

Trenco RE: J1022-5096 818 Soundside Rd Lot 37 Liberty Meadows Edenton, NC 27932 Site Information: Customer: Lot/Block: Project Name: J1022-5096 Model: Address: Subdivision: City: State: Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4 Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf This package includes 23 individual, dated Truss Design Drawings and 0 Additional Drawings. lame Date 2/16/2022 2/16/2022 2/16/2022

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special

No. 1 2 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 7 8 9 10 11 2 3 4 5 6 7 7 8 9 10 11 12 13 14 5 10 10 11 12 10 10 11 10 10 10 10 10 10 10 10 10 10	Seal# I50256509 I50256510 I50256511 I50256512 I50256513 I50256515 I50256516 I50256516 I50256517 I50256519 I50256520 I50256521 I50256522 I50256523 I50256523 I50256525 I50256526 I50256526 I50256526 I50256527 I50256526 I50256527 I50256527 I50256526 I50256527 I50256526 I50256526 I50256527 I50256526 I50256527 I50256526 I50256556 I50256557 I502567 I502567 I502567 I502567	Truss Name A1 A1GE A2 A3 B1GE B2 C1 C1-GR C1GE G1 G1GE J1 J2 J2GE J3 J3GE V1 V2	Date 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022 2/16/2022	No. 21 22 23	Seal# I50256529 I50256530 I50256531	Truss N V4 V5 V6
20	150256528	V3	2/16/2022			

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Strzyzewski, Marvin

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



February 16, 2022



WEBS 3-13=-544/300, 5-13=-140/991, 5-10=-139/991, 7-10=-544/300

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-6 to 3-4-7, Interior(1) 3-4-7 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 34-0-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 2 and 91 lb uplift at joint 2 and 91 lb uplift at

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.









			<u>33-0-0</u> <u>33-0-0</u>	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.04 BC 0.02 WB 0.13 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 20 n/r 120 Vert(CT) 0.00 20 n/r 120 Horz(CT) 0.01 20 n/a n/a	PLATES GRIP MT20 244/190 Weight: 283 lb FT = 20%

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

WFBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2x4 SPF No.2 - 11-30, 10-31, 12-29 Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 33-0-0.

(lb) - Max Horz 2=306(LC 11)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22
 Max Grav All reactions 250 lb or less at joint(s) 2, 20, 30, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-277/226, 10-11=-242/277, 11-12=-242/277

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 pole=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.





Job	Truss	Truss Type	Qty	Ply	Lot 37 Liberty Meadows		
J1022-5096	A2	COMMON	4	1		15	0256511
Camtach Inc. Fauetta	ille NC 20214			120 0 0.00	Job Reference (optional)) - Ing. Tug Eab 15 14:17:16 2022, Da	~ 1
Connech, Inc, Fayeller	/ille, NC - 20314,	ID	aTXuLo?nW09qtpROz:	2WQ0wyd	IkZW-GYOI6M0B1CDfCm	b7vtULpWR_HNWAGYgSXA4F6nzk	swn
[<u>1-2-8</u> 1-2-8	<u>8-6-0</u> 8-6-0	16-6-0 8-0-0	24-1	6-0)-0		3-0-0 34-2-8 3-6-0 1-2-8	
			Eve —			Scal	e = 1:67.5
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0 340	-	4x6 =		3x4 =		4x12	
		3x4 =		4x6 =			
	11-6-0		21-6-0		32-0-0	33-0-0	
	11-6-0		10-0-0		10-6-0	1-0-0	
	-7-0,0-1-1], [0.0-0-2, 1-1-1]						
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.65	DEFL. in Vert(LL) -0.15	(loc) 10-13	l/defl L/d >999 360	PLATES GRIP MT20 244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.60	Vert(CT) -0.26	8-10	>999 240		
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	WB 0.31 Matrix-S	Horz(CT) 0.05 Wind(LL) 0.05	8 2-13	n/a n/a >999 240	Weight: 223 lb FT = 20%	
			BRACING-				
TOP CHORD 2x6 SP No	.1		TOP CHORD	Structur	ral wood sheathing direc	tly applied or 4-11-2 oc purlins.	
BOT CHORD 2x6 SP No WEBS 2x4 SP No	.1 .2		BOT CHORD	Rigid ce	eiling directly applied or	10-0-0 oc bracing.	
WEDGE							
Right: 2x6 SP No.1							
REACTIONS. (size)	2=0-3-8, 8=0-3-8 2=245(LC 11)						
Max Uplift	2=-91(LC 12), 8=-91(LC 13)						
Max Grav	2=1525(LC 19), 8=1525(LC	20)					
FORCES. (lb) - Max. Cor	np./Max. Ten All forces 250	(lb) or less except when shown.					
BOT CHORD 2-3=-229 BOT CHORD 2-13=-22	.2/2070, 10-13=-9/1347, 8-10	=-233/1886					
WEBS 3-13=-54	4/300, 5-13=-140/991, 5-10=	-139/991, 7-10=-544/300					
NOTES-							
<ol> <li>Unbalanced roof live loa</li> <li>Wind: ASCE 7-10; Vult=</li> </ol>	ads have been considered for 130mph Vasd=103mph; TCI	this design. DL=6.0psf; BCDL=6.0psf; h=15ft; C	Cat. II; Exp C; Enclosed	d; MWFR	S (envelope)		
and C-C Exterior(2) -1-0	-6 to 3-4-7, Interior(1) 3-4-7 t	o 16-6-0, Exterior(2) 16-6-0 to 20-	10-13, Interior(1) 20-10	)-13 to 34	-0-6 zone;		
<ul><li>3) This truss has been des</li></ul>	igned for a 10.0 psf bottom c	hord live load nonconcurrent with a	any other live loads.	e grip DC	JL-1.60	STATE DISTORTS	
<ol> <li>4) * This truss has been de will fit between the botto</li> </ol>	esigned for a live load of 30.0	psf on the bottom chord in all areas	s where a rectangle 3-	6-0 tall by	/ 2-0-0 wide	CAP	
5) Provide mechanical con	nection (by others) of truss to	bearing plate capable of withstan	ding 100 lb uplift at joir	nt(s) 2, 8.		CALLER HAL	
<ol> <li>This truss is designed in referenced standard AN</li> </ol>	accordance with the 2015 In	ternational Residential Code section	ons R502.11.1 and R8	802.10.2 a	and 💦	TO MAN	
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February 16,2022



Job	Truss	Truss Type	Qty Ply	Lot 37 Liberty Meadows	
J1022-5096	A3	COMMON	9	1	150256512
Comtech, Inc, Favettev	/ille, NC - 28314,		8.430 s A	Job Reference (optional) ug 16 2021 MiTek Industries, Ir	ic. Tue Feb 15 14:17:18 2022 Page 1
r1-2-8	8-6-0	II 16-6-0	D:aTXuLo?nW09qtpROz2WQ 24-6-0	0wydkZW-DxV2X22RYpUNR4	<pre><v1iwpuxwqvaccks4i_uzmagzkswl 33-0-0<="" pre=""></v1iwpuxwqvaccks4i_uzmagzkswl></pre>
1-2-8	8-6-0	8-0-0	8-0-0	1	8-6-0
			5x5 =		Scale = 1:66.2
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		14		5 2x4 //	
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6 3x6	5 =	7 18 12 11 19 4x6 =	20 10 3x	9 21 22 4 =	3x4 =
		3x4 =	5x8	=	
	11-6-0		21-6-0	33-0-0	
Plate Offsets (X,Y) [8:0	-0-2.Edael		10-0-0	11-6-0	
	SPACING 2.0.	120		l/dofi L/d	
TCLL 20.0	Plate Grip DOL 1.1	TC 0.32	Vert(LL) -0.15 9-12	>999 360	MT20 244/190
BCLL 0.0 *	Rep Stress Incr YES	6 BC 0.54 6 WB 0.32	Horz(CT) -0.27 8-9	>999 240 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.05 2-12	>999 240	Weight: 218 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP No	.1		BRACING- TOP CHORD Struct	ural wood sheathing directly	applied or 4-10-2 oc purlins.
BOT CHORD 2x6 SP No	.1		BOT CHORD Rigid	ceiling directly applied or 10-	0-0 oc bracing.
	. <u>.</u>				
Max Horz	2=0-3-8, 8=Mechanical 2=241(LC 11)				
Max Uplift Max Grav	2=-91(LC 12), 8=-75(LC 13 2=1529(LC 19), 8=1461(LC	20)			
FORCES. (lb) - Max Cor	nn /Max Ten - All forces 25	) (lb) or less except when shown			
TOP CHORD 2-3=-230	13/425, 3-5=-2098/466, 5-7=-	2112/480, 7-8=-2319/440			
WEBS 3-12=-54	.5/300, 5-12=-141/990, 5-9=	260/1916 144/1011, 7-9=-555/308			
NOTES-					
<ol> <li>Unbalanced roof live loa</li> <li>Wind: ASCE 7-10: Vult=</li> </ol>	ds have been considered fo 130mph Vasd=103mph ⁻ TC	r this design. DI =6 0psf: BCDI =6 0psf: h=15ft: Ca	t II: Exp.C: Enclosed: MWE	RS (envelope)	
and C-C Exterior(2) -1-0	0-6 to 3-4-7, Interior(1) 3-4-7	to 16-6-0, Exterior(2) 16-6-0 to 20-10	-13, Interior(1) 20-10-13 to 3	32-11-4	
3) This truss has been des	igned for a 10.0 psf bottom of	clions shown, Lumber DOL-1.00 pla	y other live loads.		
<ol> <li>This truss has been de will fit between the botto</li> </ol>	esigned for a live load of 30.0 om chord and any other mem	bers, with BCDL = 10.0psf.	where a rectangle 3-6-0 tall	by 2-0-0 wide	2007/00/ EMPIRIONS
<ol> <li>5) Refer to girder(s) for trus</li> <li>6) Provide mechanical con</li> </ol>	ss to truss connections. nection (by others) of truss t	b bearing plate capable of withstandi	ng 100 lb uplift at ioint(s) 2.	8	CAD CAD
<ol> <li>This truss is designed in referenced standard AN</li> </ol>	accordance with the 2015 I	nternational Residential Code section	ns R502.11.1 and R802.10.2	2 and	of the second
				S 1	CENT 1
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				11°.	STATISTICS STATIST
					February 16,2022
	parameters and READ NOTES ON T				-





- Max Horz 2=301(LC 9) (lb) -
  - Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22 except 21=-107(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 20, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23,
    - 22 21

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-280/222, 10-11=-236/265, 11-12=-236/265

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated. 5) Gable requires continuous bottom chord bearing.

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22 except (jt=lb) 21=107.

10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.









	F		6-0-0						12-0-0		
Plate Off	eete (X V)	[2:0 1 7 0 1 8] [8:0 1 7 0 1 8	6-0-0						6-0-0		
	3et3 (X, T)		1								
LOADIN	G (psf)	SPACING- 2-	0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1	.15	TC (	0.14	Vert(LL)	-0.02 10-11	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1	.15	BC (	0.19	Vert(CT)	-0.03 10-11	>999	240		
BCLL	0.0 *	Rep Stress Incr Y	'ES	WB (	0.09	Horz(CT)	0.01 8	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI20	14	Matrix-8	S	Wind(LL)	0.03 10-11	>999	240	Weight: 65 lb	FT = 20%
LUMBER	۶.					BRACING-				1	

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 WFBS 2x4 SP No.2 OTHERS

REACTIONS. (size) 2=0-3-0, 8=0-3-0 Max Horz 2=-73(LC 17) Max Uplift 2=-143(LC 9), 8=-143(LC 8) Max Grav 2=550(LC 1), 8=550(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown

2-3=-617/666, 3-4=-570/687, 4-5=-561/740, 5-6=-561/740, 6-7=-570/688, 7-8=-617/666 TOP CHORD 2-14=-494/491, 13-14=-494/491, 12-13=-494/491, 11-12=-494/491, 10-11=-494/491, BOT CHORD 8-10=-494/491 WEBS 5-12=-513/328

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=143, 8=143.

9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses systems, see **AUSI/PTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=109, 4=109,

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







February 16,2022







Job	Truss	Truss Type	Qty	Ply	Lot 37 Liberty Meadows
					150256517
J1022-5096	C1-GR	COMMON GIRDER	1	2	
				<b>_</b>	Job Reference (optional)
Comtech, Inc, Fay	etteville, NC - 28314,		8	430 s Aug	16 2021 MiTek Industries, Inc. Tue Feb 15 14:17:28 2022 Page 2

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#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-6=-60, 2-6=-20

Concentrated Loads (lb)

Vert: 9=-1290(F) 11=-1290(F) 12=-1290(F) 13=-1290(F) 14=-1290(F) 15=-1290(F) 16=-1290(F) 17=-1290(F) 18=-1290(F)





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

#### February 16,2022





1	4-10-10	10-10-10	17-10-10	21-1-10	28-1-5	34-	10-10	40-10	-10	46-0-0	1	
T	4-10-10	6-0-0	7-0-0	3-3-0	6-11-11	6-	9-5	6-0-	-0	5-1-6		
Plate Offsets (X,Y) [2:0-4-0,0-1-15], [4:0-3-0,Edge], [7:0-3-0,Edge], [10:0-3-0,Edge], [12:0-4-0,0-1-15], [17:0-4-12,0-2-8], [19:0-1-8,0-2-4]												
LOADING (psf)	SP/	ACING- 2-0	)-0   <b>CSI</b> .		DEFL.	in (loc)	l/defl	L/d	PL/	ATES	GRIP	
TCLL 20.0	Plat	te Grip DOL 1.	15 TC	0.26	Vert(LL) -	0.24 15-17	>999 3	360	MT	20	244/190	

TCDL 10 BCLL 0 BCDL 10	0.0 0.0 * 0.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.44 WB 0.42 Matrix-S	Vert(CT) -0.40 Horz(CT) 0.03 Wind(LL) 0.16	) 15-17 >739 240 3 12 n/a n/a 6 15-17 >999 240	Weight: 376 lb FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SP 1-4,10- 2x8 SP 17-19∷ 2x4 SP	No.1 *Except* 13: 2x4 SP No.1 2400F 2.0E *Except* 2x6 SP No.1 No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing Rigid ceiling directly applie 8-5-9 oc bracing: 18-19 7-9-4 oc bracing: 17-18. 1 Row at midpt	directly applied or 3-11-0 oc purlins. d or 10-0-0 oc bracing, Except: 9-17, 6-19, 5-19, 6-8

REACTIONS. (size) 2=0-3-8, 12=0-3-8, 18=0-3-8 Max Horz 2=156(LC 11) Max Uplift 2=-66(LC 12), 12=-168(LC 13), 18=-120(LC 12) Max Grav 2=1250(LC 1), 12=1365(LC 24), 18=1447(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 
 TOP CHORD
 2-3=-2175/456, 3-5=-1759/445, 5-6=-1250/413, 6-7=-378/186, 7-8=-350/181, 8-9=-1235/382, 9-11=-2082/468, 11-12=-2367/451

 BOT CHORD
 2-22=-304/1880, 21-22=-304/1880, 19-21=-194/1515, 18-19=-52/1043, 17-18=-48/1031, 15-17=-245/1812, 14-15=-325/2045, 12-14=-325/2045

 WEBS
 9-17=-1058/260, 9-15=-26/593, 5-19=-875/234, 5-21=-23/446, 3-21=-393/120, 11-15=-281/104, 6-8=-846/319

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-7 to 3-2-5, Interior(1) 3-2-5 to 23-0-0, Exterior(2) 23-0-0 to 27-4-13, Interior(1) 27-4-13 to 47-2-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 12=168, 18=120.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.









						46-0-0						
Plate Officiert (X X) [6:0.1.0.Edge] [22:0.1.0.Edge] [24:0.4.0.0.4.9] [42:0.4.0.0.4.9]												
	sets (A, T)	[0.0-1-9,Euge], [22.0-1-9	,⊏ugej, [34.0-	4-0,0-4-6], [4/	2.0-4-0,0-4-0							
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	-0.00	`2́7	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	-0.00	27	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.01	26	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 376 lb	FT = 20%
	- )RD 2x4 SF	P No 1		1		BRACING-	ח! ח	Structu	ral wood	sheathing di	rectly applied or 6-0-0	oc purlins

BOT CHORD

WEBS

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No 1

OTHERS 2x4 SP No 2

#### REACTIONS. All bearings 46-0-0.

- Max Horz 2=245(LC 16) (lb) -
  - Max Uplift All uplift 100 lb or less at joint(s) 2, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 26 Max Grav All reactions 250 lb or less at joint(s) 2, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 26
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- 2-3=-322/96, 11-12=-106/275, 12-13=-127/337, 13-14=-146/387, 14-15=-146/387, TOP CHORD 15-16=-127/337, 16-17=-106/275 BOT CHORD 2-48=-82/271, 47-48=-82/271, 46-47=-82/271, 45-46=-82/271, 44-45=-82/271, 43-44=-82/271, 42-43=-82/271, 41-42=-82/271, 40-41=-82/271, 39-40=-82/271, 38-39=-82/271, 37-38=-82/271, 36-37=-82/271, 35-36=-82/271, 34-35=-82/271,
  - 33-34=-82/271, 32-33=-82/271, 31-32=-82/271, 30-31=-82/271, 29-30=-82/271, 28-29=-82/271, 26-28=-82/271

#### NOTES-

- Unbalanced roof live loads have been considered for this design.
   Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 26.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTer&® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPHI Quality Criteria**, DSB-89 and BCSI Building Component **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.

Brace must cover 90% of web length.

Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2x4 SPF No.2 - 14-38, 13-39, 12-40, 11-41

, 15-37, 16-36, 17-35





			6-0-0							
Plate Offsets (X,Y)- [2:0-2-15,Edge]										
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	<b>CSI.</b> TC 0.41	DEFL. in (loc) I/defl L/d Vert(LL) -0.01 2-4 >999 360	PLATES         GRIP           MT20         244/190						
BCLL 10.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-P	Ven(CT) -0.03 2-4 >999 240 Horz(CT) 0.00 n/a n/a Wind(LL) 0.03 2-4 >999 240	Weight: 29 lb FT = 20%						
LUMBER-			BRACING-							

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No 1

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=83(LC 8) Max Uplift 2=-132(LC 8), 4=-90(LC 8) Max Grav 2=316(LC 1), 4=215(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=132.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







			5-0-0	1						
Plate Offsets (X,Y) [2:0-2-15,Edge]										
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.26 BC 0.08 WB 0.00	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.01         2.4         >999         360           Vert(CT)         -0.01         2.4         >999         240           Horz(CT)         0.00         n/a         n/a           Word(LL)         0.04         0.4         >000         0.40	PLATES         GRIP           MT20         244/190						
BCDL 10.0		IVIdu IX-F	Wild(LL) 0.01 2-4 2999 240	Weight: 24 lb F1 - 20 %						
LUMBER-			BRACING-							

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No 1

TOP CHORD BOT CHORD

5-0-0

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=72(LC 8) Max Uplift 2=-119(LC 8), 4=-72(LC 8) Max Grav 2=277(LC 1), 4=174(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=119.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.07 BC 0.02 WB 0.03 Matrix-P	DEFL.         in         (lo           Vert(LL)         0.00         Vert(CT)         0.00           Vert(CT)         0.00         Horz(CT)         -0.00	bc) l/defl L/d 1 n/r 120 1 n/r 120 5 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 24 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF OTHERS 2x4 SF	No.1 No.1 No.2 No.2		BRACING- TOP CHORD Stru exc BOT CHORD Rig	uctural wood sheathing di ept end verticals. id ceiling directly applied o	rectly applied or 5-0-0 oc purlins, or 10-0-0 oc bracing.

REACTIONS. All bearings 5-0-0.

(lb) - Max Horz 2=104(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 5, 6, 2, 7

Max Grav All reactions 250 lb or less at joint(s) 5, 6, 2, 7

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6, 2, 7.
- 8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







			1				3-6-0				I		
Plate Off	Plate Offsets (X,Y) [2:0-2-15,Edge]												
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	-0.00	2-4	>999	240			
BCLL	0.0 ^	Rep Stress Incr	YES	VVB	0.01	Horz(CT)	0.00	0	n/a	n/a		FT - 200/	
RCDL	10.0	Code IRC2015/11	12014	Matri	x-12	vvind(LL)	0.00	2		240	vveight: 17 lb	FI = 20%	

BRACING-

TOP CHORD

BOT CHORD

3-6-0

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

**REACTIONS.** (size) 2=0-3-8, 4=0-1-8 Max Horz 2=56(LC 8)

Max Uplift 2=-69(LC 8), 4=-14(LC 12) Max Grav 2=224(LC 1), 4=107(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

Structural wood sheathing directly applied or 3-6-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Eebruary 16,2022



LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.02 WB 0.02 Matrix-P	DEFL. Vert(LL) 0.0 Vert(CT) 0.0 Horz(CT) 0.0	in (loc) 0 4 0 4 0	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES         GRIP           MT20         244/190           Weight: 15 lb         FT = 20%
LUMBER-	2 No 1		BRACING-	Structu	ural wood	choothing di	roothy applied or 2.6.0 on purling

TOP CHORD BOT CHORD 2x4 SP No.1 2x4 SP No.2 WEBS

TOP CHORD

Structural wood sheathing directly applied or 3-6-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 7=3-6-0, 2=3-6-0, 8=3-6-0

Max Horz 2=79(LC 8) Max Uplift 7=-26(LC 8), 2=-91(LC 8), 8=-38(LC 12)

Max Grav 7=52(LC 1), 2=164(LC 1), 8=125(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

ł

- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2, 8.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





**REACTIONS.** All bearings 18-5-5.

(lb) - Max Horz 1=177(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-172(LC 12), 6=-172(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=411(LC 22), 9=560(LC 19), 6=560(LC 20)

WEBS 2-9=-428/293, 4-6=-428/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 9-3-1, Exterior(2) 9-3-1 to 13-7-14, Interior(1) 13-7-14 to 18-1-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=172, 6=172.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.15 BC 0.17 WB 0.10 Matrix-S	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         n/a         -         n/a         999           Vert(CT)         n/a         -         n/a         999           Vert(CT)         n/a         -         n/a         999           Horz(CT)         0.00         5         n/a         n/a	PLATES         GRIP           MT20         244/190           Weight: 69 lb         FT = 20%
LUMBER-			BRACING-	

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-7-11.

(lb) - Max Horz 1=-149(LC 8)

2x4 SP No.2

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-143(LC 12), 6=-142(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=411(LC 19), 8=429(LC 19), 6=429(LC 20)

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-10-5, Exterior(2) 7-10-5 to 12-3-1, Interior(1) 12-3-1 to 15-3-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=143.6=142.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-8=-356/254, 4-6=-356/254



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-314/239, 4-6=-314/239

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-5-8, Exterior(2) 6-5-8 to 10-10-5, Interior(1) 10-10-5 to 12-6-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=124, 6=124.

6) Non Standard bearing condition. Review required.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=199(LC 1), 3=199(LC 1), 4=347(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







REACTIONS. (size) 1=7-2-14, 3=7-2-14, 4=7-2-14 Max Horz 1=-65(LC 8) Max Uplift 1=-23(LC 13), 3=-29(LC 13)

Max Grav 1=151(LC 1), 3=151(LC 1), 4=220(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

**REACTIONS.** (size) 1=4-5-5, 3=4-5-5, 4=4-5-5 Max Horz 1=-37(LC 8)

Max Uplift 1=-13(LC 13), 3=-16(LC 13)

Max Grav 1=86(LC 1), 3=86(LC 1), 4=125(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This trust has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- a) Provide mechanical connection (by others) or trust to bearing plate capable of withstanding 100 ib upint at joint(s) 1, 3.
   7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 4-6-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.







1	isDesign		Client: Project: Address:	Precision Custon Liberty 2.0 Brewster Cour Cameron, NC	1 Homes t 28396		Date: Input by: Job Nam	10/10/2022 David Landry e: Lot 37 Liberty Meadows	Page 2 of 1
BM1	Kerto-S L	VL 1	.750''	X 16.000"	2-Ply	- PASSI	ED	Level: Level	
	•••	•••	•	•••	•••	•	•	• • •	
	•••	· ·	•	•••	•	•••	•	••••	$\cdot \cdot \cdot = \frac{\nabla}{2} \mathbb{M} = \frac{1}{2}$
1 SPF									
					16' 1/2"				<b>1</b> (  1 3 1/2"
									•
Multi-Ply	y Analysis			<i></i>					
Fasten all Capacity	l plies using 4 ro	ws of 10d 81.7 %	Box nails	(.128x3") at 12	" o.c Maxin	num end dis	tance n	ot to exceed 6".	
Load Yield Limit pe	er Foot	267.5 PLI 327.4 PLI	:						
Yield Limit pe	er Fastener	81.9 lb.							
Edge Distan	се	1V 1 1/2"							
Min. End Dis	stance	3"							
Load Combin Duration Fac	nation	D+L 1.00							
Notes		chem	icals		6. For flat roofs p	rovide proper drainage	e to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Rei∎v Road, Suite #639
Calculated Struc structural adequ design criteria responsibility of	ctured Designs is responsible on Jacy of this component based and loadings shown It the customer and/or the contr	on the 1. LVL b is the 2. Refer actor to recar	ng & Installa eams must not b to manufact ding installatio	l <b>tion</b> a cut or dri∎ed urer's product information n requirements, multi-ol	ponding 1			Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	Fayetteville, NC USA 28314 910-864-TRUS
ensure the co application, and t Lumber 1. Dry service c	omponent suitability of the i to verify the dimensions and load conditions, unless noted otherwis	ntended faster Is appro 3. Dama 4. Desig e 5. Provi	vals vals ged Beams must n assumes top er de lateral suppo	m strength values, and code not be used lige is laterally restrained t at bearing points to avoit	9			(800) 622-5850 www.metsawood.com/us	Loom Tool I
2. LVL not to be	e treated with fire retardant or c	prrosive latera	displacement ar	d rotation	This design i	s valid until 11/3/2	2024		Connech

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

	/	Clie	ent: Precis	sion Custom Home	s		Date:	10/10	/2022				Page 3 of 14
Í	isDesign	Pro <u></u> Ado	ject: Libert tress: Brew	y 2.0 vster Court			Input by Job Na	y: David me: Lot37	Landry 7 Liberty M	leadows			
÷			Cam	eron, NC 28396			Project	#: J1022	-5097				
BM2	Kerto-S LV	Ľ 1.7	50" X 16	5.000" 2	-Ply - P	ASS	ED	Level: Le	vel				
								3					
		2											
•	• • • • • •	• •	• •	• •	• •	•	• •	•	•	• •	• •	M	$\overline{1}$
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1 SPF		•	• •	• •	• •	•	• •	•	•	• •	2 SPF	ш	<u> </u>
				20'							,		1/2"
/				20							,	1 13	172
				20									
Member	Information					Reac	tions U	NPATTE	RNED II	o (Uplift	)		
Type: Plies:	Girder		Application:	Floor		Brg	Direction	ר Li	ve	Dead	Snow	Wind	Const
Moisture C	ondition: Dry		Building Code	: IBC 2012		2	Vertical	4	.00	2409 2409	135	0	0
Deflection	_L: 480		Load Sharing:	No									
Deflection	IL: 240		Deck:	Not Checke	d								
Temperatu	re: Temp <= 100°	'F											
						Bear	ings						
						Bea	ring Leng	gth Dir.	Cap.	React D/L	b Tota	Ld. Case	Ld. Comb.
						1-8	SPF 3.50	10" Vert 10" Vert	54% 54%	2409/2	401 2811 401 2811	L	D+0.75(L+S)
Analysis	Results					<u>ر</u>							
Analysis	Actual	Location Allo	owed Caj	pacity Comb.	Case								
Unbraced	13439 ft-lb	10 343	92 ft-lb 0.99	96 D+L	L								
Ohaan	0464 lb	19/4 1/01 110	(10)	0%) 06 (24%) Dul									
Snear	2461  JD	10' 1/16" 0 48	14710 0.20 89(1/480)01;	06 (21%) D+L 21 (12%) 0 75(I +S	L S)								
TL Defl in	ch 0.415 (L/565)	10' 1/16" 0.9	78 (L/240) 0.42	25 (42%) D+0.75(I	_+S) L								
Design N	otes					1							
1 Provide may also	support to prevent latera	al movement ar	nd rotation at th	e end bearings. La ide	iteral support	1							
2 Fasten a	all plies using 3 rows of	10d Box nails (.	.128x3") at 12"	o.c. Maximum end	distance not								
3 Refer to	a o . last page of calculation	s for fasteners i	required for spe	cified loads.									
4 Girders	are designed to be supported or	ported on the bo	ottom edge only										
6 Top mus	t be laterally braced at a	a maximum of 8	3'9 7/16" o.c.										
7 Bottom i 8 Lateral s	nust be laterally braced lenderness ratio based	at end bearing on single ply w	s. idth										
ID	Load Type	Loc	ation Trib V	Vidth Side	Dead 0.9	ן ו	ive 1 S	now 1.15	Wind	1.6 Cons	t. 1.25 Co	omments	
1	Uniform			Тор	200 PLF		0 PLF	0 PLF	0 F	۲LF	0 PLF W	all Above, C10	GE
2	Tie-In	0-0-0 to 2	20-0-0 1-0-0	Far Face	15 PSF	4	0 PSF	0 PSF	0 P	SF	0 PSF Fl	oor Load	
3	Tie-In	0-0-0 to 2	20-0-0 0-6-0	Near Fac	e 27 PSF		) PSF	27 PSF	0 P	SF	0 PSF J3		
	Self Weight				12 PLF								
								<b>.</b>				- 100	
Notes Calculated Struct	ured Designs is responsible only of	chemicals f the Handling &	Installation	6. For por	flat roofs provide p iding	oroper drain	age to prevent	t Manufac Metsä W	turer Info		1001 S. Fayette	ı, ınc. Rei∎y Road, Suite≢ rille, NC	#639
structural adequa design criteria	and loadings shown. It is	the 1. LVL beams r the 2. Refer to	nust not be cut or drille manufacturer's proc	d luct information				301 Merr Norwalk	itt 7 Building CT 06851	g, 2nd F <b>l</b> oor	USA 28314 910-86/	TRUS	
ensure the con application, and to	ponent suitability of the inten overify the dimensions and loads.	nded fastening de approvals	installation requiren tails, beam strength י	nents, multi-ply values, and code				(800) 622 www.met	2-5850 sawood.cor	m/us	310-002		
1. Dry service co	nditions, unless noted otherwise	<ol> <li>Damaged Be</li> <li>Design assu</li> <li>Provide late</li> </ol>	eams must not be used mes top edge is lateral ral support at bearing	y restrained g points to avoid							12	COMT	ecH
2. LVL not to be	ueated with fire retardant or corros	ateral displa	cement and rotation	Th	is design is va <b>l</b> ic	l until 11/:	3/2024						

1	isDesign		Client: Project: Address:	Precision Custom Liberty 2.0 Brewster Court Cameron, NC 2	Homes 8396		Date: Input by: Job Name Project #:	10/10/2022 David Landry 9: Lot 37 Liberty Meadows J1022-5097		Page 4 of 14
BM2	Kerto-S	LVL	1.750"	X 16.000"	2-Ply	- PASS	ED	Level: Level		
							1			
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1 SPF	••••	•	•••	••••	•••	• ·	•	•••••	2 SPF	
					20'					1 1/2"
1					20'				1	
Multi-Ply	y Analysis									
Fasten al	l plies using 3 r	ows of 10	d Box nai <b>l</b> s	(.128x3") at 12'	' o.c Maxin	num end di	stance no	ot to exceed 6".		
Capacity Load		11.2 % 27.5 PI	_F							
Yield Limit p	er Foot	245.6 F	PLF							
Yield Linit p	el Fastellel	IV								
Edge Distan Min, End Did	nce	1 1/2" 2"								
Load Combi	ination	3 D+L								
Duration Fac	ctor	1.00								
Notes Calculated Struc design criteria responsibility of ensure the cc application, and the company of the company ensure the company and the company of the company service of the company 1. Dry service of the company 2. UVI end to b	ctured Designs is responsible usecy of this component bas and badings shown. If the customer and/or the co omponent suitability of the to verify the dimensions and I conditions, unless noted other se treated with fix reteriation	contry of the Han and on the 1, Li instrate 2, R ontractor to re- intended fa oads. a 3, D wise 5, P	nemicals diling & Installat VL beams must not be der to manufactu agarding installation steining detallas steining detallas magadel Beams must esign assumes top ad rovide lateral support	tion cut or drilled refs product information requirements, multi-ply strength values, and code not be used ge is taterally restrained at bearing points to avoid	6. For flat roofs pr ponding	rovide proper drainag	e to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (600) 622-5850 www.metsawood.com/us	Comtech, Inc. 1001 S. Reilly Ro Fayetteville, NC USA 28314 910-964-TRUS	ad, Suite #639
2. LVL not to b	pe treated with fire retardant o	or corrosive la	teral displacement and	I rotation	This design i	s valid until 11/3/	2024			meen

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1



1	isDesign	Client: Project: Address:	Precision Custom Homes Liberty 2.0 Brewster Court Cameron, NC 28396	Date Inpu Job Proj	e: 10/10/2022 It by: David Landry Name: Lot 37 Liberty M ect #: .11022-5097	Page 6 of 14 eadows
BM3	S-P-F #2	2.000" X	10.000" 2-Ply	- PASSED	Level: Level	
	• • PF End Grain	• •	• •	•	• •	2 SPF End Grain
/			10'3 1/2"			
<i>†</i>			10'3 1/2"			ł
Fasten all Capacity Load Yield Limit py Yield Limit py Yield Mode Edge Distan Min. End Dis Load Combin Duration Fac	I plies using 2 row er Foot er Fastener ce stance nation ctor	As of 10d Box nails 0.0 % 0.0 PLF 157.4 PLF 78.7 lb. IV 1 1/2" 3" 1.00	(.128x3") at 12" o.c Maxi	mum end distan	ce not to exceed 6"	
			This desing	1 is valid until 11/3/2024	Manufacturer Info	Comtech, Inc. 1001 S. Reily Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS

Process     Barry 20 Carrendon, NC 2830     Huge Arrow Instruction Program     Exact Lastity and Lastity in program       BIM     Kerto-S LVL     1.750° X 9.250°     2.PIP - PASSED       Product Line     Image: Control of the c	126	1	Cli	ient: Pr	ecision Cus	stom Homes			Date:	10/10/2	022				Page 7 of 14
Allow Breader Court Carmenton, NC 23808     Auditary Breader Court Frequent B. 102-2007       BMA Kerto-S LVL     1,750° X 9,250° 2.Ply - PASSED     Seet Level       Sevel Level     Sevel Level       SPE Loop     Sevel Level	1		Pro	oject: Lil	perty 2.0				Input by	: David I	andry				
Currantic Product         Currant         Currant <thcurrant< th="">         Currant         <thcurrant< th=""></thcurrant<></thcurrant<>	E	sDesign	Ad	Idress: Br	rewster C	ourt			Job Nai	ne: Lot 37	Liberty Meac	lows			
BMA     Kerto-S LVL     1.750" X 9.250"     2.Pip - PASSED     Non total       3	-			Ca	ameron, M	IC 28396			Project	#: J1022-	5097				
	BM4	Kerto-S LV	′L 1.	.750" )	K 9.25	50" 2	Ply -	PAS	SSED	Level: Lev	el				
3       3         2       3         3       3         3       3         3       3         3       3         3       3         3       3         3       3         3       3         3       3         4       3         4       3         4       3         4       3         4       3         4       3         4       3         4       3         4       3         4       3         4       3         4       3         4       3         4       3         4       3         4       4         4       4         4       4         4       4         4       4         4       4         4       4         4       4         4       4         4       4         4       4         4       4															
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SPF End Cein         SPF End Cein         OPT         Rember Information         Resctions UNPATTERNED Is (Uplift)         Thm:       Application:       Flore       Procession       Processin       Procession       Procession	•		•		•	•		•		•				VV	
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Reactions UNPATTERNED Ib (Uplift)       Type:     Circle     Descion Method:     Appleation:     Floor       Ples:     2       Mostare Condition: Dry     Descion Method:     ASD       Deferion LL:     480       Deferion LL:     480       Deferion LL:     480       Deferion LL:     380       Importance:     Normal - II       Tamparature:     Temp <= 100°F       Analysis Results:     Not Checked       Analysis Actual     Location Allowed Capacity Comb.       Organ     Capacity Comb.       Capacity Comparison:     Sign Morest       Straff Lib     312' 12424 Alb     0.406 (45%) Dr.L       Lib Definich 0.056 (L1520)     312' 12' 12424 Alb     0.406 (45%) Dr.L       Lib Definich 0.056 (L1520)     312' 12' 1242 Alb     0.406 (45%) Dr.L       Lib Definich 0.056 (L1520)     312' 12' 1242 Alb     0.406 (45%) Dr.L       Lib Definich 0.056 (L1520)     312' 12' 1242 Alb     0.406 (45%) Dr.L       Lib Definich 0.056 (L1520)     312' 12' 12' 12' 12' 12' 12' 12' 12' 12'	1				6'7"						1				
Reactions UNPATTERNED Ib (Uplift)         Type:       Circle       Application:       Floor       Besign Method:       ASD         Motistor Condition: Dry       Design Method:       ASD       Live 11       Vertical       2140       1903       0       0         Deletation L1:       480       Design Method:       ASD       Baseline:       Not Checked       1       Vertical       2140       1903       0       0         Integrature:       Temp <= 100°F       Deck:       Not Checked       Bearing Length       Dir.       Cap.       Reactions UNPATTERNED Ib (Uplift)         Integrature:       Temp <= 100°F       Dir.       Not Checked       Dir.       Cap.       Reactions UNPATTERNED Ib (Uplift)         Integrature:       Temp <= 100°F       Dir.       Not Checked       Dir.       Cap.       Reactions UNPATTERNED Ib (Uplift)         Integrature:       Temp <= 100°F       State															
Type:         Grider         Application:         Floor         Desciption         Line and point of the second sec	Member li	nformation						Read	ctions UI	NPATTER	NED lb (I	Jplift)			
Piles:       2       Design Method:       ASD       Vertical       2/49       1003       0       0         Deflection L1:       480       Design Method:       No       Design Method:       1       Vertical       2/149       1003       0       0         Deflection L1:       480       Design Method:       No       Design Method:       No       Design Method:       0       0       0       0         Importance:       Torm p < 100'F       Design Method:       No       Design Method:       No       Design Method:       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0 <t< th=""><th>Туре:</th><th>Girder</th><th></th><th>Application</th><th>:: F</th><th>loor</th><th></th><th>Brg</th><th>Direction</th><th>Liv</th><th>e De</th><th>ead :</th><th>Snow</th><th>Wind</th><th>Const</th></t<>	Туре:	Girder		Application	:: F	loor		Brg	Direction	Liv	e De	ead :	Snow	Wind	Const
Modelardion LUY       Ebuilding Code:       IBC 2012       2       Vertical       2.149       1903       0       0         Deflection LT:       360       Incompariance:       Not Checked       Incompariance:       Not Checked       Incompariance:       Not Checked         Importance:       Temperature:       Temperature:       Temperature:       Not Checked       Incompariance:       Not Checked       Incompariance:       Incompari	Plies:	2		Design Me	thod: A	SD		1	Vertical	214	9 1	903	0	0	0
Loade sharing: No Deck: Nort All Checked Importance: Normal -I Temperature: Temp <= 100°F Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearings Bearing Bearings Bearings Bearings B	Moisture Co	ndition: Dry		Building Co	ode: IE	3C 2012		2	Vertical	214	9 1	903	0	0	0
builde subject to less provide subject to prevent lateral movement and rotation at the end bearings. The Definition 0.056 (L7130) 33 1/2 ² 0.153 (L480) 0.344 (05%) D+L L LL Definition 0.056 (L7130) 33 1/2 ² 0.153 (L480) 0.344 (05%) D+L L LL Definition 0.056 (L7130) 33 1/2 ² 0.153 (L480) 0.344 (05%) D+L L LL Definition 0.056 (L7130) 33 1/2 ² 0.153 (L480) 0.344 (05%) D+L L LL Definition 0.056 (L7130) 33 1/2 ² 0.153 (L480) 0.344 (05%) D+L L LL Definition 0.056 (L7130) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L LL Definition 0.056 (L7130) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L LL Definition 0.056 (L7130) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L L Definition 0.056 (L7130) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L L Definition 0.056 (L7130) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 33 1/2 ² 0.244 (L680) 0.541 (51%) D+L L Definition 0.056 (L7100) 0.541	Deflection LI	L: 480		Load Shari	ng: N	0 ot Checked									
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Bearings       Bearing Length Dir. Cap. React DJL Ib Total Ld. Case Ld. Com 1. SPF 3.500° Vert 39% 1903 / 2149 4053 L D+L End Grain       Analysis     Actual     Location Allowed     Capacity Comb. Case 0 (400 (49%) D+L L Untraced 5774 fr-b     2. SPF 3.500° Vert 39% 1903 / 2149 4053 L D+L End Grain     2. SPF 3.500° Vert 39% 1903 / 2149 4053 L D+L End Grain     4053 L D+L End Grain       2. SPF 3.500° Vert 39% 1903 / 2149     4053 L     D+L End Grain     2. SPF 3.500° Vert 39% 1903 / 2149 4053 L     D+L End Grain       1. Deflicion 0.056 (L/1320) 33 1/2° 034 (4080 0.0346 (49%) D+L LD efficion 0.056 (L/1320) 33 1/2° 0.204 (4080 0.046 (49%) D+L L Deflicion 0.056 (L/1320) 33 1/2° 0.204 (4080 0.046 (49%) D+L L Deflicion 0.056 (L/1320) 33 1/2° 0.204 (4080 0.046 (49%) D+L L Deflicion 0.056 (L/1320) 33 1/2° 0.204 (4080 0.046 (49%) D+L L Deflicion 0.056 (L/1320) 33 1/2° 0.204 (4080 0.046 (49%) D+L L Deflicion 0.056 (L/1320) 31 2° 0.204 (4080 0.046 (49%) D+L L Deflicion 0.056 (L/1320) 31 2° 0.204 (4080 0.046 (49%) D+L End Stefer to laster are designed to be supported on the bolton edge only. 5 Top loads much sugnature and the inferior bearings by the building code. 5 Top loads much sugnature and the bolton edge only. 5 Top load Stefer to laster are designed to be supported on the bolton edge only. 5 Top load Stefer to laster are designed to be supported on the bolton edge only. 5 Top load Stefer to laster are designed to be supported on the bolton edge only. 5 Top load Stefer to laster are designed to be supported on the bolton edge only. 5 Top load Stefer to laster are designed to be supported on the bolton edge only. 5 Top load Stefer to laster are designed to be supported on the bolton edge only. 5 Top load Stefer to laster are designed to be supported on the bolton ed	Temperature	e: Temp <= 100°F													
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Image: set in the set in								Bea	aring Len	gth Dir.	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Comb.
Image: Securits     End Grain     Case Care     End Grain     Case Care     End Care     Case Care     End Care     Case Care     Care     Care     Care     Care     Care     Care     Care     Care     Care <thc< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>1-</td><td>SPF 3.50</td><td>0" Vert</td><td>39% 1</td><td>903 / 2149</td><td>4053</td><td>L</td><td>D+L</td></thc<>								1-	SPF 3.50	0" Vert	39% 1	903 / 2149	4053	L	D+L
Analysis Results Analysis Actual Analysis Analys	Nabycic P	oculto						_ End Gra	d ain						
Character Departure Control in Control i			ocation All	owed	Canacity	Comb	Case	2-	SPF 3.50	0" Vert	39% 1	903 / 2149	4053	L	D+L
Ubinance 5774 H-lb       33 1/2" 9934 fH-lb       OSI (59%) D+L       L         Shear       2750 lb       1' 3'4'       6907 lb       0.398 (49%) D+L       L         LL Definich       0.056 (U/1320)       3' 31 1/2'       0.153 (L480)       0.388 (49%) D+L       L         LL Definich       0.056 (U/1320)       3' 31 1/2'       0.153 (L480)       0.388 (49%) D+L       L         Lessign Notes       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1	Moment	5774 ft-lb	3'3 1/2" 12	542 ft-lb	0.460 (46%	D+L	L	Enc	d viz						
Shear       2750 lb       1' 3/4' 6 907 lb       0.398 (40%) D+L       L         LL Defl inch       0.056 (L/1202)       3'3 1/2'       0.153 (L/480)       0.564 (36%) L       L         TL Deflinch       0.105 (L/1202)       3'3 1/2'       0.153 (L/480)       0.564 (36%) D+L       L         Provide support 10 prevent lateral movement and rotation at the end bearings. Lateral support       1       L         Provide support 10 prevent lateral movement and rotation at the end bearings. Lateral support       3'3 1/2'       0.514 (51%) D+L       L         Provide support 10 prevent lateral movement and rotation at the end bearings.       C. Maximum end distance not to exceed 6''.       Stateral black supported on the bottom edge only.       Stateral black supported on the	Unbraced	5774 ft-lb	3'3 1/2" 99	34 ft-Ib	0.581 (58%	) D+L	L	Gia							
LL Defl inch 0.056 (L/1320) 3'3 1/2" 0.153 (L/480) 0.364 (36%) L L T. Defl inch 0.105 (L/700) 3'3 1/2" 0.204 (L/360) 0.514 (51%) D+L L Persign Notes Persign Notes Provide support to prevent lateral movement and rotation at the end bearings. Lateral support 1 provide support to prevent lateral movement and rotation at the end bearings. Lateral support 2 Pasten all piles using 2 rows of 10d Box nalls (128x3") at 12" o.c. Maximum end distance not 10 exceed 6". 3 Refer to last page of calculations for fasteners required for specified loads. 4 Girders are designed to be supported quality by all piles. 5 Top loads must be supported quality by all piles. 6 Top must be laterally braced at end bearings. 8 Lateral stendermess ratio based on single pily width. ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform 1 Uniform 3 Uniform 3 Uniform 3 Uniform 3 Uniform 5 Refer to the previde previde previde previde prevent materials to a rotation of the bubble of the supported on single pily width. ID Load Type Location Trib Width Side 9 Uniform 3 Uniform 5 Refer to last page of calculations for fasteners required to a rotation of the bubble of the supported on single pily width. ID 1 Uniform 3 Uniform 5 Refer to last page of calculations for fasteners required to a rotation of the bubble of the supported on single pily width. ID 1 Uniform 3 Uniform 5 Refer Weight 7 PLF 5 Refer to the previde prevent materials to a rotation of the bubble of the support of the support do not rotation of the bubble of the support of prevent materials to a rotation of the bubble of the support do not rotation of the bubble of the support do not rotation of the bubble of the support do not rotation of the bubble of the support do not rotation of the bubble of the support to not rotation of the bubble of the support to rotation of the bubble of the support to a rotation of the bubble of the support to rotation the bubble of the support to rotation the bubble of the	Shear	2750 lb	1' 3/4" 69	07 lb	0.398 (40%	) D+L	L								
TL Deflinen 0.105 (L/200) 3'3 1/2' 0.204 (L/360) 0.514 (51%) D+L L  Pesign Notes  1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support 2 Pasten all plies using 2 rows of 10d Box nalls (.128x3'') at 12'' o.c. Maximum end distance not to exceed 0''. 3 Refer to last page of calculations for fasteners required for specified loads. 4 Girders are designed to be supported on the bottom edge only. 5 Top load must be supported qualiby by all plies. 6 Top must be laterally braced at end bearings. 7 Bottom must be laterally braced at end bearings. 7 Bottom must be laterally braced at end bearings. 8 Lateral slend=mess ratio based on single ply width. 1 Uniform 1 Uniform 1 Uniform 1 Uniform 1 Top 1 20 PLF 2 Uniform 3 Girden 1 Top 1 20 PLF 3 49 PLF 3 49 PLF 9 PL 9 PL 9	LL Defl inch	h 0.056 (L/1320)	3'3 1/2" 0.1	153 (L/480)	0.364 (36%	) L	L								
Design Notes         1       Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.       2         2       Fasten all ples using 2 rows of 10d Box nalls (.126X3") at 12" o.c. Maximum end distance not to exceed 6".       3         3       Refer to last page of calculations for fasteners required for specified loads.       4. Girders are designed to be supported on the bottom edge only.       5         5       Top loads must be laterally braced at end bearings.       5       Top loads must be laterally braced at end bearings.       5         7       Bottom must be laterally braced at end bearings.       5       Lateral stendermess ratio based on single ply width.       Top       102 PLF       304 PLF       0 PLF       0 PLF       0 PLF       5       Comments         1       Uniform       Top       102 PLF       304 PLF       0 PLF       0 PLF       0 PLF       6       PLF       2       0 PLF       0 PLF       0 PLF       0 PLF       1       10       1.0       Linkel       7       7       12       0 PLF       1.0       1.0       1.0       1.0       1.0       1.0       1.0	TL Defl incl	h 0.105 (L/700)	3'3 1/2" 0.2	204 (L/360)	0.514 (51%	) D+L	L								
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may also be required at the interior beamgs by the building code.       2 Fasten all piles using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".         3 Refer to last page of calculations for fasteners required for specified loads.       4 Girders are designed to be supported on the bottom edge only.         5 Top loads must be supported equally by all piles.       5 Top loads must be supported equally braced at end bearings.         7 Bottom must be laterally braced at end bearings.       8 Lateral stendermess ratio based on single ply width.         ID       Load Type       Location Trib Width Side       Dead 0.9       Live 1       Snow 1.15       Wind 1.6       Const. 1.25       Comments         1       Uniform       Top       102 PLF       304 PLF       0 PLF       0 PLF       0 PLF       F 2         2       Uniform       Top       120 PLF       0 PLF       0 PLF       0 PLF       Wind 1.6       Const. 1.25       Comments         3       Uniform       Top       120 PLF       0 PLF       0 PLF       0 PLF       Wind 1.6       Const. 1.25       Comments         4       Uniform       Top       120 PLF       0 PLF       0 PLF       0 PLF       Wind 1.6       Contect, Inc.       Uniform       Uniform       Top       120 PLF       0 PLF       0 PLF       0 PLF       <	1 Provide s	upport to prevent lateral	movement a	and rotation a	t the end b	earings. Late	eral support	1							
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Store and reaction of the construction of the control of the co	3 Refer to la	ast page of calculations	for fasteners	required for	specified lo	ads.									
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2       Uniform       Top       349 PLF       349 PLF       0 PLF       0 PLF       0 PLF       A1         3       Uniform       Top       120 PLF       0 PLF<	1	Uniform				Тор	102 PLF	30	04 PLF	0 PLF	0 PLF	0 P	_F F2		
3       Uniform       Top       120 PLF       0	2	Uniform				Тор	349 PLF	34	49 PLF	0 PLF	0 PLF	0 P	_F A1		
Self Weight     7 LF       Votes     Comtech, Inc.       Substated Structured Designs is responsible only of the responsibility of the customer and/or the contractor to practing details, beam struct to be cut or diffed     6. For flat roofs provide proper drainage to prevent ponding     Manufacturer Info     Comtech, Inc.       1. Ub beams must not be cut or diffed     1. Ub beams must not be cut or diffed     6. For flat roofs provide proper drainage to prevent ponding     Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us     Comtech, Inc.       1. Ub parameter in divide tateral support at bearang pontits to avoid approvals     1. With a transmiss to pedge is laterally restrained     6. For flat roofs provide proper drainage to prevent ponding     Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us     Comtech, Inc. 1001 S. Really Road, Suite #639 Fayetterelle, NC USA 10-864-TRUS	3	Uniform				Top	120 PLF		0 PLF	0 PLF	0 PLF	0 P	F Wal	Above	
Notes     Chemicals     6. For flat roofs provide proper drainage to prevent pending     Manufacturer Info     Contech Inc.       Votes     Audited Structured Designs is responsible only of the customer and loadings shown, it is component sustability of the number on the contractor to measure the customer and/or the contractor to measure the customer and/or the contractor to measure the customer and/or the customer and/or the contractor to measure the customer and/or the contractor to measure the contractor to measure the customer and/or the contractor to measure the customer and/or the contractor to measure the contractor to measure the contractor to measure the customer and/or the customer and/or the customer and relation regarding installation regar	•	Self Weight				[-	7 PI F								
Notes     chemicals     6. For flat roofs provide proper drainage to prover drainage to prove drain		Sell Weight					7 1 21								
Notes     chemicals     6. For flat roofs provide proper drainage to prove ponding     Manufacturer Info     Comtech, Inc.       Ataculated Structured Designs is responsible only of the testing notarias and loadings shown. It is the segonsibility of the customer and/or the contraction     I. Musem must not be cut of dilled     Mature Info     Comtech, Inc.       1. Up, beams must not be cut of dilled     Provide lateral support at beams must not be used     Mature Info     Use and the customer analysis       1. Dry service conditions, unless noted otherwise     1. Dry service conditions, unless noted otherwise     I. Must as wood the customer and rotation     Service conditions to avoid lateral displacement and rotation     Comtech, Inc.       1. Dry service conditions, unless noted otherwise     Design is valid until 11/3/2024     Mature Info     Comtech, Inc.       1. Dry service conditions, unless noted otherwise     Design is valid until 11/3/2024     Mature Info     Comtech, Inc.       1. Dry service conditions, unless noted otherwise     Design assumes to poet used     Norwalk     CT 08851     Use analyse to avoid lateral displacement and rotation     This design is valid until 11/3/2024     Mature Info     Comtech, Inc.															
Notes     chemicals     6. For flat roofs provide proper drainage to prevent ponding     Manufacturer intro     United, flat.       Jackulated Structured Designs is responsible only of the customer and/of the contraded not drafted means multiple segonsibility of the intender proper drainage to prevent ponding     Metsä Wood     JUL Veams must not be cut or drilled     This design is valid until 11/3/2024     Metsä Wood     USA       1. Dry service conditions, urless noted otherwise     3. Damaged Beams must not be used     A beams degi is laterally restrained     Norwalk     CT 06851     1000 S.22-5850     Www.metsawood.com/us     1000 S.22-5850       1. Dry service conditions, urless noted otherwise     3. Damaged Beams must not be used     This design is valid until 11/3/2024										Manufact	uror Info		Comtech	Inc	
throtural adequacy of this component based on the leage original and loadings shown, it is the seponsibility of the customer and/or the contractor to mean the customer and/or the custome	Notes Calculated Structure	ed Designs is responsible only of th	chemicals he Handling 8	& Installation		6. For fi pondi	at roofs provide p ng	oroper drai	nage to prevent	Metsä Wo	net into		1001 S. R Fayettevil	eily Road, Suite a	#639
esponsibility of the customer and/or the contractor to insure the component suitability of the initialition requirements, multipely fastening details, beam strength values, and code approvals 1. Dry service conditions, unless noted otherwise 2. UVL not to be treated with fire relardant or corrorsion 2. UVL not to be treated with fire relardant or corrorsion 3. Damaged Beams must not be used 4. Design assumes to pedge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 5. Provide latera	structural adequacy design criteria a	y of this component based on th ind loadings shown. It is th	1e 1. LVL beams 1e 2. Refer to	must not be cut or manufacturer's	dri∎ed product inforr	nation				301 Merrit	7 Building, 2r	nd Floor	USA 28314	20110	
Approvals approv	responsibility of the ensure the comp	e customer and/or the contractor t conent suitability of the intende	to regarding ad fastening c	installation req details, beam stren	uirements, mi igth values, and	ulti-ply code				(800) 622-	5850		910-864-T	RUS	
1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive 4. How the treated with fire retardant or corrosive 5. Provide lateral support at bearing points to avoid lateral displacement and rotation This design is valid until 11/3/2024	Lumber	verify the dimensions and loads.	approvals 3. Damaged E	Beams must not be	used	4				www.mets	awood.com/us	i			
	<ol> <li>Dry service cond</li> <li>LVL not to be tr</li> </ol>	ditions, unless noted otherwise reated with fire retardant or corrosiv	<ul> <li>Design ass</li> <li>5. Provide lat</li> <li>/e lateral disc</li> </ul>	teral support at be acement and rotati	earing points to on	avoid	docion in tralita	44 النصريا	1212024				C	OMT	есн
string 21 90 417 Deward by iStruct M Dataset: 220610011		7.0 11 '0' 17' 0 1				inis	uesign is valid	runtui 11.	1312024						

1	isDesign	Client: Project: Address:	Precision Custom H Liberty 2.0 Brewster Court Cameron, NC 28	omes 396	Date: Input by: Job Nam Project #	10/10/2022 David Landry ne: Lot 37 Liberty Meadows	Page 8 of 14
BM4	Kerto-S L\	/L 1.750	" X 9.250"	2-Ply	- PASSED	Level: Level	
•	•	•	•	•	•	• =.	<u> </u>
						<1 1/2	9 1/4
	• PF End Grain	•	•	•	• 2 SPF End		
+			6'				3 1/2"
			6'7"			1	
Multi-Ply Fasten all Capacity Load Yield Limit py Yield Mode Edge Distand Min. End Diss Load Combin Duration Fac	y Analysis   plies using 2 rows er Foot er Fastener ce stance nation ctor	of 10d Box nails 0.0 % 0.0 PLF 163.7 PLF 81.9 lb. IV 1 1/2" 3" 1.00	(.128x3") at 12" (	o.c Maximu	ım end distance n	not to exceed 6".	
Notes	stured Designs is responsible only of t	chemicals he Handling & Installa	tion	<ol> <li>For flat roofs provi ponding</li> </ol>	de proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequ design criteria responsibility of ensure the co	Jacy of this component based on t and loadings shown. It is t the customer and/or the contractor mponent suitability of the intend to verify the dimensions and load	he 1. LVL beams must not be the 2. Refer to manufactu to regarding installation ed fastening details, beam	cut or dri∎ed irer's product information requirements, multi-ply n strength values, and code			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	UŚA 28314 910-964-TRUS
1. Dry service of 2. LVL not to be	conditions, unless noted otherwise e treated with fire retardant or corrosi	approvals 3. Damaged Beams must 4. Design assumes top ed 5. Provide lateral support lateral displacement and	not be used ge is laterally restrained : at bearing points to avoid d rotation	This design is v	alid until 11/3/2024	www.metsawood.com/us	соттесн
Version 21.80	417 Powered by iStruct™ Da	taset: 22061001 1					CCD I III

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.

	/	Client:	Precision Cus	tom Homes		1	Date:	10/10/20	22			Page 9 of 14
1	isDesign	Project: Address	Liberty 2.0	wirt			nput by: Iob Name	David La	ndry herty Meadows			
+	1315 631311	Address.	Cameron, N	IC 28396			Project #:	J1022-50	)97			
BM5	Kerto-S LVI	1.750	" X 11.8	75"	2-Plv	- PAS	SED	_evel: Level				
		- 11/00	× 1110		,							
									TTT			
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1217	- TTEN			at .			-	Win			IĂIĂ	11 7/8"
•	•		•	•		THE R	•	•			V V V	
	PF End Grain						2 SPF I	End Grain				
/			8'								<i>\</i>	, 3 1/2"
/			8'8 11/16"						┥			
Member	nformation					Reactio	ns UNF	PATTERN	IED lb (Upli	ft)		
Туре:	Girder	Applic	ation: Fl	oor		Brg Di	rection	Live	Dead	Snow	Wind	Const
Plies: Moisture C	2 ondition: Drv	Desigi Buildir	n Method: Al na Code: IP	SD IC 2012		1 Ve	rtical rtical	1772	2203	1044	0	0
Deflection I	LL: 480	Load	Sharing: N	0		2 10	lica	1755	2115	1055	0	0
Deflection	TL: 240	Deck:	N	ot Checked								
Temperatur	e: Normal – II re: Temp <= 100°F											
lomporata						Bearing	js					
						Bearing	Length	n Dir.	Cap. React [	D/L lb Tot	al Ld. Case	Ld. Comb.
						1 - SPF End	4.625"	Vert	32% 2203 /	2112 43	14 L	D+0.75(L+S)
Analysis I	Results					Grain						
Analysis	Actual Loo	cation Allowed	Capacity	Comb.	Case	2 - SPF End	4.063"	Vert	36% 2179/	2089 42	68 L	D+0.75(L+S)
Moment	7478 ft-lb 4	'4 5/8" 19911 ft-lb '4 5/8" 11006 ft lb	0.376 (38%)	) D+L	L	Grain						
Shear	2737 lb 1	'4 1/2" 8867 lb	0.309 (31%	) D+L	L							
LL Defl ind	ch 0.059 (L/1643) 4'4	11/16" 0.203 (L/4	30) 0.292 (29%	) 0.75(L+S)	L							
TL Defl ind	ch 0.121 (L/804) 4'4	11/16" 0.406 (L/2	40) 0.298 (30%)	) D+0.75(L+	S) L	]						
Design N	otes					4						
1 Provide may also	support to prevent lateral n be required at the interior	hovement and rotat bearings by the bu	ion at the end be ilding code.	earings. Late	ral support							
2 Fasten a to excee	all plies using 2 rows of 10d ed 6".	Box nails (.128x3"	) at 12" o.c. Max	imum end di	istance not							
3 Refer to	last page of calculations fo	r fasteners require	l for specified lo	ads.								
5 Top load	are designed to be support Is must be supported equal	ed on the bottom e ly by all plies.	age only.									
6 Top mus 7 Bottom r	it be laterally braced at end	bearings. end bearings										
8 Lateral s	slenderness ratio based on	single ply width.										
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live	1 Sno	w 1.15	Wind 1.6 Co	nst. 1.25	Comments	
1	Part. Uniform	0-0-0 to 8-8-11		Тор	135 PLF	404 P	LF	0 PLF	0 PLF		-3	
2	Uniform			Тор	120 PLF 238 PLF		LF 1 F 2	0 PLF 38 PLF			Vall Above	
5	Self Weight			lob	9 PLF	01			01 El	UT EI		
	con troigin											
Notes		chemicals		6. For fla	at roofs provide p	roper drainage	to prevent	Manufacture	er Info	Comt	ech, Inc.	****
Calculated Structu structural adequa	ured Designs is responsible only of the acy of this component based on the	Handling & Installa	tion cut or dri∎ed	pondin	ig P			Metsä Wood 301 Merritt 7	Buildina, 2nd Flo	or USA	teville, NC	
design criteria responsibility of t ensure the con	and loadings shown. It is the the customer and/or the contractor to apponent suitability of the intended	2. Refer to manufact regarding installation	rer's product inform	lation Iti-ply code				Norwalk, CT (800) 622-58	06851 50	2831- 910-8	i 64-TRUS	
application, and to Lumber	verify the dimensions and loads.	approvals 3. Damaged Beams must	not be used	COUR				www.metsaw	vood.com/us			
1. Dry service co 2. LVL not to be	nditions, unless noted otherwise treated with fire retardant or corrosive	<ol> <li>Design assumes top ed 5. Provide lateral support lateral displacement ar</li> </ol>	ge is laterally restrained t at bearing points to d rotation	avoid Thie	design is valid	until 11/3/20	24			ī	сотт	есн
Version 21 80 4	117 Powered by iStruct™ Datas	et [.] 22061001.1		1115	- seign is valu		- 1				110.00	

	./		Client:	Precision Custom	Homes		Date:	10/10/2022	Page 10 of 14
	LieDesign		Project:	Liberty 2.0			Input by:	David Landry	
	ispesign		Address:	Brewster Court			Job Nam	e: Lot 37 Liberty Meadows	
	_			Cameron, NC 2	8396		Project #	J1022-5097	
BM5	5 Kerto-S	LVL	1.750"	X 11.875	5" 2-F	'lv - P	ASSED	Level: Level	
						-,			
•	•	•	•	•	•	•	•	•	
								2"	MM
	•	•	•	•	•	•	•	• <u> </u>	
lı í	1 SPF End Grain						2 SPF	End Grain	
	,			8'				/ _	3 1/2"
				5					1 10 112
1				8'8 11/16"				1	
N.A									
INIUITI-I	Piy Analysis								
Fasten	a <b>ll</b> plies using 2 ro	ows of 10d	Box nails (.	.128x3") at 12'	' o.c Maxii	num end	d distance n	ot to exceed 6".	
Capacity		0.0 %							
Load		0.0 PLF	_						
Yield Limi	t per Foot	163.7 PL	.F						
Yield Mod		61.910. IV							
Edge Dist	ance	1 1/2"							
Min. End	Distance	3"							
Load Corr	nbination								
Duration F	Factor	1.00							
Notes		cher	micals		6. For flat roofs	provide proper o	drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Rei∎v Road. Suite #639
Calculated S	Structured Designs is responsible of	only of the Hand	ling & Installatio	on	ponding			Metsä Wood	Fayetteville, NC USA
design crite	eria and loadings shown. It	is the 2 Refe	beams must not be cu or to manufacture	it or dri∎ed r's product information				301 Memtt / Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
ensure the	component suitability of the	intended faste	iraing installation ening details, beam s	requirements, multi-ply strength values, and code				(800) 622-5850	
Lumber	and to youry the dimensions and 0	appr 3. Dan	rovals naged Beams must no	t be used				www.meisawood.com/us	
1. Dry servi 2. LVL not 1	ice conditions, unless noted otherw to be treated with fire retardant or	/ise 5. Prov corrosive	ign assumes top edge vide lateral support a	is laterally restrained it bearing points to avoid					COMTECH
		later	a displacement and re	otatión	This design	is valid until	11/3/2024		
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1	isDesign		Client: Project: Address:	Precision Custom Liberty 2.0 Brewster Court Cameron NC 2	Homes	Date Inpu Job	e: it by: Name: iect #:	10/10/2022 David Landry : Lot 37 Liberty Meadows .11022-5097	Page 12 of 1
GDH	Kerto-S	LVL	1.750"	X 11.875"	2-Ply	- PASSED	L	Level: Level	
	• •	•	• •	• •	• •	• •	•	• • •	<u>1</u> <u>1</u> <u>1</u> <u>1</u>
	• •	•	• •	• •		• •	•		
					16'10"				3 1/2"
╆───					16'10"				
Multi-Plv	Analysis								
Fasten all	plies using 2	rows of	10d Box nai <b>l</b> s	(.128x3") at 12"	o.c Maxim	um end distan	ce no	ot to exceed 6".	
Capacity Load		29. 56.0	7 % ) PLF						
Yield Limit pe Yield Limit pe	er Foot er Fastener	188 94.1	.3 PLF 1 lb.						
Yield Mode Edge Distanc	æ	IV 1 1/	2"						
Min. End Dist	tance	3"	_						
Load Combin Duration Fact	tor	D+9 1.19	5						
Notes	und Decima is server "	o only of the	chemicals Handling & Installa	tion	6. For flat roofs pro ponding	wide proper drainage to pro	event	Manufacturer Info	Comtech, Inc. 1001 S. Relly Road, Suite #639 Favetteville. NC
design criteria responsibility of t	acy of this component ba and loadings shown the customer and/or the o mponent suitability of th	ased on the It is the contractor to ne intended	1. LVL beams must not be 2. Refer to manufacture regarding installation fastening details have	ecutor drilled urer's product information requirements, multi-ply n strength values and orde				301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	USA 28314 910-864-TRUS
application, and to Lumber 1. Dry service co 2. IVI. net to be	o verify the dimensions and	l loads.	approvals 3. Damaged Beams must 4. Design assumes top ec 5. Provide lateral suppor	not be used lige is laterally restrained t at bearing points to avoid				www.metsawood.com/us	Comtech
			ateral displacement an	a rotation	This design is	valid until 11/3/2024			

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lis	Design	Client: Project: Address:	Precision Custom Ho Liberty 2.0 Brewster Court Cameron, NC 283	mes 96	Date: Input by: Job Name Project #:	10/10/2022 David Landry e: Lot 37 Liberty Meadows J1022-5097	Page 14 of 14
GDH2	S-P-F #	2 2.000"	X 12.000"	2-Ply -	PASSED	Level: Level	
•	•	• •	•	•	• •	• •	
	•	• •	•	•	• •		¥ ₩ ↓
			8'10"				, , , , , , , , , , , , , , , , , , ,
⊁			8'10"			ł	
Multi-Ply A	nalvsis						
Fasten all pl	ies using 2 rov	ws of 10d Box nails	s (.128x3") at 12" o.	.c Maximum	end distance n	ot to exceed 6".	
Capacity Load		0.0 % 0.0 PLF					
Yield Limit per F Yield Limit per F	oot astener	157.4 PLF 78.7 lb.					
Yield Mode Edge Distance		IV 1 1/2"					
Min. End Distan	ce	3"					
Duration Factor		1.00					
					[	Manufacturer Info	Comtech, Inc. 1001 S. Re∎y Road, Suite #639
							Fayetteville, NC USA 28314 910-864-TRUS
				This design is va <b>l</b> io	l until 11/3/2024		соттесн



### RE: J1022-5097

City:

Lot 37 Liberty Meadows

**Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information: Customer: Project Name: J1022-5097 Lot/Block: Address:

Model: Subdivision: State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 10 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	150256534	ET1	2/16/2022
2	150256535	ET2	2/16/2022
3	150256536	ET3	2/16/2022
4	150256537	F1	2/16/2022
5	150256538	F2	2/16/2022
6	150256539	F3	2/16/2022
7	150256540	F4	2/16/2022
8	150256541	F4A	2/16/2022
9	150256542	F5	2/16/2022
10	150256543	FG1	2/16/2022

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844 IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO





February 16, 2022



LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. ir Vert(LL) n/z Vert(CT) n/z Horz(CT) -0.00	n (loc) l/defi L/d n - n/a 999 n - n/a 999 38 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 147 lb         FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	⊃ No.1(flat) ⊃ No.1(flat) ⊃ No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	irectly applied or 6-0-0 oc purlins, or 6-0-0 oc bracing.

REACTIONS. All bearings 32-11-0.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 56, 29, 55, 54, 53, 52, 51, 50, 49, 48, 46, 45, 44, 43, 42, 41, 40, 38, 37, 36, 35, 34, 33, 32, 31, 30

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

OTHERS

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



32-11-0





I	1-4-0	1-4-0 1-4-0	1-4-0	1-4-0 1-	4-0 ' 1	-4-0 1-4-0	1-4	4-0 '	1-4-0	1-4-0	1-4-0 1-4	4-0 ' 1-0-0 '
Plate Off	sets (X,Y)	[7:0-1-8,Edge], [24:0-1-8	,Edge]									
LOADIN TCLL TCDL BCLL	<b>G</b> (psf) 40.0 10.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.00 NO	CSI. TC BC WB	0.07 0.01 0.03	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 17	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	5.0	Code IRC2015/T	PI2014	Matrix	<-S						Weight: 84 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat)				BRACING- TOP CHOF	RD	Structu	ral wood	sheathing di	rectly applied or 6-0-0	) oc purlins,		

BOT CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)OTHERS2x4 SP No.3(flat)

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc pu except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 18-4-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 32, 17, 31, 30, 29, 28, 27, 25, 24, 23, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.







Plate Offsets (X,Y)	[9:0-1-8,Edge], [24:0-1-8,Edge]						
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code. IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n (loc) l/defl a - n/a a - n/a D 18 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 89 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S	P No.1(flat) P No.1(flat)		BRACING- TOP CHORD	Structural woo	od sheathing di	irectly applied or 6-0-0	) oc purlins,

2x4 SP No.3(flat) WFBS OTHERS 2x4 SP No.3(flat) BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 19-4-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 34, 18, 33, 32, 31, 30, 29, 27, 26, 25, 24, 23, 22, 21, 20, 19

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.









L	14-1-4		32-11-0					
	14-1-4		18-9-12					
Plate Offsets (X,Y)	[6:0-3-0,0-0-0], [17:0-1-8,Edge], [27:0	1-8,Edge], [36:0-1-8,Edge]						
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.63 BC 0.82 WB 0.63 Matrix-S	DEFL. Vert(LL) -0 Vert(CT) -0 Horz(CT) 0	in (loc) .20 27 .27 26-27 .04 24	l/defl >999 >820 n/a	L/d 480 360 n/a	<b>PLATES</b> MT20 Weight: 180 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER-       BRACING-         TOP CHORD       2x4 SP No.1(flat)       TOP CHORD       Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.         BOT CHORD       2x4 SP No.3(flat)       BOT CHORD       BOT CHORD       Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.         WEBS       2x4 SP No.3(flat)       BOT CHORD       Rigid ceiling directly applied or 6-0-0 oc bracing.								oc purlins,
REACTIONS.       (size)       40=0-3-8, 24=0-3-8, 33=0-3-8         Max Grav       40=662(LC 3), 24=882(LC 4), 33=2190(LC 1)         FORCES.       (lb)       Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP 01400000       4.4.40000000 4.5.40000000 4.5.40000000 4.5.40000000000								
	-10=-392/1404, 10-11=0/2571, 11-12=0/2 4-15=-1631/0, 15-17=-2564/0, 17-18=-29 20-21=-2592/0, 21-22=-1586/0	571, 12-13=-40/475, 13-14=-16 31/0, 18-19=-2931/0, 19-20=-2	531/0, 592/0,					
BOT CHORD	39-40=0/704, 38-39=-106/1489, 37-38=-62 33-35=-1709/0, 32-33=-1291/0, 31-32=-20 27-28=0/2931, 26-27=0/2856, 25-26=0/21	1/1602, 36-37=-621/1602, 35-3 )/935, 29-31=0/2220, 28-29=0/ )3, 24-25=0/952	36=-1086/1024, 2931,					
WEBS 2	2-40=-935/0, 2-39=-59/564, 3-39=-528/98, 22-25=0/881, 21-25=-845/0, 21-26=0/542, 12-33=-1704/0, 12-32=0/1314, 10-33=-139 6-36=-690/0, 13-32=-1288/0, 13-31=0/991	4-38=-325/0, 5-38=0/648, 22-2 19-26=-358/0, 19-27=-253/331 9/0, 10-35=0/993, 7-35=-1033/ , 15-31=-835/0, 15-29=0/579, 1	4=-1266/0, , 0, 7-36=0/1214, 17-29=-718/0					
NOTES-								

1) Unbalanced floor live loads have been considered for this design.

- 2) All plates are 3x4 MT20 unless otherwise indicated.
- Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.







	15-5-12	31-11-0						
	15-5-12	1	16-5-4					
Plate Offsets (X,Y)	[15:0-1-8,Edge], [25:0-1-8,Edge], [33:0-	1-8,Edge], [34:0-1-8,Edge]						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.85 BC 0.91 WB 0.54	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.18 24-25 -0.25 24-25 0.04 22	l/defl >999 >776 n/a	L/d 480 360 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S					Weight: 167 lb	FT = 20%F, 11%E
LUMBER-       BRACING-         TOP CHORD       2x4 SP No.1(flat)       TOP CHORD       Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.         BOT CHORD       2x4 SP No.1(flat)       BOT CHORD       Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.         WEBS       2x4 SP No.3(flat)       BOT CHORD       Rigid ceiling directly applied or 6-0-0 oc bracing.         REACTIONS.       (size) 36=0-3-0, 30=0-3-8, 22=0-3-0 Max Grav 36=719(LC 3), 30=2079(LC 1), 22=795(LC 4)       BOT CHORD								
FORCES. (lb) - M TOP CHORD 2 8 1 1	Iax. Comp./Max. Ten All forces 250 (lb) or -3=-1229/0, 3-4=-1941/0, 4-5=-1941/0, 5-6= -9=-241/659, 9-10=0/2218, 10-12=0/2218, 4-15=-1712/32, 15-16=-2291/0, 16-17=-22 9-20=-1395/0	eless except when shown. =-1941/0, 6-7=-1408/202, 7-8= 12-13=-523/817, 13-14=-1712 291/0, 17-18=-2225/0, 18-19=	=-1408/202, 2/332, -2225/0,					
BOT CHORD 3 3	5-36=0/769, 34-35=0/1668, 33-34=0/1941, 0-31=-1214/0, 28-30=-1129/0, 27-28=-547/ 4-25=0/2385, 23-24=0/1918, 22-23=0/852	32-33=-35/1748, 31-32=-414 1232, 26-27=0/2291, 25-26=0	/927, )/2291,					
WEBS 2 2 1 1	2-361022/0, 2-35-0/640, 3-35610/0, 3-34=-83/372, 9-30=-1439/0, 9-31=0/1062, 20-22=-1132/0, 20-23=0/755, 19-23=-728/0, 19-24=-1/417, 17-25=-477/33, 12-30=-1508/0, 8-31=-1027/0, 8-32=0/734, 6-32=-547/0, 6-33=0/628, 5-33=-320/0, 12-28=0/1134, 13-28=-1085/0, 13-27=0/740, 15-27=-1088/0, 15-26=0/273							
NOTES-								

Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.







3x4 =

3x6 =

4x6 =

3x6 =

19-11-0										
Plate Offsets (X,Y) [6:0-1-8,Edge], [7:0-1-8,Edge]										
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.95 WB 0.55 Matrix-S	DEFL. ir Vert(LL) -0.33 Vert(CT) -0.45 Horz(CT) 0.08	n (loc) I/defi L/d 3 18-19 >724 480 5 18-19 >526 360 3 14 n/a n/a	<b>PLATES</b> MT20 M18AHS Weight: 106 lb	<b>GRIP</b> 244/190 186/179 FT = 20%F, 11%E				
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	? No.1(flat) ? No.1(flat) ? No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	ectly applied or 5-9-6 c r 2-2-0 oc bracing.	oc purlins,				
REACTIONS. (size Max G	e) 24=0-3-0, 14=0-3-0 rav 24=1075(LC 1), 14=1075(LC 1)									

 
 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-2005/0, 3-4=-3408/0, 4-5=-3408/0, 5-6=-4160/0, 6-7=-4391/0, 7-9=-4160/0, 9-10=-3408/0, 10-11=-3408/0, 11-12=-2005/0

 BOT CHORD
 23-24=0/1172, 22-23=0/2810, 20-22=0/3918, 19-20=0/4391, 18-19=0/4391, 17-18=0/4391, 16-17=0/3918, 15-16=0/2810, 14-15=0/1172

WEBS 2-24=-1557/0, 2-23=0/1160, 3-23=-1119/0, 3-22=0/814, 5-22=-693/0, 5-20=0/469, 6-20=-575/87, 12-14=-1557/0, 12-15=0/1160, 11-15=-1119/0, 11-16=0/814, 9-16=-693/0, 9-17=0/469, 7-17=-575/87

#### NOTES-

3x6 =

4x6 =

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 1.5x3 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

3x6 = 3x10 M18AHS FP =

3x4 =







L						19-8-0						
	19-8-0											
Plate Offse	ts (X,Y)	[7:0-3-0,Edge], [21:0-1-8,	Edge]									
LOADING TCLL TCDL BCLL BCDL	(psf) 40.0 10.0 0.0 5.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.00 1.00 YES 212014	<b>CSI.</b> TC BC WB Matrix	0.39 0.88 0.54 -S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.29 -0.40 0.08	(loc) 20 20 16	l/defl >806 >587 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 112 lb	<b>GRIP</b> 244/190 186/179 FT = 20%F, 11%E
LUMBER- TOP CHOF BOT CHOF WEBS	LUMBER- TOP CHORD     2x4 SP No.1(flat)     BRACING- TOP CHORD       BOT CHORD     2x4 SP No.1(flat)     TOP CHORD     Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.       WEBS     2x4 SP No.3(flat)     BOT CHORD     Rigid ceiling directly applied or 10-0-0 oc bracing.											
REACTION	NS. (size Max G	e) 25=0-3-0, 16=Mecha rav 25=1062(LC 1), 16=1	nical 068(LC 1)									
FORCES.	(lb) - Max.	Comp./Max. Ten All for	ces 250 (lb) or	less except v	when shown.							
TOP CHOP	RD 2-3=- 10-12	1976/0, 3-4=-3346/0, 4-6 2=-3354/0, 12-13=-3354/0	=-3350/0, 6-7= , 13-14=-1976	-4437/0, 7-8= /0	=-4437/0, 8-1	0=-4140/0,						
BOT CHOP	RD 24-25 17-1	5=0/1156, 22-24=0/2765, 8=0/2764, 16-17=0/1157	21-22=0/3951	, 20-21=0/443	37, 19-20=0/4	4437, 18-19=0/38	323,					
WEBS	2-25= 7-21= 10-1	2-25=-1537/0, 2-24=0/1140, 3-24=-1097/0, 3-22=0/790, 6-22=-803/0, 6-21=0/970, 7-21=-557/0, 14-16=-1541/0, 14-17=0/1139, 13-17=-1096/0, 13-18=0/802, 10-18=-637/0, 10-19=0/581, 8-19=-621/0										
NOTES-												

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x4 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Refer to girder(s) for truss to truss connections.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.







		13-11-4		1-6-0	1-2-6 1-2-6 1-6-0 0-4-0
Plate Offsets (X,Y	) [13:0-1-8,Edge], [17:0-1-8,Edge]				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.40 BC 0.38 WB 0.37 Matrix-S	DEFL.         in           Vert(LL)         -0.08           Vert(CT)         -0.11           Horz(CT)         0.03	(loc) l/defl L/d 25 >999 480 25 >999 360 19 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 115 lb         FT = 20%F, 11%E
LUMBER- TOP CHORD 25 BOT CHORD 25 WEBS 25 REACTIONS.	4 SP No.1(flat) 4 SP No.1(flat) 4 SP No.3(flat) (size) 30=0-3-0, 15=0-3-8, 19=0-3-8 ax Grav 30=731(LC 8), 15=272(LC 4), 19=	1155(LC 7)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing c except end verticals. Rigid ceiling directly applied 6-0-0 oc bracing: 18-19,17-	tirectly applied or 6-0-0 oc purlins, I or 10-0-0 oc bracing, Except: 18.

15-5-4

. 16-7-10 . 17-10-0

19-4-0 19-8-0

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1332/0, 3-4=-1332/0, 4-5=-1971/0, 5-6=-1971/0, 6-7=-1964/0, 7-8=-1964/0, 8-9=-1310/0, 9-11=-1310/0, 11-12=0/359, 12-13=0/359

 BOT CHORD
 29-30=0/776, 28-29=0/76, 27-28=0/1734, 26-27=0/1734, 25-26=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 23-25=0/2050, 25-25=0/2050, 25-25=0/2050, 25

 22-23=0/1721, 21-22=0/1721, 20-21=0/755, 19-20=0/755

 WEBS
 15-17=-104/317, 2-30=-1022/0, 2-28=0/751, 4-28=-542/0, 4-26=0/320, 8-23=0/344, 8-21=-568/0, 11-21=0/769, 11-19=-1228/0, 13-19=-573/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x6 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

13-11-4

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

6) CAUTION, Do not erect truss backwards.







L		5-6-8			5-10-	8
		5-6-8			0-4-0	)
Plate Offsets (X,Y)	[2:0-1-8,Edge], [6:0-1-8,Edge], [9:0-1-8,	D-1-8]				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.11 WB 0.18 Matrix-S	DEFL.         in           Vert(LL)         -0.01           Vert(CT)         -0.01           Horz(CT)         -0.01	(loc) l/defi L/d 6 >999 480 6 >999 360 4 n/a n/a	PLATES MT20 Weight: 32 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	2 No.1(flat) 2 No.1(flat) 2 No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	rectly applied or 5-10- or 10-0-0 oc bracing.	8 oc purlins,
REACTIONS. (size Max G	e) 8=0-3-8, 4=0-3-8 rav 8=288(LC 1), 4=294(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-294/0, 3-4=-296/0

BOT CHORD 7-8=0/294, 6-7=0/294 WEBS 2-8=-382/0, 4-6=0/383

NOTES-

1) Unbalanced floor live loads have been considered for this design.
2) Plates checked for a plus or minus 1 degree rotation about its center.
3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

4) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

5) CAUTION, Do not erect truss backwards.







REACTIONS. (size) 8=0-3-8, 5=0-3-8 Max Grav 8=455(LC 1), 5=476(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-683/0

 BOT CHORD
 7-8=0/683, 6-7=0/683, 5-6=0/683

 WEBS
 2-8=-802/0, 3-5=-801/0

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

#### LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb) Vert: 3=-172 11=-194





