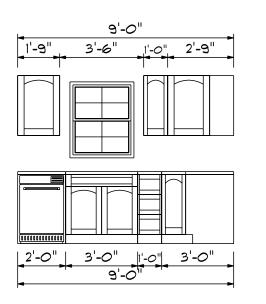
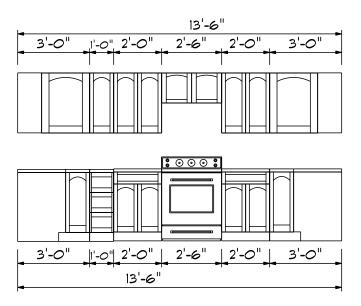
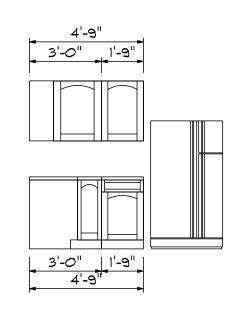


Kitchen Cabinets







1		
	2'-0"	_2'-0"
	4'-	0"
	-	

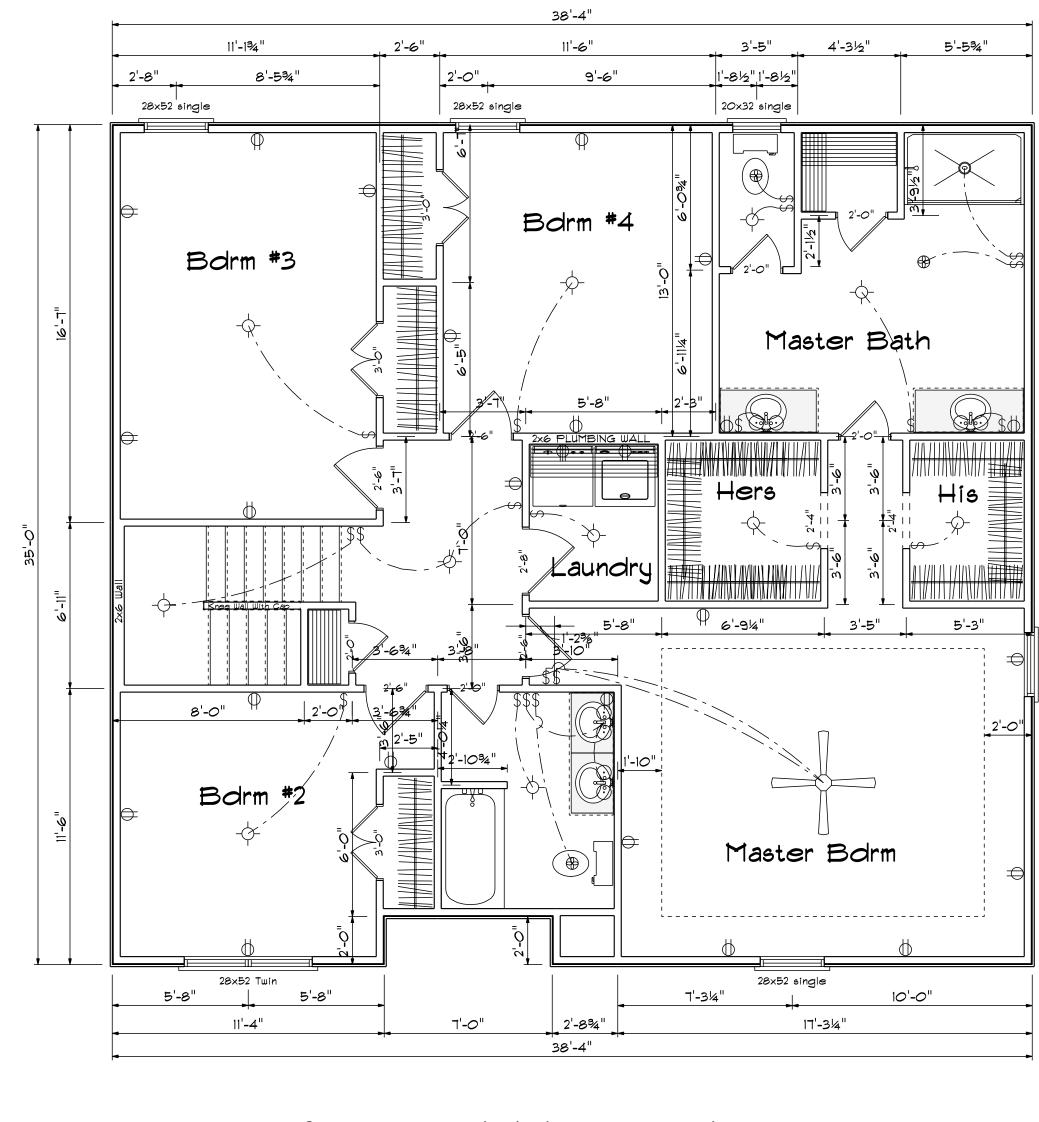
FIRST FLOOR OPENING SCHEDULE								
PRODUCT CODE	SIZE	HINGE	COUNT					
36X80 COLONIAL A	3'-0"	R	1					
7X16 GARAGE DOOR	16'-0"	U	1					
72X80 SLIDING FRENCH 2	6'-0"	NL	1					
20 colonial	2'-0"	L	1					
20 colonial	2'-0"	R	1					
30 doublehung colonial	3'-0"	LR	1					
32X80 COLONIAL A 1	2'-8"	R	1					
24X24 CASEMENT 1	2'-0" x 2'-0"	N	2					
28x52 Twin	5'-4" x 5'-2"	NA	1					
28x52 twin	5'-4" x 5'-2"	NA	1					
28x32 single	2'-8" x 3'-2"	N	1					

Areas

First Floor	929
Second Floor	1270
	=======
Total Heated	2199
Garage	413
Porch	69

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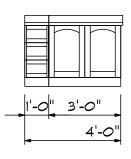
> Cypress ↓ 20 4



Second Floor Plan

Scale: 1/4"= 1'-0"





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12'-6"

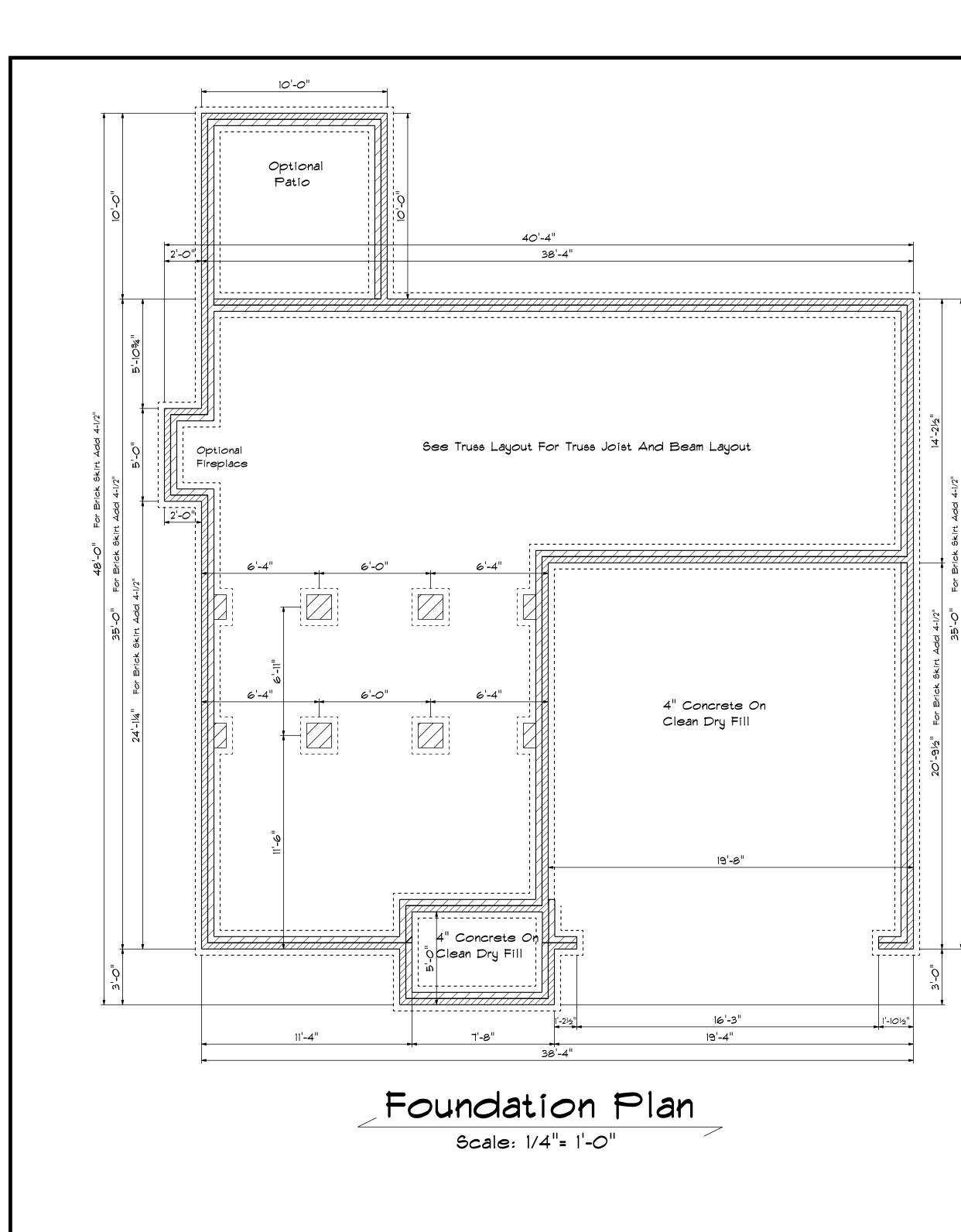
2'-0"	2'-6"				
4'-6"					

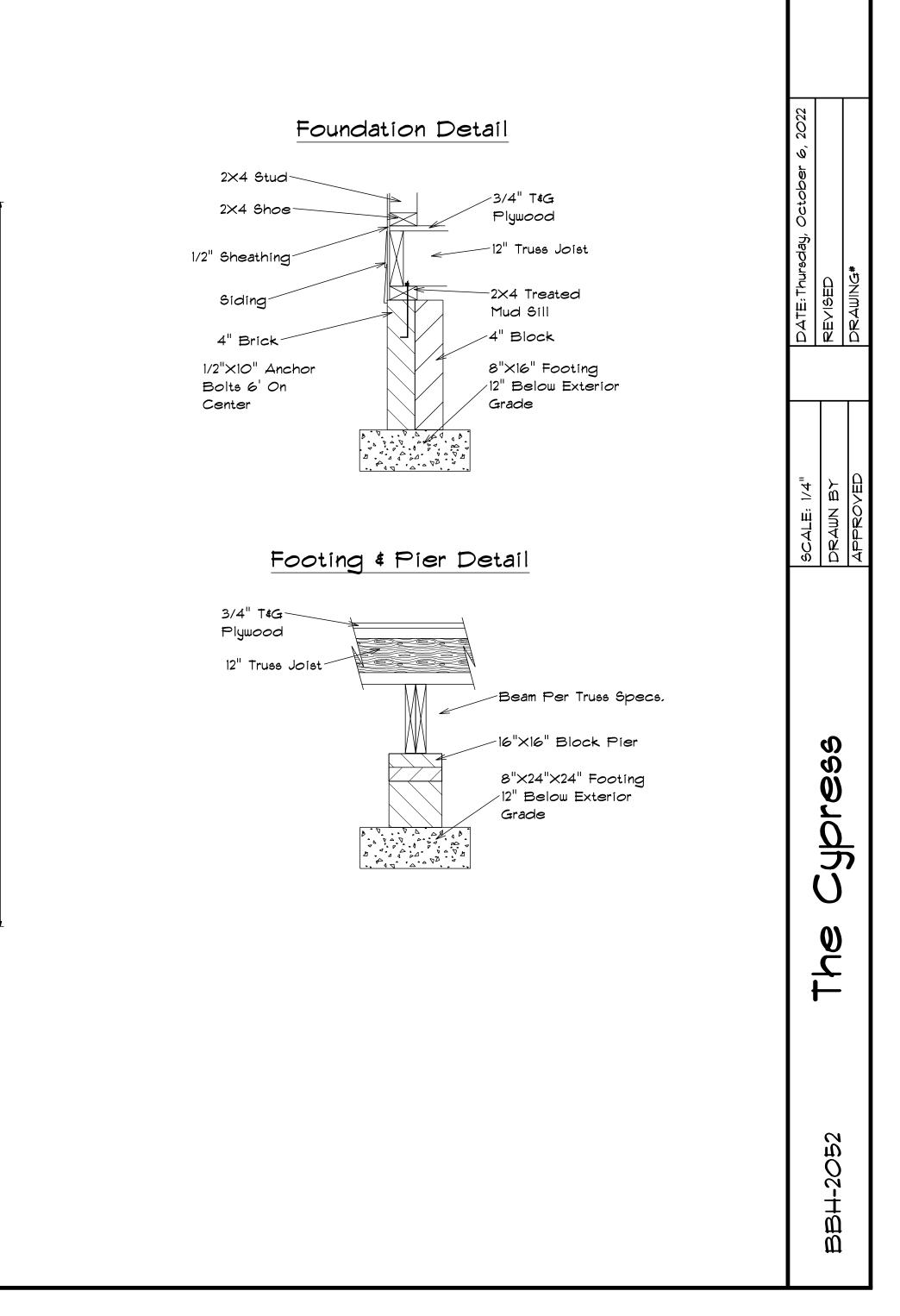
Hall Bath Cabinets

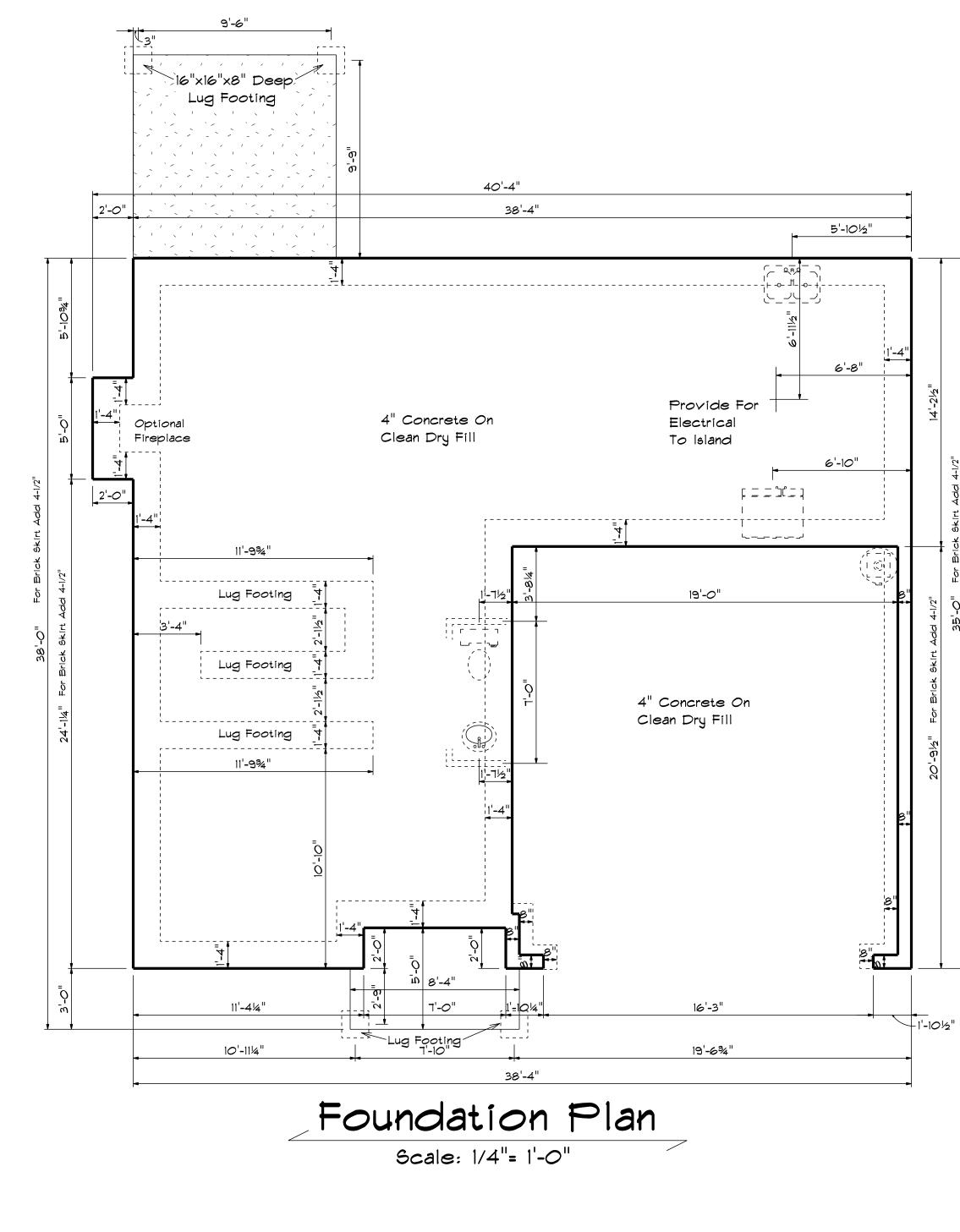
2'-6"	2'-6"					
5'-0"						

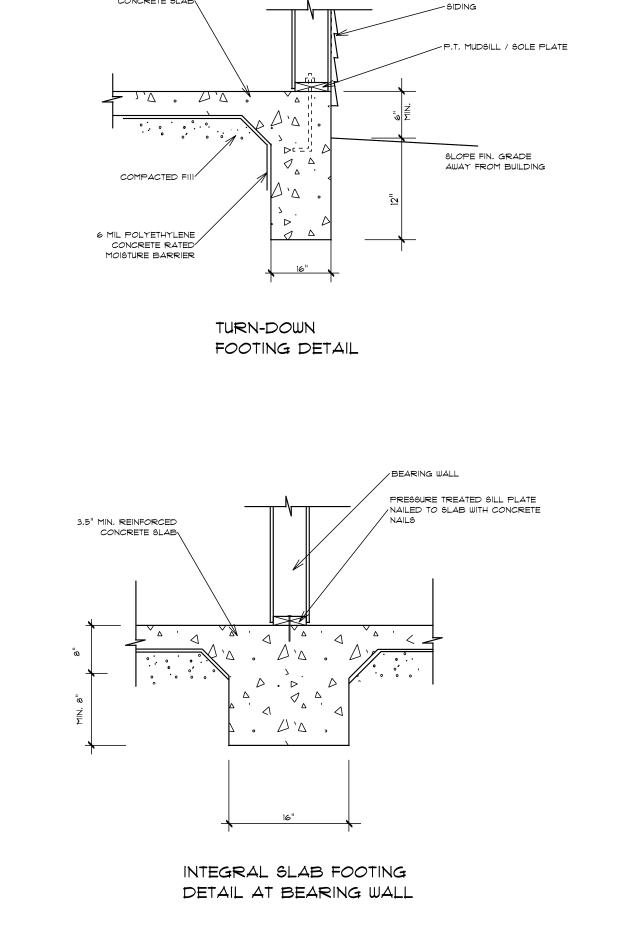
SECOND FLOOR OPENING SCHEDULE								
PRODUCT CODE	SIZE	HINGE	COUNT					
2-0 Door Unit	2'-0"	R	1					
20 cased opening	2'-0"	N	2					
20 colonial	2'-0"	R	3					
26 colonial	2'-6"	L	4					
3-0 Doublehung Door Unit	4'-0"	LR	3					
28 colonial	2'-8"	R	1					
20x32 single	2'-0" x 3'-2"	N	1					
28x52 Twin	5'-4" x 5'-2"	NA	1					
28x52 single	2'-8" x 5'-2"	N	4					











3,5" MIN, REINFORCED CONCRETE &LAB

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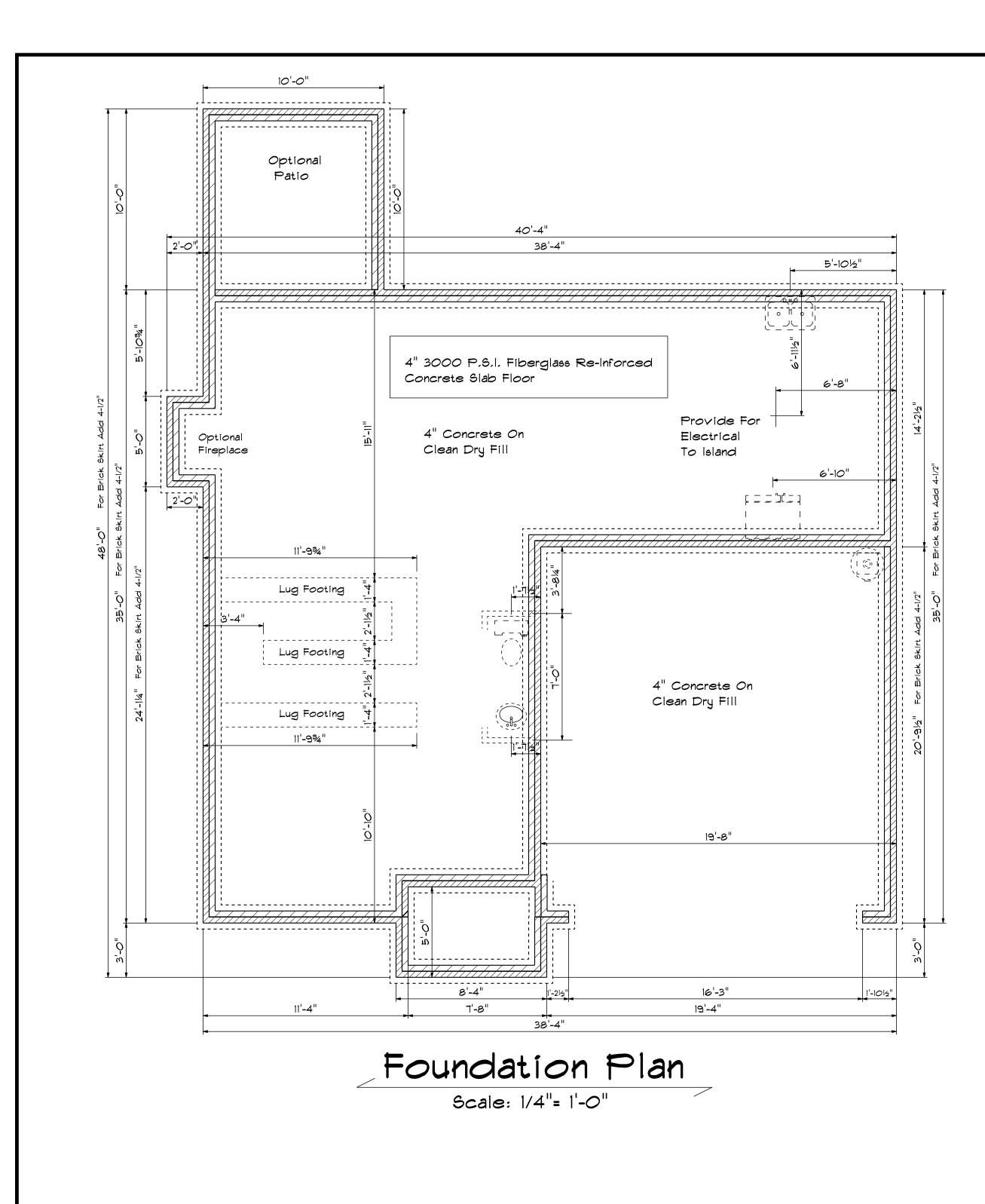
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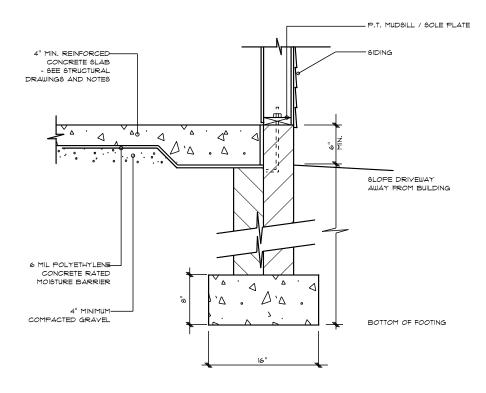
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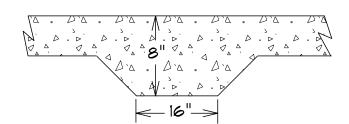
20 Ó Ö Ü Ó DATE:Thursday, (REVISED DRAWING# SCALE: 1/4" DRAWN BY APPROVED

> Cypress 17 Ø





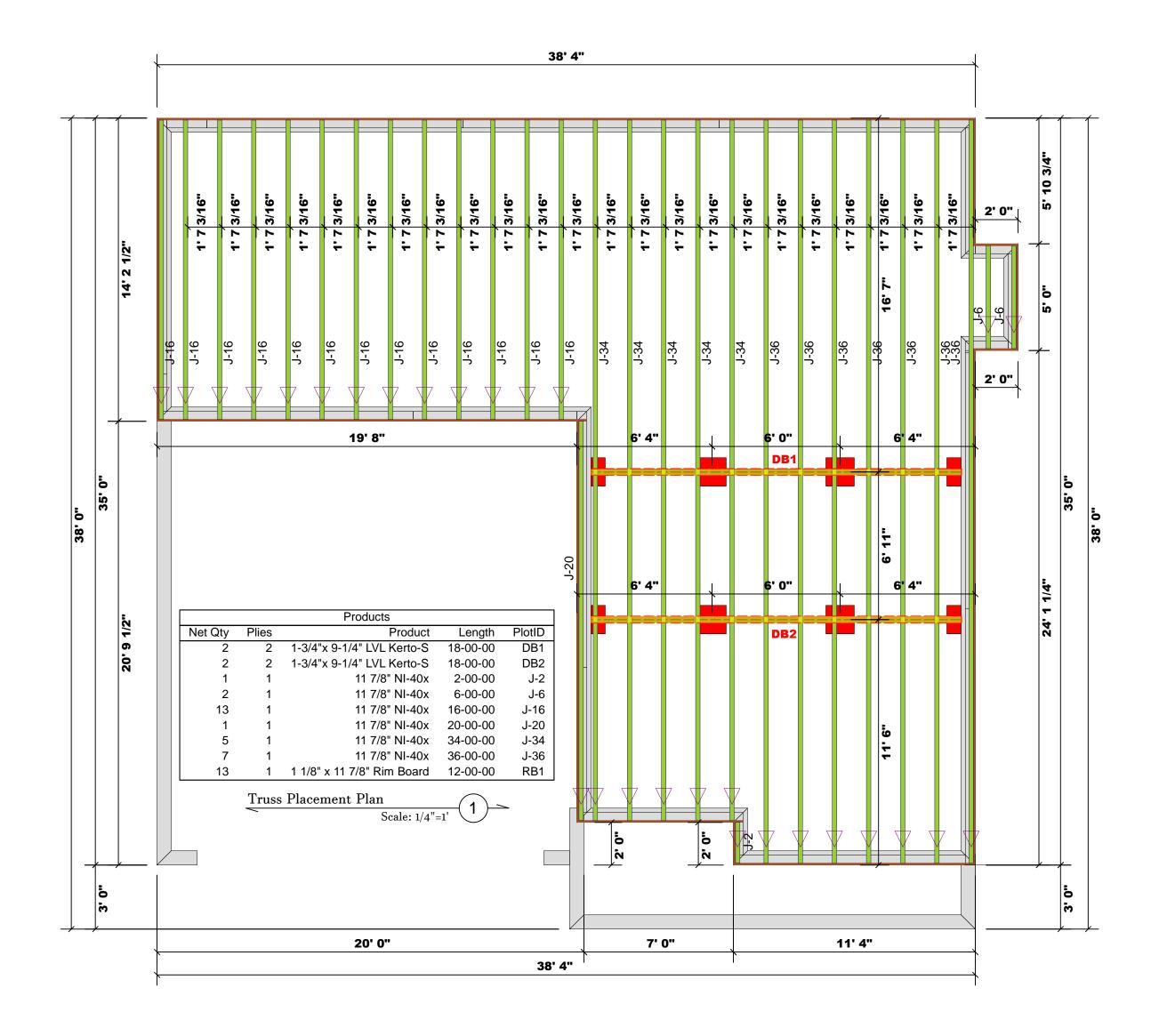
STEM WALL FOOTING DETAIL



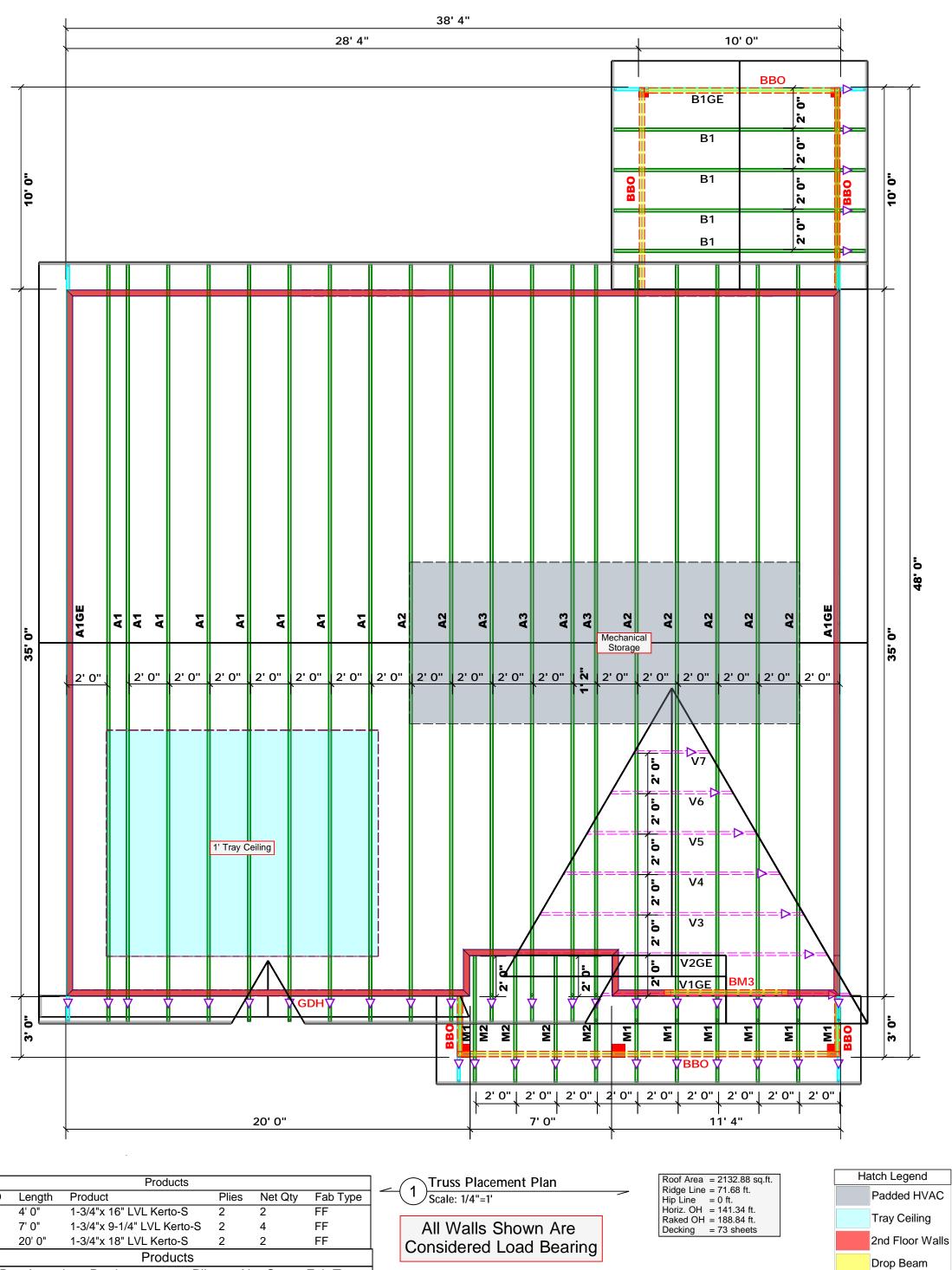
LUG FOOTING DETAIL

SCALE: 1/4" DATE: Thursday, October 6, 2022 DRAWN BY REVISED APPROVED DRAWING#

The Cypress

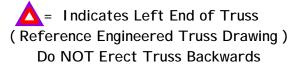


		CO	m	те					
	COMTECH ROOF & FLOOR TRUSSES & BEAMS Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444								
de rei att rei siz rei 15 rei Ta rei rei Ta	emed quiren ached quiren ze and action 000#. tained action bles.	reactions to comply nents. The Tables (nents) to number of s greater A register to design that exce A register to design that exce a stat exce	less that y with the contract derived f determin of wood s than 3000 red design the suppeds thos ed design the supped 1500	n or equa prescrip tor shall i rom the p e the mir studs req bit but no n profess port syste oprt syste 0#.	I to 3000# otive Code refer to the prescription imum fou uired to s t greater for sional sha ional shal	e e ve Code indation upport than II be y attached II be			
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		(BASED	ON TABL	ES R502.5(CK STU 1) & (b)) 0 @ EA END				
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	CITY / CO . Harnett County / Harnett	92 Melvin Lane	I-Joists Over Crawl	01/04/23	DRAWN BY David Landry	SALES REP. Marshall Naylor			
	CITY / CO.	ADDRESS	WODEL	DATE REV.	DRAWN BY	SALES REP			
	Ben Stout Real Estate	Lot 50 Liberty Meadows	Cypress	N/A		J0123-0018			
	BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #			
T d S icd p f c a d c	AKASOKTHIS IS A TRUSS PLACEMENT DIAGRAM ONLY.These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer.See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com								



Ploducis							
PlotID	Length	Product		Plies	Net Qty	Fab Type	1
BM1	4' 0"	1-3/4"x 16" LVL Kerto-	S	2	2	FF	
BM2	7' 0"	1-3/4"x 9-1/4" LVL Ker	to-S	2	4	FF	
GDH	20' 0"	1-3/4"x 18" LVL Kerto-	S	2	2	FF	
Products							
PlotID	Length	n Product	Plie	S	Net Qty	Fab Type	
BM3	8' 0"	2x12 SP No.2	2		2	FF]
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R	eilly R Fayet Phon	load Ir teville	ndustr , N.C. 0) 864	ial Par 28309 -8787 1444	k
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13600 15300	8 9				
CITY / CO. Harnett Co. / Harnett	92 Melvill Lane	Roof	. 08/10/22	DRAWN BY David Landry	SALES REP. Marshall Naylor
сі ТҮ / со	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REF
Benjamin Stout Real Estate	Lot 50 Liberty Meadows	Cypress / 2GLF, CP	N/A		J0822-4076
BUILDER	JOB NAME	PLAN	SEAL DATE N/A	QUOTE #	JOB #
These to comport design See ind identified designed perman for the support	russes ar nents to b at the sp lividual de ed on the er is resp ent braci overall st t structure	re designe be incorpo ecification esign she placemen onsible fo ng of the ructure. T e includir	ed as ind prated int n of the b rets for ea nt drawin pr tempor roof and The desig ng header	GRAM ON ividual buil o the buil ouilding de ach truss g. The bu ary and floor sys n of the tu s, beams, of the buil	ilding ding esigner. design ilding tem and russ , walls,



for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the <u>truss delivery package or online @ sbcindustry.con</u>

Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise



RE: J0822-4076 Lot 50 Liberty Meadows Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Customer: Benjamin Stout Real Estate Project Name: J0822-4076 Lot/Block: 50 Model: Cypress Address: 92 Melvill Lane City:

Subdivision: Liberty Meadows State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.3 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E16497598	A1	12/23/2021
2	E16497599	A1GE	12/23/2021
3	E16497600	A2	12/23/2021
4	E16497601	A3	12/23/2021
5	E16497602	B1	12/23/2021
6	E16497603	B1GE	12/23/2021
7	E16497604	M1	12/23/2021
8	E16497605	M2	12/23/2021
9	E16497606	V1GE	12/23/2021
10	E16497607	V2GE	12/23/2021
11	E16497608	V3	12/23/2021
12	E16497609	V4	12/23/2021
13	E16497610	V5	12/23/2021
14	E16497611	V6	12/23/2021
15	E16497612	V7	12/23/2021

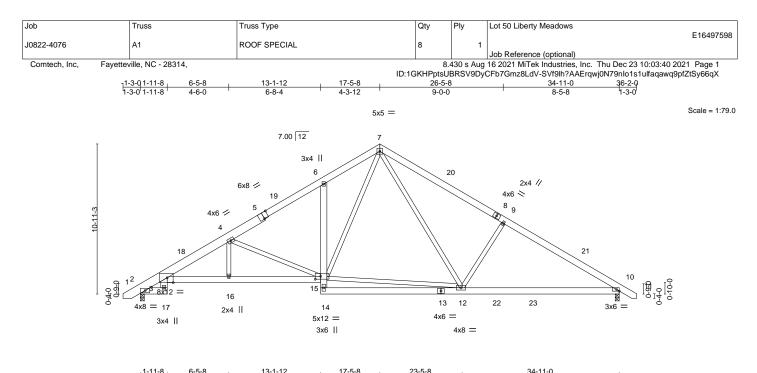
The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



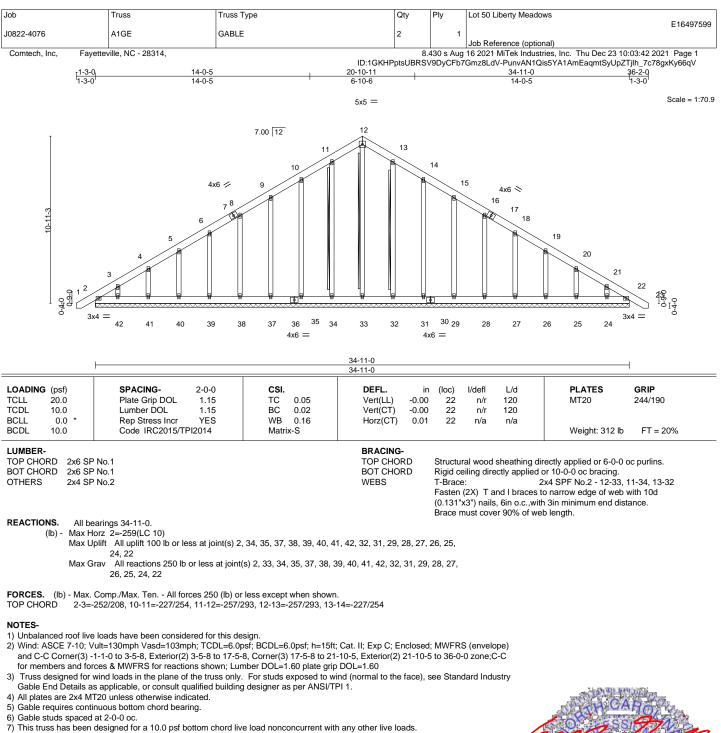
	1-11-8		13-1-12	17-5-8				34-11-0		
	1-11-8	4-6-0	6-8-4	4-3-12	6-0-0			11-5-8	1	
Plate Offsets (X,Y) [[3:0-5-4,Edge], [5	5:0-4-0,Edge], [15:0-4-	12,0-2-8]						1	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING Plate Grip Lumber DO Rep Stress	DOL 1.15 OL 1.15	CSI. TC 0.46 BC 0.57 WB 0.97	7	· · /	in (loc) 14 10-12 29 10-12 16 10	>999 >999	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0		2015/TPI2014	Matrix-S		Wind(LL) 0.			240	Weight: 281 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP No.1 *Except* 1-5: 2x8 SP 2400F 2.0E BRACING- TOP CHORD Structural wood sheathing directly applied or 5-0-6 oc purlins. BOT CHORD BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2						oc purlins.				
REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=-259(LC 10) Max Uplift 2=-93(LC 12), 10=-95(LC 13) Max Grav 2=1450(LC 1), 10=1459(LC 1) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-944/196, 3-4=-2689/491, 4-6=-1959/432, 6-7=-1921/538, 7-9=-1933/492, 9-10=-2149/442 BOT CHORD 3-16=-332/2550, 15-16=-329/2548, 6-15=-254/197, 12-14=-2/353, 10-12=-250/1758										
		5=-10/881, 7-15=-228/								
for members and for 3) This truss has been 4) * This truss has been	ult=130mph Vasd -0-11-4 to 3-5-9, I ces & MWFRS fo designed for a 10 n designed for a li ottom chord and a connection (by ott d in accordance v	d=103mph; TCDL=6.0 Interior(1) 3-5-9 to 17- r reactions shown; Lu 0.0 psf bottom chord liv ive load of 30.0psf on any other members, w hers) of truss to beari	psf; BCDL=6.0psf; 5-8, Exterior(2) 17 mber DOL=1.60 pl ve load nonconcur the bottom chord i vith BCDL = 10.0ps ng plate capable o	-5-8 to 2 late grip l rent with n all area sf. f withstar	1-10-5, Interior(1) 21- DOL=1.60 any other live loads. as where a rectangle nding 100 lb uplift at ji	10-5 to 3 3-6-0 tall bint(s) 2,	6-0-Ò zone; by 2-0-0 wi 10.	c-ć	CALL SE	



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

TDENEG

818 Soundside Road Edenton, NC 27932



8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 34, 35, 37, 38, 39, 40, 41, 42, 32, 31, 29, 28, 27, 26, 25, 24, 22.

10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

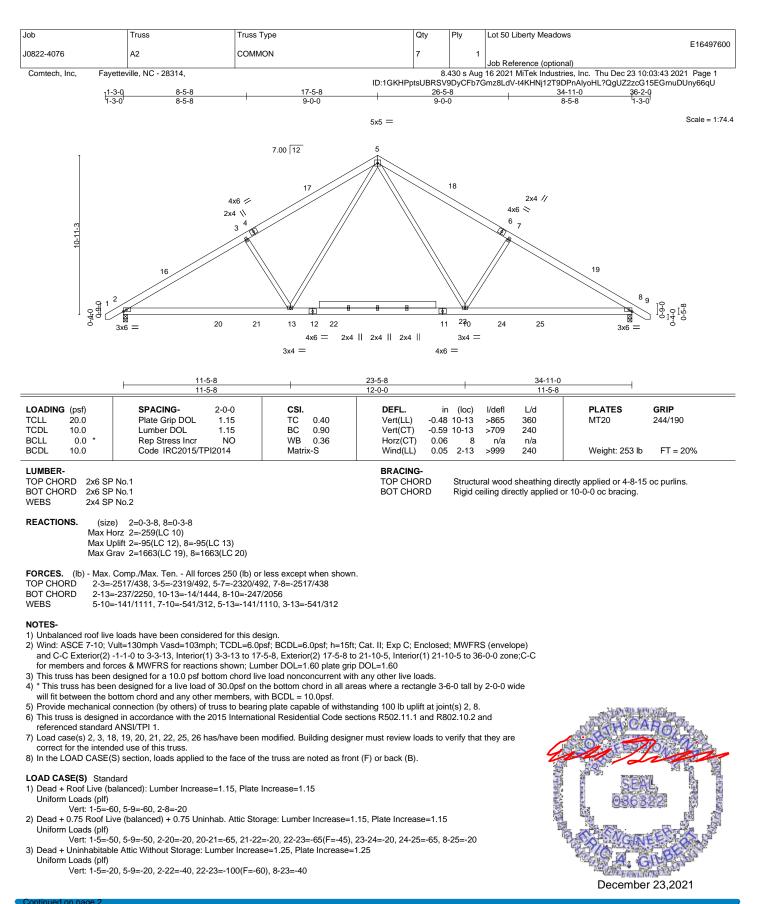
11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



December 23,2021



🗥 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange, delivery, erection and bracing of trusses and truss even and/or chord members only. Additional building design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is to here only upon parameters and properly incorporate this design is the overall building designer must verify the applicability of design parameters and properly incorporate this design is the overall building designer must verify the applicability of design parameters and properly incorporate this design is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings systems, see **ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 50 Liberty Meadows
					E16497600
J0822-4076	A2	COMMON	7	1	
					Job Reference (optional)
Comtech, Inc, Fa	etteville, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Thu Dec 23 10:03:43 2021 Page 2

ID:1GKHPptsUBRSV9DyCFb7Gmz8LdV-t4KHNj12T9DPnAlyoHL?QgUZ2zcG15EGrnuDUny66qU

LOAD CASE(S) Standard

 Dead + Uninhabitable Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-20, 5-9=-20, 2-20=-20, 20-21=-80, 21-22=-20, 22-23=-80(F=-60), 23-24=-20, 24-25=-80, 8-25=-20

19) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf) Vert: 1-2=-56, 2-5=-61, 5-8=-43, 8-9=-38, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20

Horz: 1-2=6, 2-5=11, 5-8=7, 8-9=12

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-38, 2-5=-43, 5-8=-61, 8-9=-56, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20 Horz: 1-2=-12, 2-5=-7, 5-8=-11, 8-9=-6

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-31, 2-5=-36, 5-8=-45, 8-9=-40, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20

Horz: 1-2=-19, 2-5=-14, 5-8=5, 8-9=10

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-40, 2-5=-45, 5-8=-36, 8-9=-31, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20

Horz: 1-2=-10, 2-5=-5, 5-8=14, 8-9=19 25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

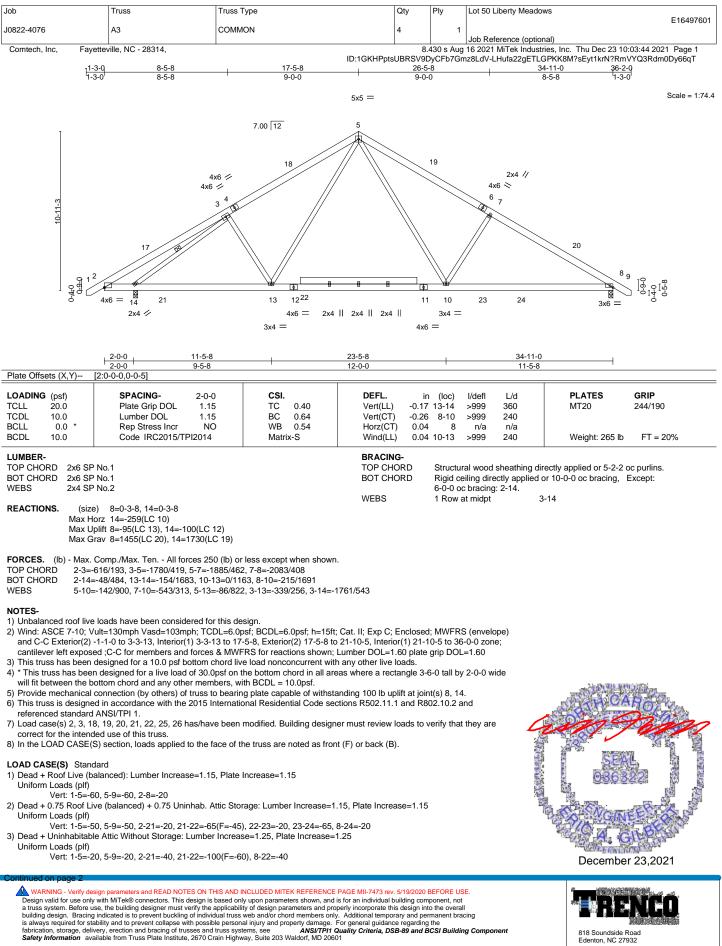
Vert: 1-5=-50, 5-9=-20, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20

26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-20, 5-9=-50, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20

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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 50 Liberty Meadows
					E16497601
J0822-4076	A3	COMMON	4	1	
					Job Reference (optional)
Comtech, Inc,	Fayetteville, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Thu Dec 23 10:03:45 2021 Page 2

ID:1GKHPptsUBRSV9DyCFb7Gmz8LdV-pTS2oO3J?nT71UvLwiNTV5ZvamKfVyoZI5NKYfy66qS

LOAD CASE(S) Standard

- 18) Dead + Uninhabitable Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-5=-20, 5-9=-20, 2-21=-20, 21-22=-80(F=-60), 22-23=-20, 23-24=-80, 8-24=-20

19) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-56, 2-5=-61, 5-8=-43, 8-9=-38, 2-14=-3, 14-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

Horz: 1-2=6, 2-5=11, 5-8=7, 8-9=12 20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-38, 2-5=-43, 5-8=-61, 8-9=-56, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

- Horz: 1-2=-12, 2-5=-7, 5-8=-11, 8-9=-6
- 21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-31, 2-5=-36, 5-8=-45, 8-9=-40, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

- Horz: 1-2=-19, 2-5=-14, 5-8=5, 8-9=10
- 22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-40, 2-5=-45, 5-8=-36, 8-9=-31, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20 Horz: 1-2=-10, 2-5=-5, 5-8=14, 8-9=19

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-50, 5-9=-20, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

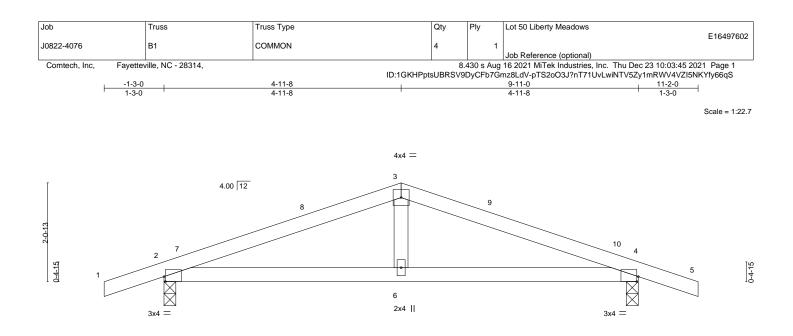
26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-20, 5-9=-50, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

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		11-8 11-8		<u>9-11-0</u> 4-11-8	
Plate Offsets (X,Y)	[2:0-0-6,Edge], [4:0-0-6,Edge]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.24 BC 0.20 WB 0.05 Matrix-S	DEFL. i Vert(LL) 0.04 Vert(CT) -0.05 Horz(CT) 0.07	3 2-6 >999 240	PLATES GRIP MT20 244/190 Weight: 37 lb FT = 20%
Max U	No.1 No.2	11	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di Rigid ceiling directly applied	rectly applied or 6-0-0 oc purlins. or 7-8-15 oc bracing.
FORCES. (lb) - Max. TOP CHORD 2-3=- BOT CHORD 2-6=-	Comp./Max. Ten All forces 250 (lb) o 654/750, 3-4=-654/750 624/567, 4-6=-624/567 293/227	r less except when shown.			
 Wind: ASCE 7-10; V and C-C Exterior(2) left and right expose This truss has been * This truss has been will fit between the b 	loads have been considered for this d ult=130mph Vasd=103mph; TCDL=6.0 -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 4- d;C-C for members and forces & MWF designed for a 10.0 psf bottom chord li n designed for a live load of 30.0psf on ottom chord and any other members. connection (by others) of truss to beari	psf; BCDL=6.0psf; h=15ft; 11-8, Exterior(2) 4-11-8 to RS for reactions shown; Lu /e load nonconcurrent with the bottom chord in all are	9-4-5, Interior(1) 9-4-5 umber DOL=1.60 plate of any other live loads. as where a rectangle 3-	to 11-2-0 zone; porch grip DOL=1.60 -6-0 tall by 2-0-0 wide	

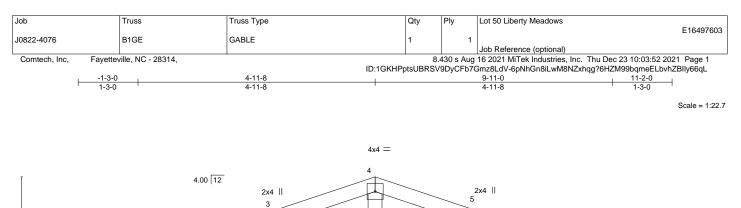
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb 2=191, 4=191.
6) This true is designed in accordance with the 2015 Interpretional Posidential Code sections RE02 11 1 and R802 10 2 and

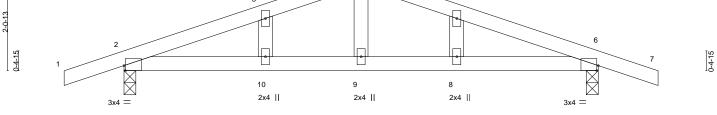
6) This trues is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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		<u>11-8</u> 11-8		<u>9-11-0</u> 4-11-8	
Plate Offsets (X,Y)	[2:0-0-6,Edge], [6:0-0-6,Edge]	11-0		4-11-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.18 BC 0.23 WB 0.04 Matrix-S	DEFL. in Vert(LL) 0.04 Vert(CT) -0.04 Horz(CT) -0.01	(loc) l/defl L/d 8 >999 240 10 >999 240 6 n/a n/a	PLATES GRIP MT20 244/190 Weight: 39 lb FT = 20%
	No.1 No.2 No.2		BRACING- TOP CHORD BOT CHORD		ing directly applied or 6-0-0 oc purlins. plied or 7-6-4 oc bracing.
Max G ORCES. (Ib) - Max. (OP CHORD 2-3=-6 OT CHORD 2-10=	rav 2=469(LC 1), 6=469(LC 1) Comp./Max. Ten All forces 250 (lb) o 555/778, 3-4=-607/789, 4-5=-607/790, -660/573, 9-10=-660/573, 8-9=-660/57 291/185	5-6=-655/778			
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; Vi gable end zone and i Lumber DOL=1.60 pl 3) Truss designed for v Gable End Details as 4) Gable studs spaced i 5) This truss has been of	vind loads in the plane of the truss only s applicable, or consult qualified buildin	psf; BCDL=6.0psf; h=15ft; ht exposed;C-C for memb- . For studs exposed to wi g designer as per ANSI/TF re load nonconcurrent with	ers and forces & MWFRS nd (normal to the face), s Pl 1. n any other live loads.	s for reactions shown; see Standard Industry	CARGE

will fit between the bottom chord and any other members. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=271, 6=271.

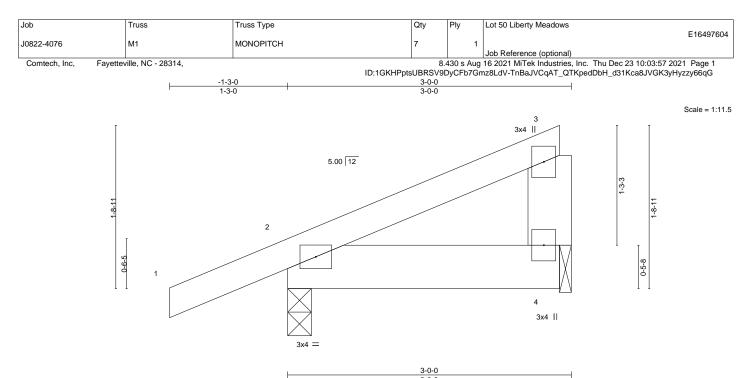
8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



December 23,2021

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							3-0-0					
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	ТС	0.09	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.02	Vert(CT)	-0.00	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-P	Wind(LL)	0.00	2-4	>999	240	Weight: 16 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=76(LC 12) Max Uplift 2=-98(LC 8), 4=-38(LC 8) Max Grav 2=210(LC 1), 4=84(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify
- capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.



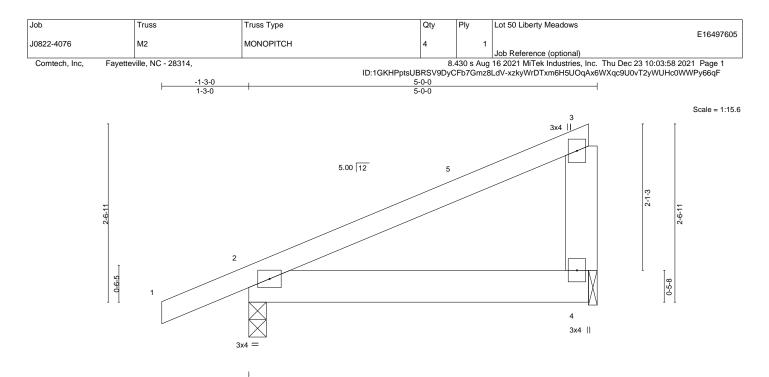
Structural wood sheathing directly applied or 3-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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LOADING	G (psf)		·0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1	.15	TC	0.26	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1	.15	BC	0.08	Vert(CT)	-0.01	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr Y	'ES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TPI20	14	Matri	x-P	Wind(LL)	0.01	2-4	>999	240	Weight: 26 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=79(LC 12)

Max Uplift 2=-83(LC 8), 4=-57(LC 8) Max Grav 2=281(LC 1), 4=174(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

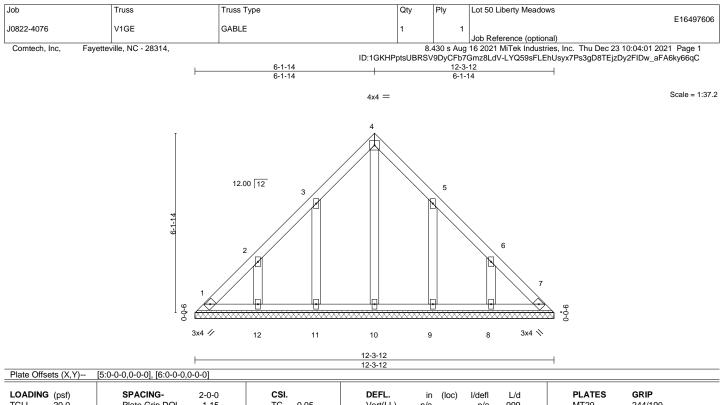
4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

- capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.05 BC 0.03 WB 0.07 Matrix-S	DEFL.in(loc)l/deflVert(LL)n/a-n/aVert(CT)n/a-n/aHorz(CT)0.007n/a	L/d 999 999 n/a	PLATES GRIP MT20 244/190 Weight: 66 lb FT = 20%
LUMBER-			BRACING-		

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-3-12.

(lb) - Max Horz 1=-174(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 11=-143(LC 12), 12=-148(LC 12), 9=-142(LC 13), 8=-149(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 11, 12, 9, 8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

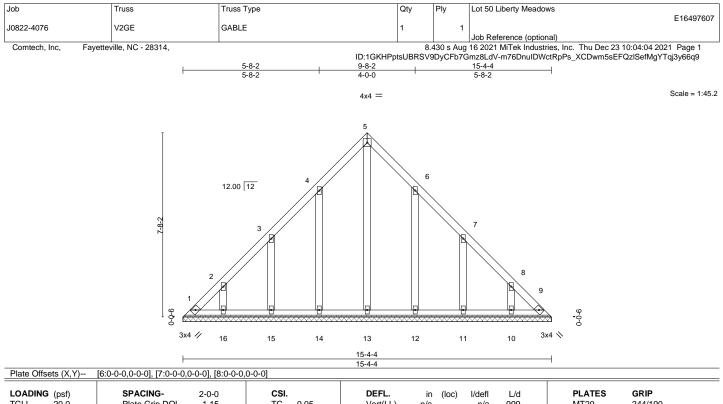
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 11=143, 12=148, 9=142, 8=149.

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.05 BC 0.03 WB 0.15 Matrix-S	Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 9 n/a n/a	M120 244/190 Weight: 92 lb FT = 20%
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) n/a - n/a 999	MT20 244/190

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.1

REACTIONS. All bearings 15-4-4

Max Horz 1=-220(LC 8) (lb) -

2x4 SP No 2

Max Uplift All uplift 100 lb or less at joint(s) 1, 9 except 14=-142(LC 12), 15=-143(LC 12), 16=-128(LC 12), 12=-140(LC 13), 11=-144(LC 13), 10=-128(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 16, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-290/181, 8-9=-255/169

NOTES-

OTHERS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9 except (jt=lb) 14=142, 15=143, 16=128, 12=140, 11=144, 10=128.

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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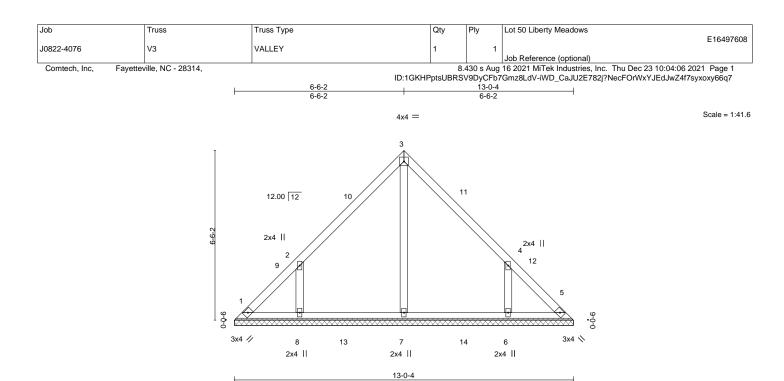


Plate Offsets (X,Y) [4:0	0-0-0,0-0-0]				
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/	/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.14	Vert(LL) n/a -	n/a 999	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.15	Vert(CT) n/a -	n/a 999	
3CLL 0.0 *	Rep Stress Incr YES	WB 0.09	Horz(CT) 0.00 5	n/a n/a	
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S	· ·		Weight: 60 lb FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD

2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. All bearings 13-0-4

(lb) - Max Horz 1=-148(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-163(LC 12), 6=-162(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=384(LC 19), 8=374(LC 19), 6=374(LC 20)

WEBS 2-8=-358/290, 4-6=-358/290

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-6-2, Exterior(2) 6-6-2 to 10-10-15, Interior(1) 10-10-15 to 12-8-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=163, 6=162,

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



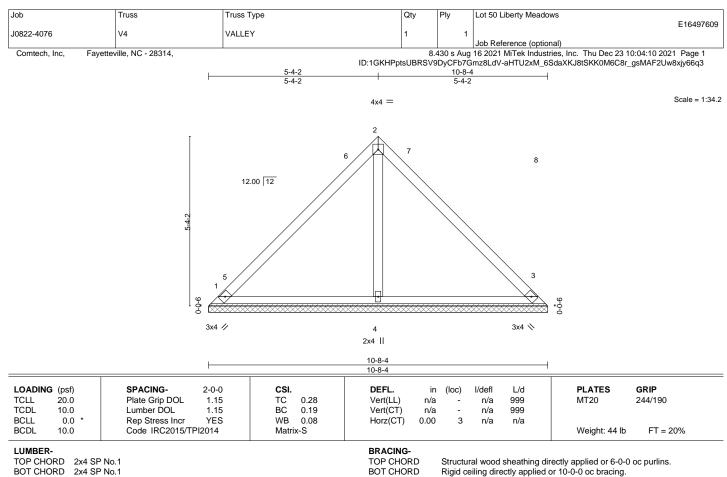
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛕 WARNING - Verify design pa rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange, delivery, erection and bracing of trusses and truss even and/or chord members only. Additional building design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is to here only upon parameters and properly incorporate this design is the overall building designer must verify the applicability of design parameters and properly incorporate this design is the overall building designer must verify the applicability of design parameters and properly incorporate this design is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings systems, see **ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=10-8-4, 3=10-8-4, 4=10-8-4

Max Horz 1=-120(LC 8)

Max Uplift 1=-30(LC 13), 3=-30(LC 13)

Max Grav 1=226(LC 1), 3=226(LC 1), 4=346(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

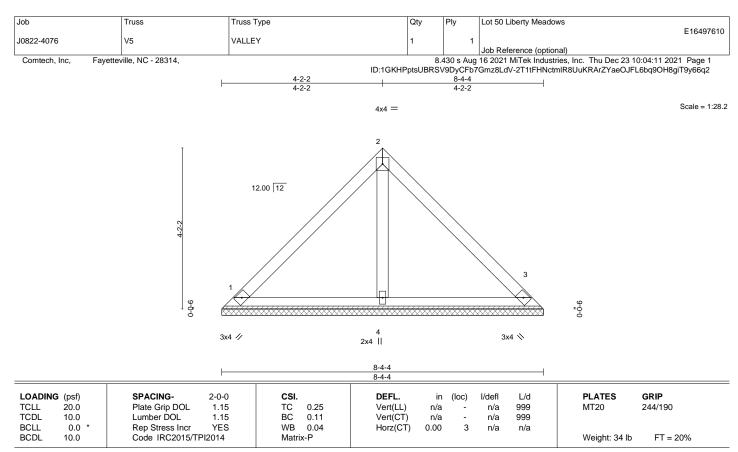
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-4-2, Exterior(2) 5-4-2 to 9-8-15, Interior(1) 9-8-15 to 10-4-0 zone;C-C for
- members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🛕 WARNING - Verify design pa rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This down the seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSETPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=8-4-4, 3=8-4-4, 4=8-4-4

Max Horz 1=92(LC 9) Max Uplift 1=-33(LC 13), 3=-33(LC 13)

Max Grav 1=186(LC 1), 3=186(LC 1), 4=239(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0ps; BCDL=6.0ps; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

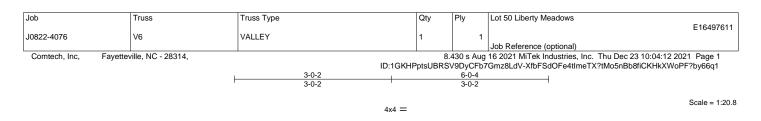


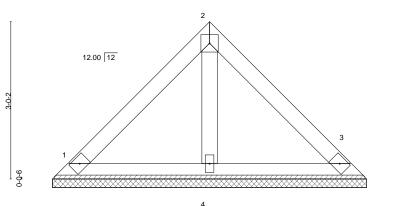
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛕 WARNING - Verify design pa rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This down the seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSETPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601









2x4 ||

LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TP	12014	Matri	x-P						Weight: 24 lb	FT = 20%
		1									-	
LUMBER	t-					BRACING-						

TOP CHORD

BOT CHORD

6-0-4

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=6-0-4, 3=6-0-4, 4=6-0-4

Max Horz 1=64(LC 9)

Max Uplift 1=-23(LC 13), 3=-23(LC 13)

Max Grav 1=129(LC 1), 3=130(LC 1), 4=166(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0ps; BCDL=6.0ps; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



0-<u>0</u>-6

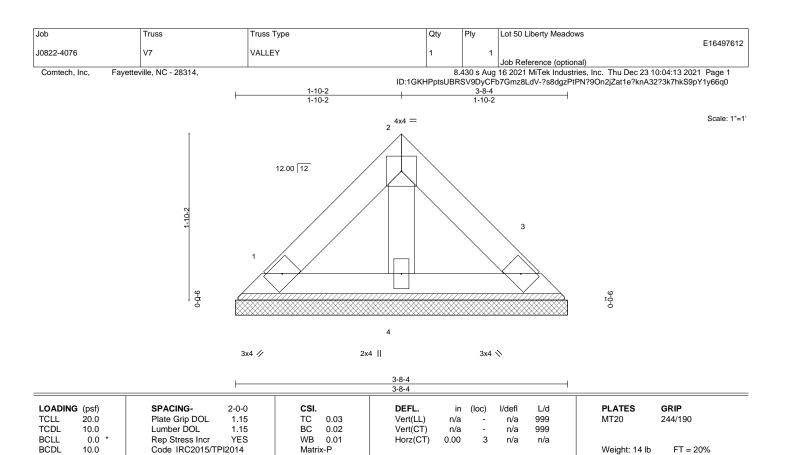
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

3x4 🔨

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling in trusses and truss systems, see **AVSUPPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

н	111	MP	۶F	R.	

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=3-8-4, 3=3-8-4, 4=3-8-4

Max Horz 1=-36(LC 8) Max Uplift 1=-13(LC 13), 3=-13(LC 13)

Max Grav 1=73(LC 1), 3=73(LC 1), 4=93(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0ps; BCDL=6.0ps; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

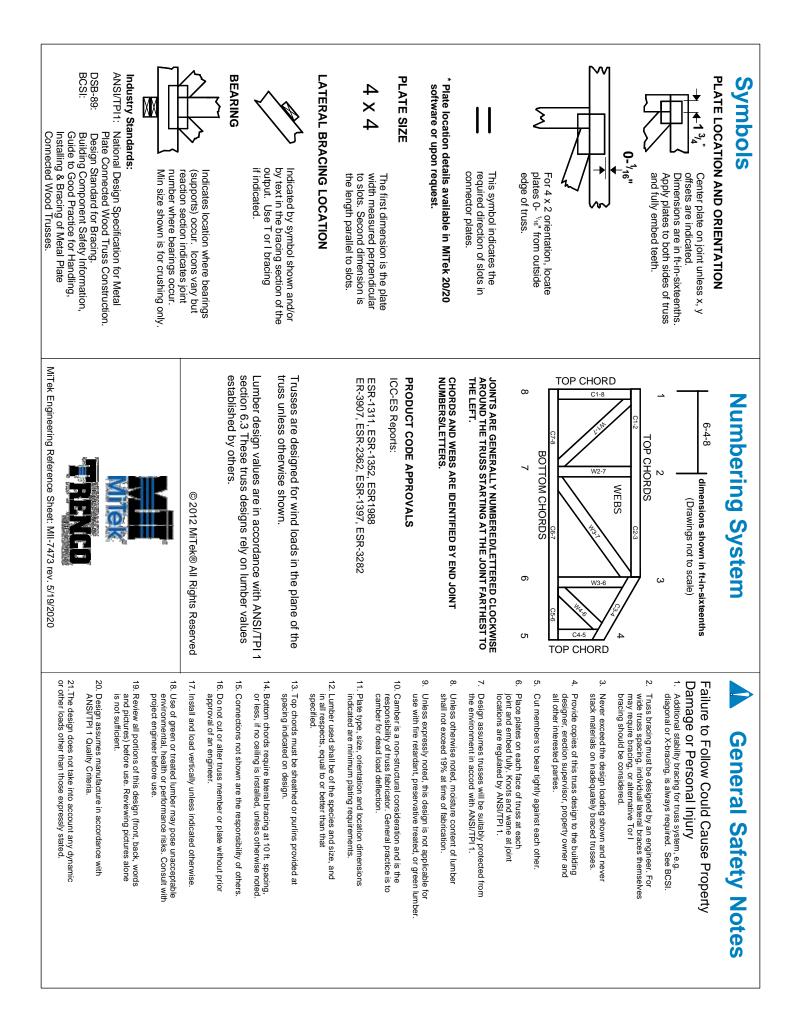


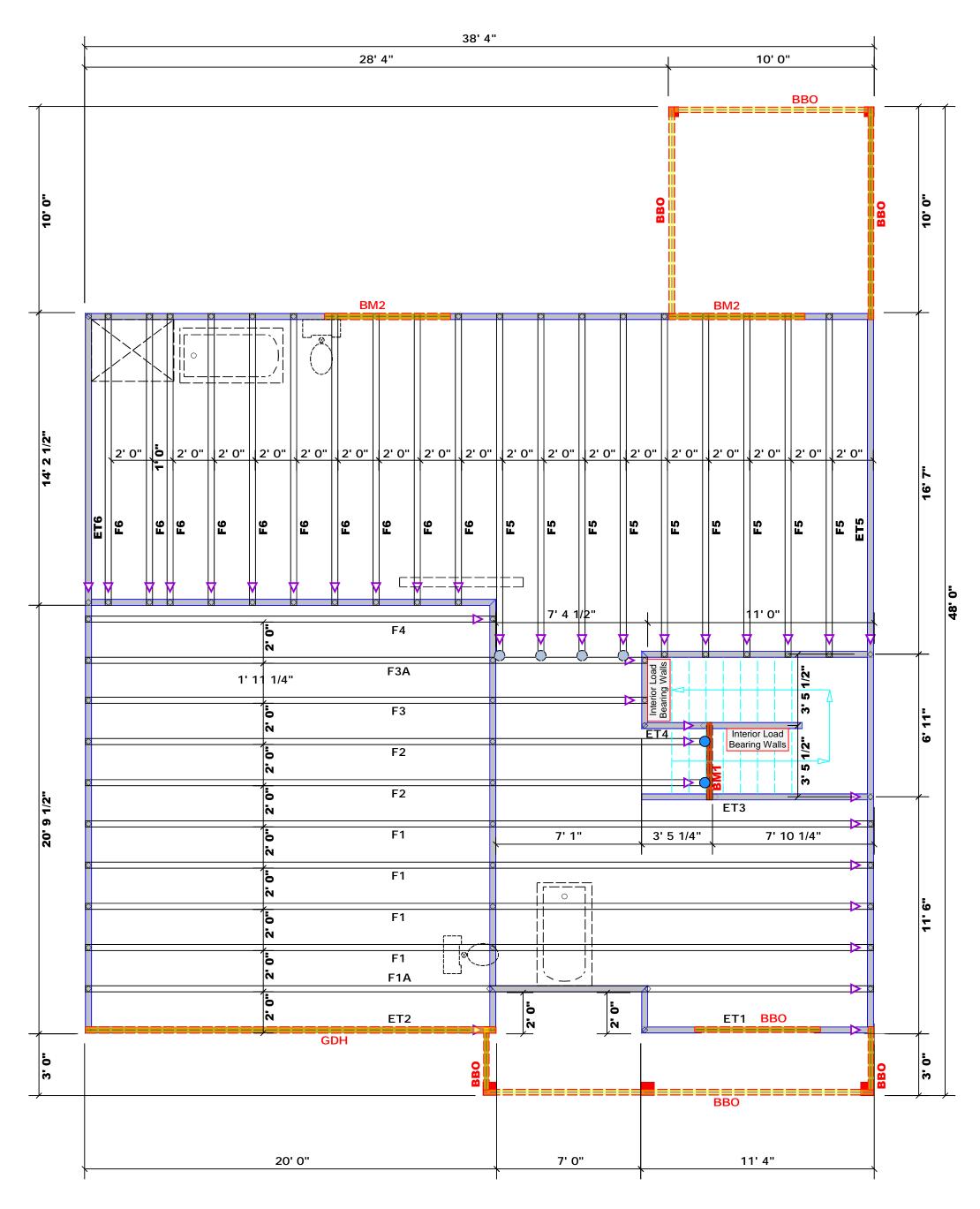
Structural wood sheathing directly applied or 3-8-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





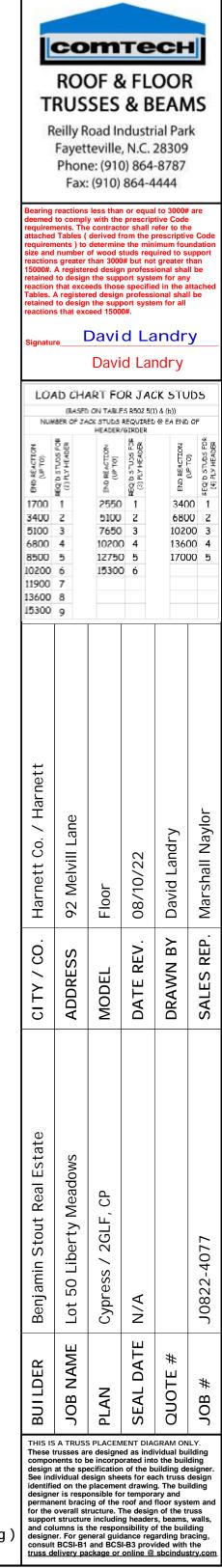


		Products				Truss Placement Plan
PlotID	Length	Product	Plies	Net Qty	Fab Type	Scale: 1/4"=1'
BM1	4' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF	
BM2	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	4	FF	All Walls Shown Are
GDH	20' 0"	1-3/4"x 18" LVL Kerto-S	2	2	FF	Considered Load Bearing
						Considered Load Dearing

Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

	Conne	ctor Info	on	Nail Information		
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	MSH422	USP	4	Varies	10d/3"	10d/3"
\bigcirc	HUS410	USP	2	NA	16d/3-1/2"	16d/3-1/2"

Plumbing Drop Notes
 Plumbing drop locations shown are NOT exact. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses. Adjust spacing as needed not to exceed 24"oc.



A = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

100	-	Client:	Benjamin Stout Rea	al Estate		Dat	te:	8/10/202	2			Page 1 of
í.	Destina		Cypress				ut by:	David La	-			
IS	Design	Address:	92 Melvill Lane						berty Meadows			
		4 760" V	46 000"		100		ject #:	J0822-40 evel: Level				
BM1 I	Kerto-S LVL	. 1./50 [°] X	16.000	2-Ply - P	A33	DED						
1 SPF	1 2 5 3'9" 3'9"	SPF										1'4"
Member In	formation				Reac	tion		ATTERN	IED lb (Uplif	t)		
Туре:	Girder	Applicati	on: Floor		Brg	Direc		Live	· ·	Snow	Wind	Cons
Plies:	2	Design M	lethod: ASD		1	Vertic		345		0	0	(
Moisture Con		Building		C 2015	2	Vertic	al	345	140	0	0	
Deflection LL: Deflection TL:		Load Sha Deck:	aring: No Not Ch	ockod								
Importance:	Normal - II	Deck.	NOL CH	eckeu								
Temperature:	Temp <= 100°F											
•	·				Bear	ings						
					Bea	ring l	Length	Dir.	Cap. React D/	'L lb Tota	Ld. Case	Ld. Comb
					1 - 5	SPF :	3.500"	Vert	9% 140 /	345 48	5 L	D+L
Analusi- P	culto	1			2 - 5	SPF 3	3.500"	Vert	9% 140 /	345 48	5 L	D+L
Analysis Re			Oran aite Oran		7							
Analysis		cation Allowed 10 1/2" 34565 ft-lb	Capacity Cor									
Moment Unbraced		'10 1/2" 34565 ft-lb '10 1/2" 29800 ft-lb	0.010 (1%) D+L 0.012 (1%) D+L									
Shear		2'1 1/2" 11947 lb	0.034 (3%) D+L									
LL Defl inch		'10 1/2" 0.083 (L/480)	. ,	 L								
LE Den mon	(L/54618)	10 112 01000 (2.100)	01000 (170) 2	-								
TL Defl inch		'10 1/2" 0.166 (L/240)	0.006 (1%) D+L	. L								
	(L/38886)				┥							
Design Not			at the set of the	a Lateral	4							
may also b 2 Fasten all p to exceed 6 3 Refer to las 4 Girders are 5 Top must b 6 Bottom mu	pport to prevent lateral e required at the interio blies using 3 rows of 10 s". It page of calculations f designed to be suppor e laterally braced at en st be laterally braced at derness ratio based or	r bearings by the build d Box nails (.128x3") a or fasteners required fo ted on the bottom edge d bearings. t end bearings.	ing code. t 12" o.c. Maximum or specified loads.									
ID	Load Type		Trib Width Side	Dead 0.9	<u>י</u> ו	_ive 1	Snow	/ 1.15	Wind 1.6 Con	st. 1.25 C	omments	
1	Uniform			Face 62 PLF		4 PLF		0 PLF	0 PLF	0 PLF F2		
	Self Weight			12 PLF								
structural adequacy design criteria and responsibility of the ensure the compor application, and to ver	Designs is responsible only of the of this component based on the loadings shown. It is the ustomer and/or the contractor to ent suitability of the intendec	 LVL beams must not be cut Refer to manufacturer regarding installation fastening details, beam st approvals 	n or drilled s product information requirements, multi-ply rength values, and code	6. For flat roofs provide p ponding	roper drain	age to p	N 3 N (lorwalk, CT 800) 622-58	l Building, 2nd Floor 06851	Fayette	Reilly Road, Suite ville, NC	#639
Lumber 1. Dry service condit	ions, unless noted otherwise	 Damaged Beams must not Design assumes top edge i 	s laterally restrained							-		
 Ury service condit LVL not to be treat 	ions, unless noted otherwise ted with fire retardant or corrosive	5 Provide lateral support at	bearing points to avoid	This design is valio	until 11/3	3/2024					COMT	есн

Client:	Benjamin Stout Real Estate	Date: 8/10/20	22 Page 2 of
Project:	Cypress	Input by: David L	andry
isDesign Address:	92 Melvill Lane	Job Name: Lot 50 I	liberty Meadows
		Project #: J0822-4	4075
BM1 Kerto-S LVL 1.750'	' X 16.000" 2-Ply -	PASSED Level: Level	21
	x 101000 _ 11j		
			,
• • • •			
<u>8</u>			WW
			1'4"
			· · · · ·
3'9"			1 13 1/2"
3'9"			
Multi-Ply Analysis			
Fasten all plies using 3 rows of 10d Box nail	ls (.128x3") at 12" o.c Maximu	Im end distance not to exc	eed 6".
Capacity 50.1 %			
Load 123.0 PLF			
Yield Limit per Foot245.6 PLFYield Limit per Fastener81.9 lb.			
Yield Mode IV			
Edge Distance 1 1/2"			
Min. End Distance 3"			
Load Combination D+L			
Duration Factor 1.00			
		do around draining to around Manufactu	rer Info Comtech, Inc.
Notes chemicals Calculated Structured Designs is responsible only of the Handling & Instal		de proper drainage to prevent Metsä Woo	1001 S. Reilly Road, Suite #639 d Fayetteville, NC
structural adequacy of this component based on the 1 LVL beams must not design criteria and loadings shown. It is the 2 Refer to manufa		301 Merritt	7 Building, 2nd Floor USA 28314
responsibility of the customer and/or the contractor to ensure the component suitability of the intended fastening details by	icturer's product information ion requirements, multi-ply eam strength values, and code	Norwalk, C (800) 622-5	1 06851 910-864-TRUS 5850
application, and to verify the dimensions and loads approvals Lumber 3. Damaged Beams mu	ust not be used		wood.com/us
1. Dry service conditions, unless noted otherwise 4. Design assumes top 5. Provide lateral sup	edge is laterally restrained port at bearing points to avoid		соттесн
2. LVL not to be treated with fire retardant or corrosive lateral displacement		valid until 11/3/2024	Conneon
Varcian 21.80.417 Powarad by iStruct™ Datacat: 22061001.1			and any second second

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

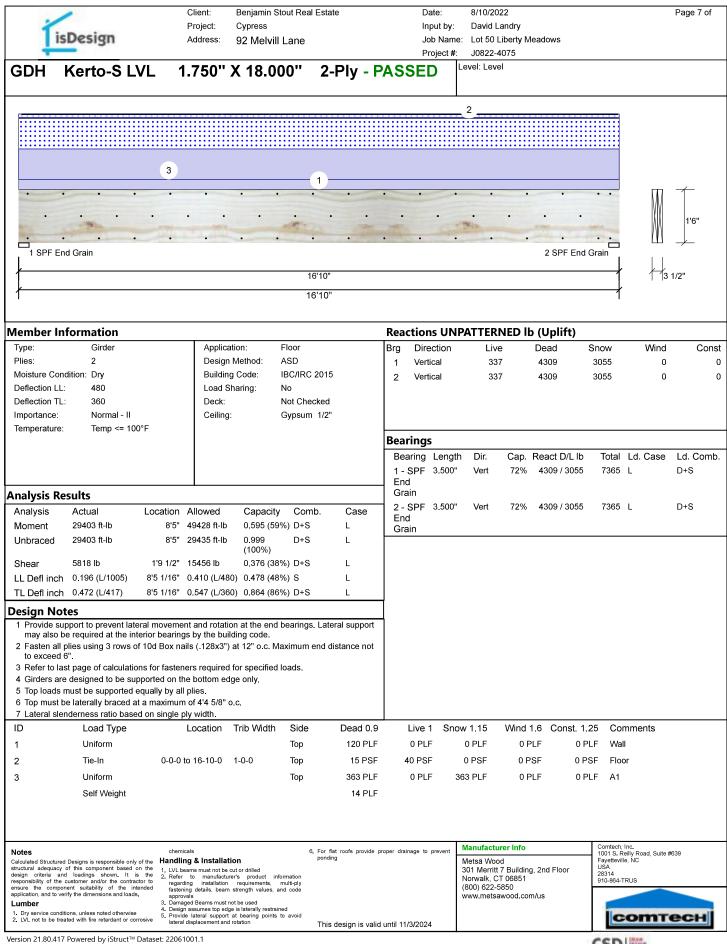
	-			Benjamin S	Stout Real	Estate		ate:	8/10/202				Page 3 of
Tis	Design		Project: Address:	Cypress 92 Melvil	llane			put by: b Name:	David La : Lot 50 Li	indry berty Meadows			
	- Congin							oject #:	J0822-40	-			
3 M 2	Kerto-S L	.VL 1	1.750"	X 9.2	250''	2-Ply -	PASSE	ED L	_evel: Level				
							3						
	(2											
•	•	•		•		•	•	-	•			M	1 T
	C. T. Man		-		atter		-		The law			XI)	9
•	Harter Party	•	A House	•		•		-	•			ĹV	
1 SPF	End Grain			0171			2 SF	PF End (Grain				
				6'7"								1	⁻ 3 1/2"
I				6'7"					I				
/lember In	formation						Reaction	s UNF	PATTERN	IED lb (Uplift)		
Type: Plies:	Girder 2		Applicat		Floor ASD		Ŭ	ection	Live		Snow	Wind	Cons
Moisture Con			Design I Building		ASD IBC/IRC	2015	1 Verti 2 Verti		1109 1109		1369 1369	0 0	
Deflection LL			Load Sh	naring:	No		2						
Deflection TL			Deck:		Not Cheo	cked							
Importance: Temperature:	Normal - II Temp <= 100)°F											
							Bearings						
							Bearing 1 - SPF	-	ı Dir. Vert	Cap. React D/L 39% 2160 / 18		I Ld. Case	Ld. Comb D+0.75(L+
Analysis Re	sults						End Grain						
Analysis	Actual	Location A		Capacity			2 - SPF End	3.500"	Vert	39% 2160 / 18	359 4019	9 L	D+0.75(L+
Moment	5726 ft-lb	3'3 1/2" 1			0%) D+0.7		Grain						
Unbraced Shear	5726 ft-lb 2727 lb	3'3 1/2" 1 1' 3/4" 7			5%) D+0.7 1%) D+0.7	. ,							
LL Defl inch).153 (L/480		,								
	0.104 (L/706)).306 (L/240										
Design No			,	<i>,</i> ,	,	. ,							
1 Provide su	pport to prevent late required at the inf				bearings.	Lateral support	-1						
2 Fasten all	plies using 2 rows o	-	-	-	1aximum e	end distance not							
to exceed 3 Refer to la	6". st page of calculatio	ns for fastene	rs required f	for specified	loads.								
	e designed to be su			je only.									
	must be supported e be laterally braced a												
7 Bottom mu	st be laterally brace	d at end bear	ings.										
ID	nderness ratio base Load Type			Trib Width	Side	Dead 0.9	Dive 1	1 Snov	w 1.15	Wind 1.6 Cons	t. 1.25 C	omments	
1	Uniform				Тор	113 PLF	= 337 PLF	=	0 PLF	0 PLF	0 PLF FS	5	
2	Uniform				Тор	416 PLF	= 0 PLF	= 4	16 PLF	0 PLF	0 PLF A2	2	
3	Uniform				Тор	120 PLF	= 0 PLF	=	0 PLF	0 PLF	0 PLF W	all	
	Self Weight					7 PLF	=						
Notes		chemica	ls		6.	For flat roofs provide	proper drainage to	prevent	Manufactur	er Info	Comtec 1001 S	h, Inc. Reilly Road, Suite i	#639
Calculated Structured structural adequacy	Designs is responsible only of this component based of	n the 1. LVL bea	g & Installations must not be cu	ut or drilled		ponding		Г	Metsä Wood 301 Merritt 7	l ' Building, 2nd Floor	Fayette USA	ville, NC	
responsibility of the	d loadings shown. It is customer and/or the contrac nent suitability of the int	the 2. Refer tor to regardin	to manufacture g installation	r's product in requirements,	multi-ply				Norwalk, CT (800) 622-58	06851	28314 910-864	I-TRUS	
ensure the compo application, and to ve Lumber	rify the dimensions and loads	approva	g details, beam ∉ Is d Beams must no		and code					vood.com/us			
	tions, unless noted otherwise	4. Design a 5. Provide	assumes top edge		ined to avoid						17	COMT	есні
	ated with fire retardant or cor	rosive	isplacement and r	otation		This design is val							

120	1	(Client:	Benjamin Stout Re	eal Estate	Da	ate:	8/10/2022	Page 4 of
1		F	Project:	Cypress			put by:	David Landry	
	isDesign	A	Address:	92 Melvill Lane	!			Lot 50 Liberty Meadows	
							roject #:	J0822-4075 evel: Level	
BM2	Kerto-S L	VL 1	1.750	' X 9.250"	2-Ply	- PASSI	ין בב		
									,
•	•	•		•	•	•		•	M T
								•	9 1/2
•	•	•		•	•	•		• — — — — — — — — — — — — — — — — — — —	
1 SF	PF End Grain					2 SI	PF End G	Grain	
1				6'7"				1	1 13 1/2"
1				6'7"				1	
Multi-Ply	7 Analysis								
Fasten all	plies using 2 rows	of 10d B	ox nails (.128x3") at 12"	o.c Maximu	um end dista	nce no	t to exceed 6".	
Capacity		0.0 %							
Load Yield Limit pe	er Foot	0.0 PLF 163.7 PLF							
Yield Limit pe		81.9 lb.							
Yield Mode Edge Distanc	ce .	IV 1 1/2"							
Min. End Dist	tance	3"							
Load Combin Duration Fact		1.00							
								Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Notes Calculated Structu	ured Designs is responsible only of		g & Installati		For flat roofs provi ponding	ide proper drainage to	prevent	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC USA
design criteria responsibility of t	acy of this component based on and loadings shown. It is the customer and/or the contracto	the 2 Refer r to regardin	ms must not be c to manufacture g installation	ut or drilled er's product information requirements, multi-ply			I	301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
ensure the con application, and to	nponent suitability of the inten o verify the dimensions and loads.	ded fastening approval	g details, beam Is	strength values, and code				(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service co	onditions, unless noted otherwise	 Design a Provide 	d Beams must no assumes top edge lateral support :	ot be used e is laterally restrained at bearing points to avoid					COMTROUM
2. LVL not to be	treated with fire retardant or corro	sive lateral di	isplacement and i	rotation	This design is v	/alid until 11/3/2024	4		COMTECH
Version 21.80.4	417 Powered by iStruct™ Da	ataset: 220610	011						CCD LORAW

Version 21.80.417 Powered by iStruct™ Dataset: 22061001

is	Design	Р	roject:	Benjamin S Cypress 92 Melvill	tout Real Estate Lane	2		Date: Input by: Job Name: Project #:	8/10/202 David La Lot 50 L J0822-4	andry iberty Me	eadows			Page 5 of
BM3	SP #2	2.000"	X 12.	000"	2-Ply -	PAS	SED	L	evel: Leve					
			1				ष व व व व							
	ind Grain	•			2 SPF Er	•							\mathbb{M}	11 1
			5'6"		2011 EI									;"
/			6'1"				ł							
lember In	formation						Reacti	ons UNP	ATTERN	JFD lb	(Uplift)			
Type: Plies: Moisture Cond Deflection LL: Deflection TL:	Girder 2 dition: Dry 480 240		Applicati Design M Building Load Sh Deck:	lethod: Code: aring:	Floor ASD IBC/IRC 2015 No Not Checked		Brg [1 \	Direction /ertical /ertical	Live 0 0		Dead 1265 1265	Snow 1265 1265	Wind 0 0	Co
Importance: Temperature:	Normal - II Temp <= 100	°F					Dearin							
								ng Length PF 3.500"	Dir. Vert	Cap. I 43%	React D/L lb 1265 / 1265	Total 2531	Ld. Case L	Ld. Cor D+S
nalysis Re	sults						Grain	PF 3.500"	Vert	43%	1265 / 1265	2531	I	D+S
Analysis Moment Unbraced Shear	Actual 3291 ft-lb 3291 ft-lb 1508 lb	Location A 3' 1/2" 4 3' 1/2" 4 1'2 3/4" 4	548 ft-lb 171 ft-lb	Capacity 0.723 (72 0.789 (79 0.333 (33	%) D+S %) D+S	Case L L	End Grain	F 0.000		4370	120371203	2001	L	
LL Defl inch	0.019 (L/3590)	3' 1/2" 0.	.141 (L/480)	0.134 (13	%) S	L								
TL Defl inch Design Not	0.038 (L/1795)	3' 1/2" 0.	.281 (L/240)	0.134 (13	%) D+S	L	l							
 Provide sup may also be 2 Fasten all p to exceed 6 Refer to las Girders are Top loads n Top must be Bottom must 	oport to prevent later e required at the inte blies using 2 rows of	erior bearings 10d Box nails ns for fastener ported on the qually by all pl end bearings. d at end bearir	by the build s (.128x3") a s required fo bottom edg lies ngs	ing code. t 12" o.c. M or specified	aximum end dis									
ID 1	Load Type Uniform	L	ocation T	Trib Width	Side Top	Dead 0.9 416 PLF			v 1.15 6 PLF	Wind 1 0 Pl	.6 Const. 1 ₋F 0 F	.25 Com PLF A2	iments	
									Manufactur	er Info		Comtech, Ir 1001 S. Rei Fayetteville USA 28314 910-864-TF		639

		tout Real Estate	Date: 8/10/2022	Page 6 of
LieDestern	Project: Cypress		Input by: David Landry	
isDesign	Address: 92 Melvill	Lane	Job Name: Lot 50 Liberty Meadows	
			Project #: J0822-4075 Level: Level	
BM3 SP #2	2.000" X 12.000"	2-Ply - PASSED		
••	• • •	• •		1
		112		
		<11/2"		11 1/4"
• •	• • •	· · · · · · · · · · · · · · · · · · ·		
L1 SPF End Grain		2 SPF End Grain		
	5'6"			3"
		/		
	6'1"	Ţ		
Multi-Ply Analysis				
	ows of 10d Box nails (.128x3") a	at 12" o.c. Maximum end dis	stance not to exceed 6"	
Capacity	0.0 %			
Load	0.0 PLF			
Yield Limit per Foot	202.6 PLF			
Yield Limit per Fastener Yield Mode	101.3 lb. IV			
Edge Distance	1 1/2"			
Min. End Distance	3"			
Load Combination	4.00			
Duration Factor	1.00			
			Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
				USA
				28314 910-864-TRUS
		This design is valid until 11/3/2	2024	COMTECH



	Client:	Benjamin Stout Re	al Estate	Date:	:	8/10/2022	Page 8 of
	Project:	Cypress		Input	by:	David Landry	
isDesign	Address:	92 Melvill Lane				Lot 50 Liberty Meadows	
				Proje		J0822-4075	
GDH Kerto-S LVL	1.750" >	K 18.000''	2-Ply -	PASSED		evel: Level	
			_				
	•	• •	• •	• •	•	• • •	· ·] 5, []]
	• •	• •	• •	• •		• • • •	"CT 1'6"
	•						
1 SPF End Grain						2 SPF End	Grain
			16'10"				1 1/3 1/2"
1			16'10"				
Multi-Ply Analysis							
Fasten all plies using 3 rows of 10c	d Roy pails (179,2") -+ 17"	o.c. Maximi	m and distance	0 00	t to overand 6"	
Capacity 0.0 %					eno		
Load 0.0 PLF							
Yield Limit per Foot 245.6 P	LF						
Yield Limit per Fastener 81.9 lb. Yield Mode IV							
Edge Distance 1 1/2"							
Min. End Distance 3" Load Combination							
Duration Factor 1.00							
	emicals		6. For flat roofs provi	de proper drainage to prev		Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
	L beams must not be cu	ıt or drilled	ponding			Metsä Wood 301 Merritt 7 Building, 2nd Floor	USA
design criteria and loadings shown. It is the 2 Re responsibility of the customer and/or the contractor to reg	fer to manufacture parding installation	r's product information requirements, multi-ply			1	Norwalk, CT 06851 800) 622-5850	28314 910-864-TRUS
application, and to verify the dimensions and loads. app	tening details, beam s provals maged Beams must no	strength values, and code				www.metsawood.com/us	
1. Dry service conditions, unless noted otherwise 5. Pro	sign assumes top edge ovide lateral support a	is laterally restrained it bearing points to avoid					соттесн
late	eral displacement and re	otad011	This design is v	alid until 11/3/2024			

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

CSD



RE: J0822-4077 Lot 50 Liberty Meadows Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Custo Lot/E

Customer: Benjamin Stout Real Estate Lot/Block: 50	Project Name: J0822-4077 Model: Cypress
Address: 92 Melvill Lane City:	Subdivision: Liberty Meadows State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.3 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 14 individual, dated Truss Design Drawings and 0 Additional Drawings.

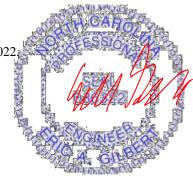
No.	Seal#	Truss Name	Date
1	E16497613	ET1	12/23/2021
2	E16497614	ET2	12/23/2021
3	E16497615	ET3	12/23/2021
4	E16497616	ET4	12/23/2021
5	E16497617	ET5	12/23/2021
6	E16497618	ET6	12/23/2021
7	E16497619	F1	12/23/2021
8	E16497620	F1A	12/23/2021
9	E16497621	F2	12/23/2021
10	E16497622	F3	12/23/2021
11	E16497623	F3A	12/23/2021
12	E16497624	F4	12/23/2021
13	E16497625	F5	12/23/2021
14	E16497626	F6	12/23/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

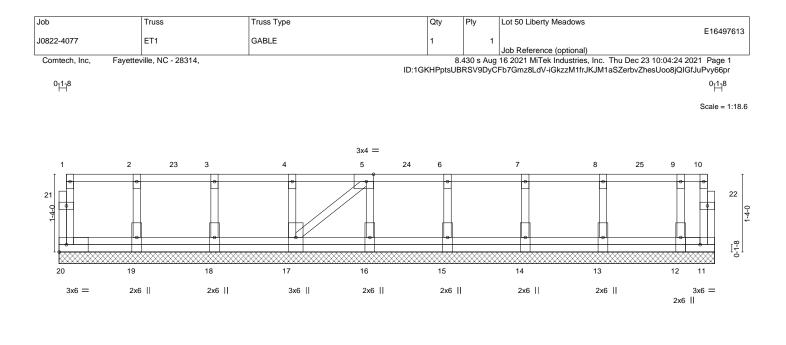
My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the

designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



December 23, 2021



L	1-4-0	2-8-0	4-0-0	5-4-0	6-8-0	8-0-0	9-4-0	10-8-0	11-3-0
	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	0-7-0
Plate	Offsets (X,Y)	[5:0-1-8,Edge]			1				
LOAI TCLL TCDL BCLL BCDL	- 10.0 . 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/	2-0-0 1.00 1.00 YES TPI2014	CSI. TC 0.07 BC 0.00 WB 0.03 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl n/a - n/a n/a - n/a 0.00 17 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 69 lb	GRIP 244/190 FT = 20%F, 11%E
	CHORD 2x4 SP CHORD 2x4 SP S 2x4 SP	No.1(flat) No.1(flat) No.3(flat) No.3(flat)			BRACING- TOP CHORD BOT CHORD	except end ver	d sheathing directly a ticals. rectly applied or 6-0-		oc purlins,

REACTIONS. All bearings 11-3-0. (lb) - Max Uplift All uplift 100 lb o

- Max Uplift All uplift 100 lb or less at joint(s) 11 Max Grav All reactions 250 lb or less at joint(s) 20, 19, 18, 17, 16, 15, 14, 13, 12

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11.

7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 11-20=-10, 1-10=-100

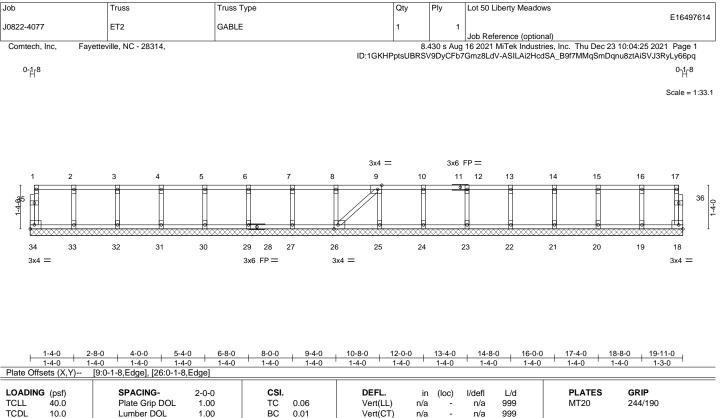
Concentrated Loads (lb) Vert: 4=-26 7=-26 23=-26 24=-26 25=-26



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932



LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. in (lo Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00 1	oc) l/defl - n/a - n/a 18 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 90 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S	P No.1(flat)		BRACING- TOP CHORD Stru	uctural wood	sheathing di	rectly applied or 6-0-0	oc purlins,

NO.1 (flat) BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WFBS 2x4 SP No.3(flat) OTHERS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 19-11-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 34, 18, 33, 32, 31, 30, 29, 27, 26, 25, 24, 23, 22, 21, 20, 19

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

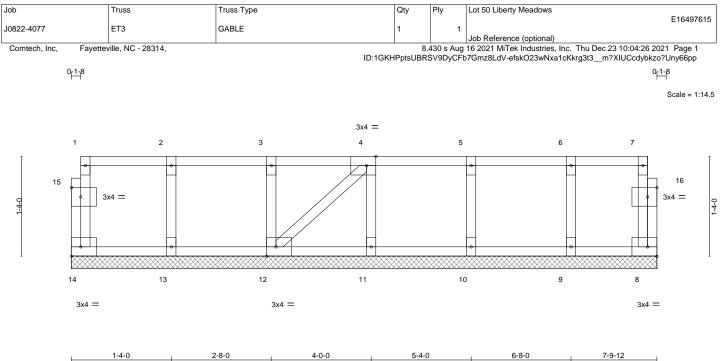
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



December 23,2021





	1-4-0 1-4-0	1-4-0	1-4-0	1-4-0	1-1-12
Plate Offsets (X,Y)	[4:0-1-8,Edge], [12:0-1-8,Edge], [15:0-	1-8,0-1-8], [16:0-1-8,0-1-8]			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-P	DEFL. in (I Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	loc) l/defl L/d - n/a 999 - n/a 999 8 n/a n/a	PLATES GRIP MT20 244/190 Weight: 39 lb FT = 20%F, 11%E
BOT CHORD 2x4 S WEBS 2x4 S	SP No.1(flat) SP No.1(flat) SP No.3(flat) SP No.3(flat)		ex	tructural wood sheathing dire coept end verticals. igid ceiling directly applied or	ctly applied or 6-0-0 oc purlins, 10-0-0 oc bracing.

REACTIONS. All bearings 7-9-12.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 14, 8, 13, 12, 11, 10, 9

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

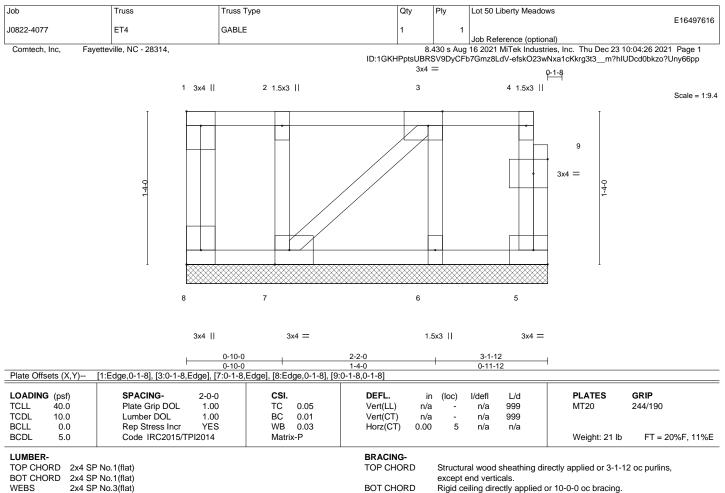
5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



December 23,2021





2x4 SP No.3(flat) WFBS 2x4 SP No.3(flat) OTHERS

REACTIONS. All bearings 3-1-12.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



December 23,2021

🛕 WARNING - Verify design pa rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This does not have a seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSETPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



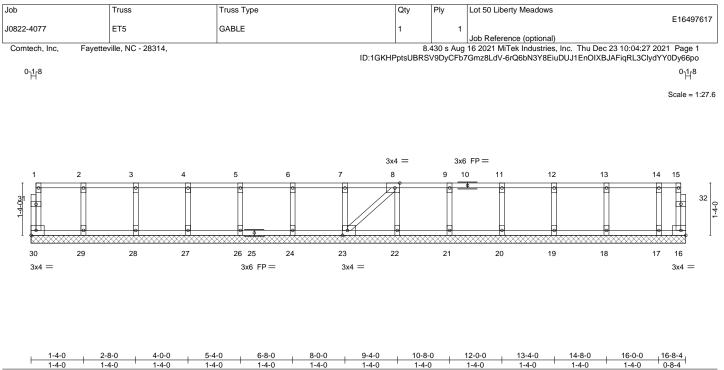


Plate Offsets (X,Y)	[8:0-1-8,Edge], [23:0-1-8,Edge]		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) n/a - n/a 999 MT20 244/190 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 16 n/a n/a Weight: 77 lb FT = 20%F, 1 FT = 20%F, 1 FT = 20%F, 1 FT = 20%F, 1
	P No.1(flat) P No.1(flat)	1	BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD WEBS 2x4 SP No.1(flat) 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat)

REACTIONS.

DNS. All bearings 16-8-4. (lb) - Max Grav All reactions 250 lb or less at joint(s) 30, 16, 29, 28, 27, 26, 24, 23, 22, 21, 20, 19, 18, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

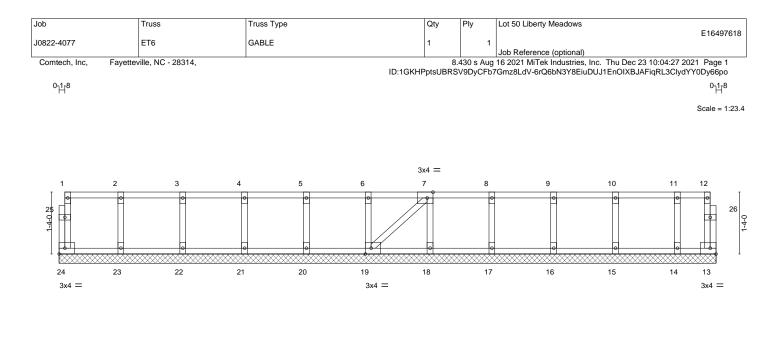
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling in trusses and truss systems, see **AVSUPPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932



1	1-4-0	2-8-0	4-0-0	5-4-0	6-8-0	8-0-0	9-4-0	10-8-0	12-0-0	13-4-0) 14-2-0
F	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	0-10-0
Plate C	Offsets (X,Y)	[7:0-1-8,Edge],	[19:0-1-8,Edge]								
LOADI TCLL TCDL BCLL BCDL	ING (psf) 40.0 10.0 0.0 5.0	SPACIN Plate Gri Lumber Rep Stre Code IR	ip DOL 1.00 DOL 1.00) T) E	CSI. FC 0.06 3C 0.01 WB 0.03 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 13	l/defl L/d n/a 999 n/a 999 n/a n/a	MT	ATES 20	GRIP 244/190 FT = 20%F, 11%E
LUMB TOP C BOT C WEBS	HORD 2x4 S HORD 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat)		I		BRACING- TOP CHOP BOT CHOP	RD Structo exception	ural wood sheathin t end verticals. ceiling directly appl	5 <i>j</i>		oc purlins,

2x4 SP No.3(flat) OTHERS REACTIONS.

DNS. All bearings 14-2-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 24, 13, 23, 22, 21, 20, 19, 18, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

All plates are 1.5x3 MT20 unless otherwise indicated.
 Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

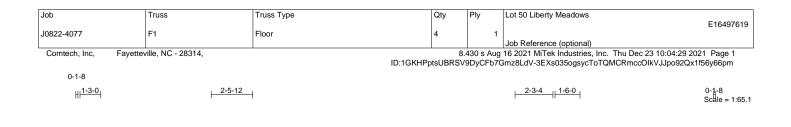
5) Gable studs spaced at 1-4-0 oc.

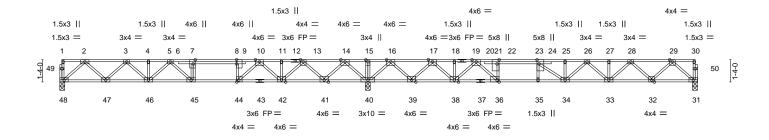
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.











	18-5-12 18-5-12			38-3-0 19-9-4					
Plate Offsets (X,Y)	[7:0-3-0,Edge], [8:0-3-0,0-0-0], [22:0-3-	0,Edge], [23:0-3-0,Edge],	[36:0-1-8,Edge], [44:0-	1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.96 BC 0.81 WB 0.71	Vert(LL) -0.2	in (loc) l/defl 27 34-35 >864 36 34-35 >655 36 31 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S				Weight: 211 lb	FT = 20%F, 11%		
BOT CHORD 2x4 S	SP No.1(flat) SP No.1(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	except end ver	icals.	rectly applied or 2-2-0 or 6-0-0 oc bracing.	oc purlins,		
8-1 15- 22- 28-	=-1560/0, 3-4=-2546/0, 4-5=-2546/0, 5-7= D=-2888/105, 10-11=-1673/754, 11-13=- 16=0/3309, 16-17=-64/1187, 17-18=-176 23=-3236/0, 23-25=-3338/0, 25-26=-3338 29=-1704/0 48=0/394 46-47=0/2157, 45-46=0/2787	673/754, 13-14=-128/140 7/545, 18-20=-1767/545, 2 8/0, 26-27=-2818/0, 27-28=	7, 14-15=0/3309, 20-22=-3258/0, =-2818/0,						
BOT CHORD 47- 41-	48=0/939, 46-47=0/2157, 45-46=0/2787, 42=-1069/990, 40-41=-1989/0, 39-40=-19	925/0, 38-39=-852/1004, 3	6-38=-294/2360,						
WEBS 2-4 13- 5-4 28 17-	36=0/3236, 34-35=0/3236, 33-34=0/3146 8=-1248/0, 2-47=0/864, 3-47=-830/0, 3-4 41=-1322/0, 13-42=0/1058, 10-42=-801/(5=-530/142, 7-45=-88/295, 8-44=-798/0, -33=0/619, 26-33=-445/18, 26-34=-20/26 39=-1403/0, 17-38=0/1136, 20-38=-907/0 34=-351/0, 23-34=-99/656	6=-14/529, 14-40=-1757/0), 10-44=0/1338, 5-46=-32 29-31=-1348/0, 29-32=0/9 2, 16-40=-1842/0, 16-39=0	, 14-41=0/1364, 8/67, 59, 28-32=-916/0, 0/1446,			1. Spotter con	antorn:		
NOTES- 1) Unbalanced floor I	ive loads have been considered for this d MT20 unless otherwise indicated.	esign.				C CAR	Rolling States		

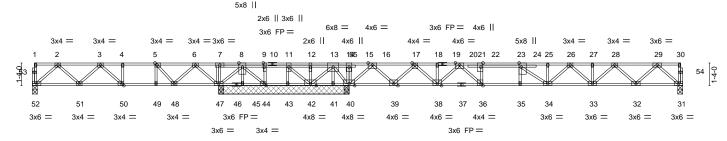
 Plates die oko m 20 andre oko m 20 and Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.





Job		Truss	Truss Type	Qty		Ply	Lot 50 Liberty Meadows	
		_	_					E16497620
J0822-4077		F1A	Floor	1		1		
							Job Reference (optional)	
Comtech, Inc,	Fayettev	/ille, NC - 28314,			8.4	30 s Aug	16 2021 MiTek Industries, Inc. Thu Dec 2	23 10:04:31 2021 Page 1
				ID:1GKHPptsUE	BRS	/9DyCFb	7Gmz8LdV-?cfdRl72CTCJi6doTcTEh1Th	7J0hHjGKtFWm9?y66pk
0-1-8								
 1-3-0 		<u>1-10-0</u>	<mark>1-2-8 1-2-8 1-2-8 1-2-8 1-2-8 -9-0</mark>				<u>2-1-8</u> <u>1-6-0</u>	0-1-8 Scale: 3/16"=1'
			U					



	10-11-8	14-9-8 18-7-8		38-3	-0			
	10-11-8	3-10-0 3-10-0		19-7	-8			
Plate Offsets (X	,Y) [5:0-1-8,Edge], [9:0-3-0,Edge], [22:0	-3-0,Edge], [23:0-3-0,Edge],	[36:0-1-8,Edge], [44:0-1	-8,Edge], [50:0-1-8,Edge]			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.71 BC 0.75 WB 0.67 Matrix-S	Vert(LL) -0.23	n (loc) l/defl L/d 3 34-35 >999 480 3 34-35 >746 360 4 31 n/a n/a	PLATES GRIP MT20 244/190 Weight: 221 lb FT = 20	0%F, 11%E		
					110igini 221 ib	,,		
BOT CHORD	2x4 SP No.1(flat) 2x4 SP No.1(flat) 2x4 SP No.3(flat)		BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.					
	All bearings 7-8-0 except (jt=length) 52=0 Max Uplift All uplift 100 lb or less at joint(Max Grav All reactions 250 lb or less at j 40=3094(LC 7), 40=3081(LC 1)	e) except 41=-793(LC 4), 42= pint(s) 42, 43, 45 except 52=	560(LC 3), 47=830(LC 3					
FORCES. (lb) TOP CHORD BOT CHORD	- Max. Comp./Max. Ten All forces 250 (lt 2-3=-901/0, 3-4=-1142/0, 4-5=-1142/0, 5- 8-9=0/380, 9-11=0/380, 11-12=0/792, 12 16-17=0/638, 17-18=-1159/0, 18-20=-112 23-25=-2972/0, 25-26=-2972/0, 26-27=-2 51-52=0/591, 50-51=0/1157, 49-50=0/1	5=-768/0, 6-7=-96/503, 7-8= 13=0/792, 13-14=0/3016, 14 9/0, 20-22=-2752/0, 22-23=- 570/0, 27-28=-2570/0, 28-29	-106/487, I-16=0/3018, 2728/0, =-1579/0					
	T CHORD 51-52=0/591, 50-51=0/1157, 49-50=0/1142, 48-49=0/1142, 47-48=0/420, 45-47=-318/0, 44-45=-318/0, 43-44=-380/0, 42-43=-380/0, 41-42=-1769/0, 40-41=-1769/0, 39-40=-1647/0, 38-39=0/350, 36-38=0/1799, 35-36=0/2728, 34-35=0/2728, 33-34=0/2834, 32-33=0/2177, 31-32=0/949							
WEBS	14-40=-299/0, 2-52=-784/0, 2-51=0/432, 5-48=-571/0, 13-40=-1985/0, 13-41=0/77 28-32=-833/0, 28-33=0/534, 26-33=-358 17-39=-1366/0, 17-38=0/1101, 20-38=-8; 25-34=-290/12, 23-34=-215/498, 13-42=	I, 11-43=-60/286, 29-31=-12 0, 16-40=-1830/0, 16-39=0/ 1/0, 20-36=0/1328, 22-36=-7	61/0, 29-32=0/876, 1403, 794/0,			-		
 All plates are Plates checked Provide mech joint 42 and 2 Recommend Strongbacks 	floor live loads have been considered for th 1.5x3 MT20 unless otherwise indicated. ed for a plus or minus 1 degree rotation aboranical connection (by others) of truss to be 275 lb uplift at joint 43. 2x6 strongbacks, on edge, spaced at 10-0- to be attached to walls at their outer ends of o not erect truss backwards.	ut its center. aring plate capable of withsta 0 oc and fastened to each tr						
Uniform Load	Live (balanced): Lumber Increase=1.00, P	ate Increase=1.00			December 23 202	4		

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 December 23,2021

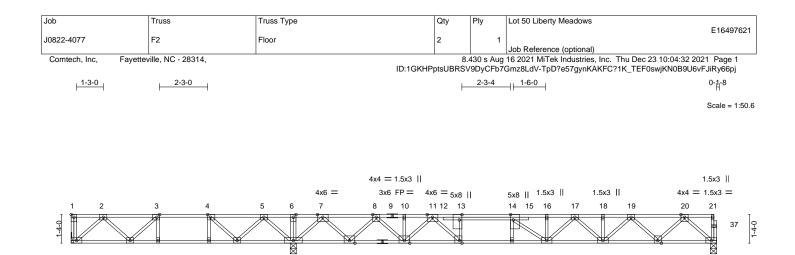


	Job	Truss	Truss Type	Qty	Ply	Lot 50 Liberty Meadows		
	J0822-4077	F1A	Floor	1	1	E16497620		
l				-		Job Reference (optional)		
Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Aug 16 2021 MiTek Industries, Inc. Thu Dec 23 10:04:31 2								
			ID:1GKHPptsUBRSV9DyCFb7Gmz8LdV-?cfdRl72CTCJi6doTcTEh1Th7J0hHjGKtFWm9?y66pk					

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 10=-69 12=-69 8=-69 55=-69





30	2	9	28	
4x6 =	3x6	FP	=	
			4x6	=

27

4x4 =

26

30-1-12

1.5x3 ||

25

3x6 =

24

3x6 =

23

4x4 =

22

3x6 =

29

30

	10-4-8 30-1-12								
10-4-8 19-9-4						9-9-4			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [3:0-1-8,	dge], [4:0-1-8,	Edge], [13:0-3-0,Ed	ge], [14:0-3-0,Edge], [27	7:0-1-8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.00 YES	CSI. TC 0.77 BC 0.85 WB 0.63	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.29 25-26 -0.39 25-26 0.06 22	l/defl >821 >604 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/T	PI2014	Matrix-S					Weight: 163 lb	FT = 20%F, 11%
BOT CHORD 2x4 S	SP No.1(flat) SP No.1(flat) SP No.3(flat)			BRACING- TOP CHOR BOT CHOR	D Structu except	t end vert	icals.	rectly applied or 5-10-1 or 6-0-0 oc bracing.	oc purlins,
Max 0 Max 0	ze) 36=Mechanical, 31= Uplift 36=-26(LC 4) Grav 36=490(LC 3), 31=1 <. Comp./Max. Ten All fo	947(LC 1), 22=	989(LC 7)	shown.					
8-10	=-721/150, 3-4=-860/441, 4 0=-2401/0, 10-11=-2401/0, 17=-3669/0, 17-18=-3045/0	11-13=-3738/0), 13-14=-3719/0, 14						
30-3	36=-18/514, 34-35=-441/8(31=-576/0, 28-30=0/1695, -24=0/2533, 22-23=0/1075				32,				
WEBS 2-36 4-33 11-2 19-2	6=-685/24, 2-35=-183/287, 3=0/301, 3-34=-264/0, 7-3 28=-774/0, 11-27=0/1261, 23=-993/0, 19-24=0/697, 1 25=-439/328	l=-1699/0, 7-30 13-27=-758/0, 2)=0/1314, 8-30=-128 20-22=-1428/0, 20-2	80/0, 8-28=0/1005, 23=0/1035,					
NOTES-									1. WR. CO.

1) Unbalanced floor live loads have been considered for this design.

33

1.5x3 ||

32

31

3x10 =

- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.4) Refer to girder(s) for truss to truss connections.
- b) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 36.
 c) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.
- Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.

36

3x6 =

35

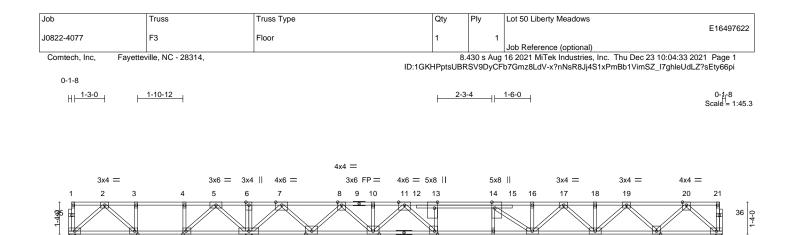
34

1.5x3 ||

10-4-8







29

4x6 =

28

3x8 M18AHS FP =

27

4x4 =

26

25

3x6 =

24

3x6 =

23

4x4 =

L	7-6-4			27-3-8		
	7-6-4			19-9-4		1
Plate Offsets (X,Y)	[13:0-3-0,Edge], [14:0-3-0,Edge], [27:0	-1-8,Edge], [32:0-1-8,Edge], [33:0-1-8,Edge]			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.85 BC 0.85 WB 0.62 Matrix-S	Vert(LL) -0.29	n (loc) I/defl L/d 9 25-26 >817 480 9 25-26 >598 360 6 22 n/a n/a	PLATES MT20 M18AHS Weight: 149 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2x4 SI	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	, ,,	oc purlins,
	e) 34=0-3-8, 31=0-3-8, 22=0-3-8 Jplift 34=-112(LC 4) Grav 34=328(LC 3), 31=1806(LC 1), 22:	=996(LC 7)				
FORCES. (Ib) - Max.	Comp./Max. Ten All forces 250 (lb) o	r less except when shown.				

 TOP CHORD
 2-3=-381/510, 3-4=-381/510, 4-5=-381/510, 5-6=0/1504, 6-7=0/1504, 7-8=-907/0, 8-10=-2479/0, 10-11=-2479/0, 11-13=-3798/0, 13-14=-3780/0, 14-16=-3711/0, 16-17=-3711/0, 17-18=-3074/0, 18-19=-3074/0, 19-20=-1833/0

 BOT CHORD
 33-34=-159/298, 32-33=-510/381, 31-32=-1003/24, 29-30=0/1785, 27-29=0/3008,

 26-27=0/3780, 25-26=0/3780, 24-25=0/3468, 23-24=0/2554, 22-23=0/1082

 WEBS
 2-34=-393/211, 2-33=-476/113, 5-31=-838/0, 5-32=0/903, 4-32=-461/0, 7-31=-1673/0, 7-30=0/1302, 8-30=-1244/0, 8-29=0/966, 20-22=-1438/0, 20-23=0/1045, 19-23=-1002/0, 19-24=0/706, 17-24=-536/0, 17-25=0/330, 11-29=-743/0, 11-27=0/1229, 13-27=-738/0, 14-25=-493/278

 \boxtimes

31

3x10 =

30

4x6 =

NOTES-

Ň

34

3x6 =

33

3x4 =

32

3x4 =

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 1.5x3 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 34.
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.



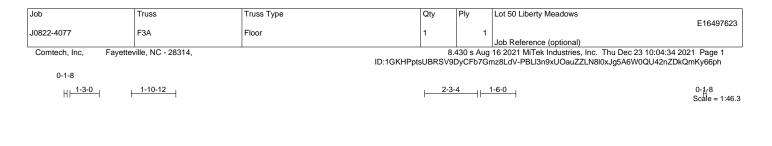
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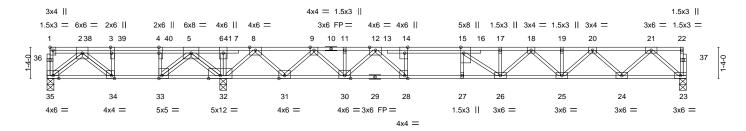
22

3x6 =

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling in trusses and truss systems, see **AVSUPPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 2000111001 20,2021







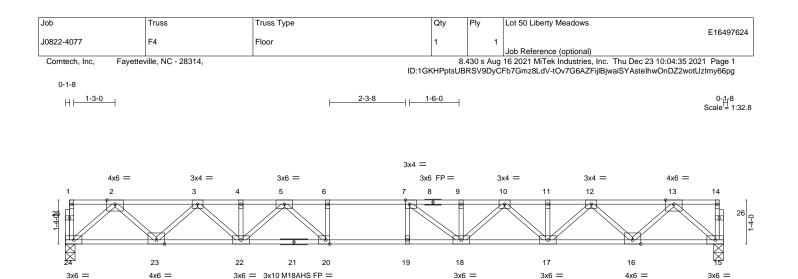
 	7-6-4			27-3-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [3:0-3-0,Edge], [4:0-3-0	,Edge], [14:0-3-0,Edge], [[15:0-3-0,Edge], [28:0-1-8		,Edge], [34:0-	1-8,Edge]	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO	CSI. TC 0.79 BC 0.88 WB 0.66	Vert(LL) -0.25	(loc) l/defl 26-27 >928 26-27 >677 23 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S				Weight: 169 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF		1	BRACING- TOP CHORD BOT CHORD	except end vert	cals. ectly applied o	ectly applied or 6-0-0 o or 10-0-0 oc bracing, 1 2.30-31.	
REACTIONS. (siz	e) 35=0-3-8, 32=0-3-8, 23=0-3-8			o o o oo biddiiq	. 02 00,01 02	.,	
Max G	Grav 35=1662(LC 3), 32=3831(LC 1), 23	3=915(LC 7)					
TOP CHORD 2-3= 9-11: 17-1: BOT CHORD 2-3= 9-12: 28-3 WEBS 6-32 3-34 21-2: 12-2:	Comp./Max. Ten All forces 250 (lb) o -2360/0, 3-4=-2360/0, 4-5=-2360/0, 5-6 =-1518/0, 11-12=-1518/0, 12-14=-3071/ 8=-3209/0, 18-19=-2730/0, 19-20=-2730 5=0/1750, 33-34=0/2360, 32-33=-583/8 0=0/2135, 27-28=0/3047, 26-27=0/3047 =-874/0, 2-35=-2265/0, 2-34=-2/810, 5- =-536/0, 8-32=-1779/0, 8-31=0/1395, - 4=0/929, 20-24=-886/0, 20-25=0/589, 11 8=0/1364, 14-28=-816/0, 15-26=-269/47	=0/2626, 6-8=0/2626, 8-9 0, 14-15=-3047/0, 15-17= 1/0, 20-21=-1659/0 93, 31-32=-1290/0, 30-31 7, 25-26=0/3034, 24-25=0, 32=-2781/0, 5-33=0/2696 31=-1342/0, 9-30=0/1070 8-25=-413/0, 17-26=-297/	=0/427, 3209/0, =-107/736, /2296, 23-24=0/991 , 4-33=-1680/0, 0, 21-23=-1317/0,				
 Plates checked for a Recommend 2x6 st Strongbacks to be a CAUTION, Do not e Hanger(s) or other of down at 3-2-4, and device(s) is the resp 	connection device(s) shall be provided s 878 lb down at 5-2-4, and 857 lb down	its center. bc and fastened to each to sstrained by other means. ufficient to support conce at 7-2-4 on top chord. T	ntrated load(s) 878 lb dow he design/selection of suc	vn at 1-2-4, 878		CA CA	Alexa.
LOAD CASE(S) Stan 1) Dead + Floor Live (I Uniform Loads (plf)	dard balanced): Lumber Increase=1.00, Plate	Increase=1.00				0363	

Vert: 23-35=-10, 1-22=-100

Concentrated Loads (lb) Vert: 38=-798(B) 39=-798(B) 40=-798(B) 41=-798(B)

A, GIL THE FIRE OF STATE December 23,2021





3x4 =

			19-11-0 19-11-0				
Plate Offsets (X,Y)	[7:0-1-8,Edge], [20:0-1-8,Edge]						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.99 BC 0.74 WB 0.55 Matrix-S	Vert(CT) -0	in (loc) l/def 0.35 18-19 >683 0.47 18-19 >498 0.07 15 n/a	8 480 8 360	PLATES MT20 M18AHS Weight: 105 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2x4 S 15-21 WEBS 2x4 S REACTIONS. (siz	P No.1(flat) P No.1(flat) *Except* : 2x4 SP 2400F 2.0E(flat) P No.3(flat) ze) 24=0-3-8, 15=0-3-8 Grav 24=1075(LC 1), 15=1075(LC 1)		BRACING- TOP CHORD BOT CHORD			ectly applied, except (r 10-0-0 oc bracing.	end verticals.
TOP CHORD 2-3= 9-10 BOT CHORD 23-2 16- WEBS 2-24 6-20	. Comp./Max. Ten All forces 250 (lb) o 2007/0, 3-4=-3409/0, 4-5=-3409/0, 5-6= 4232/0, 10-11=-3412/0, 11-12=-3412/ 24=0/1174, 22-23=0/2804, 20-22=0/3891 17=0/2808, 15-16=0/1172 I=-1560/0, 2-23=0/1159, 3-23=-1108/0, 3 395/0, 13-15=-1558/0, 13-16=0/1159, 18=0/460, 9-18=-251/64, 7-18=-606/291	4323/0, 6-7=-4323/0, 7-5 0, 12-13=-2005/0 , 19-20=0/4323, 18-19=0/ -22=0/823, 5-22=-655/0, 5	9=-4232/0, 4323, 17-18=0/3894, 5-20=0/865,				
NOTES- 1) Unbalanced floor liv	ve loads have been considered for this d	esign.					

2) All plates are MT20 plates unless otherwise indicated.

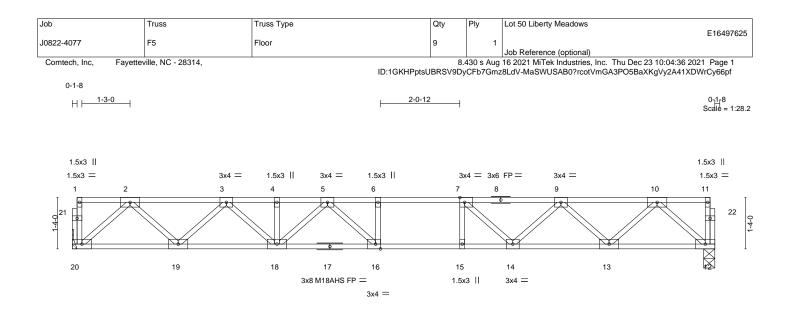
3) All plates are 1.5x3 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.







L			16-8-4				
			16-8-4				1
Plate Offsets (X,Y)	[7:0-1-8,Edge], [16:0-1-8,Edge]						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.54 BC 0.91 WB 0.43 Matrix-S	Vert(LL) -0.22	(loc) l/defl 16-18 >892 16-18 >666 12 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 87 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2x4 WEBS 2x4 REACTIONS. (SP No.1(flat) SP No.1(flat) SP No.3(flat) size) 20=Mechanical, 12=0-3-8 (Grav 20=898(LC 1), 12=898(LC 1)		BRACING- TOP CHORD BOT CHORD	except end vert	icals.	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
TOP CHORD 2 9- BOT CHORD 19 12 WEBS 2	ax. Comp./Max. Ten All forces 250 (lb) of 3=-1619/0, 3-4=-2661/0, 4-5=-2661/0, 5-6= 10=-1624/0 -20=0/970, 18-19=0/2244, 16-18=0/2936, -13=0/970 20=-1289/0, 2-19=0/903, 3-19=-869/0, 3-18 13=-861/0, 9-14=0/557, 5-18=-374/0, 5-16	3022/0, 6-7=-3022/0, 7-5 15-16=0/3022, 14-15=0/3 8=0/567, 10-12=-1289/0,	9=-2612/0, 022, 13-14=0/2243,				

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x6 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Refer to girder(s) for truss to truss connections.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

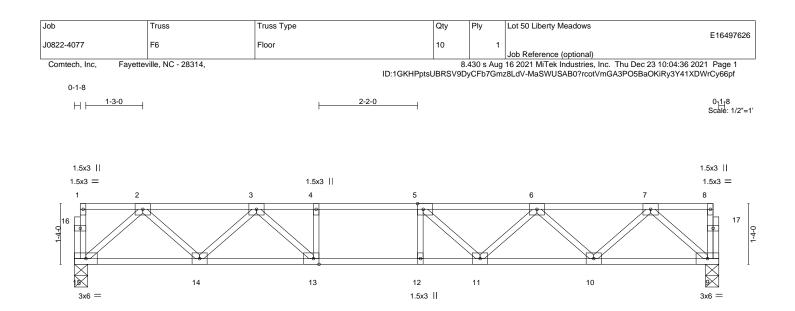
Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932



L			14-2-0			
1			14-2-0			1
Plate Offsets (X,Y)	[5:0-1-8,Edge], [13:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.54 BC 0.78 WB 0.34	Vert(LL) -0.15	n (loc) l/defl L/d 5 11-12 >999 480 0 11-12 >854 360 3 9 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	1012(01) 0.00	, <u> </u>	Weight: 73 lb	FT = 20%F, 11%
	' ? No.1(flat) ? No.1(flat)		BRACING- TOP CHORD	Structural wood sheathing dir except end verticals.	ectly applied or 6-0-0	oc purlins,
	° No.3(flat)		BOT CHORD	Rigid ceiling directly applied o	or 10-0-0 oc bracing.	
REACTIONS. (size	e) 15=0-3-8, 9=0-3-8					

Max Grav 15=759(LC 1), 9=759(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1309/0, 3-4=-2153/0, 4-5=-2153/0, 5-6=-2019/0, 6-7=-1326/0

 BOT CHORD
 14-15=0/813, 13-14=0/1799, 12-13=0/2153, 11-12=0/2153, 10-11=0/1821, 9-10=0/806

 WEBS
 2-15=-1080/0, 2-14=-0/690, 3-14=-683/0, 3-13=0/649, 7-9=-1070/0, 7-10=0/723, 000/0

6-10=-689/0, 6-11=0/352, 5-11=-383/24, 4-13=-299/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



December 23,2021



