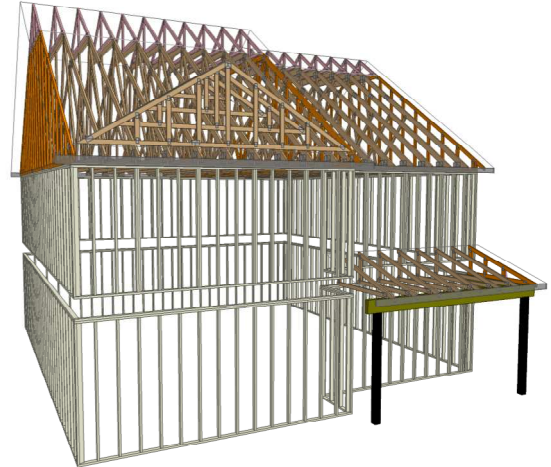




Kempsville Sanford Component Plant  
298 Harvey Faulk Rd  
Sanford, NC 27332

Phone #:919-775-1450



**Builder: DRB HOMES NC**

**Model: DEVON 8 - 91 FaNC**

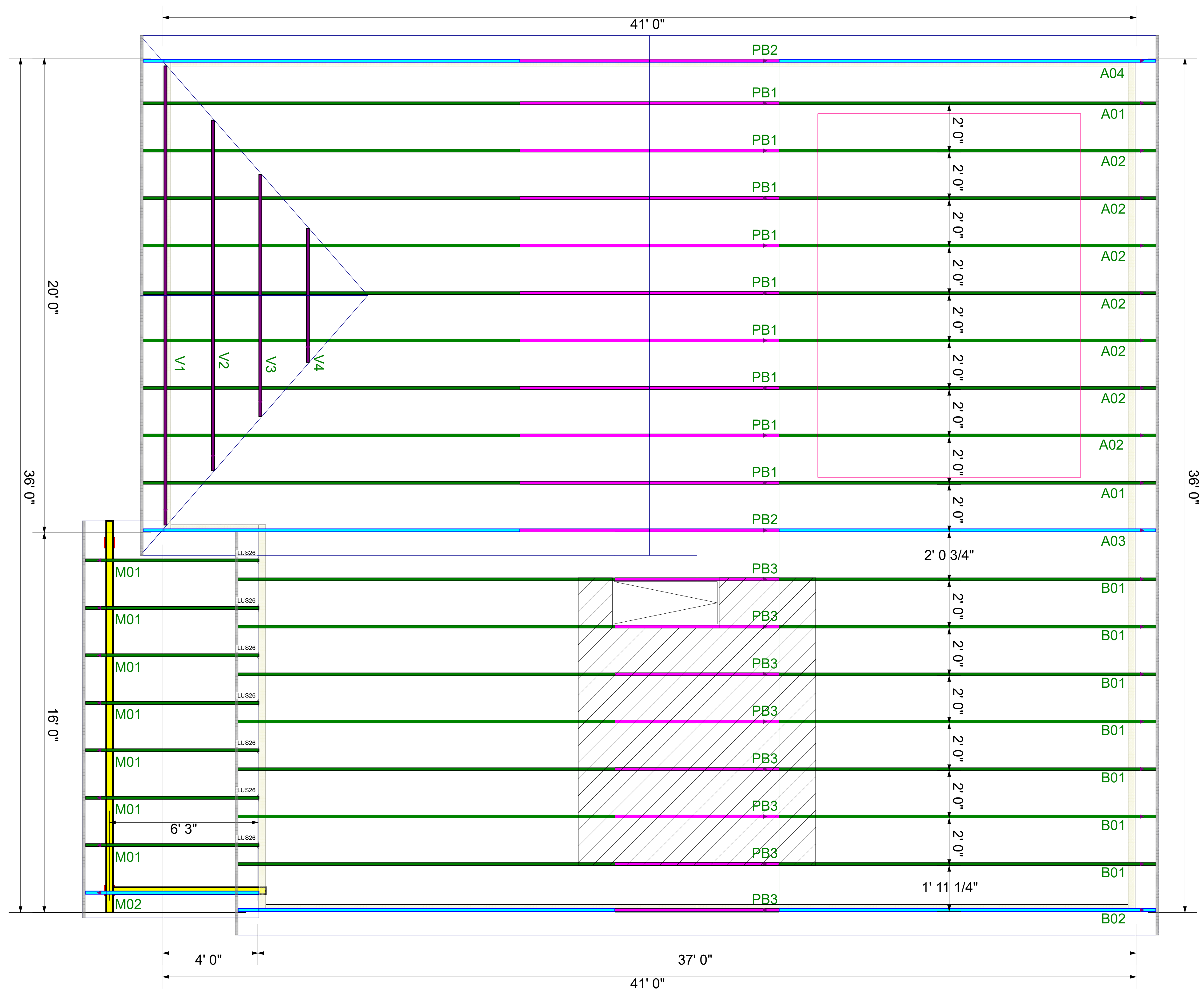
#### THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.

General Notes: \*\* CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

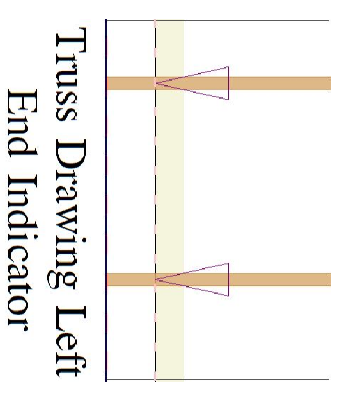
\*\* ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

Truss Connector Total List			
Qty	Product	Manuf	
7	LUS26	Simpson	



\*\* FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.

\*\* DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT.



\*\* TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

\*\* PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

\*\* REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.

\*\* GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS.

\*\* DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.

\*\* TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.

Scale:	NTS
Date:	10/5/2022
Designer:	ND
Project Number:	22090050
Sheet Number:	1/1

DRB HOMES NC  
 DEVON 8 - 91 FaNC  
**COMPONENT  
 PLACEMENT PLAN**



**THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.** These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See Individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available from the Truss Plate Institute, 583 D'Onifrio Drive: Madison, WI 53179

Revisions	
Name	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name

**Trenco**

818 Soundside Rd  
Edenton, NC 27932

Re: 22090050  
DRB GROUP - 91 FaNC

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: I54556803 thru I54556817

My license renewal date for the state of North Carolina is December 31, 2022.

North Carolina COA: C-0844



October 5, 2022

Gilbert, Eric

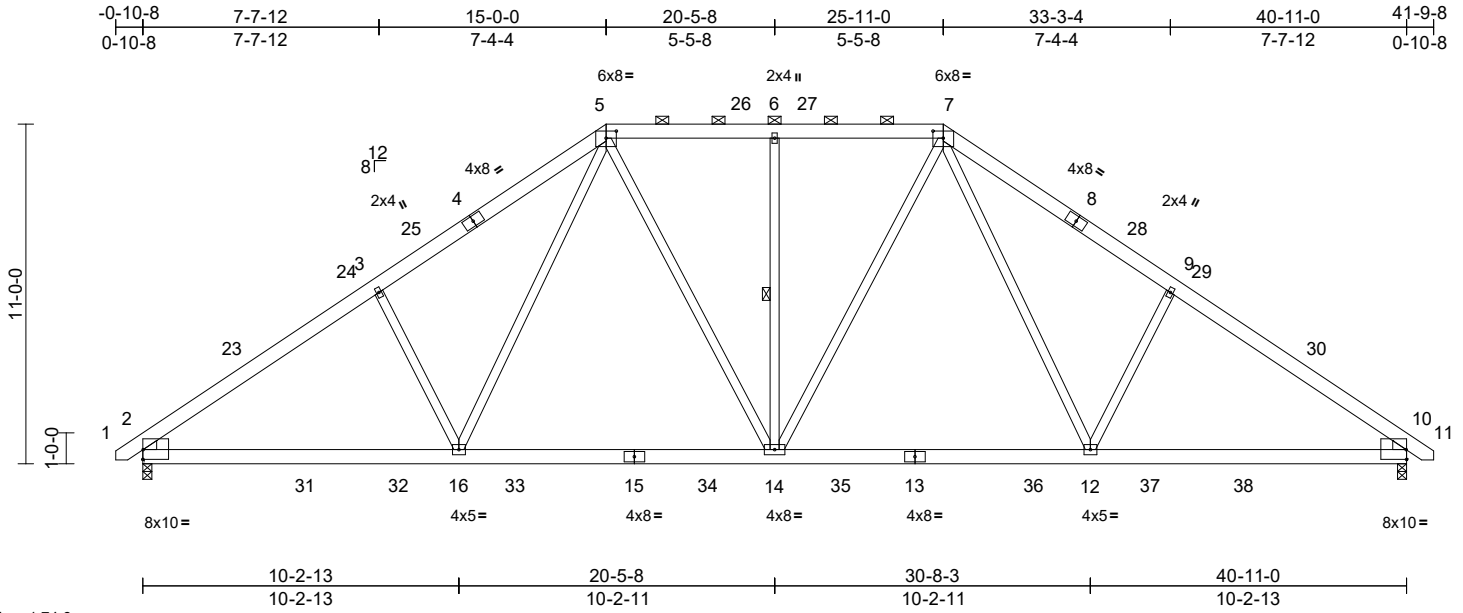
**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job 22090050	Truss A01	Truss Type Piggyback Base	Qty 2	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556803
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:07:56  
ID:sKQsrFmzhKldS3ROH4IW00yEKdi-RfC?PsB70Hq3NSgPqnL8w3uITxBGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:74.6

Plate Offsets (X, Y): [2:Edge,0-3-13], [5:0-4-0,0-2-12], [7:0-4-0,0-2-12], [10:Edge,0-3-13]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP		
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	-0.18	12-14	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.85	Vert(CT)	-0.30	12-14	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(CT)	0.09	10	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											

Weight: 311 lb FT = 20%

**LUMBER**

TOP CHORD 2x6 SP No.2  
 BOT CHORD 2x6 SP No.2  
 WEBS 2x4 SP No.2 \*Except\* 16-3,12-9:2x4 SP No.3  
 WEDGE Left: 2x4 SP No.3  
 Right: 2x4 SP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 3-8-4 oc purlins, except 2-0-0 oc purlins (5-1-4 max.): 5-7.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 WEBS 1 Row at midpt 6-14

**REACTIONS**

(size) 2=0-3-8, 10=0-3-8  
 Max Horiz 2=247 (LC 13)  
 Max Uplift 2=-170 (LC 14), 10=-170 (LC 15)  
 Max Grav 2=1980 (LC 45), 10=1980 (LC 45)

**FORCES**

(lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/23, 2-3=-2803/262, 3-5=-2656/349, 5-6=-1916/313, 6-7=-1916/313, 7-9=-2656/349, 9-10=-2803/262, 10-11=0/23  
 BOT CHORD 2-16=-259/2322, 14-16=-77/1748, 12-14=-11/1748, 10-12=-98/2238  
 WEBS 5-16=-143/907, 3-16=-501/279, 5-14=-134/545, 6-14=-546/159, 7-14=-134/545, 7-12=-144/908, 9-12=-501/279

**NOTES**

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-8-3 to 3-4-15, Interior (1) 3-4-15 to 9-2-9, Exterior(2R) 9-2-9 to 31-8-7, Interior (1) 31-8-7 to 37-6-1, Exterior(2E) 37-6-1 to 41-7-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.

- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



October 5, 2022

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



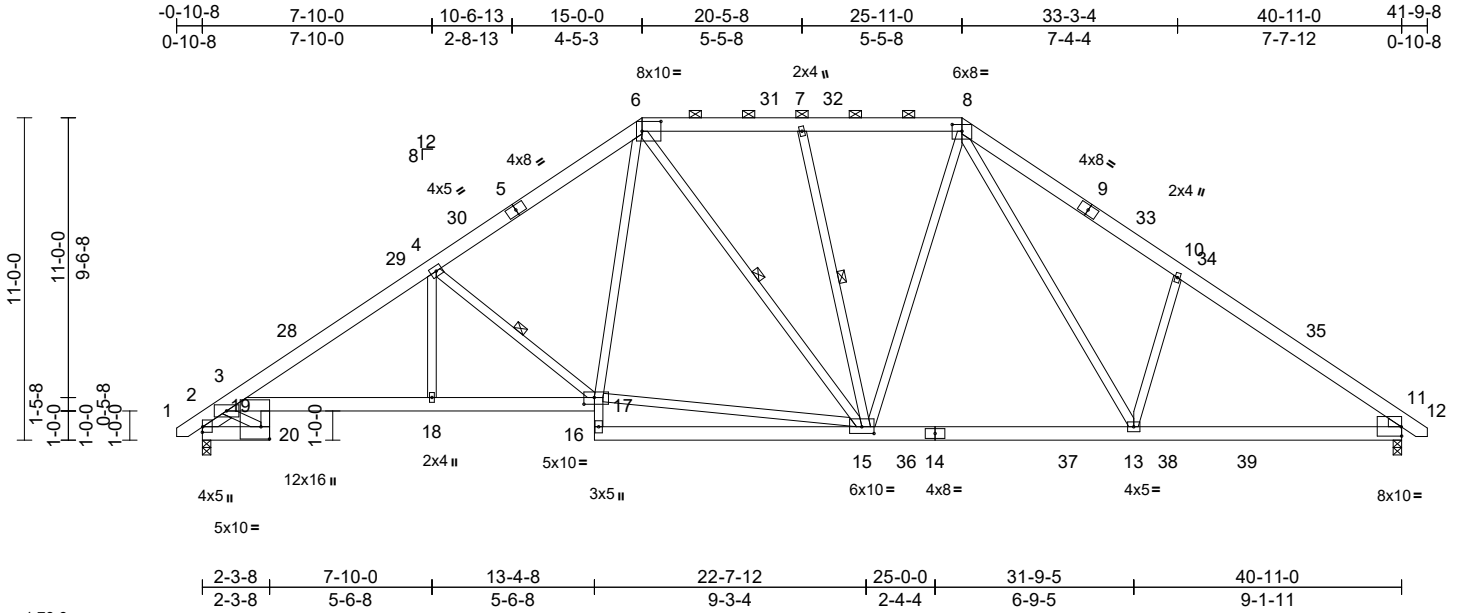
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss A02	Truss Type Piggyback Base	Qty 7	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556804
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:07:58  
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Page: 1



Scale = 1:78.6

Plate Offsets (X, Y): [2:Edge,0-0-3], [6:0-7-12,0-4-0], [8:0-4-0,0-2-12], [11:Edge,0-3-13], [15:0-5-0,0-2-12], [17:0-4-4,0-2-12], [19:Edge,0-3-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	-0.18	13-15	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.96	Vert(CT)	-0.29	13-15	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(CT)	0.18	11	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 339 lb	FT = 20%

LUMBER	
TOP CHORD	2x6 SP No.2
BOT CHORD	2x6 SP No.2 *Except* 20-19,17-16:2x4 SP No.3, 3-17:2x6 SP 2400F 2.0E
WEBS	2x4 SP No.3 *Except* 15-7,15-8,15-6,13-8:2x4 SP No.2
WEDGE	Right: 2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 0-11-13
BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-6-8 oc purlins, except 2-0-0 oc purlins (5-2-3 max.): 6-8. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 19-20.
BOT CHORD	2-2-0 oc bracing: 19-20.
WEBS	1 Row at midpt 4-17, 7-15, 6-15
REACTIONS	(size)
	2=0-3-8, 11=0-3-8
	Max Horiz 2=247 (LC 13)
	Max Uplift 2=-171 (LC 14), 11=-170 (LC 15)
	Max Grav 2=1887 (LC 45), 11=1921 (LC 45)
FORCES	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/28, 2-3=-1100/87, 3-4=-3149/316, 4-6=-2459/310, 6-7=-1846/326, 7-8=-1773/304, 8-10=-2642/396, 10-11=-2720/256, 11-12=0/23
BOT CHORD	2-20=-235/1143, 19-20=-231/1218, 3-19=-278/2648, 18-19=-300/2711, 17-18=-300/2711, 16-17=0/157, 15-16=0/257, 13-15=-8/1671, 11-13=-92/2168

WEBS	
	6-17=-97/996, 4-17=-1058/268, 4-18=0/505, 7-15=-567/163, 8-15=-103/522, 6-15=-205/331, 15-17=-84/1516, 8-13=-204/941, 10-13=-522/292, 3-20=-1385/286
NOTES	
1)	Unbalanced roof live loads have been considered for this design.
2)	Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-8-3 to 3-4-15, Interior (1) 3-4-15 to 9-2-9, Exterior(2R) 9-2-9 to 31-8-7, Interior (1) 31-8-7 to 37-6-1, Exterior(2E) 37-6-1 to 41-7-3 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3)	TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4)	Unbalanced snow loads have been considered for this design.
5)	This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6)	Provide adequate drainage to prevent water ponding.
7)	This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
  - 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 11. This connection is for uplift only and does not consider lateral forces.
  - 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
  - 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- LOAD CASE(S)** Standard



October 5, 2022

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



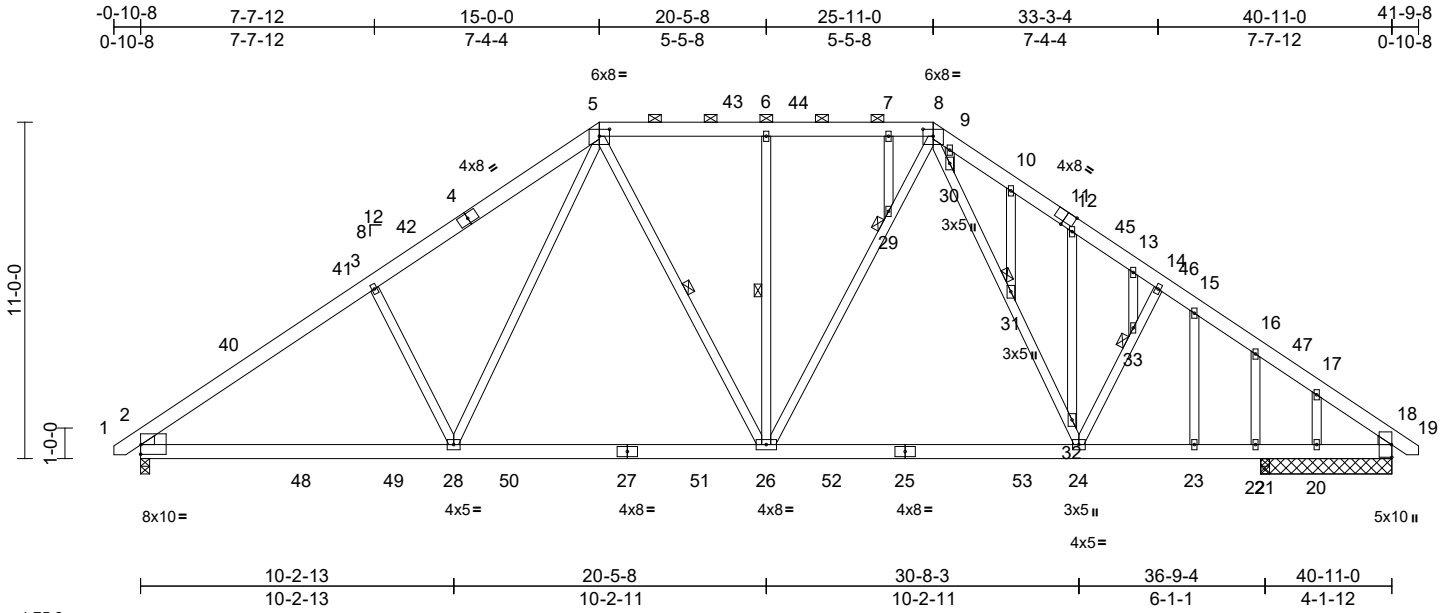
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss A03	Truss Type Piggyback Base	Qty 1	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556805
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:07:59  
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Page: 1



Scale = 1:75.3

Plate Offsets (X, Y): [2:Edge,0-3-13], [5:0-4-0,0-2-12], [8:0-4-0,0-2-12], [11:0-3-15,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	-0.17	26-28	>999	240
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.86	Vert(CT)	-0.28	26-28	>999	180
TCDL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(CT)	0.08	18	n/a	n/a
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH						
BCDL	10.0									
										Weight: 349 lb FT = 20%

LUMBER	TOP CHORD	1-2=0/23, 2-3=-2667/255, 3-5=-2521/343,	2) Wind: ASCE 7-16; Vult=130mph (3-second gust)
TOP CHORD	2x6 SP No.2	5-6=-1779/298, 6-7=-1779/298,	Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft;
BOT CHORD	2x6 SP No.2	7-8=-1779/298, 8-9=-2103/327,	Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior
WEBS	2x4 SP No.2 *Except* 28-3,24-14:2x4 SP No.3	9-10=-2116/321, 10-12=-2239/315,	zone and C-C Exterior(2E) -0-8-3 to 3-4-15, Interior (1)
OTHERS	2x4 SP No.3	12-13=-2091/224, 13-14=-2090/204,	3-4-15 to 9-2-9, Exterior(2R) 9-2-9 to 31-8-7, Interior (1)
WEDGE	Left: 2x4 SP No.3 Right: 2x4 SP No.3	14-15=-2189/200, 15-16=-2058/180,	31-8-7 to 37-6-1, Exterior(2E) 37-6-1 to 41-7-3 zone;
BRACING		16-17=-1875/129, 17-18=-2075/93,	cantilever left and right exposed; end vertical left and
TOP CHORD	Structural wood sheathing directly applied or 3-10-3 oc purlins, except 2-0-0 oc purlins (5-3-12 max.): 5-8.	18-19=0/23	right exposed; C-C for members and forces & MWFRS
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	BOT CHORD	for reactions shown; Lumber DOL=1.60 plate grip
WEBS	1 Row at midpt 5-26, 6-26	2-28=-253/2221, 26-28=-88/1632,	DOL=1.60
JOINTS	1 Brace at Jt(s): 29, 31, 33	24-26=0/1545, 23-24=-22/1632,	
REACTIONS	(size)	22-23=-22/1632, 21-22=-22/1632,	
	2=0-3-8, 18=4-3-8, 20=4-3-8,	20-21=-22/1632, 18-20=-22/1632	
	21=0-3-8, 37=4-3-8	WEBS	
	Max Horiz 2=247 (LC 13)	5-28=-145/916, 3-28=-504/279,	
	Max Uplift 2=-166 (LC 14), 18=-29 (LC 15),	5-26=-132/473, 6-26=-519/158,	
	20=-996 (LC 45), 21=-122 (LC 15),	26-29=-106/585, 8-29=-119/631,	
	37=-29 (LC 15)	8-30=-143/794, 30-31=-120/637,	
	Max Grav 2=1899 (LC 45), 18=1689 (LC 45),	31-32=-136/710, 24-32=-13/270,	
	20=6 (LC 14), 21=1324 (LC 45),	24-33=-22/364, 14-33=-13/396, 7-29=-53/14,	
	37=1689 (LC 45)	9-30=-170/25, 10-31=-17/81,	
FORCES	(lb) - Maximum Compression/Maximum Tension	12-32=-468/146, 13-33=-74/16,	
		15-23=-337/24, 16-22=-443/111,	
		17-20=-16/282	
		NOTES	
		1) Unbalanced roof live loads have been considered for this design.	



October 5, 2022

Continued on page 2

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss A03	Truss Type Piggyback Base	Qty 1	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional) I54556805
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:07:59  
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Page: 2

- 4) TCLK: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 996 lb uplift at joint 20.
- 13) N/A
  
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



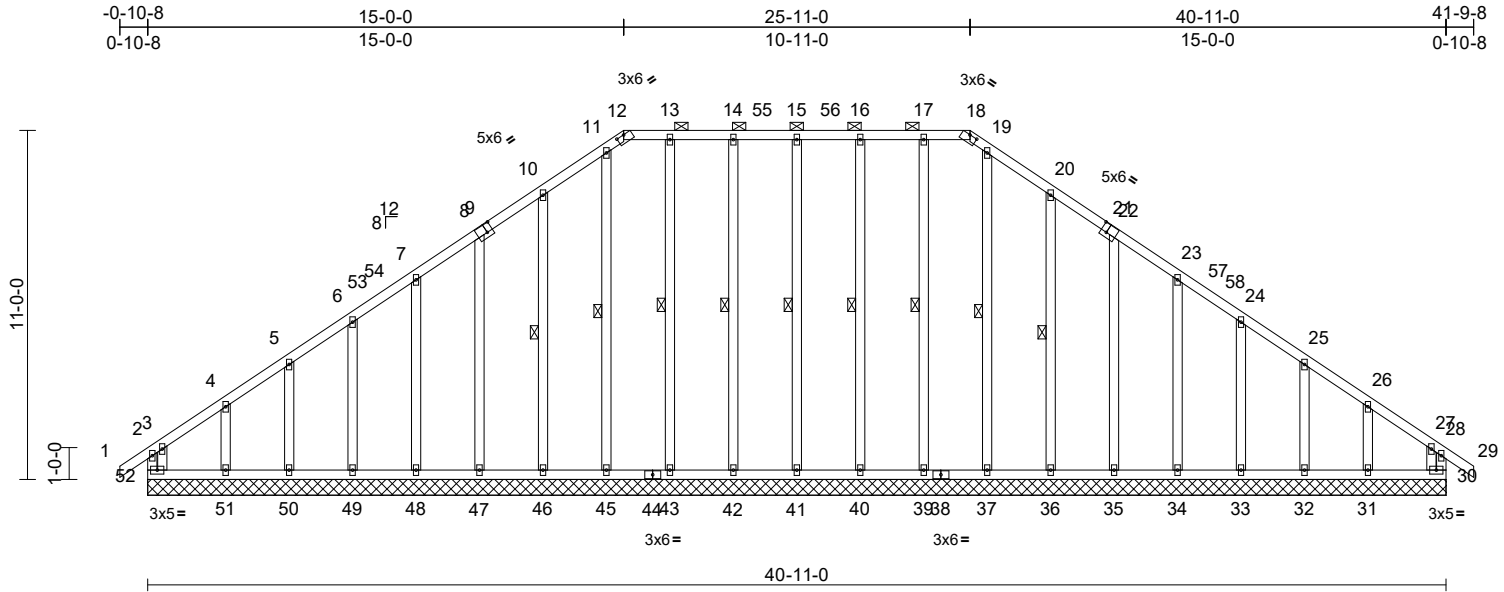
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss A04	Truss Type Piggyback Base Supported Gable	Qty 1	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556806
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:00  
ID:vwN7V4GFd7cdU1xiHzEyyEKZB-RfC?PsB70Hq3NSgPqnL8w3uTXbGKWrCDoi7J4zJC7f

Page: 1



Scale = 1:72.6

Plate Offsets (X, Y): [9:0-2-4,0-3-4], [12:0-3-0,0-0-2], [18:0-3-0,0-0-2], [21:0-2-4,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.16	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(CT)	0.01	30	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 343 lb	FT = 20%

LUMBER		Max Uplift		BOT CHORD
TOP CHORD	2x4 SP No.2	30=-51 (LC 11), 31=-141 (LC 15), 32=-34 (LC 15), 33=-64 (LC 15), 34=-57 (LC 15), 35=-56 (LC 15), 36=-78 (LC 15), 39=-14 (LC 11), 40=-31 (LC 10), 41=-25 (LC 11), 42=-31 (LC 10), 43=-15 (LC 11), 46=-76 (LC 14), 47=-56 (LC 14), 48=-57 (LC 14), 49=-65 (LC 14), 50=-29 (LC 14), 51=-157 (LC 14), 52=-108 (LC 10)		51-52=-113/140, 50-51=-113/140, 49-50=-113/140, 48-49=-113/140, 47-48=-113/140, 46-47=-113/140, 45-46=-113/140, 43-45=-113/140, 42-43=-113/140, 41-42=-113/140, 40-41=-113/140, 39-40=-113/140, 37-39=-113/140, 36-37=-113/140, 35-36=-113/140, 34-35=-113/140, 33-34=-113/140, 32-33=-113/140, 31-32=-113/140, 30-31=-113/140
BOT CHORD	2x4 SP No.2	30=192 (LC 24), 31=228 (LC 49), 32=158 (LC 22), 33=176 (LC 43), 34=230 (LC 43), 35=230 (LC 43), 36=237 (LC 43), 37=191 (LC 43), 39=221 (LC 38), 40=227 (LC 38), 41=225 (LC 38), 42=227 (LC 38), 43=221 (LC 38), 45=202 (LC 53), 46=237 (LC 41), 47=230 (LC 41), 48=230 (LC 41), 49=177 (LC 47), 50=158 (LC 21), 51=251 (LC 47), 52=236 (LC 25)		15-41=-185/49, 14-42=-187/60, 13-43=-181/39, 11-45=-162/20, 10-46=-197/100, 8-47=-190/80, 7-48=-190/81, 6-49=-136/86, 5-50=-120/65, 4-51=-181/153, 3-52=-289/250, 16-40=-187/60, 17-39=-181/38, 19-37=-151/77, 20-36=-197/102, 22-35=-190/80, 23-34=-190/81, 24-33=-136/86, 25-32=-120/65, 26-31=-165/143, 27-30=-211/158
WEBS	2x4 SP No.3			
OTHERS	2x4 SP No.3 *Except* 41-15,42-14,43-13,45-11,40-16,39-17,37-19: 2x4 SP No.2			

BRACING		Max Grav	NOTES
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 12-18.		
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.		
WEBS	1 Row at midpt 15-41, 14-42, 13-43, 11-45, 10-46, 16-40, 17-39, 19-37, 20-36		
REACTIONS (size)	30=40-11-0, 31=40-11-0, 32=40-11-0, 33=40-11-0, 34=40-11-0, 35=40-11-0, 36=40-11-0, 37=40-11-0, 39=40-11-0, 40=40-11-0, 41=40-11-0, 42=40-11-0, 43=40-11-0, 45=40-11-0, 46=40-11-0, 47=40-11-0, 48=40-11-0, 49=40-11-0, 50=40-11-0, 51=40-11-0, 52=40-11-0		
	Max Horiz	52=-276 (LC 12)	
FORCES	(lb) - Maximum Compression/Maximum Tension		
TOP CHORD		2-52=-179/165, 1-2=0/34, 2-3=-25/55, 3-4=-208/196, 4-5=-151/142, 5-6=-144/146, 6-7=-125/164, 7-8=-125/197, 8-10=-157/241, 10-11=-196/307, 11-12=-172/261, 12-13=-169/279, 13-14=-169/279, 14-15=-169/279, 15-16=-169/279, 16-17=-169/279, 17-18=-169/279, 18-19=-172/261, 19-20=-196/307, 20-22=-157/241, 22-23=-125/187, 23-24=-94/131, 24-25=-88/97, 25-26=-94/82, 26-27=-148/123, 27-28=-26/55, 28-29=0/34, 28-30=-152/165	



October 5, 2022

Continued on page 2

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road  
Edenton, NC 27932



Job 22090050	Truss A04	Truss Type Piggyback Base Supported Gable	Qty 1	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	I54556806
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:00  
ID:vwN7V4GFd7jcdU1xiHzIEyyEKZB-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 2

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust)  
Vasd=103mph; TC DL=6.0psf; BC DL=6.0psf; h=25ft;  
Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 3-2-10, Exterior (2N) 3-2-10 to 10-10-14, Corner(3R) 10-10-14 to 19-1-2, Exterior(2N) 19-1-2 to 21-9-14, Corner(3R) 21-9-14 to 30-0-2, Exterior(2N) 30-0-2 to 37-8-6, Corner(3E) 37-8-6 to 41-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) T CLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 11) Gable studs spaced at 2-0-0 oc.
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 13) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 52, 51 lb uplift at joint 30, 25 lb uplift at joint 41, 31 lb uplift at joint 42, 15 lb uplift at joint 43, 76 lb uplift at joint 46, 56 lb uplift at joint 47, 57 lb uplift at joint 48, 65 lb uplift at joint 49, 29 lb uplift at joint 50, 157 lb uplift at joint 51, 31 lb uplift at joint 40, 14 lb uplift at joint 39, 78 lb uplift at joint 36, 56 lb uplift at joint 35, 57 lb uplift at joint 34, 64 lb uplift at joint 33, 34 lb uplift at joint 32 and 141 lb uplift at joint 31.
- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



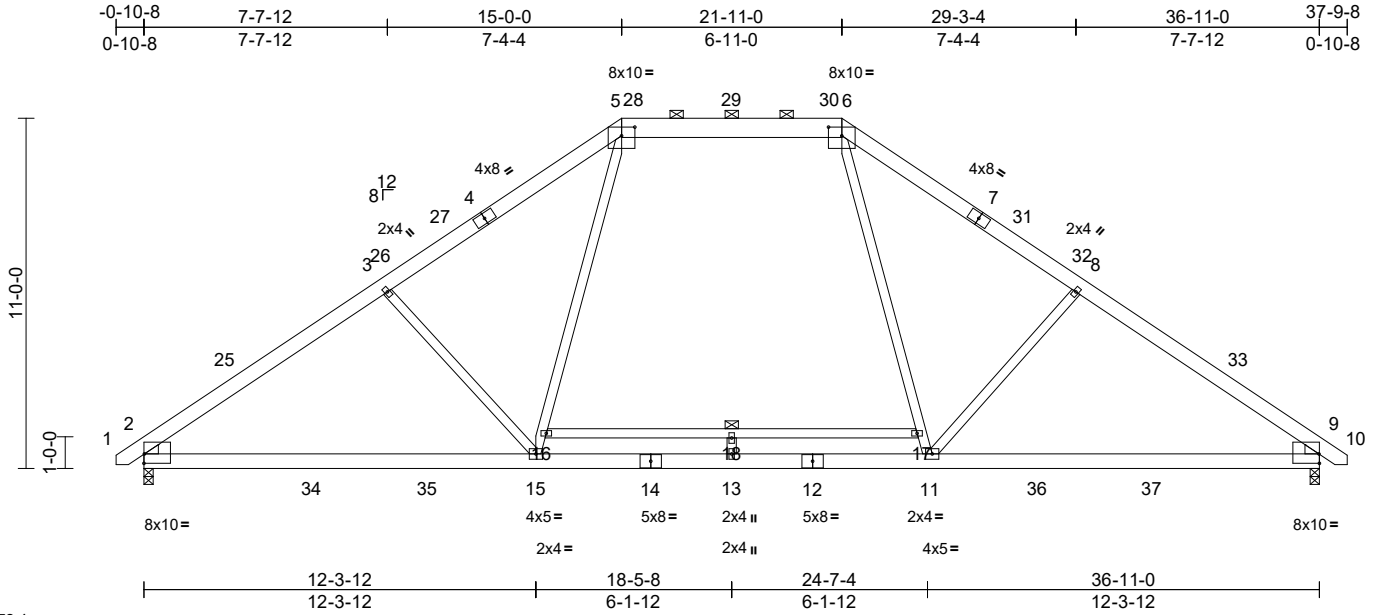
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss B01	Truss Type Piggyback Base	Qty 7	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556807
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:01  
ID:Z2hxjd1mnUfgg9L2u?c3IFyEKYc-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:72.4

Plate Offsets (X, Y): [2:Edge,0-3-9], [5:0-5-0,0-3-4], [6:0-5-0,0-3-4], [9:Edge,0-3-9]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.75	Vert(LL)	-0.37	15-21	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.67	Vert(CT)	-0.47	13-15	>943	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(CT)	0.07	9	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 268 lb	FT = 20%

**LUMBER**  
TOP CHORD 2x6 SP No.2 \*Except\* 5-6:2x8 SP 2400F 2.0E  
BOT CHORD 2x6 SP 2400F 2.0E \*Except\* 14-12:2x6 SP No.2  
WEBS 2x4 SP No.2 \*Except\* 15-3,11-8,18-13:2x4 SP No.3  
WEDGE Left: 2x4 SP No.3  
Right: 2x4 SP No.3

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 3-5-2 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 5-6.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
WEBS 1 Row at midpt 16-17

**REACTIONS** (size) 2=0-3-8, 9=0-3-8  
Max Horiz 2=-245 (LC 12)  
Max Uplift 2=-51 (LC 14), 9=-51 (LC 15)  
Max Grav 2=1874 (LC 47), 9=1874 (LC 49)

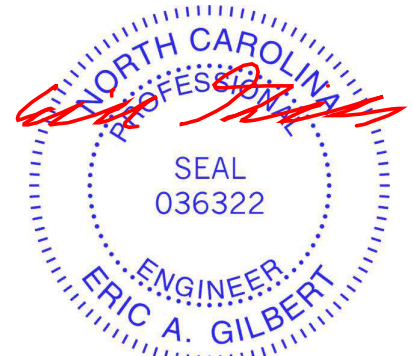
**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/23, 2-3=-2598/43, 3-5=-2353/52, 5-6=-1608/138, 6-8=-2357/52, 8-9=-2600/42, 9-10=0/23  
BOT CHORD 2-15=-156/2199, 13-15=0/1625, 11-13=0/1625, 9-11=0/2077  
WEBS 15-16=0/930, 5-16=0/949, 3-15=-543/335, 6-17=0/950, 11-17=0/932, 8-11=-542/336, 16-18=-15/0, 17-18=-15/0, 13-18=0/33

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-8-3 to 3-0-2, Interior (1) 3-0-2 to 9-9-6, Exterior(2R) 9-9-6 to 27-1-10, Interior (1) 27-1-10 to 33-10-14, Exterior(2E) 33-10-14 to 37-7-3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 18-5-8 from left end, supported at two points, 5-0-0 apart.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



October 5, 2022

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road  
Edenton, NC 27932



Job	Truss	Truss Type	Qty	Ply	DRB GROUP - 91 FaNC	I54556808
22090050	B02	Piggyback Base Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:01

Page: 2

ID:Gnt?WUm0QEK0VXWmzJcNG6yEKXF-RFC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?F

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust)  
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft;  
Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-9-13, Exterior (2N) 2-9-13 to 11-3-11, Corner(3R) 11-3-11 to 25-7-5, Exterior(2N) 25-7-5 to 34-1-3, Corner(3E) 34-1-3 to 37-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCELL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 11) Gable studs spaced at 2-0-0 oc.
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 13) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 48, 52 lb uplift at joint 28, 35 lb uplift at joint 38, 14 lb uplift at joint 39, 74 lb uplift at joint 42, 54 lb uplift at joint 43, 55 lb uplift at joint 44, 63 lb uplift at joint 45, 29 lb uplift at joint 46, 150 lb uplift at joint 47, 13 lb uplift at joint 37, 76 lb uplift at joint 34, 54 lb uplift at joint 33, 55 lb uplift at joint 32, 62 lb uplift at joint 31, 33 lb uplift at joint 30 and 135 lb uplift at joint 29.
- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

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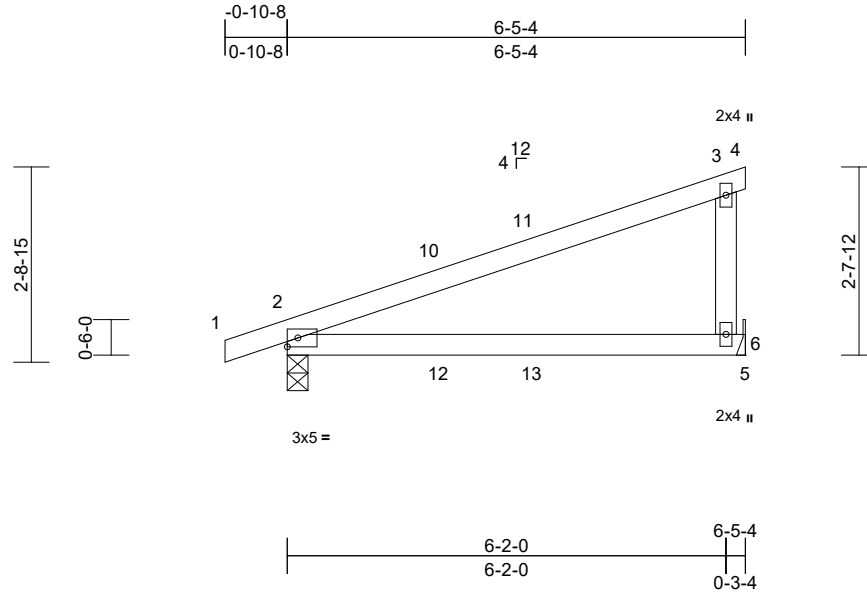
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss M01	Truss Type Monopitch	Qty 7	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556809
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:02  
ID:AuleCvrrSaYOgDpthWIENyEKVs-RfC?PsB70Hq3NSgPqnL8w3lTXbGKWrCDoi7J4zJC?f

Page: 1



Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	0.25	6-9	>291	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	Vert(CT)	0.20	6-9	>365	180		
TCDL	10.0	Rep Stress Incr	YES	WB	Horz(CT)	-0.03	2	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP							
BCDL	10.0									Weight: 24 lb	FT = 20%

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (size) 2=0-3-8, 6= Mechanical  
Max Horiz 2=95 (LC 13)  
Max Uplift 2=-119 (LC 10), 6=-101 (LC 10)  
Max Grav 2=392 (LC 21), 6=362 (LC 21)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/17, 2-3=-100/115, 3-4=-8/0, 3-6=-266/202  
BOT CHORD 2-6=-93/144, 5-6=0/0

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 6.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard

**NOTES**

- Wind: ASCE 7-16; Vult=130mph (3-second gust)  
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft;  
Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 3-5-4, Exterior(2E) 3-5-4 to 6-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.



October 5, 2022

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



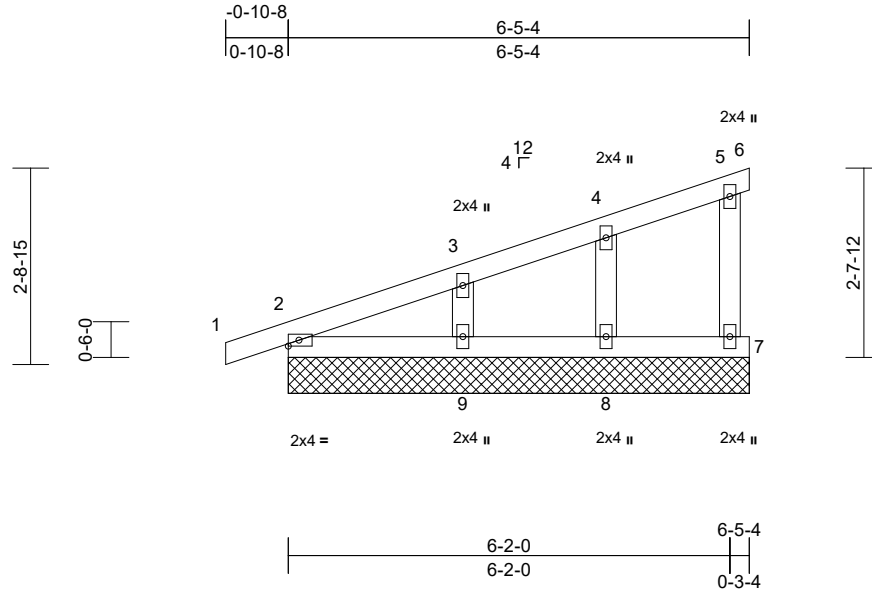
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss M02	Truss Type Monopitch Supported Gable	Qty 1	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556810
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:02  
ID:aTRnxtklVwzXgYSMerx\_WyEKVp-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?F

Page: 1



Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	6	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 27 lb	FT = 20%

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3  
OTHERS 2x4 SP No.3

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

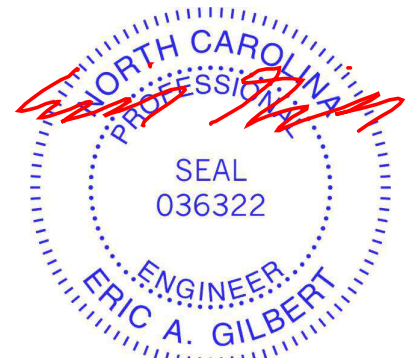
**REACTIONS** (size)  
2=6-5-4, 6=6-5-4, 7=6-5-4,  
8=6-5-4, 9=6-5-4, 10=6-5-4  
Max Horiz 2=94 (LC 13), 10=94 (LC 13)  
Max Uplift 2=-31 (LC 10), 6=-17 (LC 21),  
7=-23 (LC 14), 8=-29 (LC 10),  
9=-47 (LC 14), 10=-31 (LC 10)  
Max Grav 2=172 (LC 21), 6=8 (LC 14), 7=122  
(LC 21), 8=204 (LC 21), 9=267 (LC 21),  
10=172 (LC 21)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/17, 2-3=-110/36, 3-4=-52/26,  
4-5=-42/25, 5-6=-13/7, 5-7=-108/92  
BOT CHORD 2-9=-46/51, 8-9=-38/51, 7-8=-38/51  
WEBS 4-8=-170/162, 3-9=-205/193

- NOTES**
- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)  
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft;  
Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 6-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 2, 17 lb uplift at joint 6, 23 lb uplift at joint 7, 29 lb uplift at joint 8, 47 lb uplift at joint 9 and 31 lb uplift at joint 2.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard



October 5, 2022

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

**ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component**

**Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



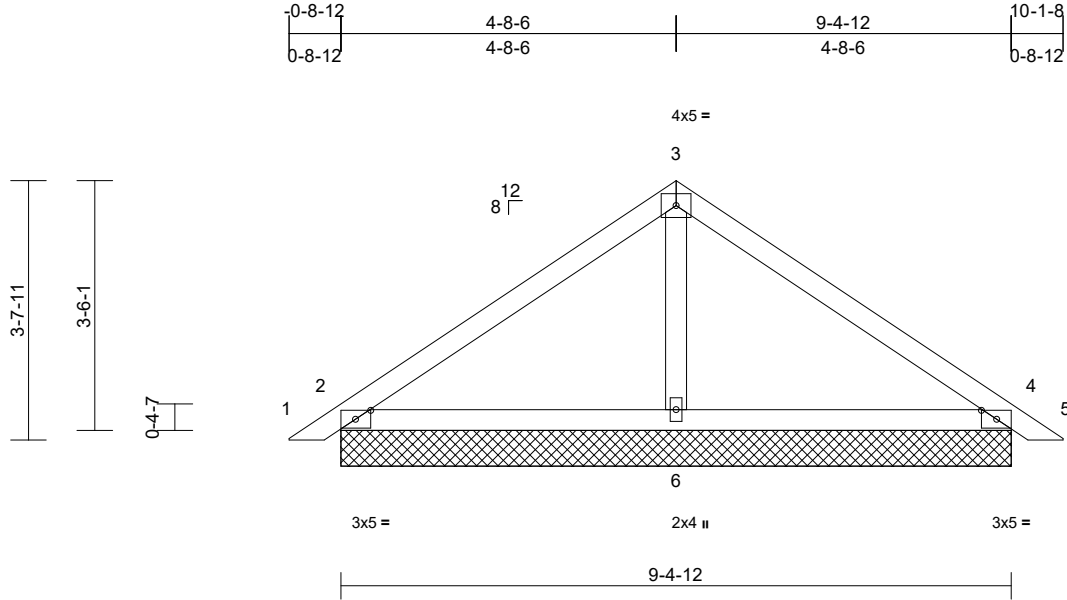
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss PB1	Truss Type Piggyback	Qty 9	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556811
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:03  
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Page: 1



Scale = 1:32.3

Plate Offsets (X, Y): [2:0-2-9,0-1-8], [4:0-2-9,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.42	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
										Weight: 38 lb	FT = 20%	

**LUMBER**

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS**

(size) 2=9-4-12, 4=9-4-12, 6=9-4-12, 7=9-4-12, 11=9-4-12  
Max Horiz 2=81 (LC 13), 7=81 (LC 13)  
Max Uplift 2=-44 (LC 14), 4=-55 (LC 15), 7=-44 (LC 14), 11=-55 (LC 15)  
Max Grav 2=345 (LC 21), 4=345 (LC 22), 6=344 (LC 21), 7=345 (LC 21), 11=345 (LC 22)

**FORCES**

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/16, 2-3=-239/106, 3-4=-239/106, 4-5=0/16  
BOT CHORD 2-6=-31/107, 4-6=-15/107  
WEBS 3-6=-171/38

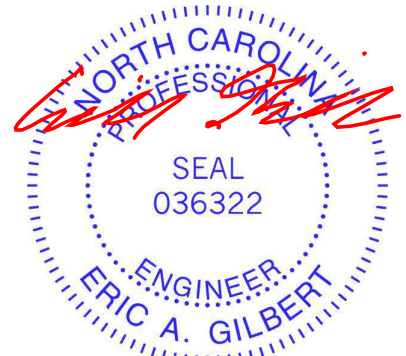
**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-3-5 to 3-3-5, Exterior(2R) 3-3-5 to 7-7-11, Exterior(2E) 7-7-11 to 10-7-11 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 4-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) N/A

- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

**LOAD CASE(S)** Standard



October 5, 2022

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



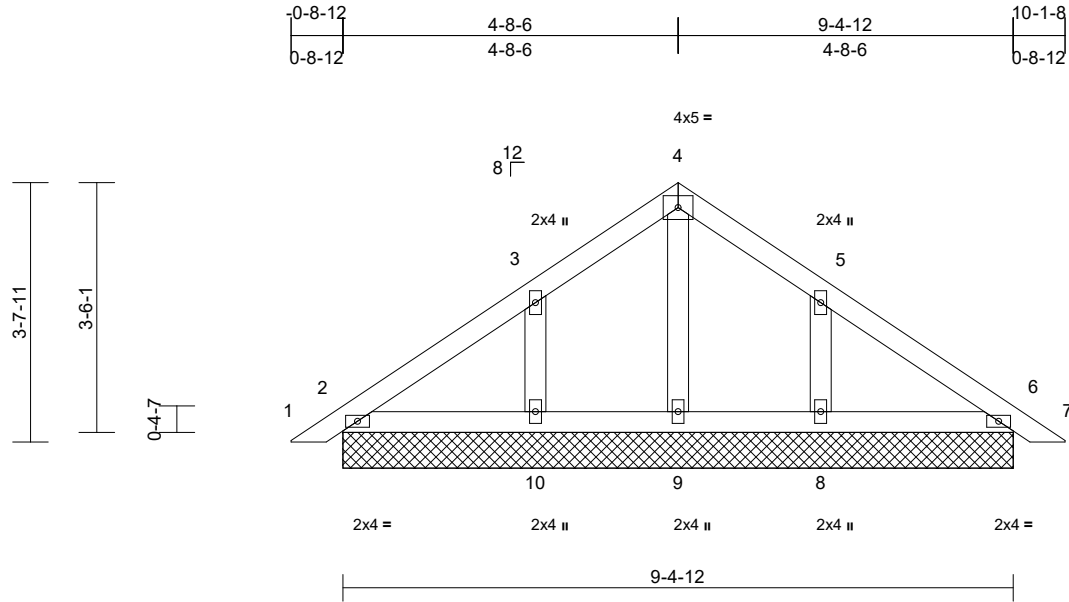
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss PB2	Truss Type Piggyback	Qty 2	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556812
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:03  
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Page: 1



Scale = 1:32.3

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	6	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 42 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS**

(size) 2=9-4-12, 6=9-4-12, 8=9-4-12, 9=9-4-12, 10=9-4-12, 11=9-4-12, 15=9-4-12  
Max Horiz 2=-81 (LC 12), 11=-81 (LC 12)  
Max Uplift 2=-10 (LC 15), 6=-10 (LC 15), 8=-85 (LC 15), 10=-86 (LC 14), 11=-10 (LC 15), 15=-10 (LC 15)  
Max Grav 2=180 (LC 21), 6=180 (LC 22), 8=350 (LC 22), 9=96 (LC 27), 10=350 (LC 21), 11=180 (LC 21), 15=180 (LC 22)

**FORCES**

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/16, 2-3=-74/53, 3-4=-98/91, 4-5=-98/91, 5-6=-67/49, 6-7=0/16  
BOT CHORD 2-10=-21/62, 9-10=-21/62, 8-9=-21/62, 6-8=-21/62  
WEBS 4-9=-72/0, 3-10=-265/126, 5-8=-265/126

**NOTES**

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-3-5 to 3-5-8, Exterior(2R) 3-5-8 to 7-5-8, Exterior(2E) 7-5-8 to 10-7-11 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- N/A

13) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

**LOAD CASE(S)** Standard

12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



October 5, 2022

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818 Soundside Road  
Edenton, NC 27932

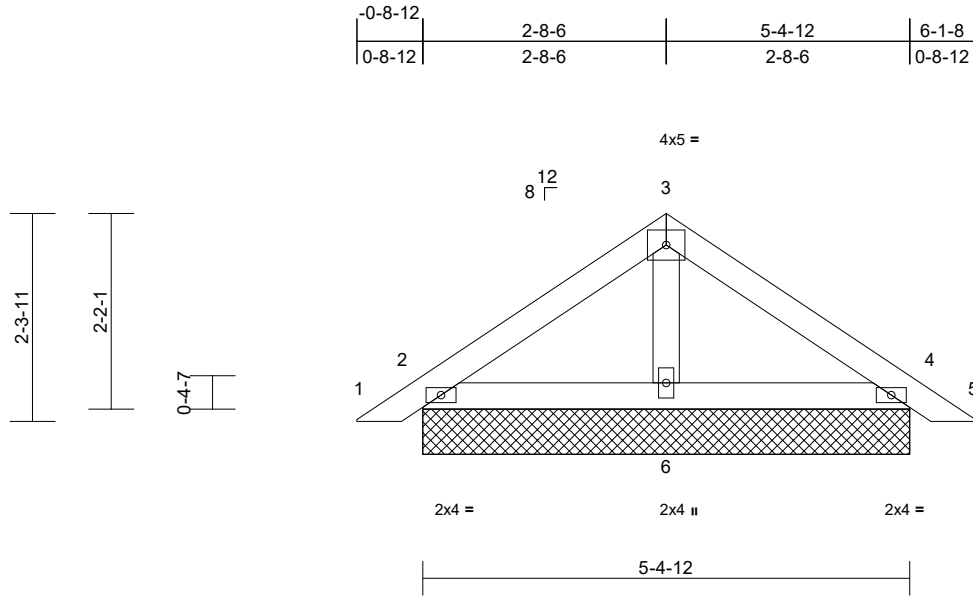


Job 22090050	Truss PB3	Truss Type Piggyback	Qty 8	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556813
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:03  
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Page: 1



Scale = 1:25.5

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.14	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP								
BCDL	10.0											
										Weight: 23 lb	FT = 20%	

**LUMBER**

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS**

(size) 2=5-4-12, 4=5-4-12, 6=5-4-12, 7=5-4-12, 11=5-4-12  
Max Horiz 2=-50 (LC 12), 7=-50 (LC 12)  
Max Uplift 2=-29 (LC 14), 4=-36 (LC 15), 7=-29 (LC 14), 11=-36 (LC 15)  
Max Grav 2=212 (LC 21), 4=212 (LC 22), 6=195 (LC 21), 7=212 (LC 21), 11=212 (LC 22)

**FORCES**

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/25, 2-3=-107/71, 3-4=-107/71, 4-5=0/25  
BOT CHORD 2-6=-12/45, 4-6=-3/45  
WEBS 3-6=-84/18

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- N/A

- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

**LOAD CASE(S)** Standard



October 5, 2022

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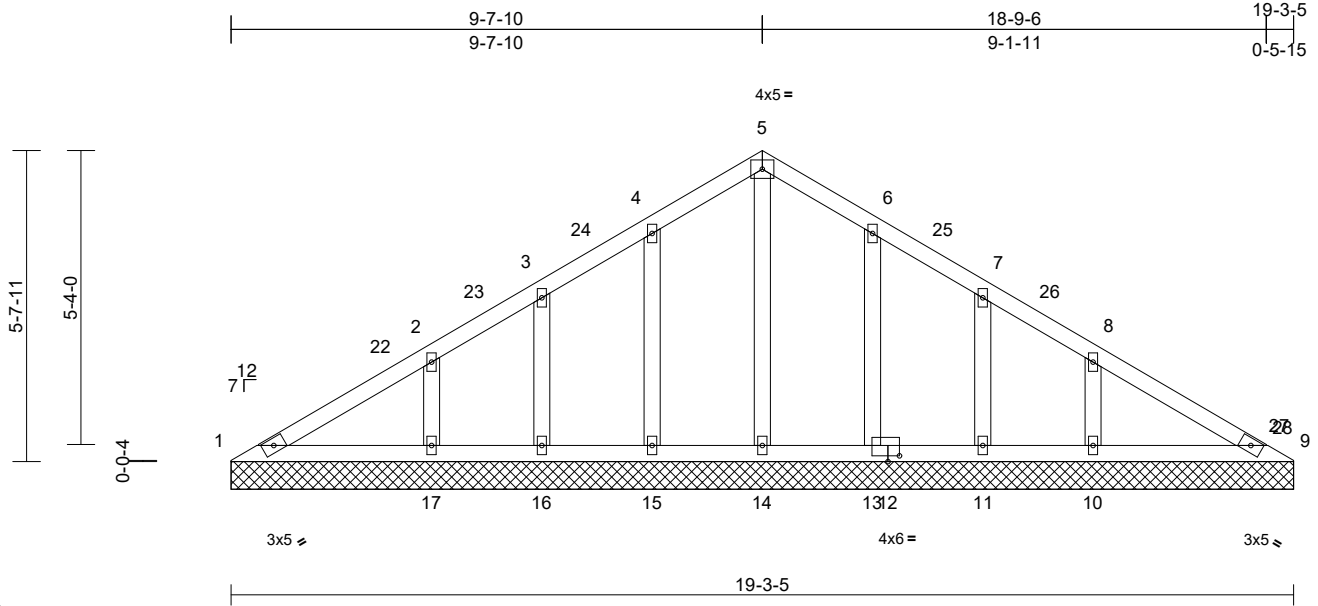
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss V1	Truss Type Valley	Qty 1	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556814
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Carter Components (Sanford), Sanford, NC - 27332,

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Page: 1



Scale = 1:41.8  
Plate Offsets (X, Y): [12:0-2-8,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.13	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.13	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horiz(TL)	0.00	9	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
										Weight: 92 lb	FT = 20%	

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (size)  
1=19-3-5, 9=19-3-5, 10=19-3-5, 11=19-3-5, 13=19-3-5, 14=19-3-5, 15=19-3-5, 16=19-3-5, 17=19-3-5  
Max Horiz 1=128 (LC 11)  
Max Uplift 1=-5 (LC 15), 10=-75 (LC 15), 11=-42 (LC 15), 13=-54 (LC 15), 15=-56 (LC 14), 16=-40 (LC 14), 17=-79 (LC 14)  
Max Grav 1=103 (LC 24), 9=73 (LC 34), 10=290 (LC 24), 11=177 (LC 21), 13=267 (LC 21), 14=200 (LC 1), 15=268 (LC 20), 16=175 (LC 20), 17=296 (LC 23)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-145/164, 2-3=-55/117, 3-4=-27/120, 4-5=-28/124, 5-6=-28/117, 6-7=-3/98, 7-8=-3/81, 8-9=-109/124  
BOT CHORD 1-17=-79/139, 16-17=-79/85, 15-16=-79/85, 14-15=-79/85, 13-14=-79/85, 11-13=-79/85, 10-11=-79/85, 9-10=-79/95  
WEBS 5-14=-164/0, 4-15=-221/79, 3-16=-161/67, 2-17=-194/96, 6-13=-220/77, 7-11=-162/68, 8-10=-191/94

**NOTES**  
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-7 to 3-0-7, Interior (1) 3-0-7 to 6-8-1, Exterior(2R) 6-8-1 to 12-8-1, Interior (1) 12-8-1 to 15-8-1, Exterior(2E) 15-8-1 to 18-9-11 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 1, 56 lb uplift at joint 15, 40 lb uplift at joint 16, 79 lb uplift at joint 17, 54 lb uplift at joint 13, 42 lb uplift at joint 11 and 75 lb uplift at joint 10.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard



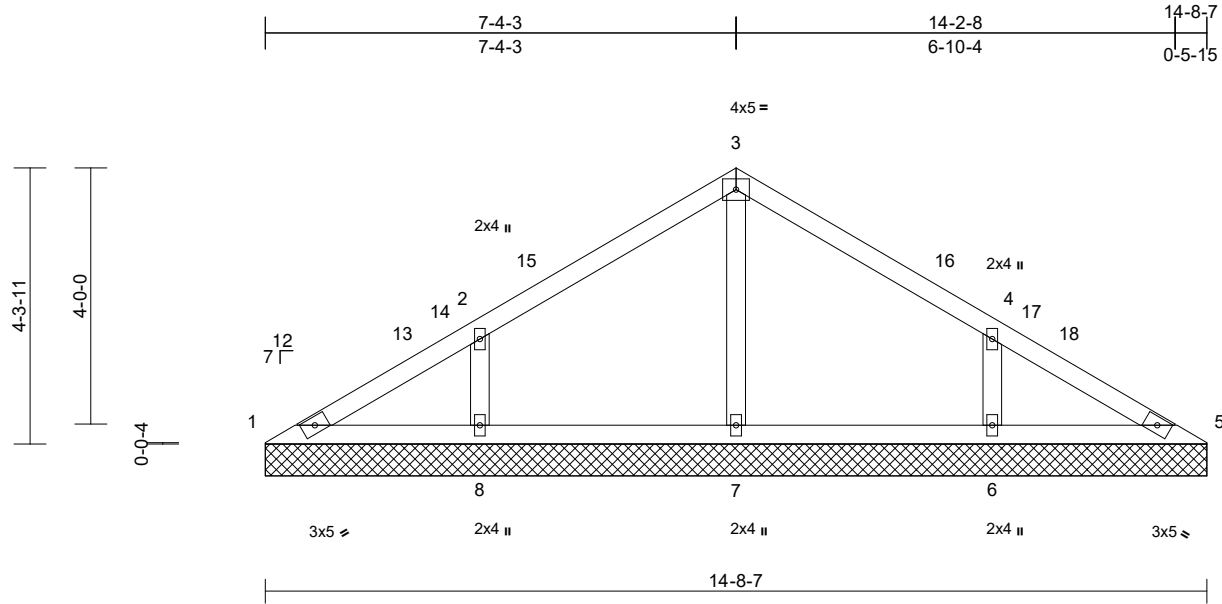
October 5, 2022

Job 22090050	Truss V2	Truss Type Valley	Qty 1	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556815
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Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Sep 22 2022 Print: 8.530 S Sep 22 2022 MiTek Industries, Inc. Wed Oct 05 08:08:04  
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Page: 1



Scale = 1:36

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.30	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.11	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horiz(TL)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 55 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS**

(size) 1=14-8-7, 5=14-8-7, 6=14-8-7, 7=14-8-7, 8=14-8-7  
Max Horiz 1=-97 (LC 10)  
Max Uplift 1=-10 (LC 15), 6=-107 (LC 15), 8=-109 (LC 14)  
Max Grav 1=92 (LC 24), 5=89 (LC 34), 6=472 (LC 21), 7=327 (LC 20), 8=472 (LC 20)

**FORCES**

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-129/124, 2-3=-103/108, 3-4=-103/95, 4-5=-114/93  
BOT CHORD 1-8=-51/119, 7-8=-51/59, 6-7=-51/59, 5-6=-51/91  
WEBS 3-7=-249/22, 2-8=-388/147, 4-6=-388/146

**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-7 to 3-0-7, Interior (1) 3-0-7 to 4-4-10, Exterior(2R) 4-4-10 to 10-4-10, Interior (1) 10-4-10 to 11-8-14, Exterior(2E) 11-8-14 to 14-8-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1, 109 lb uplift at joint 8 and 107 lb uplift at joint 6.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard



October 5, 2022

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



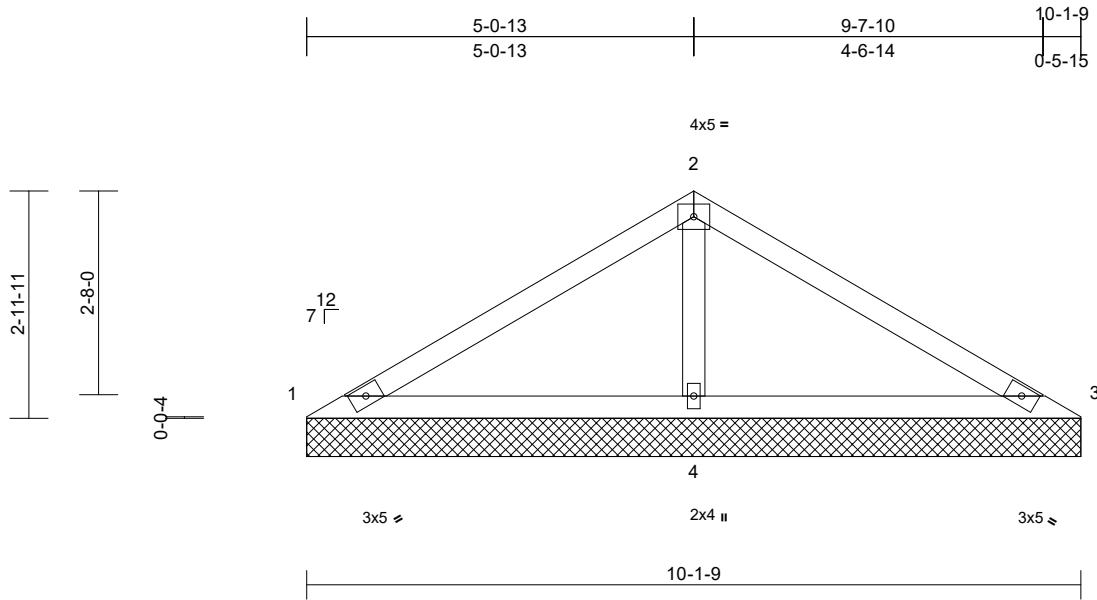
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss V3	Truss Type Valley	Qty 1	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556816
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Carter Components (Sanford), Sanford, NC - 27332,

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Page: 1



Scale = 1:30.2

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.46	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.43	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.14	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 34 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS**

(size) 1=10-1-9, 3=10-1-9, 4=10-1-9  
Max Horiz 1=-66 (LC 10)  
Max Uplift 1=-46 (LC 21), 3=-46 (LC 20), 4=-73 (LC 14)  
Max Grav 1=122 (LC 20), 3=122 (LC 21), 4=787 (LC 21)

**FORCES**

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-117/419, 2-3=-117/419  
BOT CHORD 1-4=-270/141, 3-4=-270/141  
WEBS 2-4=-607/209

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-7 to 3-0-7, Exterior(2R) 3-0-7 to 7-2-0, Exterior(2E) 7-2-0 to 10-2-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1, 46 lb uplift at joint 3 and 73 lb uplift at joint 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard



October 5, 2022

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



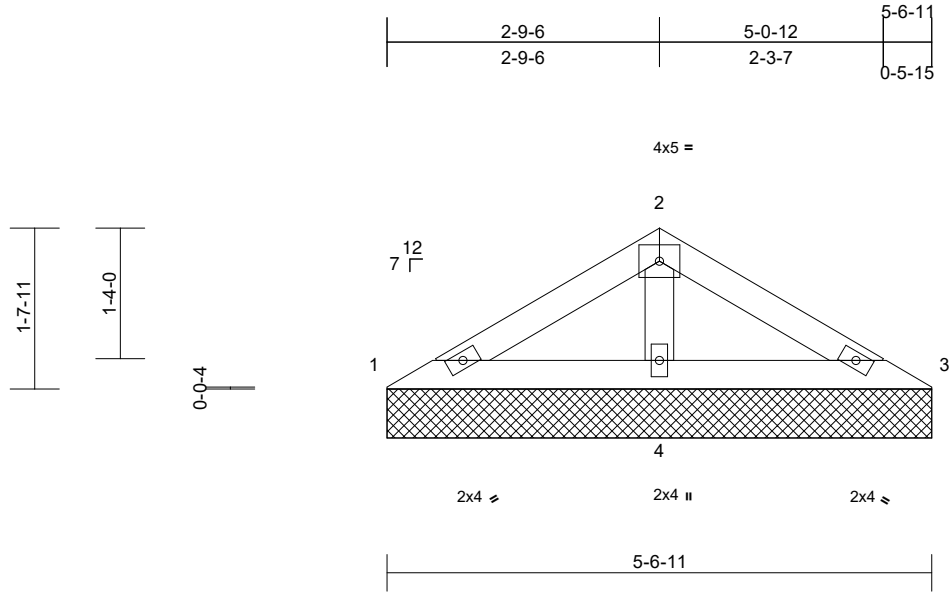
818 Soundside Road  
Edenton, NC 27932

Job 22090050	Truss V4	Truss Type Valley	Qty 1	Ply 1	DRB GROUP - 91 FaNC Job Reference (optional)	154556817
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Carter Components (Sanford), Sanford, NC - 27332,

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Page: 1



Scale = 1:23.5

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.13	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horiz(TL)	0.00	4	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP							
BCDL	10.0									Weight: 18 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-11 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

#### REACTIONS

(size) 1=5-6-11, 3=5-6-11, 4=5-6-11  
Max Horiz 1=-35 (LC 12)  
Max Uplift 1=-7 (LC 14), 3=-13 (LC 15), 4=-27 (LC 14)  
Max Grav 1=94 (LC 20), 3=94 (LC 21), 4=338 (LC 20)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-97/138, 2-3=-97/138  
BOT CHORD 1-4=-105/85, 3-4=-105/85  
WEBS 2-4=-213/106

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 1, 13 lb uplift at joint 3 and 27 lb uplift at joint 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



October 5, 2022

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component

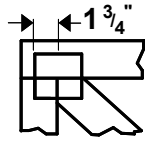
Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ENGINEERING BY  
**TRENCO**  
A MiTek Affiliate

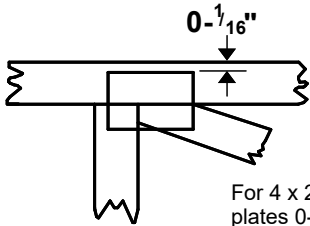
818 Soundside Road  
Edenton, NC 27932

# Symbols

## PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

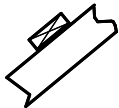
\* Plate location details available in MiTek 20/20 software or upon request.

## PLATE SIZE

4 x 4

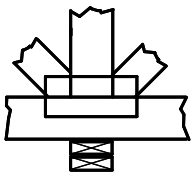
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

## BEARING

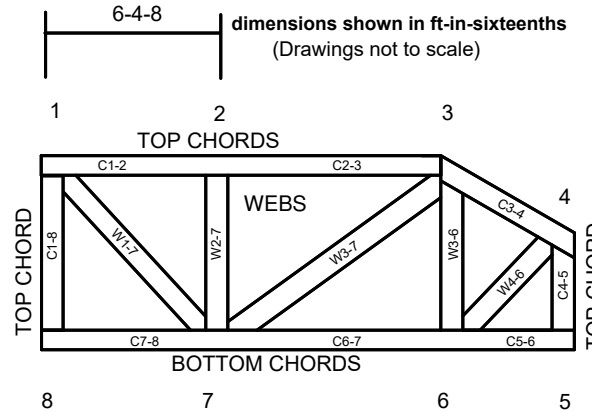


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

## Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988  
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

# General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.