

## RE: 19-055962T ON TOP BUILDERS/03 FLOWERS HILL

Trenco 818 Soundside Rd Edenton, NC 27932

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.2 Wind Speed: 115 mph Floor Load: N/A psf

This package includes 36 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T16665683	a1	4/1/2019	27	T16665709	vc5	4/1/2019
2	T16665684	a2	4/1/2019	28	T16665710	vc6	4/1/2019
3	T16665685	a4	4/1/2019	29	T16665711	vd1	4/1/2019
4	T16665686	a5	4/1/2019	30	T16665712	vd2	4/1/2019
5	T16665687	a6	4/1/2019	31	T16665713	vd3	4/1/2019
6	T16665688	a7	4/1/2019	32	T16665714	vd4	4/1/2019
7	T16665689	a8	4/1/2019	33	T16665715	ve1	4/1/2019
8	T16665690	a9	4/1/2019	34	T16665716	ve2	4/1/2019
9	T16665691	a10	4/1/2019	35	T16665717	ve3	4/1/2019
10	T16665692	a10s	4/1/2019	36	T16665718	ve4	4/1/2019
11	T16665693	b1	4/1/2019				
12	T16665694	b2	4/1/2019				
13	T16665695	c1	4/1/2019				
14	T16665696	c1g	4/1/2019				
15	T16665697	c2	4/1/2019				
16	T16665698	d1	4/1/2019				
17	T16665699	d2	4/1/2019				
18	T16665700	e1	4/1/2019				
19	T16665701	eg1	4/1/2019				
20	T16665702	f2	4/1/2019				
21	T16665703	fg1	4/1/2019				
22	T16665704	mt1	4/1/2019				
23	T16665705	vc1	4/1/2019				
24	T16665706	vc2	4/1/2019				
25	T16665707	vc3	4/1/2019				

4/1/2019

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

vc4

based on the parameters provided by Stock Building Supply.

Truss Design Engineer's Name: Albani, Thomas

My license renewal date for the state of North Carolina is December 31, 2019

North Carolina COA: C-0844

T16665708

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IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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0-0 <u>-8</u>	7-4-0	11-4-0	18-4-0	27-3-14	36-3-12	45-0-0
0-0-8	7-3-8	4-0-0	7-0-0	8-11-14	8-11-14	8-8-4
Plate Offsets (X,Y)	[B:0-1-5,0-3-0], [N:0-2	2-4,0-0-10], [T:	0-2-12,0-3-4]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DO Lumber DOL Rep Stress In Code IRC201	2-0-0 IL 1.00 1.15 cr YES 5/TPI2014	CSI. TC 0.68 BC 0.74 WB 0.99 Matrix-MS	DEFL. in Vert(LL) 0.17 Vert(CT) -0.62 Horz(CT) 0.25	(loc) l/defl L/d P-X >591 240 R-S >704 180 P n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 267 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF SLIDER Left 2x	2 1650F 1.5E or 2x4 S 2 1650F 1.5E or 2x4 S 2 No.2 4 SP No.3 2-5-0, Rigi	6P No.1 or 2x4 6P No.1 or 2x4 ht 2x4 SP No.3	SP SS SP SS 1-11-0	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing di Rigid ceiling directly applied o 6-0-0 oc bracing: N-P. 1 Row at midpt F- MiTek recommends that St be installed during truss ere Installation quide	rectly applied or 2-3-11 oc purlins. or 10-0-0 oc bracing, Except: S, H-S, I-R, J-P abilizers and required cross bracing action, in accordance with Stabilizer

REACTIONS. (lb/size) P=2245/0-3-8, N=33/0-3-0, B=1421/0-3-8 Max Horz B=-152(LC 11) Max UpliftP=-10(LC 10), N=-193(LC 23), B=-60(LC 10) Max Grav P=2246(LC 2), N=230(LC 22), B=1421(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD B-C=-1231/43, C-D=-3771/506, D-E=-3989/486, E-F=-2829/410, F-G=-1487/295, G-H=-1389/330, H-I=-1446/429, I-J=-1052/350, J-K=-14/895, K-L=-50/745, L-M=-65/787, M-N=-252/263
- BOT CHORD
   B-U=-379/3343, T-U=-310/3497, S-T=-205/2644, S-AD=0/916, AD-AE=0/916, R-AE=0/916, R-AF=-13/697, Q-AF=-13/697, Q-AG=-13/697, P-AG=-13/697, N-P=-611/112

   WEBS
   E-U=-35/858, E-T=-1148/142, F-T=-53/1326, F-S=-1687/236, H-S=-367/173, D-125/858, E-T=-1148/142, P-T=-53/1326, F-S=-1687/236, H-S=-367/173,
- I-S=-199/1019, I-R=-364/87, J-R=0/606, J-P=-2105/67, L-P=-458/223, D-U=0/331

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Paorian a triainformatic approximation of a partial designed and a provide a p

5) Bearing at joint(s) B considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) P, B except (jt=lb) N=193.

LOAD CASE(S) Standard



### April 1,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
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a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
tabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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### ......

LUMBER-		BRACING-		
TOP CHORD	2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS	TOP CHORD	Structural wood sheathi	ng directly applied or 2-2-0 oc purlins.
BOT CHORD	2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS *Except*	BOT CHORD	Rigid ceiling directly app	blied or 10-0-0 oc bracing, Except:
	B-R: 2x6 SP No.2		2-2-0 oc bracing: B-R	
WEBS	2x4 SP No.2		6-0-0 oc bracing: K-M.	
WEDGE		WEBS	1 Row at midpt	H-O, E-P, F-P, H-N
Left: 2x4 SP N	No.3		MiTek recommends th	at Stabilizers and required cross bracing
SLIDER	Right 2x4 SP No.3 1-11-0		be installed during trus Installation guide.	ss erection, in accordance with Stabilizer
REACTIONS.	. (Ib/size) M=2145/0-3-8, B=1560/0-3-8			

### Max Horz B=-152(LC 15) Max UpliftM=-70(LC 11), B=-70(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD B-C=-4693/520, C-D=-4610/436, D-E=-3310/363, E-F=-1809/293, F-G=-1766/392,

G-H=-1571/294, H-I=-700/92, I-J=-261/670, J-K=-159/330

- BOT CHORD B-R=-406/4210, Q-R=-270/4037, P-Q=-176/3063, P-Z=0/1238, Z-AA=0/1238, O-AA=0/1238, O-AB=0/1281, AB-AC=0/1281, N-AC=0/1281, M-N=0/271, K-M=-500/312
- WEBS G-P=-198/1017, G-O=-76/363, D-R=-35/994, D-Q=-1276/138, E-Q=-42/1510,
  - E-P=-1851/224, F-P=-357/171, H-N=-1200/215, I-N=-31/1092, I-M=-2063/346

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) All plates are MT20 plates unless otherwise indicated.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) Bearing at joint(s) B considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity
- of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) M, B.

LOAD CASE(S) Standard



April 1,2019



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### REACTIONS. (lb/size) L=2146/0-3-8, A=1506/0-3-8 Max Horz A=-157(LC 11) Max UpliftL=-70(LC 11), A=-58(LC 10)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD A-B=-4708/531, B-C=-4620/443, C-D=-3314/366, D-E=-1811/294, E-F=-1768/393,

F-G=-1572/295, G-H=-700/93, H-I=-261/670, I-J=-159/330

- BOT CHORD A-Q=-417/4226, P-Q=-272/4046, O-P=-177/3067, O-Y=0/1238, Y-Z=0/1238, N-Z=0/1238,
- N-AA=0/1282, AA-AB=0/1282, M-AB=0/1282, L-M=0/271, J-L=-500/312
- WEBS F-O=-198/1019, F-N=-76/363, C-Q=-37/1000, C-P=-1281/142, D-P=-45/1514,
  - D-O=-1854/226, E-O=-357/171, G-M=-1200/215, H-M=-31/1092, H-L=-2063/347

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) All plates are MT20 plates unless otherwise indicated.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
  6) Bearing at joint(s) A considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity
- of bearing an journey in considere parametre grain value doing interint in angle to grain formula. Building designer should verify capability of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) L, A.

LOAD CASE(S) Standard



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TENSINEERING BY AMITEK Attiliate 818 Soundside Road Edenton, NC 27932

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7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) N, A except (jt=lb) K=137.

LOAD CASE(S) Standard



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Plate Offsets (X Y)	[A:0-3-1 Edge] [I:0-3-0 0-3-0] [W:0-3-0	0-3-0] [AE:0-3-1 Edge] [A	45-0-0 45-0-0 N:0-4-0 0-6-01 [AT:0-	4-0 0-6-01 [BA:0-4-0 0-6-0]	
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING-         2-0-0           Plate Grip DOL         1.00           Lumber DOL         1.15	<b>CSI.</b> TC 0.12 BC 0.02	DEFL. ii Vert(LL) n/a Vert(CT) n/a	n (loc) l/defl L/d a - n/a 999 a - n/a 999	PLATES         GRIP           MT20         244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr NO Code IRC2015/TPI2014	WB 0.09 Matrix-S	Horz(CT) 0.00	) AE n/a n/a	Weight: 477 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x8 SP OTHERS 2x4 SP P-AT,C	1650F 1.5E or 2x4 SP No.1 or 2x4 SP 2250F 1.9E or 2x8 SP DSS or 2x8 SP No.3 *Except* -AU,N-AV,M-AW,L-AX,Q-AS,R-AR,S-AG	35 35 2,T-AP: 2x4 SP No.2	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing di Rigid ceiling directly applied 1 Row at midpt	rectly applied or 6-0-0 oc purlins. or 10-0-0 oc bracing. P-AT, O-AU, N-AV, M-AW, L-AX, Q-AS, R-AR, S-AQ, T-AP
REACTIONS. All be (lb) - Max H Max U	earings 45-0-0. orz A=148(LC 27) plift All uplift 100 lb or less at joint(s) Al AP. AO. AN. AM. AL. AK. AJ. Al. AF	J, AV, AW, AX, AY, AZ, BA I. AG except AF=-118(LC §	A, BB, BC, BD, BE, BF	, BG, BH, AR, AQ,	

Max Grav All reactions 250 lb or less at joint(s) A, AT, AU, AV, AW, AX, AY, AZ, BA, BB, BC, BD, BE, BF, AS, AR, AQ, AP, AO, AN, AM, AL, AK, AJ, AI, AH, AG, AE except BH=329(LC 19), AF=444(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.33

 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 Gable requires continuous bottom chord bearing.

- 5) Gable studs spaced at 1-4-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) AU, AV, AW, AX, AY, AZ, BA, BB, BC, BD, BE, BF, BG, BH, AR, AQ, AP, AO, AN, AM, AL, AK, AJ, AI, AH, AG except (jt=lb) AF=118.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 57 lb down and 35 lb up at 26-10-12, 57 lb down and 35 lb up at 32-10-12, 57 lb down and 35 lb up at 32-10-12, 57 lb down and 35 lb up at 34-10-12, 57 lb down and 35 lb up at 34-10-12, 57 lb down and 35 lb up at 34-10-12, 57 lb down and 35 lb up at 36-10-12, 57 lb down and 35 lb up at 38-10-12, and 57 lb down and 35 lb up at 42-10-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.00 Uniform Loads (plf) Vert: A-P=-60, P-AE=-60, A-AE=-20

ven: A-P=-00, P-AE=-60, A-AE=-20

### Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	ON TOP BUILDERS/03 FLOWERS HILL			
					T166656	391		
19-055962T	A10	Common Girder	1	1				
					Job Reference (optional)			
BMC (Middlesex, NC),	Middlesex, NC - 27557,		8	.220 s Nov	16 2018 MiTek Industries, Inc. Mon Apr 1 11:52:31 2019 Page 2			
		ID:E6b4hmUIQbiiYq6F0jA?rGzZib7-?BJqrOTtj3di4?5wvYsYA?bifkCiKDEmmxeV?LzV5Qk						

### LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: AO=-53 AL=-53 AI=-53 AF=-53 BI=-53 BJ=-53 BK=-53 BL=-53 BM=-53

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Max Uplift E=-99(LC 10), A=-21(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD A-B=-1193/152, B-C=-935/135

BOT CHORD A-F=-134/998, E-F=-74/533

WEBS B-F=-355/173, C-F=-13/590, C-E=-739/187

### NOTES-

 Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) E, A.



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### LUMBER-

 TOP CHORD
 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS

 BOT CHORD
 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS

 WEBS
 2x4 SP No.2

 WEDGE
 2x4 SP No.2

BRACING-TOP CHORD Struct BOT CHORD Rigid WEBS 1 Roy

Structural wood sheathing directly applied. Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt J-K

Left: 2x4 SP No.2, Right: 2x4 SP No.3

### REACTIONS. (lb/size) B=1434/0-3-8, F=1380/Mechanical Max Horz B=109(LC 10) Max Uplift B=-58(LC 10), F=-46(LC 11)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD B-C=-2441/338, C-D=-2218/357, D-E=-2219/358, E-F=-2459/339
- BOT CHORD B-I=-219/2107, H-I=-71/1527, G-H=-71/1527, F-G=-223/2110
- WEBS D-K=-93/899, G-K=-81/737, E-G=-430/208, I-J=-80/734, D-J=-92/896, C-I=-425/202

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, F.

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.00

Uniform Loads (plf)

Vert: A-D=-60, D-F=-60, M-P=-20, J-K=-40(F)



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Plate Offsets (X,Y)	[B:0-0-0,0-1-9], [B:0-2-9,Edge], [X:0-2-9	,Edge], [X:Edge,0-1-9], [A	<u>30-0-0</u> <u>30-0-0</u> H:0-3-0,0-3-0]						4
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	l/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.00	TC 0.07	Vert(LL)	-0.00	(.00) A	n/r	120	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.05	Vert(CT)	0.00	А	n/r	120	-	
CLL 0.0 *	Rep Stress Incr YES	WB 0.12	Horz(CT)	0.00	Х	n/a	n/a		
CDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	-0.00	А	n/r	90	Weight: 221 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS BOT CHORD 2x4 SP No.3

OTHERS

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

#### REACTIONS. All bearings 30-0-0.

(lb) - Max Horz B=106(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) B, AI, AJ, AK, AL, AM, AN, AO, AP, AQ, AG, AF, AE, AD, AC, AB, AA, Z, Y

Max Grav All reactions 250 lb or less at joint(s) B, AH, AI, AJ, AK, AL, AM, AN, AO, AP, AQ, AG, AF, X, AE, AD, AC, AB, AA, Z, Y

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 1-4-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, AI, AJ, AK, AL, AM, AN, AO, AP, AQ, AG, AF, AE, AD, AC, AB, AA, Z, Y.



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1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.00 Uniform Loads (plf)

Vert: A-C=-60, C-F=-60, J-M=-20

### Continued on page

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Job	Truss	Truss Type	Qty	Ply	ON TOP BUILDERS/03 FLOWERS HILL	
					T1	6665696
19-055962T	C1G	Roof Special Girder	1	2		
				~	Job Reference (optional)	
BMC (Middlesex, NC),	Middlesex, NC - 27557,		8	.220 s No	v 16 2018 MiTek Industries, Inc. Mon Apr 1 11:52:59 2019 Pa	age 2
			ID:E6b4h	mUIQbiiY	q6F0jA?rGzZib7-Ba0PjypR7f9jRIEIYRM7IvISF0i2uphtDIQ_jz	zV5QI

### LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: H=-595(B) L=-325(B) P=-344(B) Q=-460(B) R=-595(B) S=-595(B) T=-595(B) U=-595(B) V=-595(B) W=-871(B=-746)

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BRACING-

TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS BOT CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS

2x4 SP No.3 OTHERS

Left 2x4 SP No.3 1-6-7, Right 2x4 SP No.3 1-6-7 SLIDER

REACTIONS.

All bearings 21-0-0. (lb) - Max Horz B=-147(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) B, AB, AC, AD, AE, AF, AG, Z, Y, W, V, U, T Max Grav All reactions 250 lb or less at joint(s) B, AA, AB, AC, AD, AE, AF, AG, Z, R, Y, W, V, U, T

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 1-4-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, AB, AC, AD, AE, AF, AG, Z, Y, W, V, U, T.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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NOTES-

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3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, G.



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- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 1-4-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) V, K, R, S, T, U, P
- , O, N, M.





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REACTIONS. All bearings 12-4-0.

(lb) - Max Horz B=91(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) B, R, S, T, P, O, N

Max Grav All reactions 250 lb or less at joint(s) B, L, Q, R, S, T, P, O, N

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 1-4-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, R, S, T, P, O, N.



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Uniform Loads (plf) Vert: A-C=-60, C-E=-60, H-K=-20

### Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	ON TOP BUILDERS/03 FLOWERS HILL	
						T16665701
19-055962T	EG1	Roof Special Girder	1	2		
				<b>_</b>	Job Reference (optional)	
BMC (Middlesex, NC),	Middlesex, NC - 27557,		8	.220 s Nov	16 2018 MiTek Industries, Inc. Mon Apr 1 11:53:08 2019	Page 2
		ID:	E6b4hmU	IQbiiYq6F(	)jA?rGzZib7-QI2pc1w5?QIS0hQ1aq1EApd?belbVtiCHB5yc	bizV5Q9

### LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: F=-1360(B) G=-1360(B) M=-1366(B) N=-1360(B) O=-1360(B) P=-1360(B)

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L		10-0-0						18-5-	8	
	()( )()	10-0-0	0.4.401	I				8-5-8	3	I
Plate Offsets	(X,Y)	[B:0-2-0,Edge], [C:0-3-0,0-3-0], [D:0-2-8	,0-1-12]						1	
LOADING (p. TCLL 20 TCDL 10 BCLL 0 BCDL 10	sf) ).0 ).0 ).0 * ).0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.77 BC 0.53 WB 0.23 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.18 -0.40 0.01	(loc) F-l F-l F	l/defl >640 >297 n/a	L/d 240 180 n/a	<b>PLATES</b> MT20 Weight: 55 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD         2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS           BOT CHORD         2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS           WEBS         2x4 SP No.2           REACTIONS.         (Ib/size)           E=203/Mechanical, B=403/0-3-0, F=747/0-3-8           Max Harz         B=124/(C-7)			BRACING- TOP CHOR BOT CHOR	<ul> <li>Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.</li> <li>Rigid ceiling directly applied or 10-0-0 oc bracing.</li> </ul>					oc purlins,	
FORCES. (I TOP CHORD BOT CHORD WEBS	Max U b) - Max. B-C= B-F= C-F=	<ul> <li>IDE 124(LC 7)</li> <li>Iplift E=-68(LC 6), B=-20(LC 6), F=-155(</li> <li>Comp./Max. Ten All forces 250 (lb) or</li> <li>-530/2, D-F=-488/253</li> <li>-70/508</li> <li>-572/160</li> </ul>	LC 10) less except when shown.							
NOTES- 1) Wind: ASC	E 7-10; V	/ult=115mph (3-second gust) Vasd=91m	ph; TCDL=6.0psf; BCDL=6.	0psf; h=30ft; Ca	at. II; Ex	pB;En	closed;			

 Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0pst; BCDL=6.0pst; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) E, B except (jt=lb) F=155.



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L	10-0-0						18-5-	8	
	10-0-0						8-5-8	3	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.80 BC 0.21 WB 0.13 Matrix-S	DEFL. Vert(LL) - Vert(CT) - Horz(CT) -	in •0.03 •0.07 •0.00	(loc) B-L B-L H	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 55 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP OTHERS 2x4 SP	BRACING- TOP CHORD BOT CHORD	IG-         IORD       Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.         IORD       Rigid ceiling directly applied or 6-0-0 oc bracing.					oc purlins,		
REACTIONS. All be (lb) - Max H Max U Max G	earings 10-0-0 except (jt=length) H=Mec orz B=124(LC 7) plift All uplift 100 lb or less at joint(s) H rav All reactions 250 lb or less at joint(	hanical. , B, L except I=-233(LC 10 s) H, B, J except I=713(L0	0), J=-232(LC 1), K= C 1), L=589(LC 1)	=-151(L	LC 1)				
FORCES. (lb) - Max. TOP CHORD G-I=- WEBS F-J=-	Comp./Max. Ten All forces 250 (lb) or 730/361 169/313, C-L=-408/185	less except when shown.							

### NOTES-

 Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 1-4-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) H, B, L except (jt=lb) l=233, J=232, K=151.



April 1,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







Scale = 1.13 7

2-3-8 Plate Offsets (X,Y)--[C:0-2-4,0-2-0], [D:Edge,0-1-14] LOADING (psf) SPACING-CSI. DEFL. PLATES GRIP 2-0-0 in (loc)l/defl I/d 20.0 Plate Grip DOL 1.00 тс 0.04 Vert(LL) -0.00 B-D 240 244/190 >999 MT20 BC 180 10.0 Lumber DOL 1.15 0.04 Vert(CT) -0.00 B-D >999 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) -0.00 D n/a n/a 10.0 Code IRC2015/TPI2014 Matrix-P Weight: 15 lb FT = 20%

BRACING-

2 - 4 - 0

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS BOT CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS 2x4 SP No.2 WEBS SLIDER Left 2x4 SP No.3 2-5-12

TOP CHORD Structural wood sheathing directly applied or 2-4-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) B=150/0-3-8, D=73/0-1-8 Max Horz B=55(LC 7) Max Uplift B=-11(LC 10), D=-15(LC 10) Max Grav B=150(LC 1), D=81(LC 17)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, D.



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🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and KEAD NOTES ON THIS AND INCLODED INTER REFERENCE FACE INTERVISED. INCLOSE DEL ONE OCL Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







April 1,2019

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2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, H, F.



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- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right
- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) \* This truss has been designed for a live load of 20.0ps on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



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Max Uplift A=-14(LC 10), C=-19(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



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3x4 🥢

3x4 🔌

Plate Offsets (X Y)	0-0-6 [B:0-2-0 Edge]		3-7-8						
		001	DEEL		4 )			DI ATEO	0010
TCLL 20.0	Plate Grip DOL 1.00	TC 0.02	Vert(LL)	n/a	(loc) -	i/defi n/a	L/d 999	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.06	Vert(CT)	n/a	-	n/a	999	-	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00	С	n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 10 lb	FT = 20%
UMBER-			BRACING-						

. . .

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-7-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=106/3-6-12, C=106/3-6-12 Max Horz A=-17(LC 6) Max Uplift A=-2(LC 10), C=-2(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED WITER REFERENCE PAGE MIL-14's rev. 10/03/2013 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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TOP CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS BOT CHORD



- MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right
- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0ps on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.

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SEAL 15544 April 1,2019



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





	0-Q <sub>1</sub> 6	5-1-10	
	0-0-6	5-1-4	
Plate Offsets (X,Y)	[B:0-2-0,Edge]		

LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.06 BC 0.16 WB 0.00 Matrix-P	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         n/a         -         n/a         999           Vert(CT)         n/a         -         n/a         999           Horz(CT)         0.00         C         n/a         n/a	PLATES         GRIP           MT20         244/190           Weight: 15 lb         FT = 20%
LUMBER-			BRACING-	

 TOP CHORD
 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS

 BOT CHORD
 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS

TOP CHORD Structur BOT CHORD Rigid ce

Structural wood sheathing directly applied or 5-1-10 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=167/5-0-14, C=167/5-0-14 Max Horz A=27(LC 9) Max Uplift A=-3(LC 10), C=-3(LC 11)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





3x4 🥢

3x4 📎

0-0-6 0-0-6 2 - 1 - 102-1-4 Plate Offsets (X,Y)-- [B:0-2-0,Edge]

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.00 BC 0.01 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - C	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190
LUMBER-	2 1650F 1.5E or 2x4 SP No.1 or 2x4 SP	SS	BRACING-	D S	Structur	al wood	sheathing di	rectly applied or 2-1-	10 oc purlins.

TOP CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS BOT CHORD

Structural wood sheathing directly applied or 2-1-10 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

(lb/size) A=47/2-0-14, C=47/2-0-14 REACTIONS. Max Horz A=-8(LC 8) Max Uplift A=-1(LC 10), C=-1(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



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- MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right
- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a loss potentiation into the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.

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SEAL 15544 April 1,2019



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



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	S (X, I)	[D.0-2-0,Euge]		1		Ι						
LOADING (p	psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20	0.0	Plate Grip DOL	1.00	тс	0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10	0.0	Lumber DOL	1.15	BC	0.15	Vert(CT)	n/a	-	n/a	999		
BCLL (	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	С	n/a	n/a		
BCDL 10	0.0	Code IRC2015/TF	912014	Matrix	k-P						Weight: 15 lb	FT = 20%
LUMBER-					BRACING-	BRACING-						
TOP CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS				TOP CHOR	TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins.							

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS BOT CHORD

REACTIONS. (lb/size) A=162/4-11-8, C=162/4-11-8 Max Horz A=-26(LC 8) Max Uplift A=-3(LC 10), C=-3(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



April 1,2019

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5x8 =

	0- <u>0-</u> 6 0-0-6		2-0-4 1-11-14					
Plate Offsets (X,Y)	[A:0-2-15,0-0-0], [A:0-4-0,0-3-7], [B:0-0-0	0,0-2-2], [C:0-2-15,0-0-0]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.00 BC 0.01 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) l/de - n/ - n/ C n/	fl L/d a 999 a 999 a n/a	PLATES MT20 Weight: 5 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS				RD S	Structural wo	ood sheathing di	rectly applied or 2-0-	4 oc purlins.

2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS TOP CHORD 2x4 SP 1650F 1.5E or 2x4 SP No.1 or 2x4 SP SS BOT CHORD

Structural wood sheathing directly applied or 2-0-4 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=42/1-11-8, C=42/1-11-8 Max Horz A=-7(LC 6) Max Uplift A=-1(LC 10), C=-1(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, C.



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