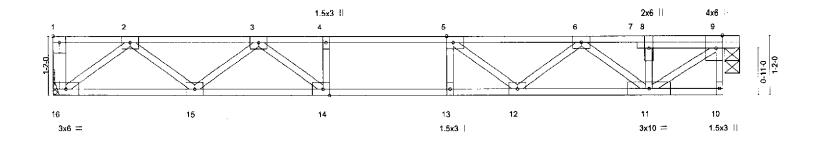


	Job	Truss	Truss Type	Qty	Ply	Moore Residence	
ı						; IS	56 4 96101
	J0223-0539	F2	Floor	1	1		
					ļ	Job Reference (optional)	
	Comtech, inc. Favettey	/ille. NC - 28314.			3.430 s Jar	6 2022 MiTek Industries, Inc. Mon Feb 6 10:56:48 2023 P	age 1

ID:H?WdV7YlehtS?ynQPhAFS1zqDSo-Lek1pQ?5XWUQFc1pB7vloBQFTXcr5U77rcJlJqzntij

1-3-0 2-3-8 0-4-0

Scale = 1:22.5



						3-0-8						13-4-8
Plate Offse	ets (X,Y)	[1:Edge,0-1-8], [5:0-1-8,E	Edge], [9:0-3-	0,Edge], [14:0		3-0-8						0-4-0
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.41	Vert(LL)	-0.12	12-13	>999	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.61	Vert(CT)	-0.15	13	>999	360		
BCLL	0.0	Rep Stress Incr	YES	WB	0.50	Horz(CT)	0.01	9	n/a	n/a		
BCDL	5.0	Code IRC2015/TF	PI2014	Matri	⊹ S						Weight: 68 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

2x4 SP No.3(flat) WEBS

REACTIONS. (size) 16=Mechanical, 9=0-3-8

Max Grav 16=707(LC 1), 9=707(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown,

TOP CHORD 2-3=-1379/0, 3-4=-2139/0, 4-5=-2139/0, 5-6=-1816/0, 6-8=-850/0, 8-9=-850/0

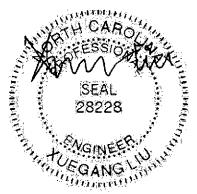
BOT CHORD 15-16=0/867, 14-15=0/1864, 13-14=0/2139, 12-13=0/2139, 11-12=0/1472 WEBS

9-11=0/1043, 2-16=-1088/0, 2-15=0/667, 3-15=-630/0, 3-14=0/559, 6-11=-794/0,

6-12=0/467, 5-12=-535/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 7) CAUTION, Do not erect truss backwards.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

February 7,2023

🛦 WARK 🚧 Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIH7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITTPT Quality Criteria, DSB-89 and BCSI Building Component Safety information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801



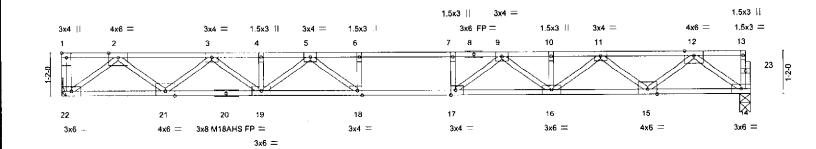
818 Soundside Road

Moore Residence Job Truss Type Qty Truss 156496102 Floor J0223-0539 F3 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Feb 6 10:56:49 2023 Page 1 Comtech, Inc. Fayetteville, NC - 28314,

ID:H?WdV7YlehtS?ynQPhAFS1zqDSo-qqlP1m0klqcHtmc?krQXKPzlaxtVqwmG4G3srGzntii

2-4-8

Scale = 1:30.8



						18-4-8					
						18-4-8					
Plate Offse	ets (X,Y)	[1:Edge,0-1-8], [17:0-1-8	,Edge], [18:0-	1-8,Edge]				·····			
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc) l/defi	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.84	Vert(LL)	-0.32 17-18	>688	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.91	Vert(CT)	-0.44 17-1	>499	360	M18AHS	186/179
BCLL	0.0	Rep Stress Incr	YES	WB	0.54	Horz(CT)	0.08 1	t n/a	n/a		
BCDL	5.0	Code IRC2015/Ti	Pi2014	Matri	⊹ -S					Weight: 93 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

1-3-0

WEBS 2x4 SP No.3(flat)

BRACING-

TOP CHORD

Structural wood sheathing directly applied or 2-2-0 oc purlins.

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 22=Mechanical, 14=0-3-8 Max Grav 22=997(LC 1), 14=991(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2110/0, 3-4=-3538/0, 4-5=-3538/0, 5-6=-4278/0, 6-7=-4278/0, 7-9=-4278/0, TOP CHORD

9-10=-3538/0, 10-11=-3538/0, 11-12=-2110/0 21-22=0/1247, 19-21=0/2939, 18-19=0/3975, 17-18=0/4278, 16-17=0/3975, 15-16=0/2940,

BOT CHORD 14-15=0/1246

2-22=-1565/0, 2-21=0/1123, 3-21=-1079/0, 3-19=0/764, 5-19=-558/0, 5-18=-36/740,

6-18=-346/0, 12-14=-1561/0, 12-15=0/1124, 11-15=-1080/0, 11-16=0/764, 9-16=-558/0,

9-17=-36/740, 7-17=-346/0

NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.



February 7,2023

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 5/19/2020 BEFORE USE



818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qtv Moore Residence Ply 156496103 J0223-0539 F3A Floor Job Reference (optional)

Comtech, Inc.

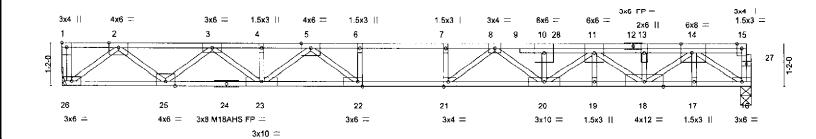
⊢ 1-3-0 →

Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Feb 6 10:56:51 2023 Page 1 ID:H?WdV7YlehtS?ynQPhAFS1zqDSo-mDQ9SS1_qRs_63IOsFS?Pq2dKkZ8lk?ZYaYzw9zntig

2-2-0 1-2-8 1-2-8 1-2-8 0-1-8 1-2-8

Scale = 1:30.8



						18-4-8 18-4-8						
Plate Offsets (X,Y) [1:Edge,0-1-8], [14:0-3-8,Edge], [21:0-1-8,Edge]												
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.95	Vert(LL)	-0.35 20-21	>618	480	MT20	244/190	
TCDL	10.0	Lumber DOL	1.00	BC	0.90	Vert(CT)	-0.49 20-21	>444	360	M18AHS	186/179	
BCLL	0.0	Rep Stress Incr	NO	W8	0.87	Horz(CT)	0.08 16	n/a	n/a			
BCDL	5.0	Code IRC2015/TF	PI2014	Matrix	c-S	, ,				Weight: 104 lb	FT = 20%F, 11%E	

LUMBER-

TOP CHORD BOT CHORD

2x4 SP 2400F 2.0E(flat) 2x4 SP 2400F 2.0E(flat)

WEBS 2x4 SP No.3(flat) BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-10-15 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 26=Mechanical, 16=0-3-8 Max Grav 26=1114(LC 1), 16=1296(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2407/0, 3-4=-4108/0, 4-5=-4108/0, 5-6=-5288/0, 6-7=-5288/0, 7-8=-5288/0,

8-10=-5275/0, 10-11=-5275/0, 11-13=-3201/0, 13-14=-3201/0

BOT CHORD 25-26=0/1403, 23-25=0/3367, 22-23=0/4705, 21-22=0/5288, 20-21=0/5280, 19-20=0/4454,

18-19=0/4454, 17-18=0/1725, 16-17=0/1725

2-26=-1760/0, 2-25=0/1307, 3-25=-1249/0, 3-23=0/947, 5-23=-761/0, 5-22=0/1079,

6-22=-478/0, 14-16=-2101/0, 14-18=0/1832, 11-18=-1555/0, 11-20=0/1018,

10-20=-518/0, 8-21=-406/297

NOTES-

WEBS

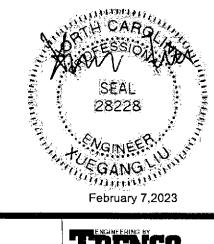
- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center,
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.

Vert: 28=-423(B)

- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 503 lb down at 13-2-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-26=-10, 1-15=-100 Concentrated Loads (lb)



February 7,2023

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 (ev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and parament bracing is slaways required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932

Job Truss Truss Type Qty Ply Moore Residence 156496104 J0223-0539 FG1 FLOOR 1 | Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Feb 6 10:56:52 2023 Page 1 Comtech, Inc. Fayetteville, NC - 28314, 3x4 =ID:H?WdV7YlehtS?ynQPhAFS1zqDSo-EPzXfn2cbl_rkDKaQz_Ey1bz68091LsimEHWSbzntif 1-0-8 0-6-0 1-3-0 3x4 |I ⊢ Scale = 1:7.3 0-11-0 3x6 = 1.5x3 || 1.5x3 💠 3x6 == 8 7 5

Plate Offsi	4.4.7	[1:Edge,0-1-8], [2:0-1-8,8	-090] [0.0 . 0	i daol		Т						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
TÇLL	40.0	Plate Grip DOL	1.00	TC	0.24	Vert(LL)	-0.01	5-6	>999	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.40	Vert(CT)	-0.02	5-6	>999	360		21,00
BCLL	0.0	Rep Stress Incr	NO	WB	0.19	Horz(CT)	0.00	5	n/a	n/a		
BCDL	5.0	Code IRC2015/TI	PI2014	Matri	x-S			-	,		Weight: 20 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

(size) 8=Mechanical, 5=Mechanical

Max Grav 8=446(LC 1), 5=523(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-705/0

BOT CHORD 7-8=0/705, 6-7=0/705, 5-6=0/705

WEBS 3-5=-804/0, 2-8=-838/0

NOTES.

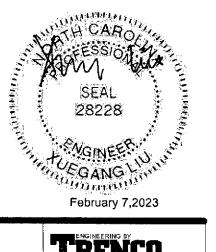
- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 5-8=-10, 1-4=-100

Concentrated Loads (lb)

Vert: 3=-607



Structural wood sheathing directly applied or 3-6-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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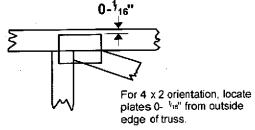
818 Soundside Road Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

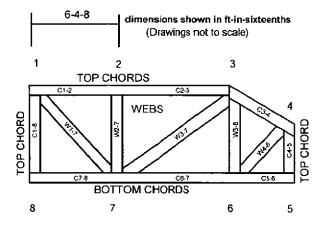
DSB-89: Design Standard for Bracing.

BCSI:

Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate

Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, eraction supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.