

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 243_2939_C KB Home.243.2939.C

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I48185412 thru I48185454

My license renewal date for the state of North Carolina is December 31, 2021.

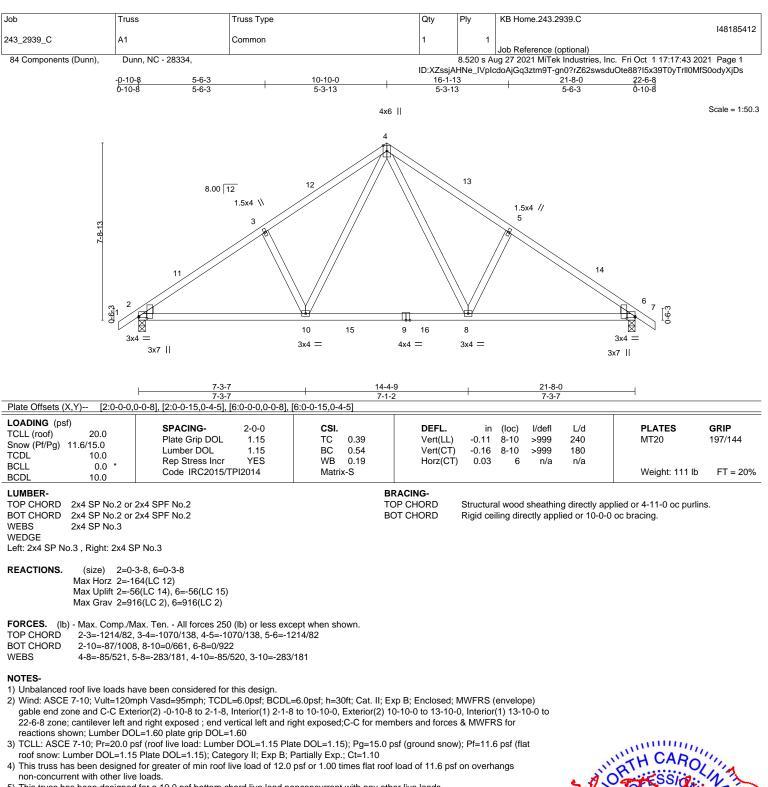
North Carolina COA: C-0844



October 4,2021

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



AMITEK AM

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

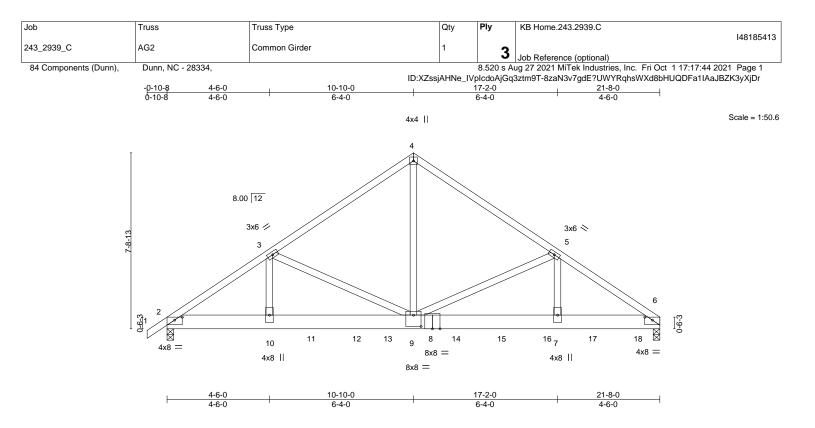


Plate Offsets (X,Y) [2:0-4-0,0	0-1-9], [6:0-4-0,0-1-9], [9:0-4-0,0-6-0]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNO	CSI. TC 0.63 BC 0.89 WB 0.87	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.09 9-10 -0.17 9-10 0.04 6	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 436 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 3 BOT CHORD 2x8 SP No.2 WEBS 2x4 SP No.3	2x4 SPF No.2	Т		Structural wood Rigid ceiling di			lied or 6-0-0 oc purlins oc bracing.	
TOP CHORD 2-3=-8772/106 BOT CHORD 2-10=-925/714	ax. Ten All forces 250 (lb) or less exc 57, 3-4=-6161/648, 4-5=-6162/648, 5-6= 1, 9-10=-925/7141, 7-9=-575/7376, 6-7 1, 5-9=-2611/279, 5-7=-91/2565, 3-9=-2	-9034/763 =-575/7376						
 Top chords connected as follo Bottom chords connected as Webs connected as follows: 2 All loads are considered equaply connections have been pr Unbalanced roof live loads hat Wind: ASCE 7-10; Vult=120m gable end zone; cantilever lef TCLL: ASCE 7-10; Pr=20.0 proof snow: Lumber DOL=1.15 This truss has been designed non-concurrent with other live This truss has been designed will fit between the bottom chords 	follows: 2x8 - 2 rows staggered at 0-5-0 2x4 - 1 row at 0-9-0 oc. Illy applied to all plies, except if noted a ovided to distribute only loads noted as ave been considered for this design. ph Vasd=95mph; TCDL=6.0psf; BCDL t and right exposed ; end vertical left ar sf (roof live load: Lumber DOL=1.15 Pla i Plate DOL=1.15); Category II; Exp B; I for greater of min roof live load of 12.0 e loads. for a 10.0 psf bottom chord live load not ed for a live load of 20.0psf on the bottom	oc. s front (F) or back (B) fac (F) or (B), unless otherw =6.0psf; h=30ft; Cat. II; E d right exposed; Lumber te DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10 psf or 1.00 times flat root onconcurrent with any oth m chord in all areas wher	ise indicated. xp B; Enclosed; M DOL=1.60 plate g osf (ground snow) f load of 11.6 psf o her live loads. e a rectangle 3-6-	IWFRS (envelo prip DOL=1.60 ; Pf=11.6 psf (fi on overhangs 0 tall by 2-0-0 v	pe) lat vide		SEAL 044925	Ne Swamming

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=505, 2=628.

Continued on page 2

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mmm October 4,2021

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Job	Truss	Truss Type	Qty	Ply	KB Home.243.2939.C
					I48185413
243_2939_C	AG2	Common Girder	1	2	
				3	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			8.520 s A	ug 27 2021 MiTek Industries, Inc. Fri Oct 1 17:17:45 2021 Page 2
		ID:XZ	ssjAHNe_	VplcdoAjG	q3ztm9T-d98IGF8IOX7L8i01FZ1mAM8SEpZUJUYJpzx6sWyXjDq

NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1324 lb down and 350 lb up at 4-3-12, 1026 lb down and 118 lb up at 6-3-12, 972 lb down and 113 lb up at 8-3-12, 925 lb down and 108 lb up at 9-8-4, 920 lb down and 72 lb up at 11-11-4, 920 lb down and 72 lb up at 12-8-4, 920 lb down and 12-8-4design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

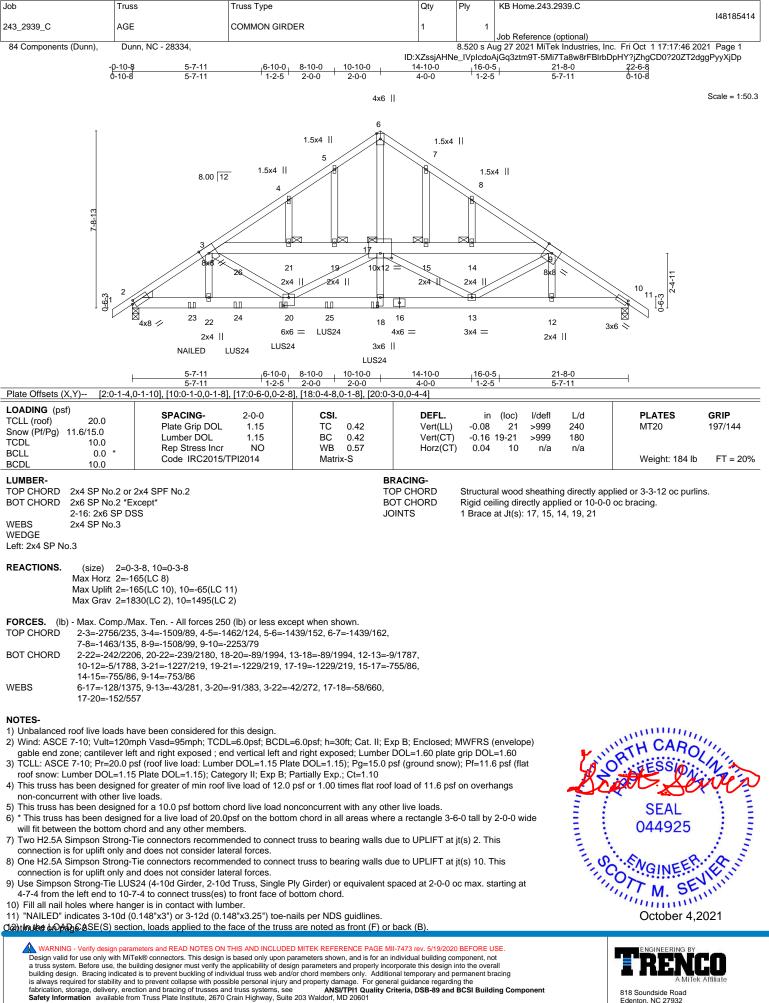
Vert: 1-4=-43, 4-6=-43, 2-6=-20

Concentrated Loads (lb)

Vert: 8=-722(B) 10=-1289(B) 11=-1026(B) 12=-972(B) 13=-925(B) 14=-722(B) 15=-722(B) 16=-722(B) 17=-722(B) 18=-723(B)

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818 Soundside Road Edenton, NC 27932

Truss	Truss Type	Qty	Ply	KB Home.243.2939.C
				I48185414
AGE	COMMON GIRDER	1	1	
				Job Reference (optional)
Dunn, NC - 28334,			8.520 s A	ug 27 2021 MiTek Industries, Inc. Fri Oct 1 17:17:46 2021 Page 2
	AGE	AGE COMMON GIRDER	AGE COMMON GIRDER 1	AGE COMMON GIRDER 1 1

ID:XZssjAHNe_IVplcdoAjGq3ztm9T-5Mi7Ta8w8rFBlrbDpHY?jZhgCD0?20ZT2dggPyyXjDp

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

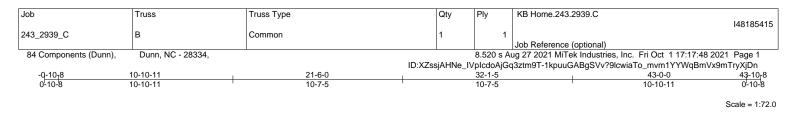
Uniform Loads (plf) Vert: 1-6=-43, 6-11=-43, 2-10=-20, 3-9=-20

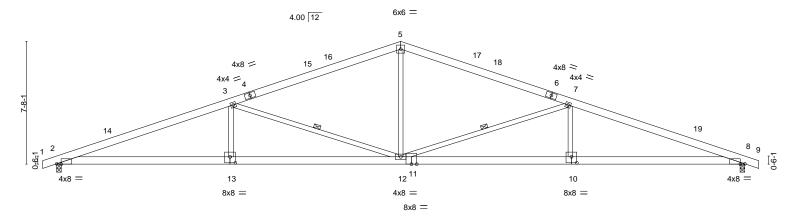
Concentrated Loads (lb)

Vert: 20=-143(F) 18=-412(F) 23=-176(F) 24=-143(F) 25=-143(F)

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10-10-		10-7-5		10-7-5	1		10-10-11	1
Plate Offsets (X,Y) [2:0-3-6,I	Edge], [8:0-3-6,Edge], [10:0-4-0,0-4-8],	[13:0-4-0,0-4-8]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.87 BC 0.99 WB 0.95 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.26 12-13 -0.57 12-13 0.18 8	l/defl >999 >904 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 263 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 REACTIONS. (size) 2=0-	3-8. 8=0-3-8	T	SOT CHORD	Structural wood Rigid ceiling dire 1 Row at midpt	ectly app			
Max Horz 2=10 Max Uplift 2=-1 Max Grav 2=17 FORCES. (lb) - Max. Comp./M TOP CHORD 2-3=-4272/307 BOT CHORD 2-13=-286/395	,	4272/308 , 8-10=-204/3959						
 Wind: ASCE 7-10; Vult=120m gable end zone and C-C Exte 43-10-8 zone; cantilever left a reactions shown; Lumber DO TCLL: ASCE 7-10; Pr=20.0 p 	we been considered for this design. hph Vasd=95mph; TCDL=6.0psf; BCDL rior(2) -0-10-8 to 3-5-2, Interior(1) 3-5-2 and right exposed ; end vertical left and L=1.60 plate grip DOL=1.60 sf (roof live load: Lumber DOL=1.15 Plate Plate DOL=1.15); Category II; Exp B;	2 to 21-6-0, Exterior(2) 2 right exposed;C-C for me ate DOL=1.15); Pg=15.0	1-6-0 to 25-9-10, li embers and forces	nterior(1) 25-9-1 s & MWFRS for	0 to			

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

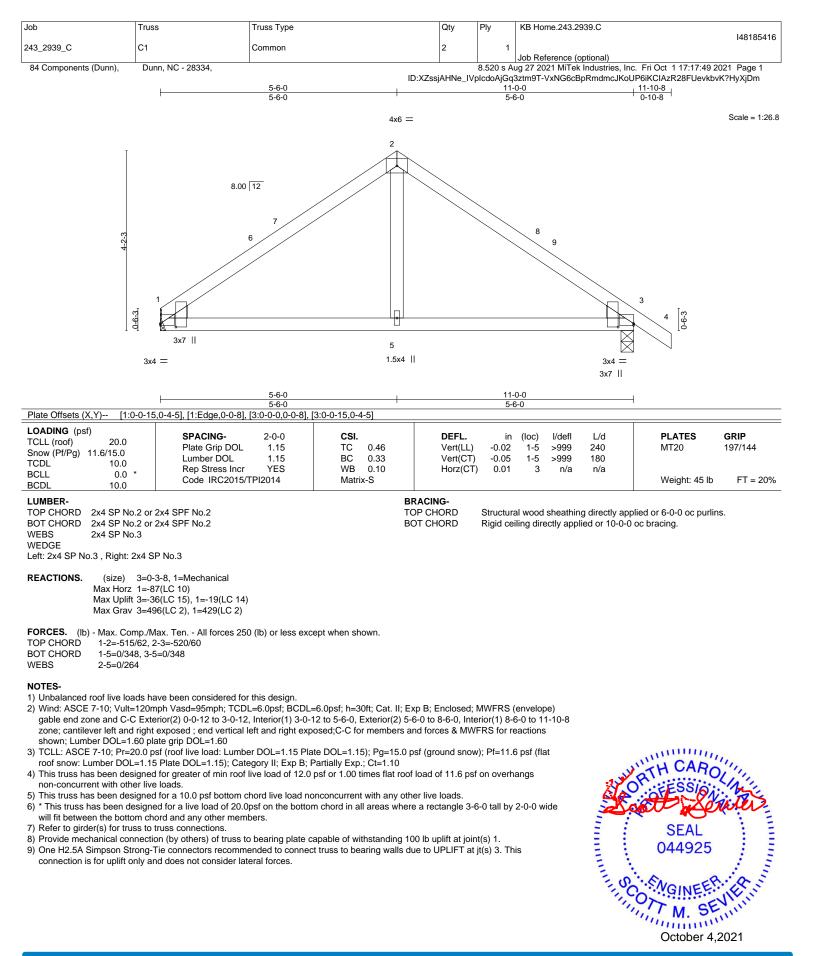
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.



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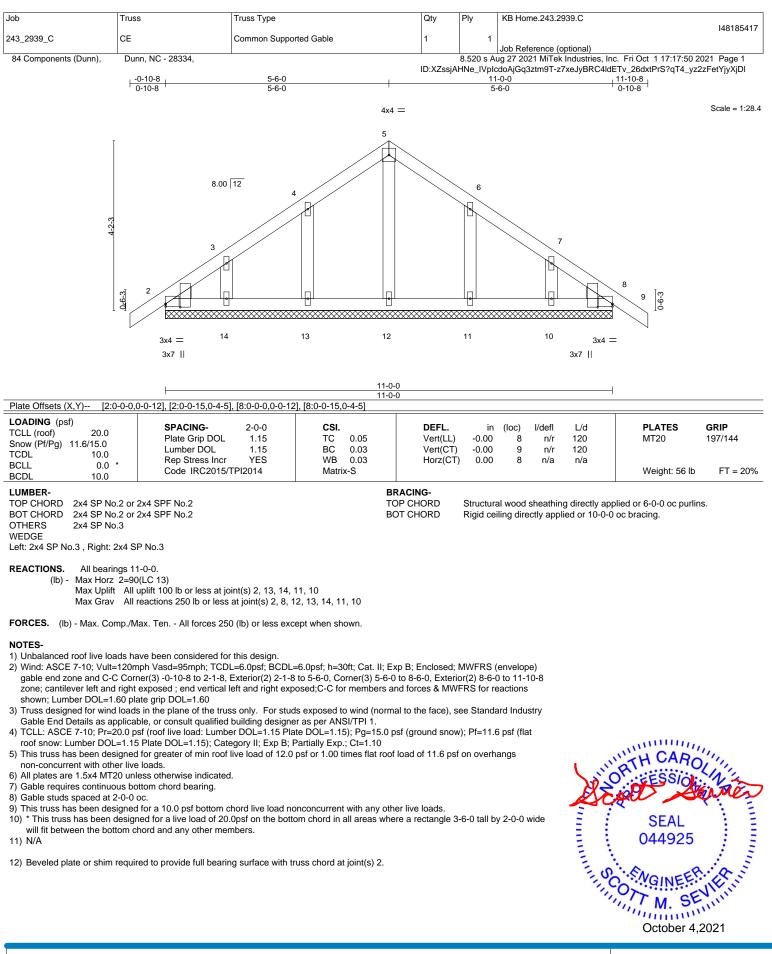
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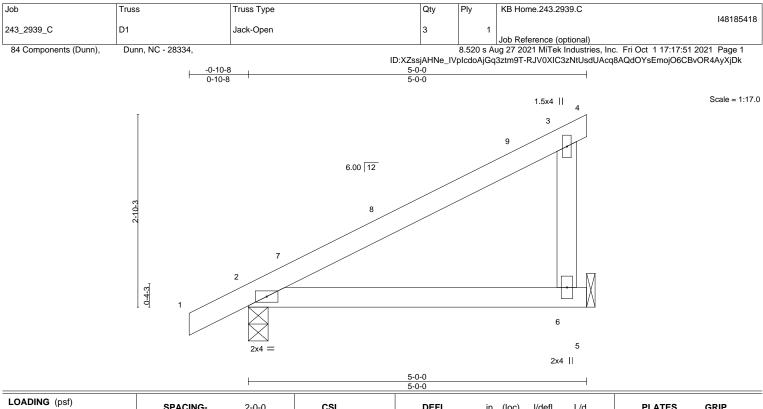
TRENCO A MITek Affiliat 818 Soundside Road

Edenton, NC 27932



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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.37 BC 0.25 WB 0.04	DEFL. in (loc) l/defl L/d Vert(LL) -0.03 2-6 >999 240 Vert(CT) -0.05 2-6 >999 180 Horz(CT) 0.00 n/a n/a	PLATES GRIP MT20 197/144
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 21 lb FT = 20%

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LUMBER-
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 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 6=Mechanical Max Horz 2=94(LC 16) Max Uplift 2=-16(LC 16), 6=-42(LC 16) Max Grav 2=250(LC 2), 6=188(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

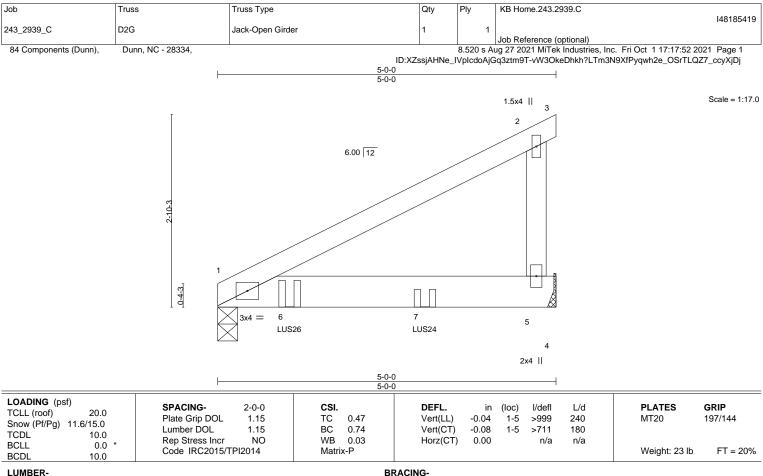
NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
 connection is for uplift only and does not consider lateral forces.
- 9) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces.



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TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 5=Mechanical Max Horz 1=80(LC 12) Max Uplift 1=-35(LC 12), 5=-69(LC 12) Max Grav 1=657(LC 2), 5=538(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat 2)
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members. 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This
- connection is for uplift only and does not consider lateral forces.
- 9) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent at 1-0-12 from the left end to connect truss(es) to front face of bottom chord.
- 10) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent at 3-0-12 from the left end to connect truss(es) to front face of bottom chord.
- 11) Fill all nail holes where hanger is in contact with lumber.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf)

Vert: 1-3=-43, 1-4=-20 Concentrated Loads (lb) Vert: 6=-319(F) 7=-318(F)

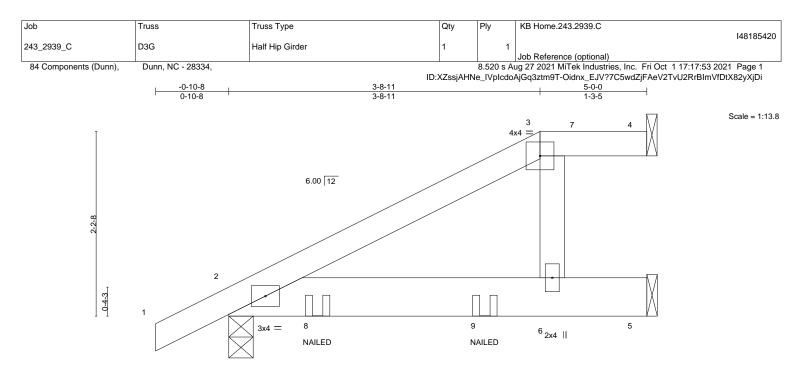


Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



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		<u>3-8-11</u> 3-8-11					4-11-3 1-2-9	<u>5-0-</u> 0 0-0-13	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.29 BC 0.28 WB 0.03 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.02 -0.03 0.01	(loc) 2-6 2-6 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 24 lb	GRIP 197/144 FT = 20%

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x6 SP No.2

2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 5-0-0 oc purlins, except 2-0-0 oc purlins: 3-4.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical Max Horz 2=74(LC 12) Max Uplift 4=-13(LC 8), 2=-66(LC 12), 5=-35(LC 12) Max Grav 4=46(LC 31), 2=375(LC 32), 5=201(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
 One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 15) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 3-4=-53, 2-5=-20

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	KB Home.243.2939.C
					I48185420
243_2939_C	D3G	Half Hip Girder	1	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			8.520 s A	ug 27 2021 MiTek Industries, Inc. Fri Oct 1 17:17:53 2021 Page 2

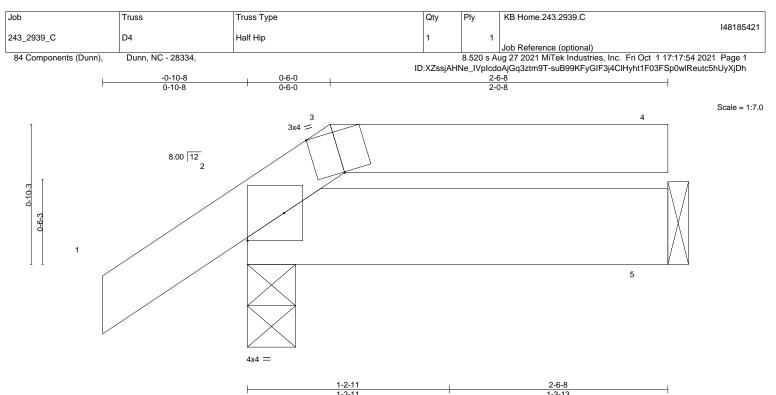
ID:XZssjAHNe_IVpIcdoAjGq3ztm9T-Oidnx_EJV?7C5wdZjFAeV2TvU2RrBImVfDtX82yXjDi

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 8=-60(B) 9=-51(B)

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		1-2-11				1-3-13		
Plate Offsets (X,Y) [2:Edge,0	0-2-0], [3:0-2-0,Edge]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.36 BC 0.16 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.00 -0.00	2-5 >9 2-5 >9	defl L/d 999 240 999 180 n/a n/a	PLATES MT20 Weight: 12 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 1 BOT CHORD 2x6 SP No.2	2x4 SPF No.2		ACING- P CHORD	Structural v 2-0-0 oc pu			pplied or 2-6-8 oc purlir	ns, except

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 5=Mechanical Max Horz 2=29(LC 14) Max Uplift 2=-27(LC 14), 5=-24(LC 10)

Max Grav 2=168(LC 2), 5=91(LC 31)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

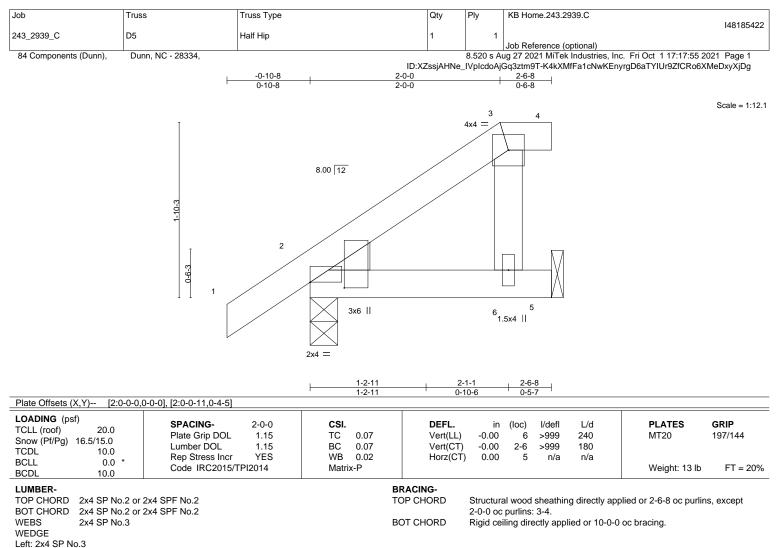
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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REACTIONS. (size) 2=0-3-8, 5=Mechanical

Max Horz 2=62(LC 14) Max Uplift 2=-14(LC 14), 5=-16(LC 14) Max Grav 2=168(LC 2), 5=84(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

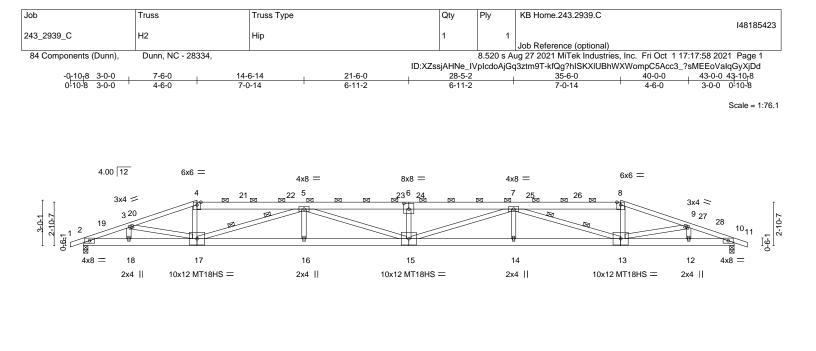
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This
 connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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28 5 2

25.6.0

10.0.0

12.0.0

F		7-6-0 14-6-14	21-6-0	28-5-2			35-6-0		40-0-0 43-0	
		I-6-0 7-0-14	6-11-2	6-11-2			7-0-14	1	4-6-0 3-0-	0 '
Plate Offsets ()	X,Y) [6:0-4-0,0	0-4-8]								
LOADING (psi TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	20.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.91 BC 0.92 WB 0.84 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.74 -1.43 0.21	(loc) 15 15 10	l/defl >696 >358 n/a	L/d 240 180 n/a	PLATES MT20 MT18HS Weight: 264 It	GRIP 197/144 244/190 FT = 20%
LUMBER-				BRACING-						
TOP CHORD	2x4 SP No.2 or 2 4-6,6-8: 2x6 SP	2x4 SPF No.2 *Except* No.2		TOP CHORD	Structura 2-0-0 oc				pplied or 2-2-0 oc purli	ns, except
BOT CHORD	2x6 SP No.2 *Ex	1		BOT CHORD	Rigid cei 2-2-0 oc	0			-0 oc bracing, Except:	
WEBS	2x4 SP No.3			WEBS	2 Rows	at 1/3 p	ts	5-17, 7-	13	
REACTIONS.	Max Horz 2=38 Max Uplift 2=-2	3-8, 10=0-3-8 i(LC 20) 16(LC 12), 10=-216(LC 13) '70(LC 2), 10=1770(LC 2)								
FORCES. (Ib TOP CHORD	2-3=-4099/436	ax. Ten All forces 250 (lb) or less exc 5, 3-4=-4425/481, 4-5=-4204/474, 5-6=-7 4, 8-9=-4425/481, 9-10=-4099/438		/810,						
BOT CHORD	2-18=-407/378	36, 17-18=-407/3786, 16-17=-697/7044, 044, 12-13=-378/3786, 10-12=-378/3786	,	1-15=-667/7044,						
WEBS		0, 5-17=-3014/343, 5-16=0/269, 5-15=-10 0, 7-14=0/269, 7-13=-3014/343, 8-13=-3	,	/						
2) Wind: ASCE	7-10; Vult=120m	ive been considered for this design. hph Vasd=95mph; TCDL=6.0psf; BCDL= rior(2) -0-10-8 to 3-5-2 Interior(1) 3-5-2								<u>.</u>

21.6.0

- gable end zone and C-C Exterior(2) -0-10-8 to 3-5-2, Interior(1) 3-5-2 to 7-6-0, Exterior(2) 7-6-0 to 13-7-0, Interior(1) 13-7-0 to 35-6-0, Exterior(2) 35-6-0 to 41-7-0, Interior(1) 41-7-0 to 43-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.

200

760

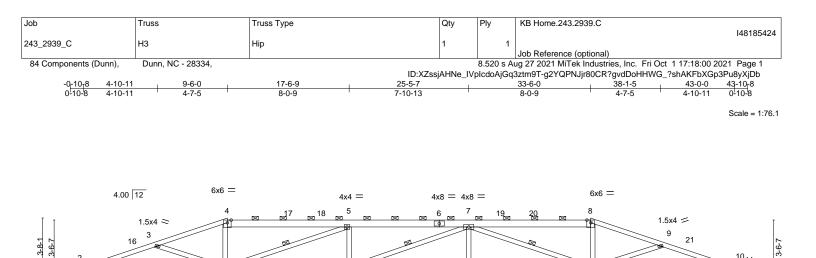
14-6-14

- 7) All plates are MT20 plates unless otherwise indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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14

8x8 =

13

2x4 ||

12

8x12 =

ł	9-6		17-6-9		25-5-			3-6-0		-	43-0-0	
	9-6		8-0-9	- 41 54 4 0 4	7-10-		8	-0-9		I	9-6-0	I
Plate Offsets (· · ·	,0-0-9], [10:0-0-1:	5,0-0-9], [12:0-5-12,0-	o-4], [14:0-4∙	0,0-5-4], [15:	J-5-12,0-5-4]						
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	20.0	SPACING- Plate Grip Lumber DC Rep Stress Code IRC	DOL 1.15 DL 1.15	CSI. TC BC WB Matri	0.72 0.88 0.92 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in () -0.49 13 -0.98 13 0.18	8-14	l/defl >999 >521 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 257 lb	GRIP 197/144 FT = 20%
LUMBER-					BF	ACING-						
TOP CHORD	2x4 SP No.2 or 2	2x4 SPF No.2 *Ex	<cept*< td=""><td></td><td>тс</td><td>P CHORD</td><td>Structural v</td><td>wood s</td><td>heathin</td><td>g directly a</td><td>pplied or 2-3-5 oc purlins</td><td>, except</td></cept*<>		тс	P CHORD	Structural v	wood s	heathin	g directly a	pplied or 2-3-5 oc purlins	, except
	4-6,6-8: 2x6 SP						2-0-0 oc pu					
BOT CHORD	2x6 SP No.2 *Ex					T CHORD			tly appl		-0 oc bracing.	
WEBS	14-15,12-14: 2x6 2x4 SP No.3	SP DSS			VVI	EBS	1 Row at m	nidpt		5-15, 7-	14, 7-12	
REACTIONS.		,										
FORCES. (It TOP CHORD	2-3=-4343/516		es 250 (lb) or less exc 4-5=-3942/428, 5-7=-\$									
BOT CHORD WEBS		, 4-15=-5/917, 5-	93, 13-14=-533/5906, 15=-2209/285, 5-14=0		,							
2) Wind: ASCE gable end z , Exterior(2)	E 7-10; Vult=120m one and C-C Exte 33-6-0 to 39-7-0,	ph Vasd=95mph; rior(2) -0-10-8 to Interior(1) 39-7-0	red for this design. ; TCDL=6.0psf; BCDL= 3-5-2, Interior(1) 3-5-2 to 43-10-8 zone; cant RS for reactions showr	to 9-6-0, Ex ilever left an	terior(2) 9-6-0 d right expose	to 15-7-0, Inter ed ; end vertical	ior(1) 15-7-	0 to 33				11.

.60 plate grip DOL=1.60 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

ų į

4x8

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

15

8x12 =

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



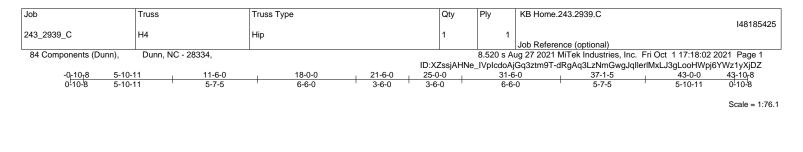
818 Soundside Road Edenton, NC 27932

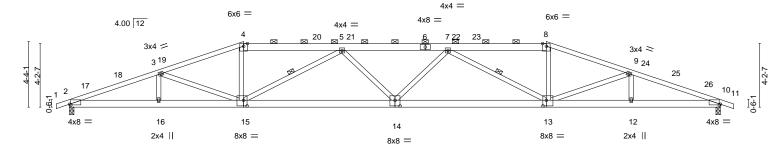
¹⁰11

4x8

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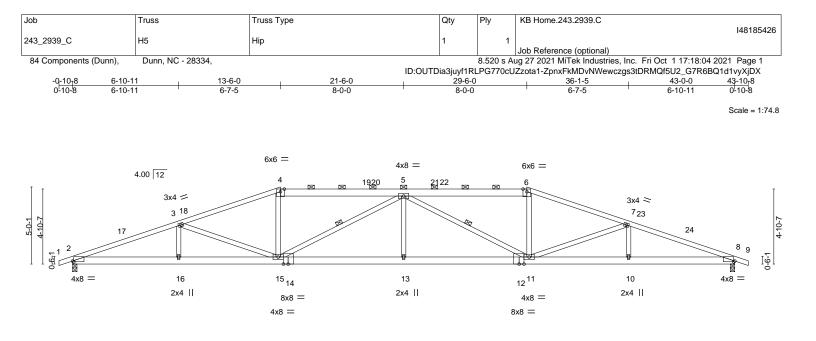


5-10-11	11-6-0	21-6-0		1-6-0		37-1-5	43-0-0	_
Plate Offsets (X,Y) [2:0-0-1	<u> </u>	10-0-0 -4-8] [14:0-4-0 0-4-8] [15		0-0-0		5-7-5	5-10-11	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.82 BC 0.93 WB 0.44	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.41 14 -0.83 14-15 0.19 10	l/defl >999 >617 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 254 lb	FT = 20%
LUMBER-		B	RACING-					
TOP CHORD 2x4 SP No.2 or	2x4 SPF No 2 *Except*		OP CHORD	Structural wood	sheathin	a directly app	blied or 2-2-0 oc purlins	except
4-6,6-8: 2x6 SF				2-0-0 oc purlins				, encopt
BOT CHORD 2x6 SP No.2		В	OT CHORD	Rigid ceiling dire	·	,	oc bracing.	
WEBS 2x4 SP No.3		N	/EBS	1 Row at midpt		5-15, 7-13	3	
Max Horz 2=5 Max Uplift 10=	0-3-8, 2=0-3-8 7(LC 20) -206(LC 13), 2=-206(LC 12) 1770(LC 2), 2=1770(LC 2)							
TOP CHORD 2-3=-4376/45	Max. Ten All forces 250 (lb) or less ex 0, 3-4=-3980/413, 4-5=-3749/410, 5-7= 3, 9-10=-4376/450),					
	60, 15-16=-415/4060, 14-15=-447/4747	7, 13-14=-416/4747, 12-13	8=-371/4060,					
WEBS 3-15=-510/14	8, 4-15=-14/898, 5-15=-1303/217, 5-14 16, 8-13=-14/898, 9-13=-510/149	=0/284, 7-14=0/284,						
NOTES-								
 Wind: ASCE 7-10; Vult=120, gable end zone and C-C Ext 31-6-0, Exterior(2) 31-6-0 to exposed;C-C for members a TCLL: ASCE 7-10; Pr=20.0, roof snow: Lumber DOL=1.1 governs. Rain surcharge ap Unbalanced snow loads hav This truss has been designe non-concurrent with other liv Provide adequate drainage t This truss has been designe will fit between the bottom cl One H2.5A Simpson Strong-connection is for uplift only a 		2 to 11-6-0, Exterior(2) 11 he; cantilever left and righ n; Lumber DOL=1.60 platt ate DOL=1.60 platt Partially Exp.; Ct=1.10, Li s less than 0.500/12 in ac 0 psf or 1.00 times flat roo nonconcurrent with any oth om chord in all areas when ct truss to bearing walls d	-6-0 to 17-7-0, in t exposed ; end v te grip DOL=1.60 psf (ground snow u=50-0-0; Min. fla cordance with IBi f load of 11.6 psf her live loads. re a rectangle 3-6 ue to UPLIFT at j	terior(1) 17-7-0 t retrical left and rig); Pf=16.5 psf (fla tt roof snow load C 1608.3.4. on overhangs -0 tall by 2-0-0 w t(s) 10 and 2. Th	o´ ght at vide	Street Street	SEAL 044925	North Manual Contraction

October 4,2021

ENGINEERING BY EREPACED A MITek Affiliate 818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



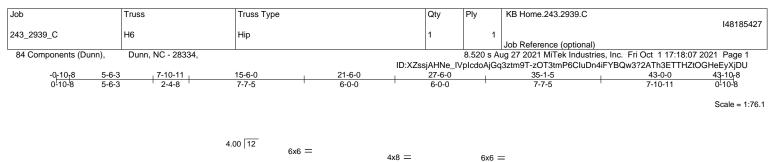
6-10-11	13-6-0	21-6-0	29-6-0			36-1	-5	43-0-0	1
6-10-11	6-7-5	8-0-0	8-0-0		1	6-7	-5	6-10-11	
Plate Offsets (X,Y) [2:0-0-15	5,0-0-9], [8:0-0-15,0-0-9], [12:0-3-12,0-0	-0], [14:0-3-12,0-0-0]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.78	DEFL. Vert(LL)	in -0.34	(loc) 13	l/defl >999	L/d 240	PLATES MT20	GRIP 244/190
TCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.88 WB 0.67	Vert(CT) Horz(CT)	-0.69 0.18	13 8	>740 n/a	180 n/a		
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	1012(01)	0.10	0	n/a	n/a	Weight: 253 lb	FT = 20%
LUMBER- 10.0 TOP CHORD 2x4 SP No.1 *E: 4-6: 2x6 SP No. 80T CHORD BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3		T	BOT CHORD	2-0-0 oc	purlins ling dire	(3-6-2 m	ax.): 4-6.	pplied or 2-2-0 oc purlins 0 oc bracing. 1	, except
	,								
TOP CHORD 2-3=-4381/430 7-8=-4381/430		3505/386, 6-7=-3764/38	,						
8-10=-346/40	9, 4-15=0/766, 5-15=-988/154, 5-13=0/3								
 Wind: ASCE 7-10; Vult=120n gable end zone and C-C Exte 29-6-0, Exterior(2) 29-6-0 to 3 exposed;C-C for members ar TCLL: ASCE 7-10; Pr=20.0 p roof snow: Lumber DOL=1.15 governs. Rain surcharge app Unbalanced snow loads have This truss has been designed non-concurrent with other live Provide adequate drainage to 7) This truss has been designed will fit between the bottom ch One H2.5A Simpson Strong-connection is for uplift only ar 	o prevent water ponding. I for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the botto	2 to 13-6-0, Exterior(2) 1: te; cantilever left and right the; cantilever left and right the; DOL=1.60 plate te DOL=1.15); Pg=15.0 Partially Exp.; Ct=1.10, L te less than 0.500/12 in act psf or 1.00 times flat room conconcurrent with any ot m chord in all areas whe ct truss to bearing walls of	3-6-0 to 19-7-0, in ht exposed ; end v itte grip DOL=1.60 psf (ground snow); u=50-0-0; Min. fla ccordance with IBC of load of 11.6 psf ther live loads. ite a rectangle 3-6 due to UPLIFT at j	terior(1) 1 ertical left); Pf=16.5 t roof snc C 1608.3. on overha -0 tall by t(s) 2 and	19-7-0 t t and rig 5 psf (fla w load 4. angs 2-0-0 w I 8. This	oʻ ght at		SEAL 044925	With Community

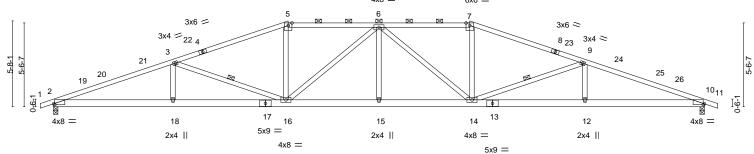
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



818 Soundside Road Edenton, NC 27932

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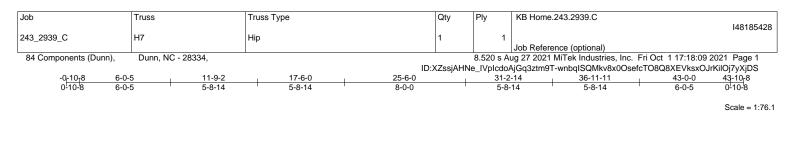


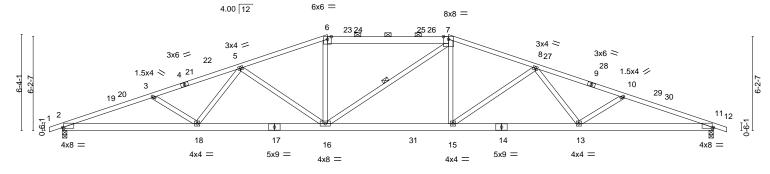
7-10		21-6-0	27-6-0		35-1-5	43-0-0	
7-10 Plate Offsets (X,Y) [2:0-0-	<u>11 ' 7-7-5</u> 15,0-0-9], [10:0-0-15,0-0-9]	6-0-0	6-0-0		7-7-5	7-10-11	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.77 BC 0.91	DEFL. Vert(LL) Vert(CT)		>844 180	PLATES MT20	GRIP 197/144
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.73 Matrix-S	Horz(CT)	0.17 10	n/a n/a	Weight: 244 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP DSS	Except* lo.2 or 2x4 SPF No.2	T ⁱ B	RACING- OP CHORD OT CHORD /EBS	2-0-0 oc purlin:	s (2-6-11 max.): 5-7 rectly applied or 10)-0-0 oc bracing.	s, except
Max Horz 2= Max Uplift 2=	0-3-8, 10=0-3-8 -77(LC 21) -191(LC 12), 10=-191(LC 13) 1770(LC 2), 10=1770(LC 2)						
TOP CHORD 2-3=-4358/4 9-10=-4358 BOT CHORD 2-18=-375/4 10-12=-317 WEBS 3-18=0/322	039, 16-18=-375/4039, 15-16=-233/3565	3239/348, 7-9=-3505/341 . 14-15=-233/3565, 12-14	4=-317/4039,				
 2) Wind: ASCE 7-10; Vult=12 gable end zone and C-C E 27-6-0, Exterior(2) 27-6-0 t exposed;C-C for members 3) TCLL: ASCE 7-10; Pr=20.0 roof snow: Lumber DOL=1 governs. Rain surcharge a 4) Unbalanced snow loads ha 5) This truss has been design non-concurrent with other I 6) Provide adequate drainage 7) This truss has been design will fit between the bottom 9) One H2.5A Simpson Stron connection is for uplift only 		2 to 15-6-0, Exterior(2) 15 e; cantilever left and righ n; Lumber DOL=1.60 platit te DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10, Li eless than 0.500/12 in ac psf or 1.00 times flat roo ponconcurrent with any oth m chord in all areas when ct truss to bearing walls d	5-6-0 to 21-6-0, ir t exposed ; end v te grip DOL=1.60 psf (ground snow u=50-0-0; Min. fla cordance with IB f load of 11.6 psf ner live loads. re a rectangle 3-6 lue to UPLIFT at	hterior(1) 21-6-0 vertical left and r r); Pf=16.5 psf (f at roof snow load C 1608.3.4. on overhangs 6-0 tall by 2-0-0 jt(s) 2 and 10. T	to ight lat d	SEAL 044925	A CONTRACT OF THE PARTY OF THE

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



October 4,2021



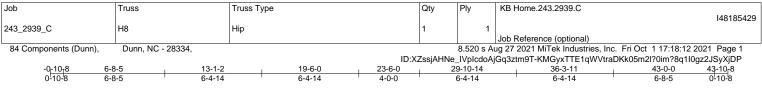


	8-10-1		17-6-0			25-6-0			34-1-5			43-0-0	
Plate Offsets	<u>8-10-1</u>	<u>1</u> D-0-9], [11:0-0-15,0-0-9]	8-7-5	1		8-0-0			8-7-5		-	8-10-11	
		5-0-9], [11.0-0-13,0-0-9]											
LOADING (p TCLL (roof) Snow (Pf/Pg) TCDL	20.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.76 0.90 0.69		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.31 -0.64 0.16	(loc) 15 15-16 11	l/defl >999 >806 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCLL BCDL	0.0 * 10.0	Code IRC2015/TP		Matri				0.16		n/a	n/a	Weight: 252 lb	FT = 20%
LUMBER- TOP CHORD		x4 SPF No.2 *Except*					CHORD	2-0-0 oc	purlins iling dire	(3-3-1 m ectly app	ax.): 6-7.	applied or 2-2-0 oc purlins 0-0 oc bracing.	i, except
REACTIONS.	Max Horz 2=86(Max Uplift 2=-18	,											
FORCES. (I TOP CHORD BOT CHORD WEBS	2-3=-4358/414, 8-10=-4079/344 2-18=-402/4043 3-18=-295/163,	x. Ten All forces 250 (3-5=-4078/344, 5-6=-32 4, 10-11=-4358/414 8, 16-18=-290/3584, 15-1 5-18=0/473, 5-16=-756/ 8-13=0/475, 10-13=-295	257/316, 6-7=-3 16=-150/3039, 161, 6-16=0/6	3041/325, 7- 13-15=-237	8=-3256/3 /3584, 11	-13=-33							
 Wind: ASC gable end: 25-6-0, Ext exposed;C TCLL: ASC roof snow: governs. F Unbalance This truss I non-concu Provide ad This truss I 8) * This truss I will fit betw One H2.5A connection 	E 7-10; Vult=120mp zone and C-C Exter terior(2) 25-6-0 to 31 i-C for members and CE 7-10; Pr=20.0 psi Lumber DOL=1.15 Rain surcharge appli d snow loads have I has been designed f rent with other live I lequate drainage to has been designed f s has been designed es has been designed reen the bottom chou A Simpson Strong-Ti h is for uplift only and	ve been considered for the bh Vasd=95mph; TCDL= ior(2) -0-10-8 to 3-5-2, Ir I-7-0, Interior(1) 31-7-0 t I forces & MWFRS for re f (roof live load: Lumber Plate DOL=1.15); Categ ied to all exposed surfac been considered for this for greater of min roof liv loads. prevent water ponding. for a 10.0 psf bottom cho d for a live load of 20.0ps rd and any other membe e connectors recommen d does not consider later on does not depict the siz	6.0psf; BCDL: tterior(1) 3-5-2 o 43-10-8 zon pactions showr DOL=1.15 Pla ory II; Exp B; F es with slopes design. e load of 12.0 ord live load nc ef on the botton ors, with BCDL ded to connec al forces.	to 17-6-0, E e; cantilever b; Lumber D te DOL=1.1 Partially Exp less than 0. psf or 1.00 t enconcurrent n chord in a = 10.0psf. t truss to be	Exterior(2) left and ri DL=1.60 p 5); Pg=15 ;; Ct=1.10 500/12 in imes flat r t with any II areas will aring wall	17-6-0 ght exp late gri .0 psf (g , Lu=50 accord oof load other lin here a r s due to	to 23-7-0, Int vosed ; end ve p DOL=1.60)-0-0; Min. flat ance with IBC d of 11.6 psf of ve loads. rectangle 3-6- b UPLIFT at jt	erior(1) 2 eritical lef ; Pf=16.5 t roof snc 2 1608.3 on overh -0 tall by c(s) 2 and	23-7-0 t t and rig 5 psf (fla bw load .4. angs 2-0-0 w 1 11. Th	o ght at vide	and the second s	SEAL 044925	A CONTRACT OF THE STATE

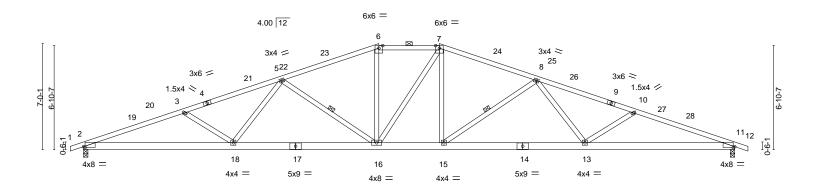
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TRENGINEERING BY A MiTek Affiliate 818 Soundside Road Edenton, NC 27932

October 4,2021



Scale = 1:76.1



	9-10-11	19-6-		3-6-0	33-1-5		43-0-0	
Plate Offsets (<u>9-10-11</u> (X,Y) [2:0-1-3,0-0-9], [11:0	9-7-	5 4	-0-0	9-7-5	I	9-10-11	I
		-1-3,0-0-9]						
LOADING (ps TCLL (roof) Snow (Pf/Pg)	20.0 Plate	CING- 2-0-0 © Grip DOL 1.15 ber DOL 1.15	CSI. TC 0.91 BC 0.94	DEFL. Vert(LL) Vert(CT)	in (loc) -0.33 13-15 -0.67 13-15	l/defl L/d >999 240 >760 180	PLATES MT20	GRIP 197/144
TCDL BCLL BCDL	10.0 Rep	Stress Incr YES e IRC2015/TPI2014	WB 0.31 Matrix-S	Horz(CT)		n/a n/a	Weight: 249 lb	FT = 20%
LUMBER- TOP CHORD	2x4 SP No.2 or 2x4 SPF No	b.2		BRACING- TOP CHORD		sheathing directly a		
	2x6 SP No.2					(3-3-13 max.): 6-7.		
WEBS	2x4 SP No.3			BOT CHORD WEBS	Rigid ceiling dire 1 Row at midpt	ectly applied or 2-2- 5-16. 8	U	
REACTIONS.	(size) 2=0-3-8, 11=0-3 Max Horz 2=96(LC 16) Max Uplift 2=-172(LC 12), Max Grav 2=1770(LC 2), 1	11=-172(LC 13)			r row at mapt	0 10, 0		
FORCES. (III TOP CHORD BOT CHORD WEBS	8-10=-4080/305, 10-11=-4 2-18=-378/4067, 16-18=-2	9/305, 5-6=-3024/281, 6-7=- 4365/385 252/3533, 15-16=-98/2785, 75, 5-16=-900/180, 6-16=-3/	2787/292, 7-8=-3022/28 13-15=-196/3533, 11-13	3=-305/4068				
 Wind: ASCI gable end z 43-10-8 zor reactions sl TCLL: ASC roof snow: I governs. R Unbalancee This truss h Provide add This truss h 	d roof live loads have been cc E 7-10; Vult=120mph Vasd=9 cone and C-C Exterior(2) -0-1 ne; cantilever left and right ex nown; Lumber DOL=1.60 plat E 7-10; Pr=20.0 psf (roof live Lumber DOL=1.15 Plate DOL ain surcharge applied to all e d snow loads have been cons has been designed for greater rent with other live loads. equate drainage to prevent wa has been designed for a 10.0 has been designed for a live	5mph; TCDL=6.0psf; BCDL D-8 to 3-5-2, Interior(1) 3-5-2 posed ; end vertical left and e grip DOL=1.60 load: Lumber DOL=1.15 Pla =1.15); Category II; Exp B; xposed surfaces with slopes idered for this design. of min roof live load of 12.0 ater ponding. psf bottom chord live load of	2 to 19-6-0, Exterior(2) 1 right exposed;C-C for n ate DOL=1.15); Pg=15.0 Partially Exp.; Ct=1.10, less than 0.500/12 in a psf or 1.00 times flat ro ponconcurrent with any o	19-6-0 to 29-7-0, In nembers and force 0 psf (ground snow Lu=50-0-0; Min. fla accordance with IB4 pof load of 11.6 psf other live loads.	terior(1) 29-7-0 tr s & MWFRS for); Pf=16.5 psf (fla tr toof snow load C 1608.3.4. on overhangs	oʻ at	CONTH CARO SEAL	
9) One H2.5A connection	een the bottom chord and any Simpson Strong-Tie connect is for uplift only and does not purlin representation does not	ors recommended to connect consider lateral forces.	Ū		.,		044925	10



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Job	г	russ	Truss Type		Qty	Ply	KB Home.243.2	2939.C]
243_2939_C		łG	Hip Girder		1					I48185430
84 Components (E		Dunn, NC - 28334,			i	2	Job Reference (c. Fri Oct 1 17:18:21 20	21 Dogo 1
						_IVpIcdoA		pYZuvbfESEm	xM7gCdydXlKuXlnnc5a	08QyXjDG
	5-6-0 5-6-0	<u>+ 11-11-14</u> 6-5-14	18-3-15 6-4-2		-8-1 4-2	<u>31-0-2</u> 6-4-2		37-6-0 6-5-14	43-0-0 5-6-0	43-10 ₁ 8 0-10-8
										Scale = 1:73.2
	-	NAIL		NAILED NAILED NA	ILED NAILED NAILEI	כ	NAILED		NAILED	
4.00 12	8x		x4 NAILED		x8 = 2x4		NAILED $5x12 = 1$		6x6 =	
			4 00 00 04	⊴ ^{25 5} 26 27 ⊠	6 28 ⁷ 29	30 31	32 ⁸ 33	3 <u>4</u> 35	9 36	
						\square		n n		
4x8 =		10	18 40 41 17	40	45		40 13 50		10	4x8 =
470	37 NAILED	38 39	40 - 17	42 ¹⁶ 43 44 8HS = NAILED NA	40 47	14 48	49 00 2ve II	51 52	53 6ve —	1X0 —
	INAILED	NAILED NAILED	ED NAILED NAILED	3x6		2 MT18HS		NAILED NAILEL	NAILED NAILED	
				NAILED	NAILEI					
	5-6-0 5-6-0	+ <u>11-11-14</u> 6-5-14	+ <u>18-3-15</u> 6-4-2		-8-1 4-2	<u>31-0-2</u> 6-4-2		37-6-0 6-5-14	43-0-0	
Plate Offsets (X,Y		-0,0-2-6], [8:0-2-12,0-2-4],			4-2	0-4-2		0-0-14	5-6-0	
LOADING (psf)		SPACING-	2-0-0	CSI.	DEFL.	in	(loc) l/defl	L/d	PLATES	GRIP
TCLL (roof) Snow (Pf/Pg) 16	20.0 6.5/15.0	Plate Grip DOL	1.15	TC 0.91	Vert(LL)	-0.91	15-16 >565	240	MT20	197/144
TCDL	10.0	Lumber DOL Rep Stress Incr	1.15 NO	BC 0.66 WB 0.79	Vert(CT Horz(CT	·	15-16 >280 10 n/a	180 n/a	MT18HS	244/190
BCLL BCDL	0.0 * 10.0	Code IRC2015/	TPI2014	Matrix-S					Weight: 572 lb	FT = 20%
LUMBER-					BRACING-					
	2x4 SP No.2 3-6,6-9: 2x6	or 2x4 SPF No.2 *Except* SP No 2			TOP CHORD	Structur except	al wood sheathii	ng directly app	blied or 3-11-0 oc purlir	IS,
BOT CHORD 2	2x8 SP DSS					2-0-0 oc	purlins (2-6-11			
	2x4 SP No.3 3-18,8-12: 2:	*Except* k4 SP No.2 or 2x4 SPF No	.2		BOT CHORD WEBS	Rigid ce 1 Row a	iling directly app t midpt	8-12 8-12	oc bracing.	
N	Max Horz 2 Max Uplift 2	=0-3-8, 10=0-3-8 =30(LC 62) =-461(LC 8), 10=-461(LC =2999(LC 2), 10=3004(LC								
		o./Max. Ten All forces 25 1197, 3-4=-13799/2093, 4			8=-16846/2540.					
	8-9=-7526	/1129, 9-10=-8191/1190								
		2/7690, 18-19=-1107/7657 98/14259, 12-13=-2098/14			432,					
), 3-18=-1002/6446, 4-18= 107, 7-15=-585/259, 8-15=								
	9-12=-191/		111/2/10,010-0	010, 012-1020,	1000,					
NOTES-										
		d together with 10d (0.131 follows: 2x4 - 1 row at 0-9			~~					
Bottom chords	s connected	as follows: 2x8 - 2 rows st			00.				WHY CAR	11,
		vs: 2x4 - 1 row at 0-9-0 oc. qually applied to all plies,	except if noted as fr	ont (F) or back (B)	face in the LOAD (CASE(S) s	ection. Plv to	J.	R	JAN.
ply connections	ns have bee	n provided to distribute onl	y loads noted as (F)			, (OL(O) 0		112	SEESSION	The
		s have been considered fo 20mph Vasd=95mph; TCD		0psf; h=30ft; Cat. I	I; Exp B; Enclosed;	MWFRS	(envelope)	Her.	an Le	me
gable end zone	e; cantileve	r left and right exposed ; e	nd vertical left and r	ight exposed; Lum	ber DOL=1.60 plate	e grip DOL	=1.60	- E - E	SEAL	1 E
		0 psf (roof live load: Lumb 1.15 Plate DOL=1.15); Cat					applied to all		044925	- 1 E
		pes less than 0.500/12 in a ave been considered for the		2 1608.3.4.				Contraction of the second s		1 3
7) This truss has	been desig	ned for greater of min roof		f or 1.00 times flat	roof load of 11.6 ps	f on overh	angs	5.0	NGINEER	A.S.
non-concurrent 8) Provide adequa		live loads. e to prevent water ponding	1.					14	OTTOF	Main
		unless otherwise indicate gned for a 10.0 psf bottom		concurrent with or	w other live loads				M. SV	111
11) * This truss ha	nas been de	signed for a live load of 20	.0psf on the bottom			3-6-0 tall b	y 2-0-0 wide		October 4,2	021
Contivilletit betwee	an 2 he botto	m chord and any other me	nbers.					1		
		parameters and READ NOTES ON MiTek® connectors. This design							ENGINEERING BY	
a truss system.	 Before use, tl 	The building designer must verify the cated is to prevent buckling of indi	e applicability of design p	parameters and properly	y incorporate this design	into the over	all		REN	JU
is always requi	ired for stability	and to prevent collapse with pos erection and bracing of trusses a	sible personal injury and	property damage. For		ing the	-		A MiTek	Affiliate
		e from Truss Plate Institute, 2670		3 Waldorf, MD 20601	,		3ponom		818 Soundside Road Edenton, NC 27932	

Job	Truss	Truss Type	Qty	Ply	KB Home.243.2939.C
					148185430
243_2939_C	HG	Hip Girder	1	2	
				2	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			8.520 s A	ug 27 2021 MiTek Industries, Inc. Fri Oct 1 17:18:22 2021 Page 2
		ID:XZs	siAHNe IV	/plcdoAiGc	3ztm9T-1Htk1uaWgvn54NL8wrCRA9AiVkEmUE0mKEOggtvXiDF

NOTES-

- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 15) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 112 lb down and 77 lb up at 5-5-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

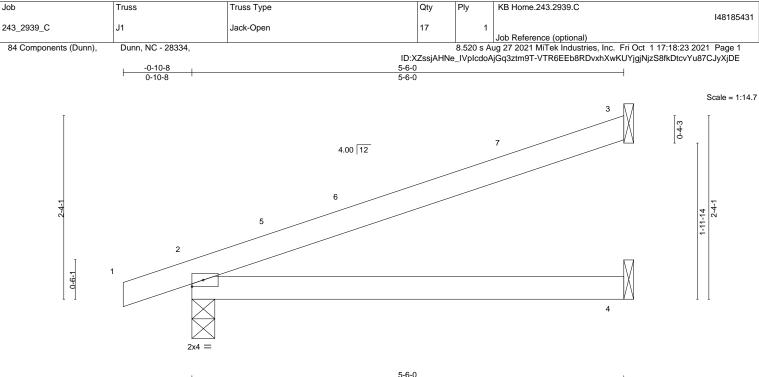
- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf) Vert: 1-3=-43, 3-9=-53, 9-11=-43, 2-10=-20

Concentrated Loads (lb)

Vert: 3=-67(B) 6=-62(B) 9=-67(B) 19=-33(B) 12=-33(B) 20=-62(B) 21=-62(B) 22=-62(B) 23=-62(B) 24=-62(B) 25=-62(B) 25= 47=-33(B) 48=-33(B) 49=-33(B) 50=-33(B) 51=-33(B) 52=-33(B) 53=-146(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			5-6-0				
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.51 BC 0.35 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.05 2-4 -0.09 2-4 -0.00 3	L/d 240 180 n/a	PLATES MT20 Weight: 18 lb	GRIP 197/144 FT = 20%

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=70(LC 12) Max Uplift 3=-66(LC 16), 2=-47(LC 12)

Max Grav 3=153(LC 2), 2=279(LC 2), 4=106(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

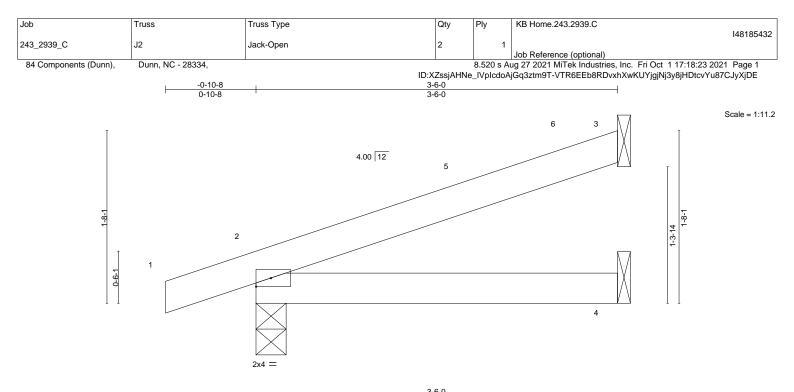
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **MSIVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



			3-6-0						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.16 BC 0.13 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.01 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 12 lb	GRIP 197/144 FT = 20%

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=48(LC 12) Max Uplift 3=-41(LC 16), 2=-43(LC 12)

Max Grav 3=89(LC 2), 2=202(LC 2), 4=66(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

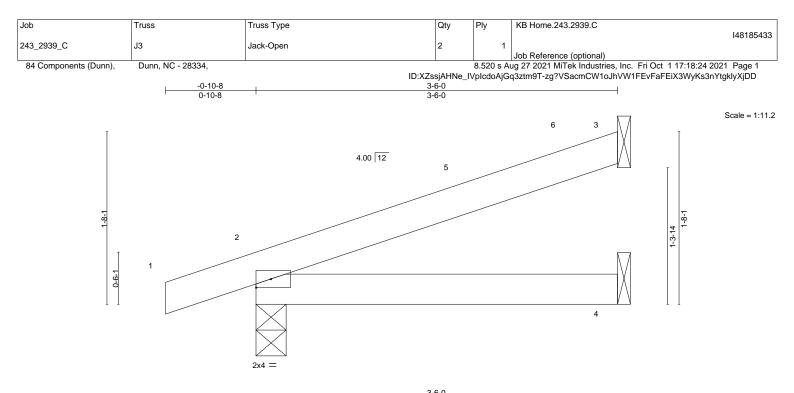
7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **MSIVTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



			3-6-0						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.16 BC 0.13 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.01 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	RACING-					Weight: 12 lb	FT = 20%

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 3=Mechanical, 2=0-3-8, 4=Mechanical (size)

Max Horz 2=48(LC 12) Max Uplift 3=-41(LC 16), 2=-43(LC 12)

Max Grav 3=89(LC 2), 2=202(LC 2), 4=66(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-5-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

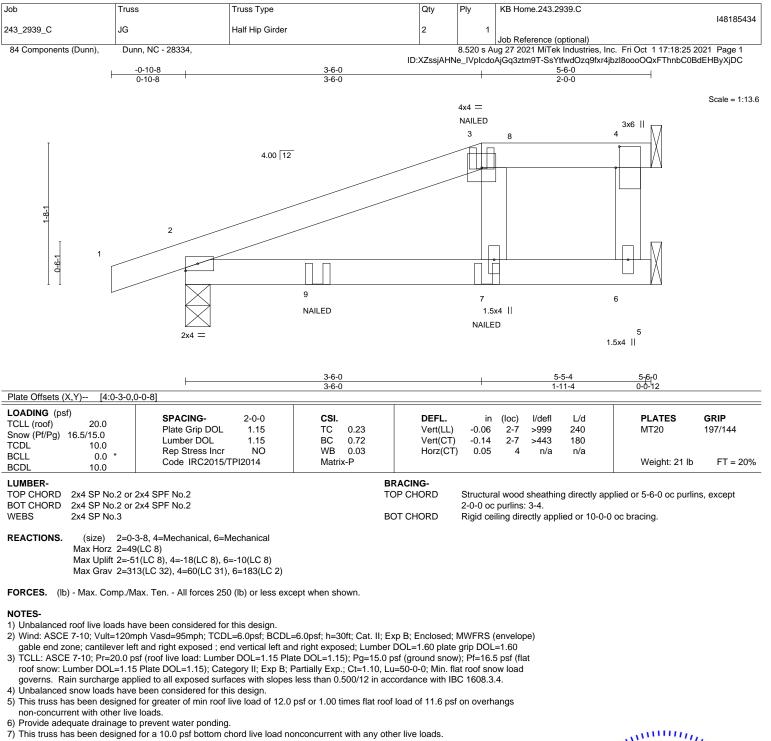
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Refer to girder(s) for truss to truss connections.

10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

11) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4 and 6. This connection is for uplift only and does not consider lateral forces.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

13) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

15) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-43, 3-4=-53, 2-5=-20

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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Job	Truss	Truss Type	Qty	Ply	KB Home.243.2939.C
					148185434
243_2939_C	JG	Half Hip Girder	2	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			8.520 s Ai	ug 27 2021 MiTek Industries, Inc. Fri Oct 1 17:18:26 2021 Page 2

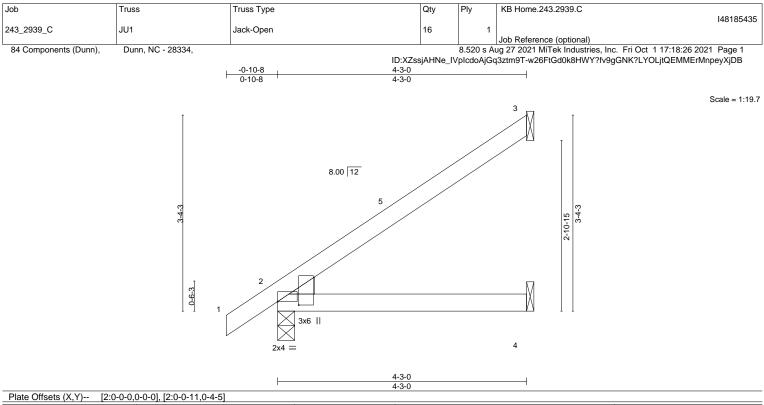
ID:XZssjAHNe_IVpIcdoAjGq3ztm9T-w26FtGd0k8HWY?fv9gGNK?LZALbiQErMErMnpeyXjDB

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 3=-16(F) 7=-13(F) 9=-13(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.28 BC 0.20 WB 0.00	Vert(CT) -	in -0.02 -0.03 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 16 lb	FT = 20%

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEDGE
 2x4 SP No.2 or 2x4 SPF No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-3-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Left: 2x4 SP No.3

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=109(LC 14) Max Uplift 3=-76(LC 14), 2=-1(LC 14) Max Grav 3=122(LC 26), 2=231(LC 2), 4=81(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

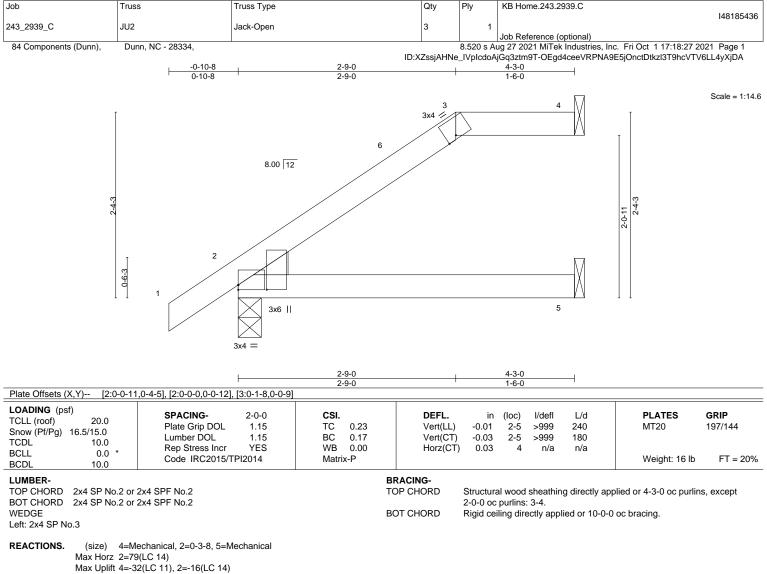
NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-2-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
 connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MSIVTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Max Grav 4=106(LC 2), 2=231(LC 2), 5=76(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

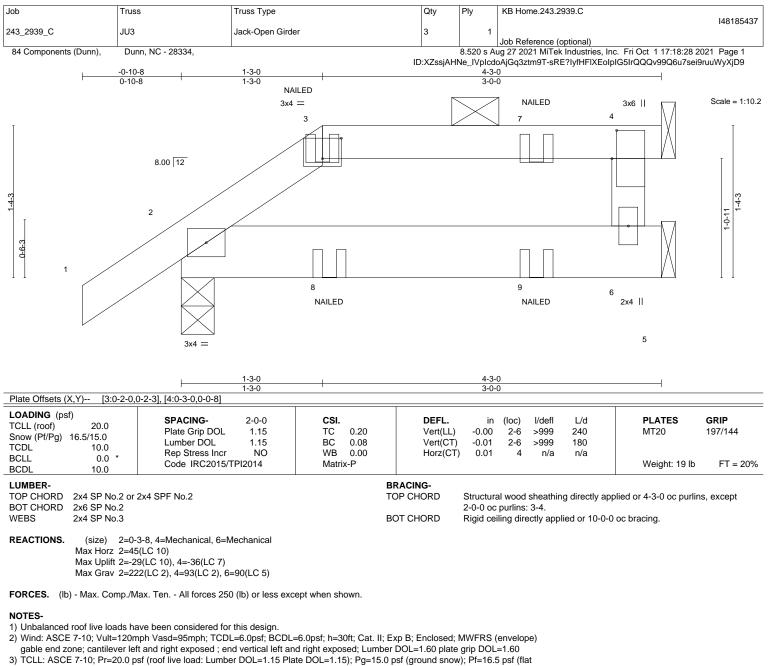
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 2-9-0, Exterior(2) 2-9-0 to 4-2-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 10) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs
- (a) This trust has been designed to greater of min fool live load of 12.0 ps of 1.00 times hat fool load of 11.0 ps of ov non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
 One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This
 connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 15) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

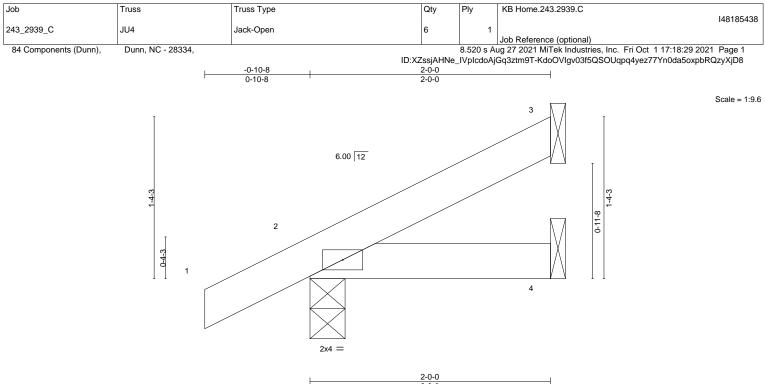
LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-3=-43, 3-4=-53, 2-5=-20

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		1	2-0-0	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.06 BC 0.04 WB 0.00	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.00 2 >999 240 MT20 197/144 Vert(CT) -0.00 2-4 >999 180 Horz(CT) 197/144	
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Weight: 8 lb FT = 20	ე%

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=45(LC 16)

Max Uplift 3=-25(LC 16), 2=-19(LC 16) Max Grav 3=46(LC 2), 2=145(LC 2), 4=39(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

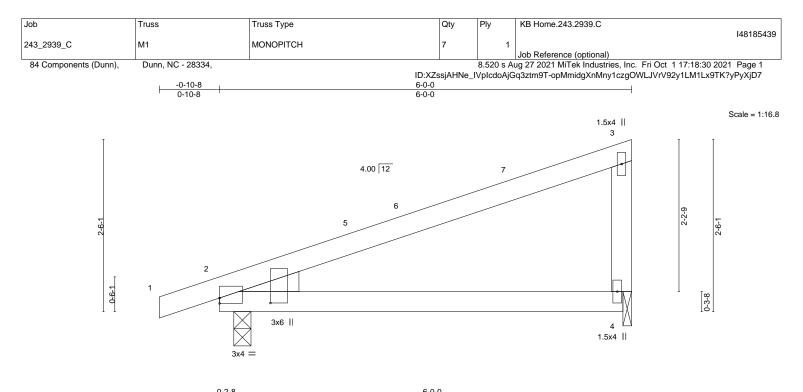


Plate Offsets (X.Y) [2:0-0-0.	<u>0-2-8</u> 0-2-8 0-0-15], [2:0-0-14,0-8-14]		5-9-8					1	
		1							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.62 BC 0.42 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.06 -0.13 0.00	(loc) 2-4 2-4 4	l/defl >999 >532 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 24 lb	FT = 20%
LUMBER-			ACING-						

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEDGE			

Left: 2x4 SP No.3

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=79(LC 13)

Max Uplift 2=-54(LC 12), 4=-31(LC 16) Max Grav 2=294(LC 2), 4=224(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

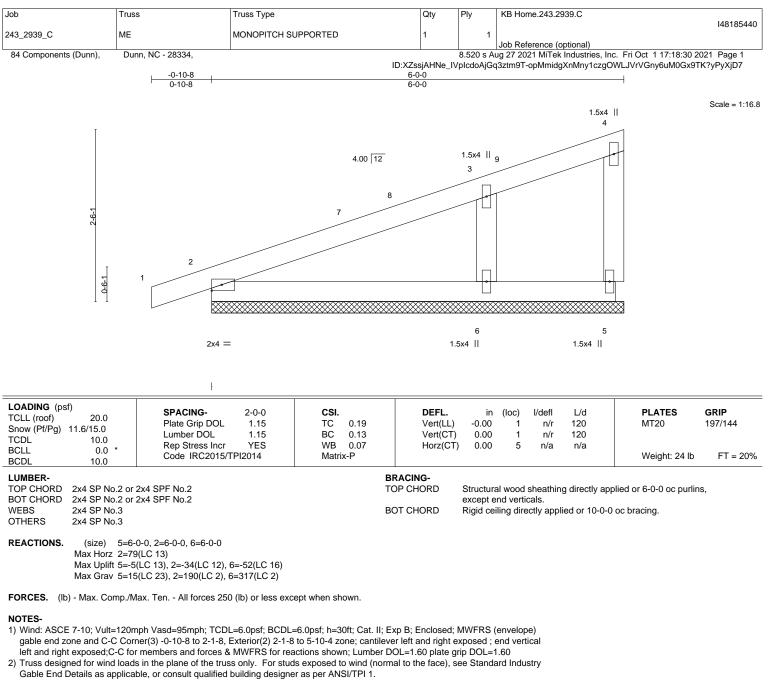
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

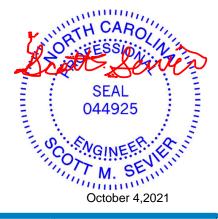


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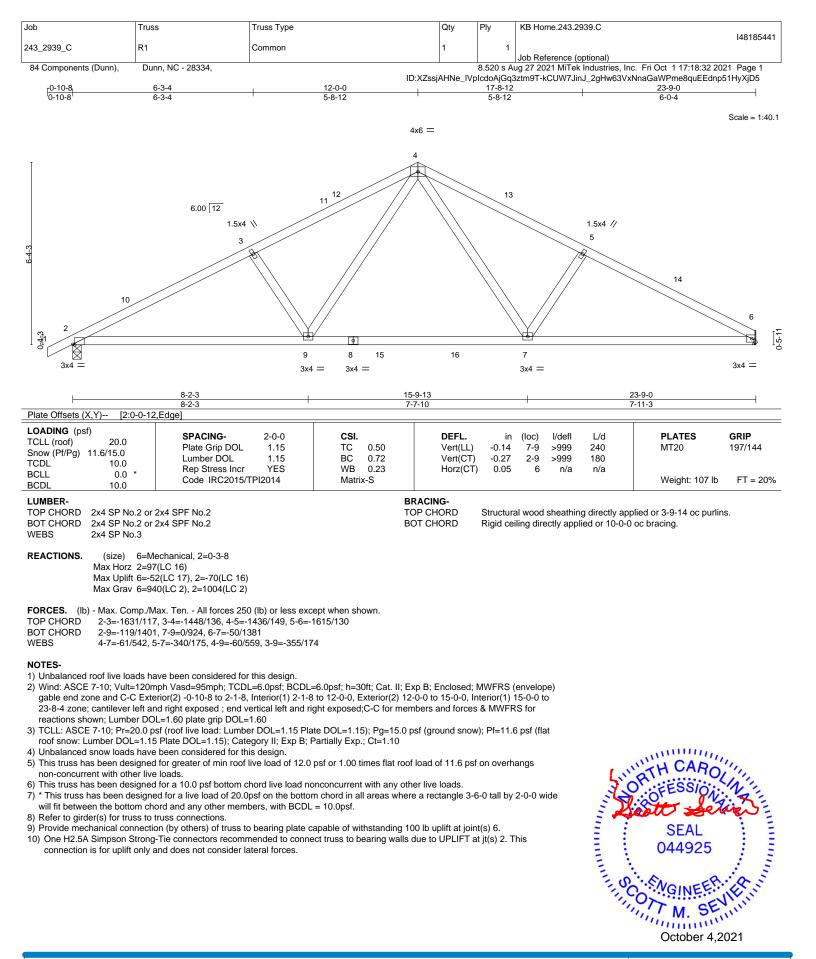


- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



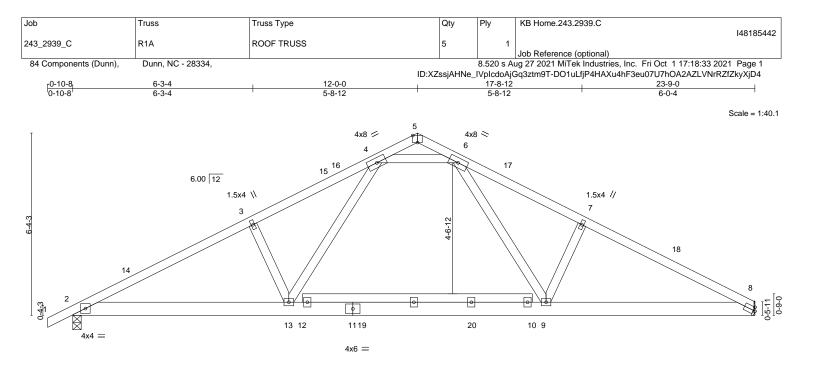
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Edenton, NC 27932

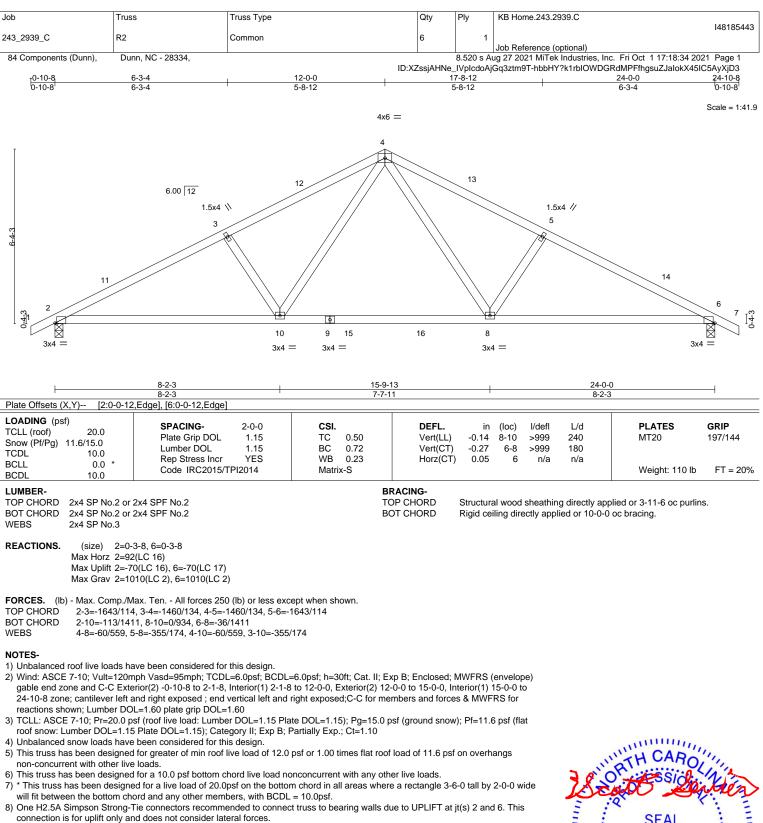


 	7-6-4		-5-12		<u>23-9-0</u> 7-3-4	
Plate Offsets (X,Y) [5:0-2-0,		0-	11-0		7-3-4	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.49 BC 0.41 WB 0.23 Matrix-S	DEFL. in (loc) Vert(LL) -0.09 2-13 Vert(CT) -0.14 2-13 Horz(CT) 0.03 8	>999 240 >999 180	PLATES MT20 Weight: 139 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or BOT CHORD 2x6 SP No.2 *E 10-12: 2x4 SP No.3		TC		d sheathing directly appl irectly applied or 10-0-0 o		
Max Horz 2=98 Max Uplift 8=-5	echanical, 2=0-3-8 3(LC 16) 2(LC 17), 2=-70(LC 16) 40(LC 2), 2=1004(LC 2)					
TOP CHORD 2-3=-1675/104 BOT CHORD 2-13=-97/1424	lax. Ten All forces 250 (lb) or less exc 4, 3-4=-1529/133, 6-7=-1516/152, 7-8=- 4, 9-13=-9/1033, 8-9=-43/1408 7-9=-335/171, 4-13=-44/558, 3-13=-347,	1660/122				
 Wind: ASCE 7-10; Vult=120n gable end zone and C-C Exte 23-8-4 zone; cantilever left ar reactions shown; Lumber DOJ TCLL: ASCE 7-10; Pr=20.0 p roof snow: Lumber DOL=1.15 Unbalanced snow loads have 5) This truss has been designed non-concurrent with other live 6) All plates are 3x4 MT20 unles This truss has been designed will fit between the bottom ch Refer to girder(s) for truss to 10) Provide mechanical connec 1) One H2.5A Simpson Strong connection is for uplift only a 	sf (roof live load: Lumber DOL=1.15 Plate 5 Plate DOL=1.15); Category II; Exp B; I 6 been considered for this design. 1 for greater of min roof live load of 12.0 6 loads. 5s otherwise indicated. 4 for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the botton ord and any other members, with BCDL	to 12-0-0, Exterior(2) 12 ght exposed;C-C for men te DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10 psf or 1.00 times flat roof perconcurrent with any oth m chord in all areas when = 10.0psf. capable of withstanding f	0-0 to 15-0-0, Interior(1) 15-0-0 abers and forces & MWFRS for sf (ground snow); Pf=11.6 psf (load of 11.6 psf on overhangs er live loads. e a rectangle 3-6-0 tall by 2-0-0 100 lb uplift at joint(s) 8.	flat	SEAL 044925	North Marine



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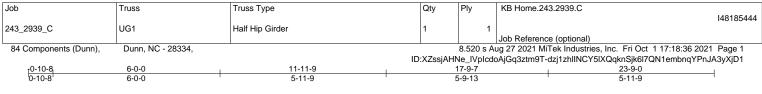
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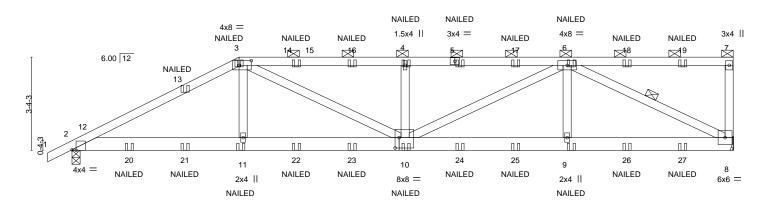


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Scale = 1:41.4



		1-11-9 5-11-9	<u> </u>		<u>23-9-0</u> 5-11-9	<u>23-</u> 9-3 0-0-3
-	2,Edge], [3:0-5-4,0-2-0], [10:0-1-12,0-4-		0010		0110	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNO	CSI. TC 0.82 BC 0.62 WB 0.64	DEFL. ir Vert(LL) 0.12 Vert(CT) -0.22 Horz(CT) 0.05	2 10 >999 240 2 10 >999 180	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S			Weight: 136 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 *E 3-5: 2x4 SP No BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3	•	Ti B	except OT CHORD Rigid c	0	y applied or 2-7-6 oc purlin c purlins (3-0-5 max.): 3-7. 11-5 oc bracing.	S,
Max Horz 2=1	lechanical, 2=0-3-8 07(LC 9) 330(LC 9), 2=-215(LC 12)					

Max Grav 8=1344(LC 2), 2=1373(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2432/470, 3-4=-2782/670, 4-6=-2782/670

BOT CHORD 2-11=-482/2091. 10-11=-476/2103. 9-10=-546/2119. 8-9=-546/2119

WEBS 3-11=0/409, 3-10=-296/840, 4-10=-529/308, 6-10=-184/744, 6-9=0/382, 6-8=-2322/568

NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=330.

10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 12) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

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Truss	Truss Type	Qty	Ply	KB Home.243.2939.C
				I48185444
UG1	Half Hip Girder	1	1	
				Job Reference (optional)
Dunn, NC - 28334,			8.520 s A	ug 27 2021 MiTek Industries, Inc. Fri Oct 1 17:18:36 2021 Page 2
	UG1	UG1 Half Hip Girder	UG1 Half Hip Girder 1	UG1 Half Hip Girder 1 1

ID:XZssjAHNe_IVpIcdoAjGq3ztm9T-dzj1zhIINCY5IXQqknSjk6l7QN1embnqYPnJA3yXjD1

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

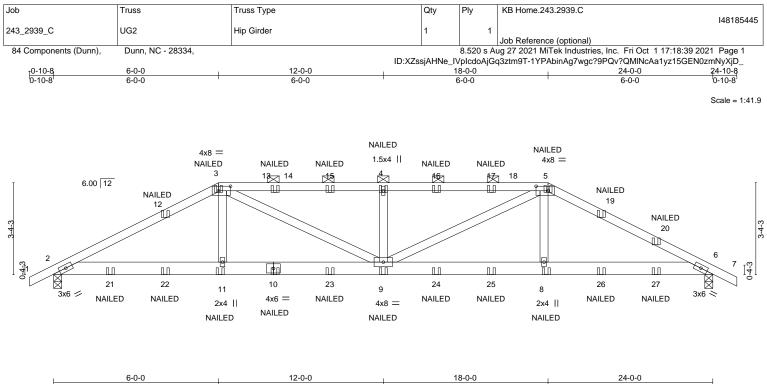
Uniform Loads (plf) Vert: 1-3=-43, 3-7=-53, 2-8=-20

Concentrated Loads (lb)

Vert: 3=-33(F) 5=-28(F) 11=-20(F) 10=-20(F) 4=-28(F) 9=-20(F) 6=-28(F) 13=-45(F) 15=-28(F) 16=-28(F) 17=-28(F) 18=-28(F) 19=-28(F) 20=-36(F) 21=-25(F) 22=-20(F) 23=-20(F) 24=-20(F) 25=-20(F) 26=-20(F) 27=-20(F) 27=-20(F) 26=-20(F) 27=-20(F) 27=-2

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6-0	-0	6-0-0	6-0-0	I	6-0-0	
Plate Offsets (X,Y) [3:0-5-4,0)-2-0], [5:0-5-4,0-2-0]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.91 BC 0.64 WB 0.35 Matrix-S	DEFL.inVert(LL)0.12Vert(CT)-0.23Horz(CT)0.05	(loc) l/defl L/d 9 >999 240 9 >999 180 6 n/a n/a	PLATES MT20 Weight: 127 lb	GRIP 197/144 FT = 20%
LUMBER-	2x4 SPF No.2 *Except*		ACING- P CHORD Structural except	I wood sheathing direc	tly applied or 2-3-11 oc purli	ns,

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2 *Except*	TOP CHORD	Structural wood sheathing directly applied or 2-3-11 oc purlins,
	3-5: 2x4 SP No.1		except
BOT CHORD	2x6 SP No.2		2-0-0 oc purlins (3-1-0 max.): 3-5.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=-49(LC 63) Max Uplift 2=-206(LC 12), 6=-217(LC 13) Max Grav 2=1384(LC 2), 6=1412(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2459/442, 3-4=-2837/629, 4-5=-2837/629, 5-6=-2486/453

BOT CHORD 2-11=-378/2115. 9-11=-372/2127. 8-9=-356/2141. 6-8=-362/2129

WEBS 3-11=0/412, 3-9=-277/849, 4-9=-574/334, 5-9=-260/819, 5-8=0/414

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope)

gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0 Rain surcharge applied to all

exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-43, 3-5=-53, 5-7=-43, 2-6=-20

Continued on page 2

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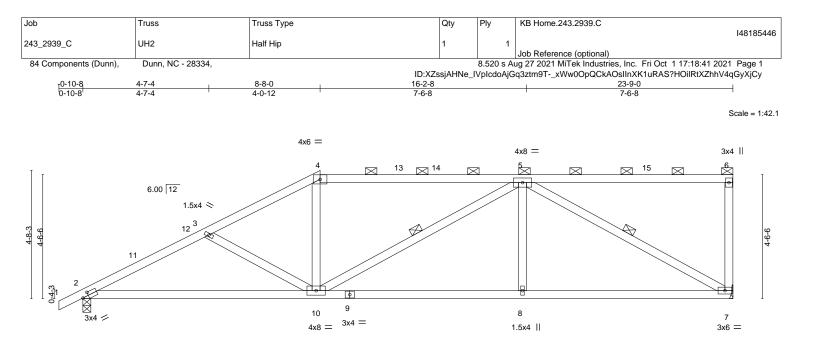
Job	Truss	Truss Type	Qty	Ply	KB Home.243.2939.C			
243 2939 C	UG2	Hip Girder	1	1	I48185445			
			-		Job Reference (optional)			
84 Components (Dunn),	Dunn, NC - 28334,			8.520 s A	ug 27 2021 MiTek Industries, Inc. Fri Oct 1 17:18:39 2021 Page 2			
		ID:XZssjAHNe_IVpIcdoAjGq3ztm9T-1YPAbinAg7wgc?9PQv?QMINcAa1yz15GEN0zmNyXjD_						

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 3=-33(B) 5=-33(B) 10=-20(B) 11=-20(B) 9=-20(B) 4=-28(B) 8=-20(B) 12=-45(B) 14=-28(B) 15=-28(B) 16=-28(B) 17=-28(B) 19=-45(B) 20=-39(B) 21=-36(B) 22=-25(B) 23=-20(B) 24=-20(B) 25=-20(B) 26=-25(B) 27=-36(B)

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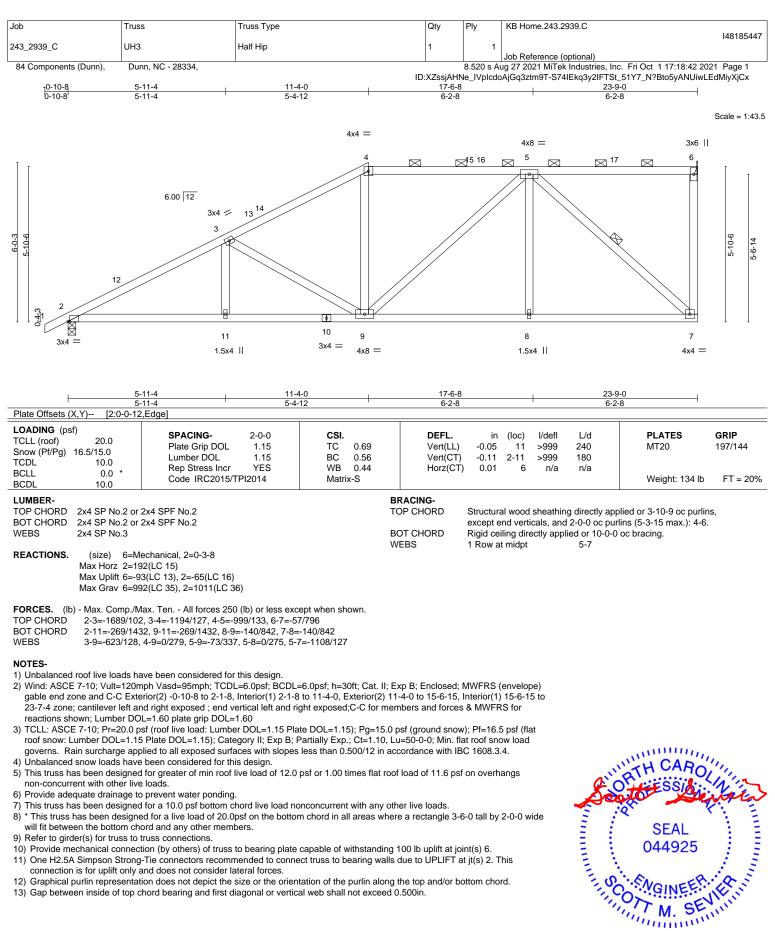
	<u>8-8-0</u> 8-8-0		<u>16-2-8</u> 7-6-8			<u>23-9-0</u> 7-6-8		
Plate Offsets (X,Y) [2:0-2-10								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.75 BC 0.75 WB 0.61 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.14 2-10 -0.30 2-10 0.05 7	>999 >953	L/d 240 180 n/a	PLATES MT20 Weight: 123 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 4-6: 2x4 SP No BOT CHORD 2x4 SP No.2 or WEBS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD WEBS		ticals, and 2- rectly applied	-0-0 oc purlins	d or 4-1-14 oc purlin (4-8-0 max.): 4-6. bracing.	S,
Max Horz 2=1 Max Uplift 7=-9	1echanical, 2=0-3-8 48(LC 15) 98(LC 13), 2=-47(LC 16) 046(LC 35), 2=1001(LC 2)							
TOP CHORD 2-3=-1668/15 BOT CHORD 2-10=-289/14	Max. Ten All forces 250 (lb) or less exc 0, 3-4=-1427/118, 4-5=-1228/124 42, 8-10=-158/1348, 7-8=-158/1348 0, 4-10=0/369, 5-8=0/312, 5-7=-1502/14							
NOTES-								

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 8-8-0, Exterior(2) 8-8-0 to 12-10-15, Interior(1) 12-10-15 to 23-7-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
 connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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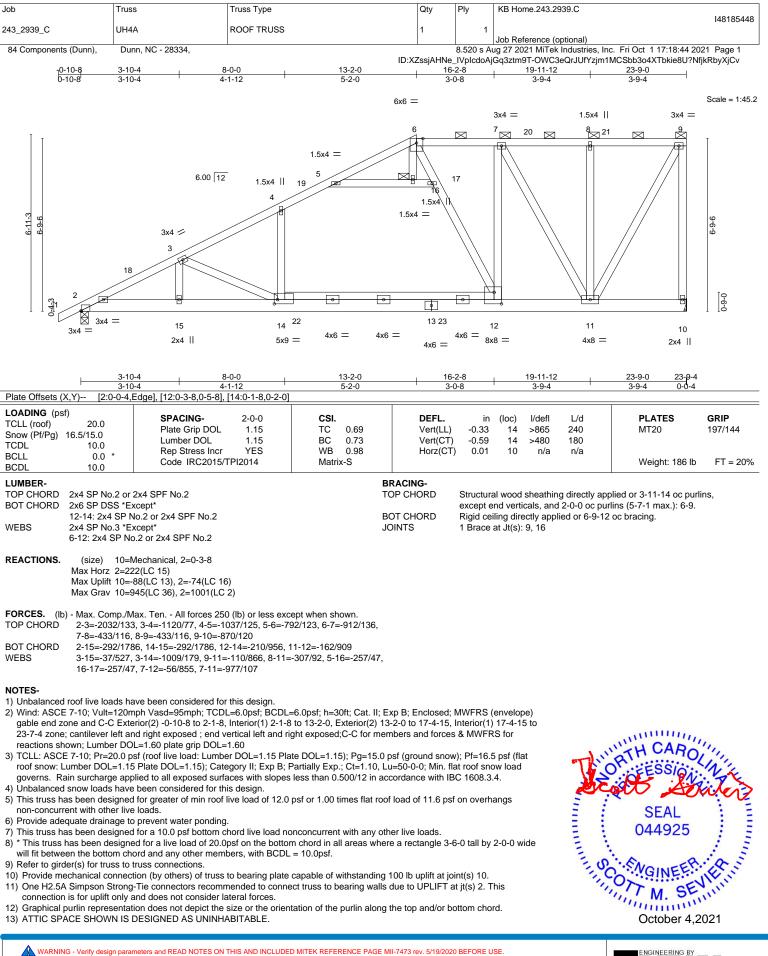




October 4,2021

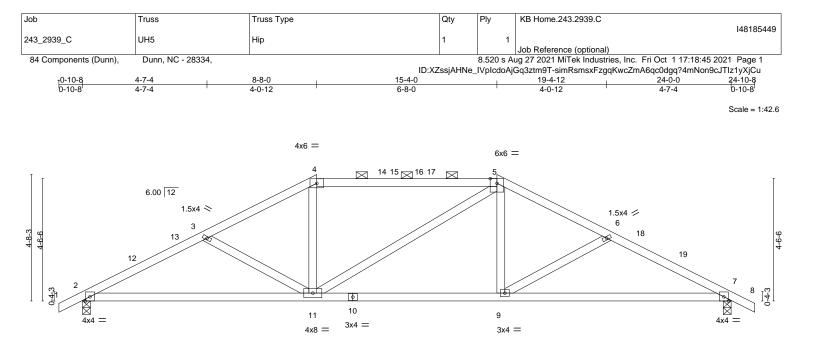
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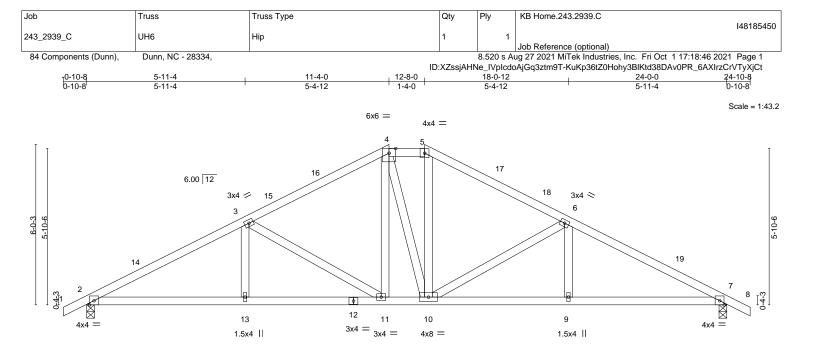


	8-8-0	15-4-0					24-0-0		_
•	8-8-0	6-8-0					8-8-0	1	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.78 BC 0.74 WB 0.15	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.15 -0.32 0.05	7-9	l/defl >999 >897 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	()					Weight: 114 lb	FT = 20%
LUMBER-		TOP	:	2-0-0 oc p	urlins (3-10-5 r		blied or 4-1-7 oc purlins oc bracing.	, except
TOP CHORD 2-3=-1686/19 BOT CHORD 2-11=-121/14	Aax. Ten All forces 250 (lb) or less exc 6, 3-4=-1444/154, 4-5=-1243/168, 5-6=- 55, 9-11=-27/1243, 7-9=-125/1455 9, 4-11=0/361, 5-9=0/361, 6-9=-338/139	1444/154, 6-7=-1686/196							
 Wind: ASCE 7-10; Vult=120n gable end zone and C-C Ext 15-4-0, Exterior(2) 15-4-0 to exposed;C-C for members an 3 TCLL: ASCE 7-10; Pr=20.0 p roof snow: Lumber DOL=1.1 governs. Rain surcharge app 4) Unbalanced snow loads have 5) This truss has been designed 6) Provide adequate drainage to 7) This truss has been designed 8) * This truss has been designed 8) * This truss has been designed 9) One H2.5A Simpson Strong- connection is for uplift only an 	o prevent water ponding. d for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the bottor	to 8-8-0, Exterior(2) 8-8-0 t one; cantilever left and right to DOL=1.60 plate g te DOL=1.15); Pg=15.0 psf Partially Exp.; Ct=1.10, Lu=5 less than 0.500/12 in accor psf or 1.00 times flat roof loc onconcurrent with any other n chord in all areas where a st truss to bearing walls due	o 12-10-15, Interest exposed ; end (ground snow); 0-0-0; Min. flat dance with IBC ad of 11.6 psf c live loads. rectangle 3-6- to UPLIFT at jt	erior(1) 12 I vertical le ; Pf=16.5 g t roof snow C 1608.3.4. on overhar -0 tall by 2- t(s) 2 and 7	2-10-15 oft and r psf (flat v load ngs -0-0 wic 7. This	to right	State	SEAL 044925	The work

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October 4,2021



		<u>11-4-0</u> <u>12-8-0</u> 5-4-12 1-4-0	<u>18-0-12</u> 5-4-12	24-0-0 5-11-4	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.62 WB 0.43 Matrix-S	DEFL. in (loc) Vert(LL) -0.08 11 Vert(CT) -0.15 11-13 Horz(CT) 0.06 7	I/defi L/d PLATES >999 240 MT20 >999 180 n/a n/a Weight: 128 lb	GRIP 197/144 FT = 20%
	r 2x4 SPF No.2 r 2x4 SPF No.2	TOF	2-0-0 oc purlins	sheathing directly applied or 3-6-3 oc purlin (5-3-8 max.): 4-5. ctly applied or 10-0-0 oc bracing.	s, except
REACTIONS. (size) 2=0	0-3-8, 7=0-3-8				

Max Horz 2=-86(LC 17) Max Uplift 2=-66(LC 16), 7=-66(LC 17) Max Grav 2=1146(LC 39), 7=1146(LC 39)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-1982/125, 3-4=-1377/145, 4-5=-1138/156, 5-6=-1380/144, 6-7=-1981/125
- BOT CHORD 2-13=-88/1681, 11-13=-88/1681, 10-11=0/1135, 9-10=-53/1680, 7-9=-53/1680
- WEBS 3-13=0/251, 3-11=-619/123, 4-11=-21/362, 5-10=-25/364, 6-10=-614/124

NOTES-

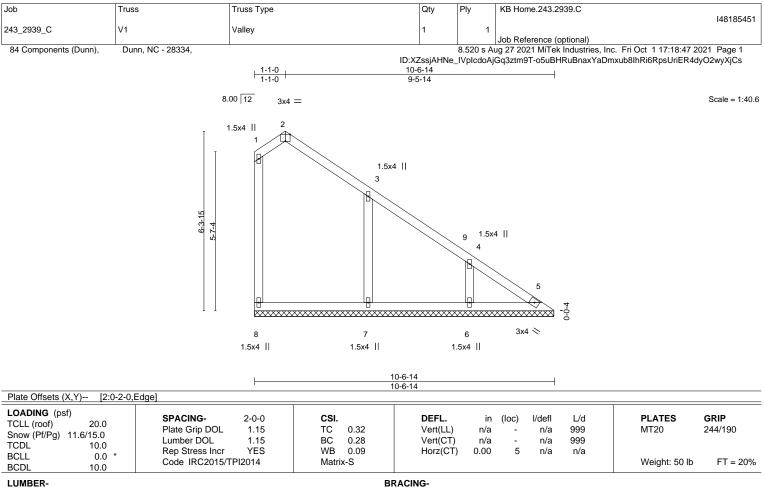
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 11-4-0, Exterior(2) 11-4-0 to 16-10-15, Interior(1) 16-10-15 to 24-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; PI=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.3	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.3		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. All bearings 10-6-8.

(lb) - Max Horz 8=-186(LC 10)

3-7=-262/153

Max Uplift All uplift 100 lb or less at joint(s) 8, 5, 6, 7

Max Grav All reactions 250 lb or less at joint(s) 8, 5 except 6=261(LC 30), 7=408(LC 26)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 4-0-0, Interior(1) 4-0-0 to 10-1-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

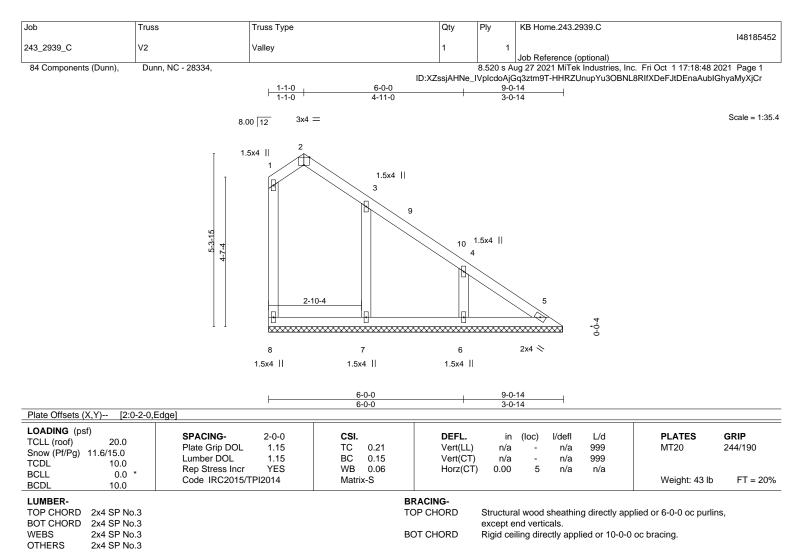
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



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REACTIONS. All bearings 9-0-8.

(lb) - Max Horz 8=-154(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 8, 5, 7, 6

Max Grav All reactions 250 lb or less at joint(s) 8, 5, 6 except 7=310(LC 26)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 4-1-0, Interior(1) 4-1-0 to 8-7-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

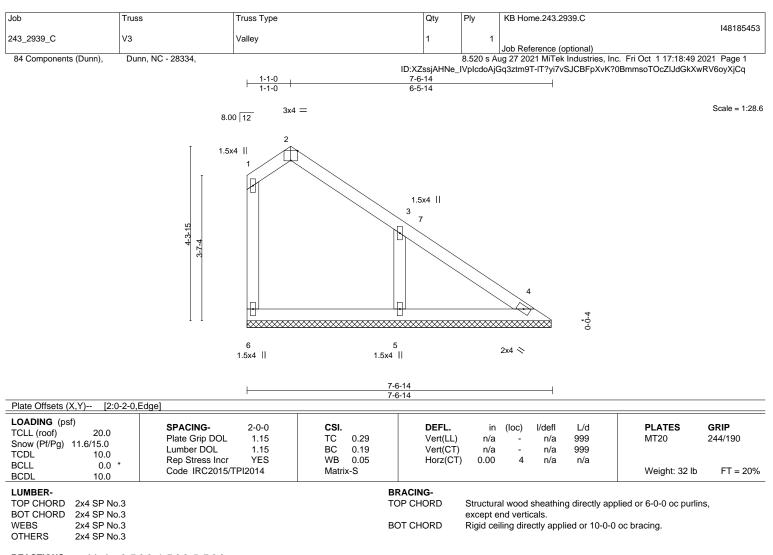
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



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A MITek Affilia 818 Soundside Road Edenton, NC 27932



REACTIONS. (size) 6=7-6-8, 4=7-6-8, 5=7-6-8 Max Horz 6=-122(LC 10) Max Uplift 6=-9(LC 14), 5=-80(LC 15) Max Grav 6=124(LC 2), 4=121(LC 25), 5=335(LC 26)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 3-5=-251/142

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 3-9-7, Interior(1) 3-9-7 to 7-1-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

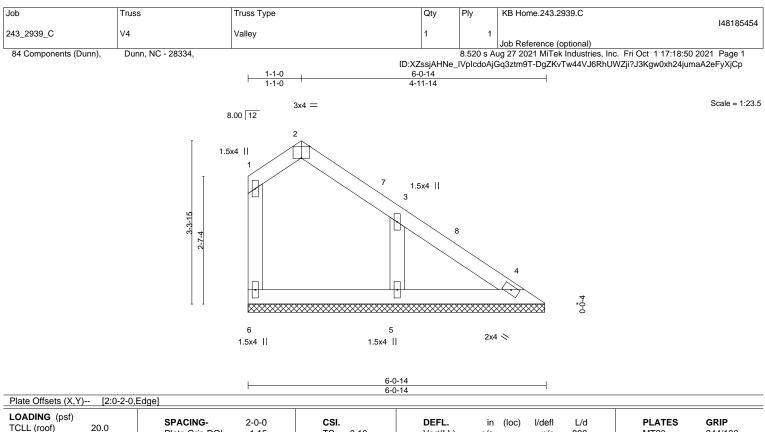
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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TCLL (roof) Snow (Pf/Pg) 11.6 TCDL BCLL BCDL	20.0 5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES VI2014	CSI. TC BC WB Matrix	0.18 0.12 0.04 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 25 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4	4 SP No.3 4 SP No.3 4 SP No.3 4 SP No.3 4 SP No.3				T	e	except ei	nd vertio	als.	g directly app ed or 10-0-0	blied or 6-0-0 oc purlin oc bracing.	S,

REACTIONS. (size) 6=6-0-8, 4=6-0-8, 5=6-0-8 Max Horz 6=-90(LC 10) Max Uplift 6=-7(LC 14), 5=-58(LC 15) Max Grav 6=101(LC 2), 4=93(LC 25), 5=258(LC 26)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 4-1-0, Interior(1) 4-1-0 to 5-7-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

1001 Show: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Part

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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