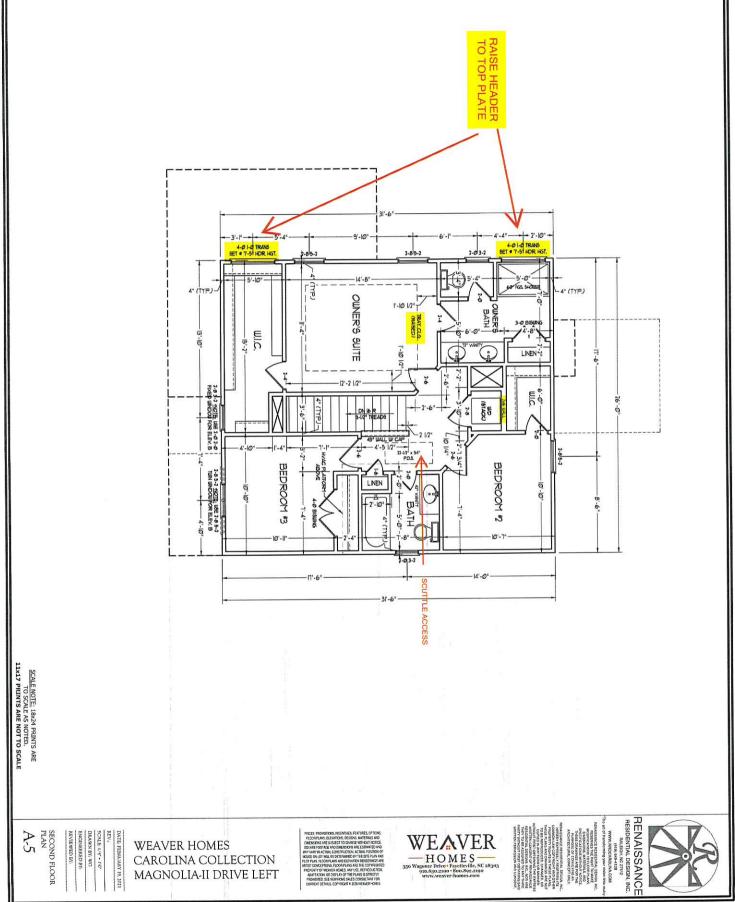


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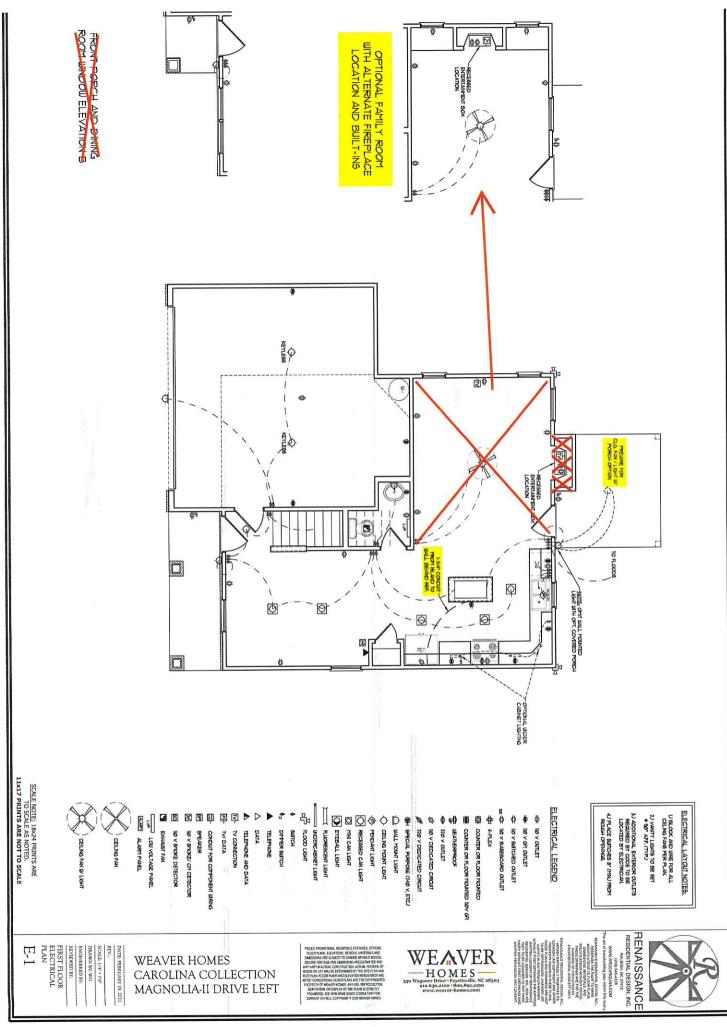
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SQUARE FOOTAGE (IFS.)

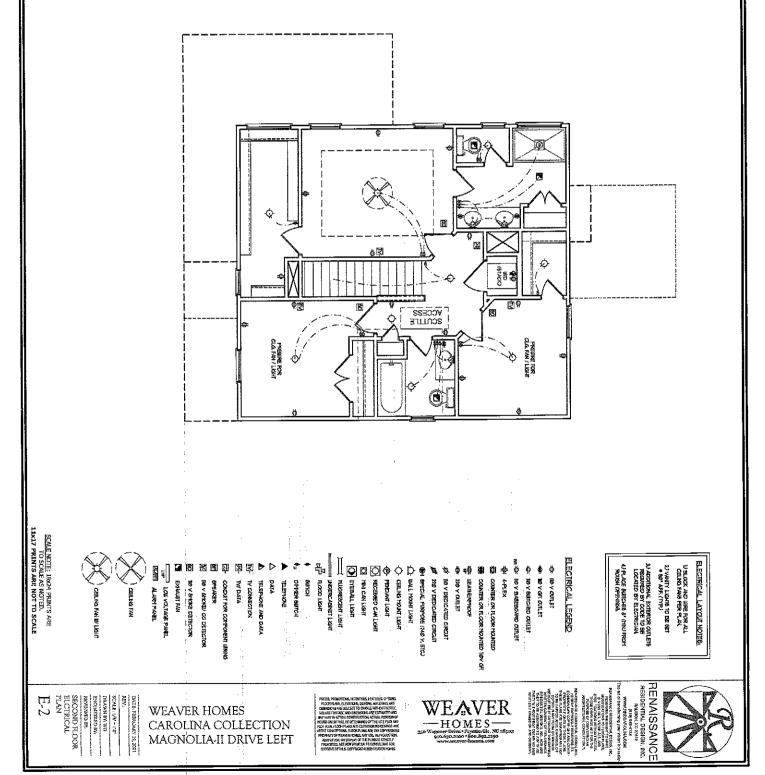




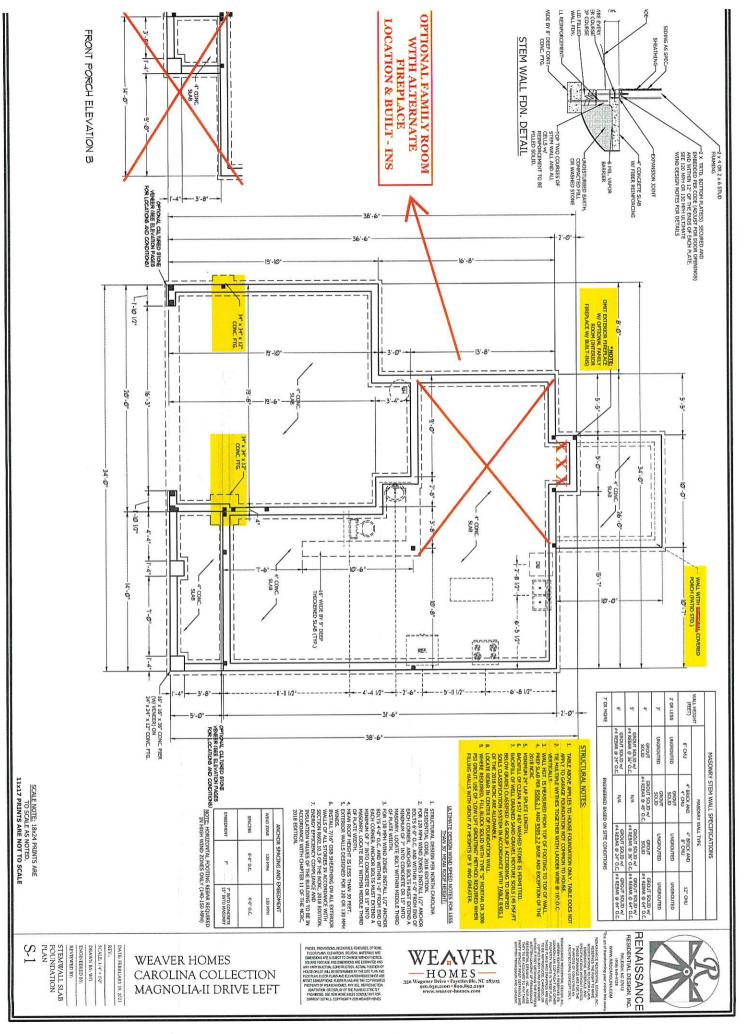




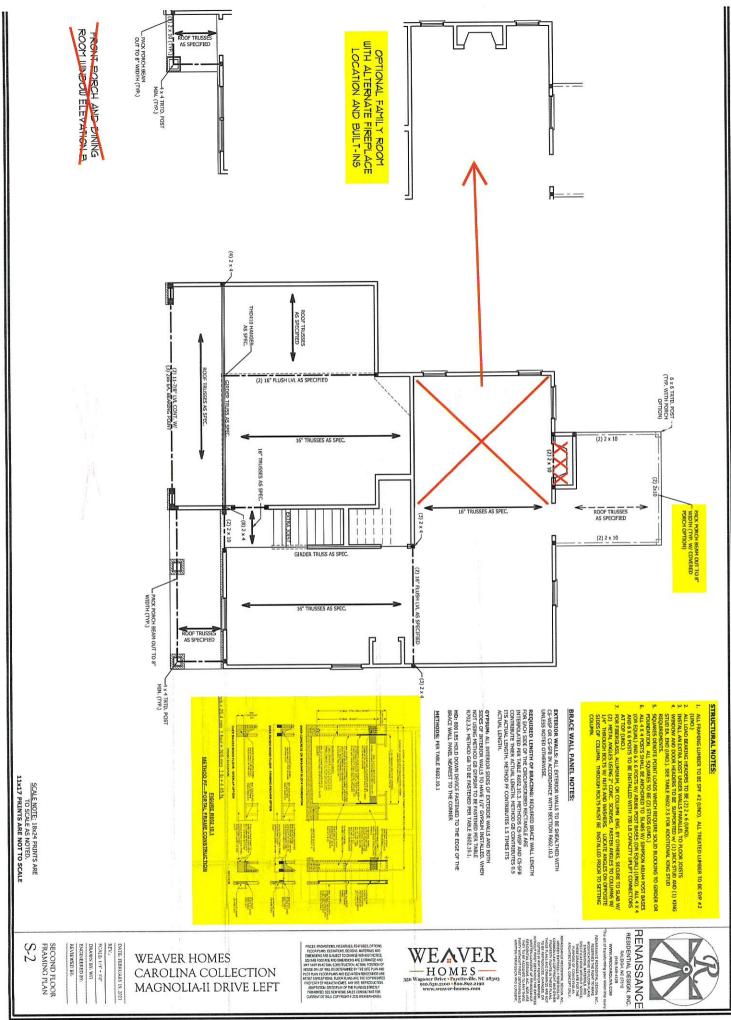




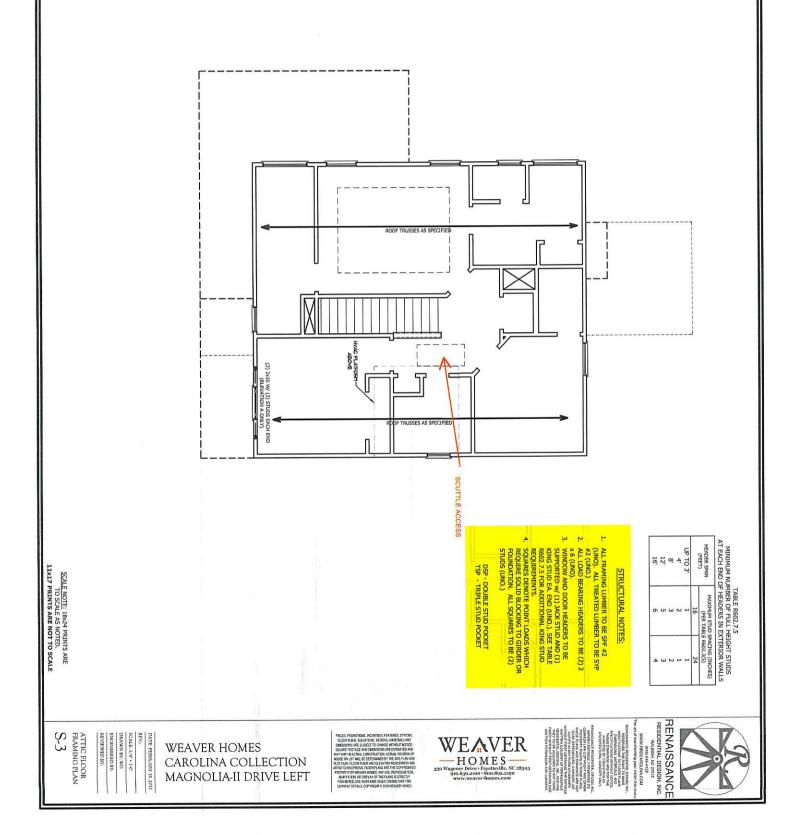




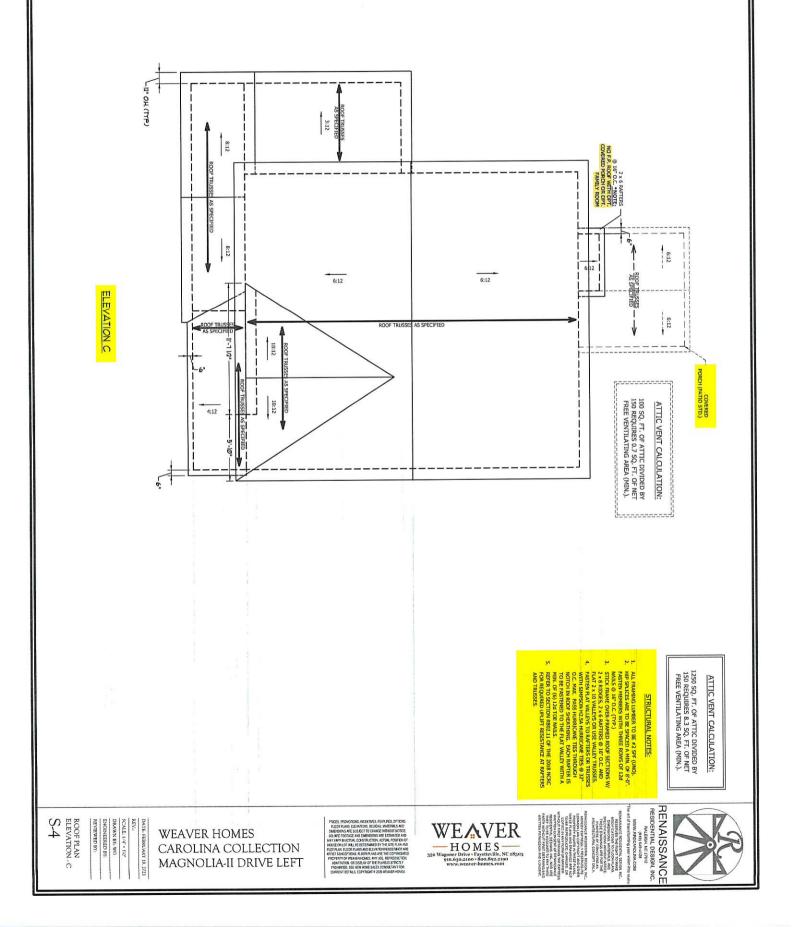


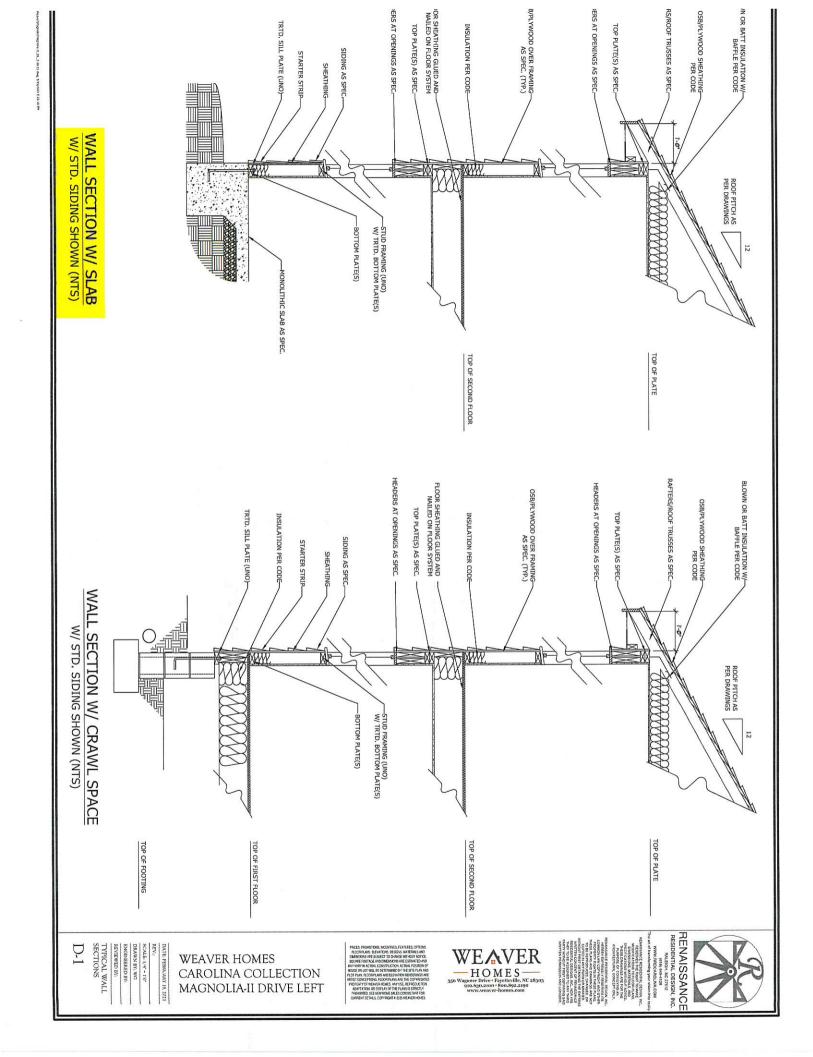


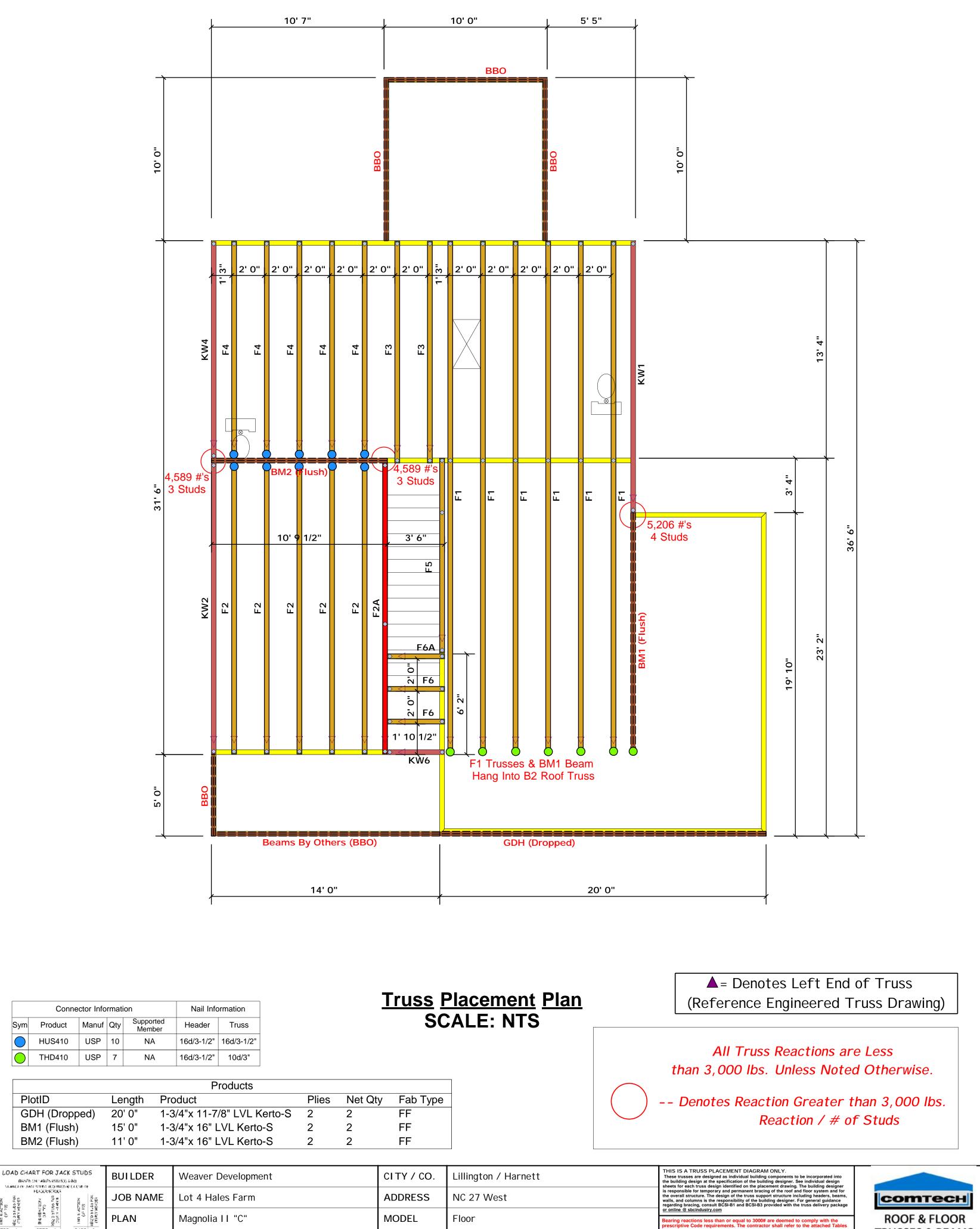












Sym

ABC, DISTUDS FOR (7) NIV HEADER

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7650 3

10200 4

12750 5

15300 6

3400

6600 Z

10200 3

13600 4

17000 5

Magnolia II "C"

Seal Date

Quote #

J0822-4191

MODEL

DATE REV.

DRAWN BY

SALES REP.

Floor

11

Christine Shivy

Lenny Norris

PLAN

SEAL DATE

QUOTE #

JOB #

END REAC

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ROOF & FLOOR TRUSSES & BEAMS

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Christine Shivy

Christine Shivy

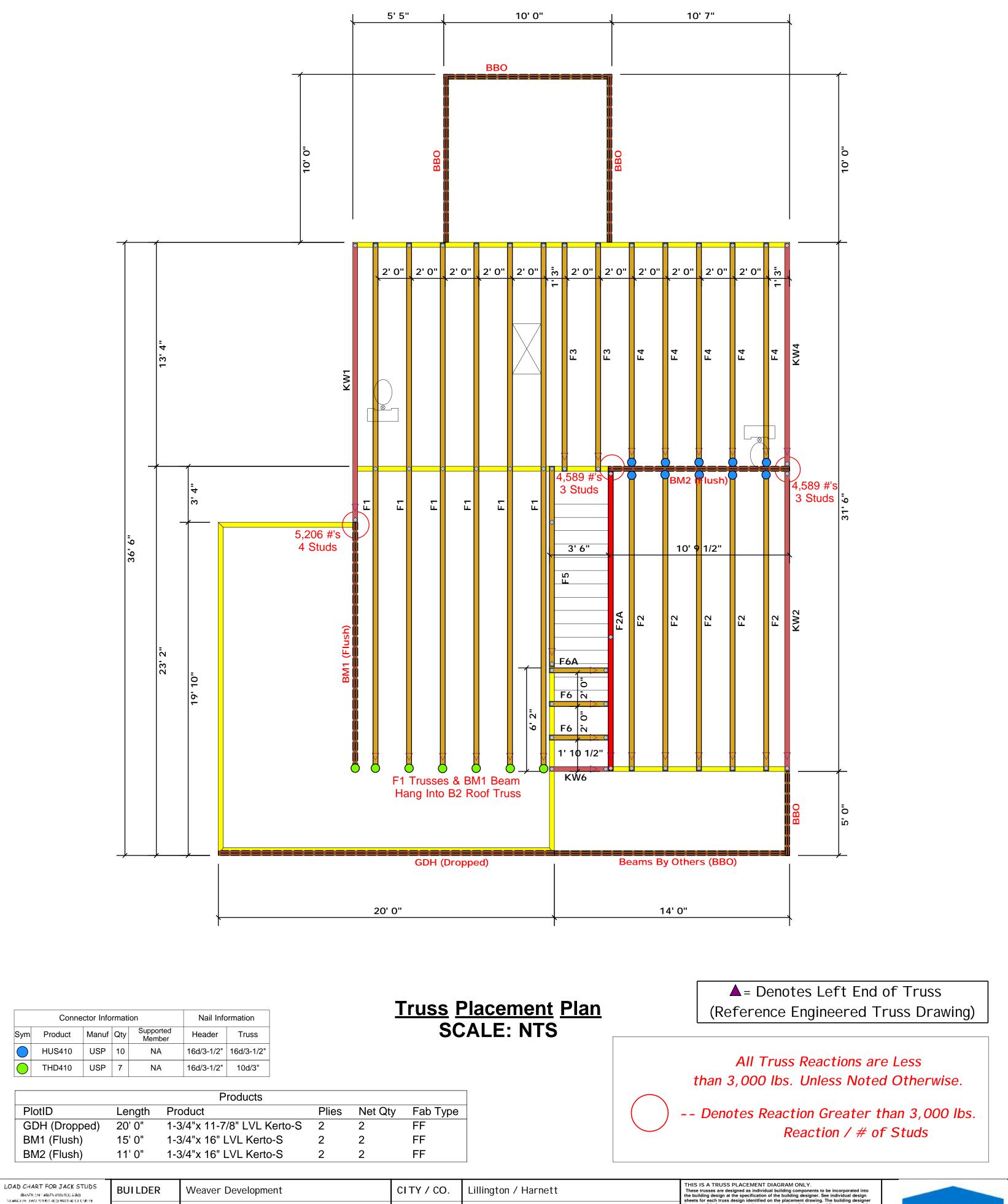
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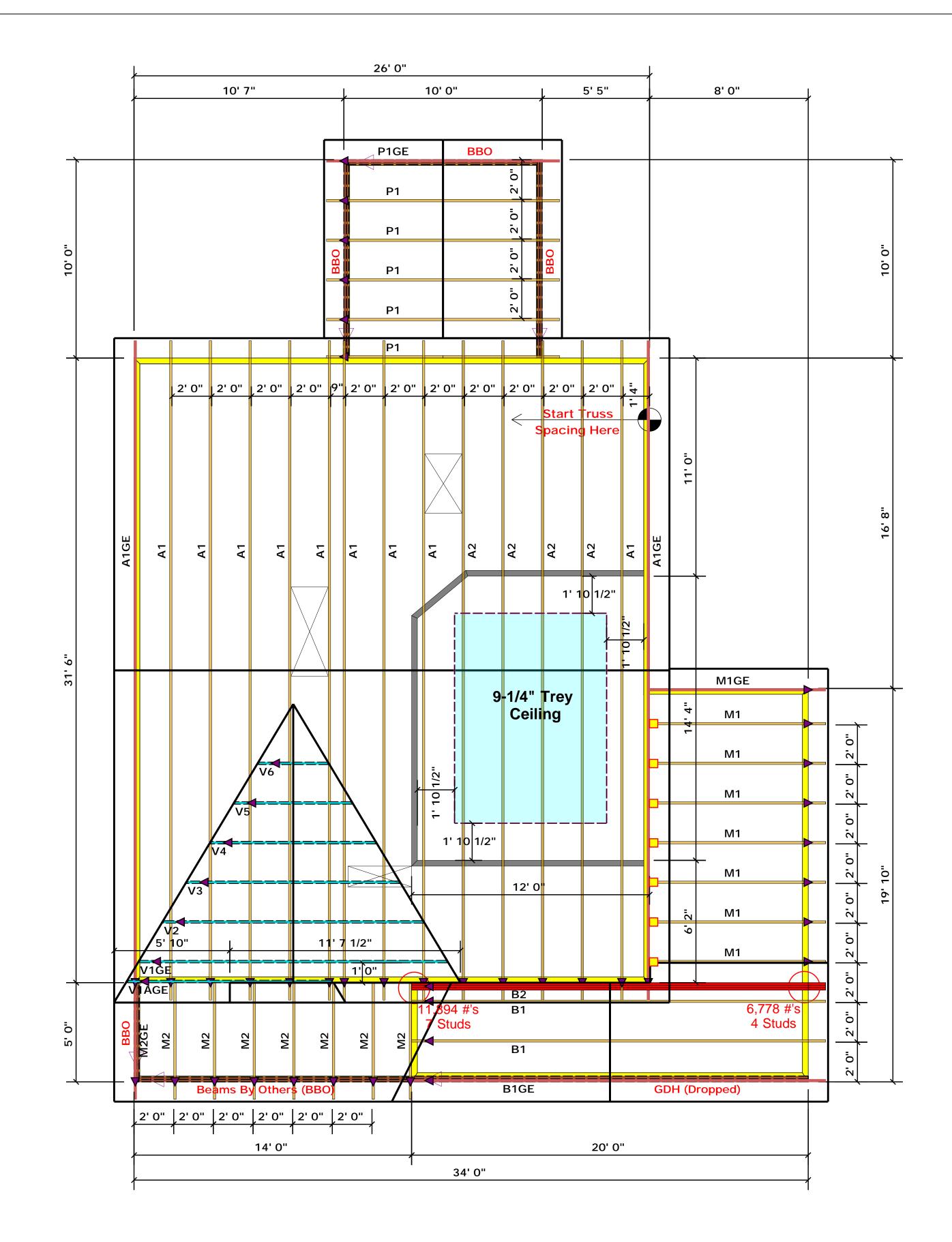
nal shall be

ins that exceed

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444



LOAD CHART FOR JAC (045Fb ON 140 F5 8502 5(1) MURICE OF JACK STUTE BCD/05(16)	4.000	BUILDER	Weaver Development	CITY/CO.	Lillington / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	JOB NAME	Lot 4 Hales Farm	ADDRESS	NC 27 West	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
<u>z %6 Z %6</u>	UN STAC	PLAN	Magnolia I I "C"	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 2550 1 3400 2 5100 2 5100 3 7650 3	3400 1 6600 2 10200 3	SEAL DATE	Seal Date	DATE REV.	/ /	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park
6800 4 10200 4 8500 5 12750 5 10200 6 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Christine Shivy	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Christine Shivy	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9	<u> </u>	JOB #	J0822-4191	SALES REP.	Lenny Norris	Christine Shivy	Fax: (910) 864-4444



	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	JUS24	USP	7	NA	10d/3"	10d/3"

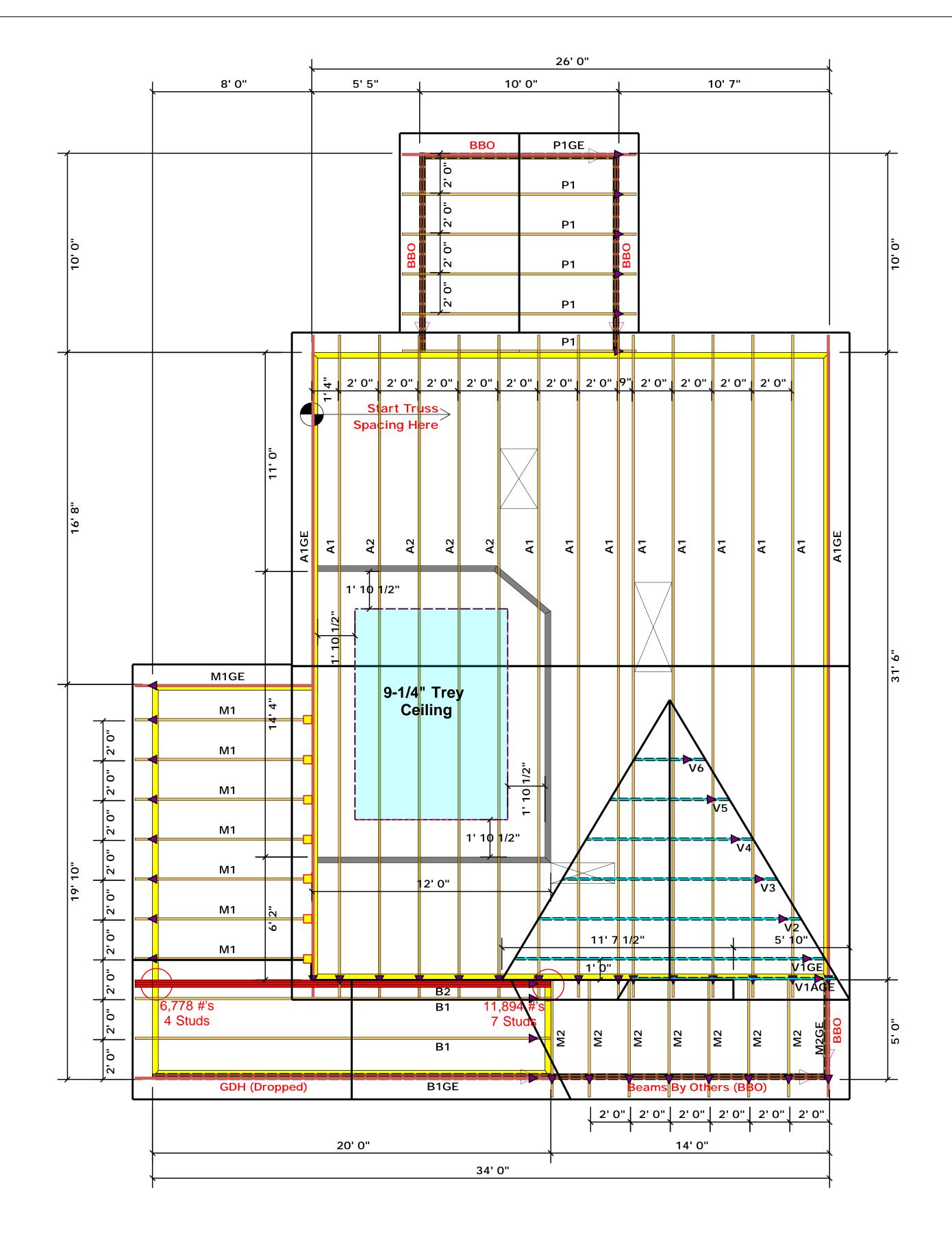
▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing)

<u>Truss Placement Plan</u> SCALE: NTS

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

LOAD CHART FOR JACK STUDS (0456 DN 14045 (2005)) 4.00) MARKET JACK THEN BOARD (4.000 DF	BUILDER	Weaver Development	СІТҮ / СО.	Lillington / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
	JOB NAME	Lot 4 Hales Farm	ADDRESS	NC 27 West	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
IND REAL PROPERTY OF THE PROPE	PLAN	Magnolia I I "C"	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 2550 1 3400 ? 3400 2 5100 2 6600 2 5100 3 7650 3 10200 3	SEAL DATE	Seal Date	DATE REV.	/ /	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 300% but not greater than 1500%. A registered design professional shall be retained to design the support system for any reaction that exceeds those	Reilly Road Industrial Park
680C 4 10200 4 13600 4 850C 5 12750 5 17000 5 1020C 6 15500 6	QUOTE #		DRAWN BY	Christine Shivy	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Christine Shivy	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9	JOB #	J0822-4190	SALES REP.	Lenny Norris	Christine Shivy	Fax: (910) 864-4444



	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header Truss	
	JUS24	USP	7	NA	10d/3"	10d/3"

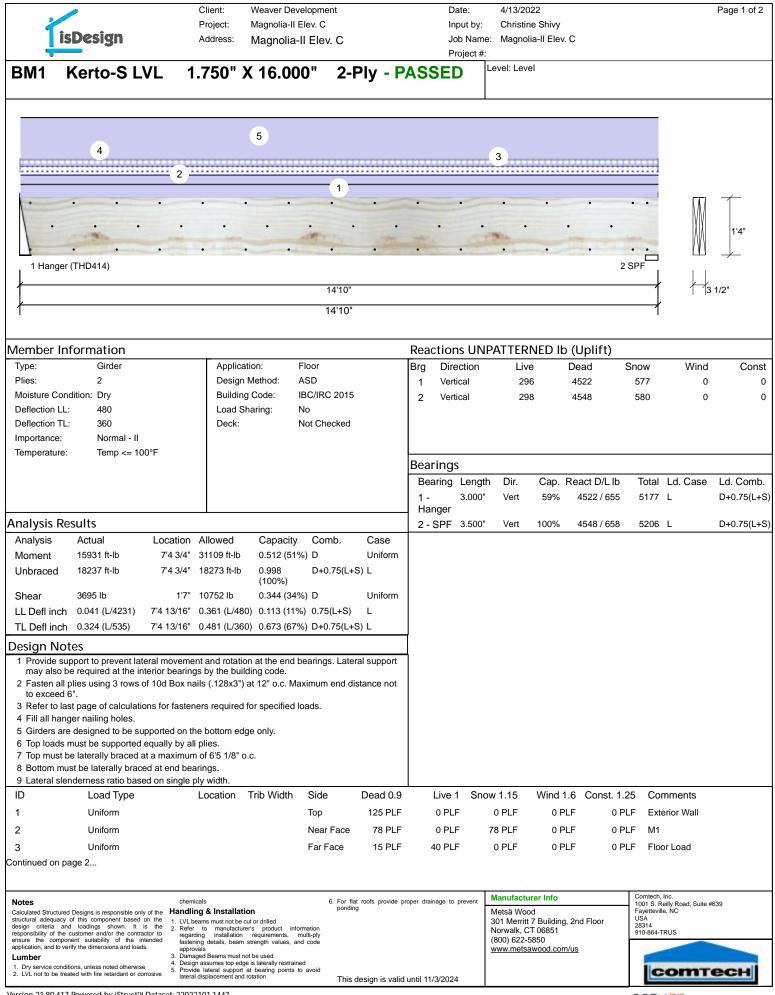
▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing)

<u>Truss</u> <u>Placement</u> <u>Plan</u> SCALE: NTS

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

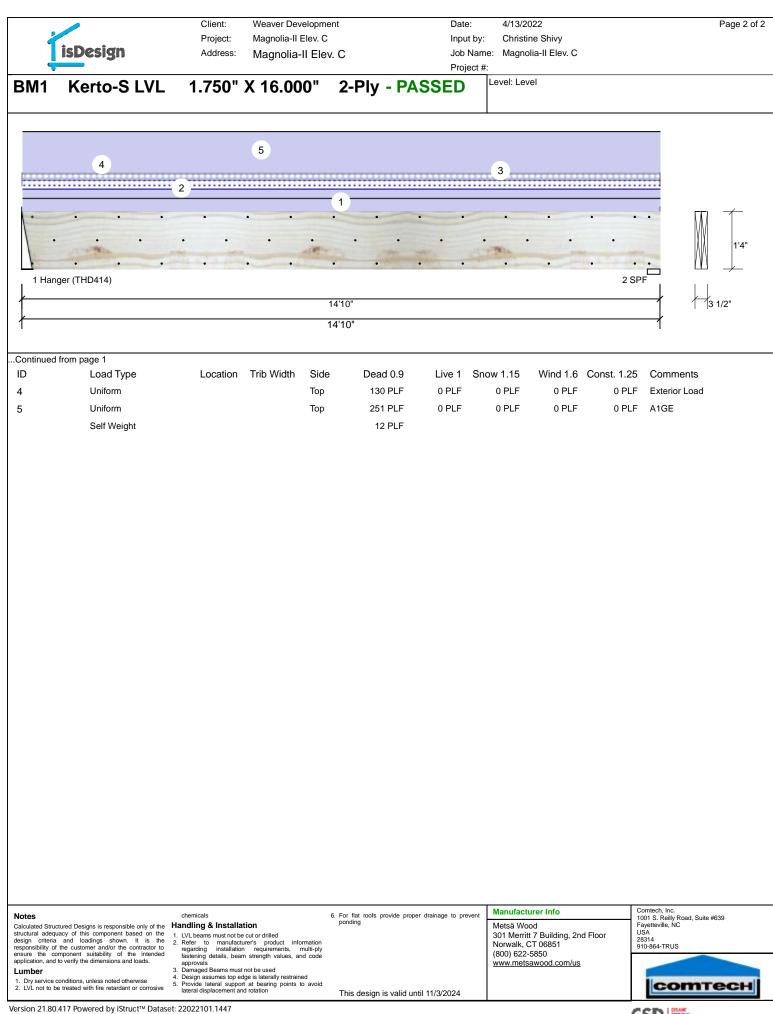
-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

(045)	HART FOR JAC FN ON 1 ABLES (2502-5) MAGE STUDG (COURTE)	A (60)	BUILDER	Weaver Development	CITY/CO.	Lillington / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
CTICN 00 105 FUC		N LOS CON	JOB NAME	Lot 4 Hales Farm	ADDRESS	NC 27 West	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
no sia COR(V)	nur and grad drag	Un the state	PLAN	Magnolia I I "C"	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	Seal Date	DATE REV.	//	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	Reilly Road Industrial Park
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #		DRAWN BY	Christine Shivy	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Christine Shivy	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9			JOB #	J0822-4190	SALES REP.	Lenny Norris	Christine Shivy	Fax: (910) 864-4444



Version 21.80.417 Powered by iStruct™ Dataset: 22022101.1447

CSD 🛤



isDesign		Weaver Development		Date:	4/13/2022	2		Page 1 of
ic Decian	Project:	Magnolia-II Elev. C		Input by	: Christine	Shivy		Ū.
Ispesizii	Address:	Magnolia-II Elev. C	;	Job Nar	•	II Elev. C		
		<u> </u>		Project	#: Level: Level			
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Vember Information	_				NPATTERN	ED lb (Uplift)		
Type: Girder Plies: 2		ation: Floor n Method: ASD		Brg Direction	Live	Dead		ind Con
Moisture Condition: Dry	-	n Method: ASD ng Code: IBC/IRC 20)15	1 Vertical 2 Vertical	3389 3389	1200 1200	0 0	0 0
Deflection LL: 480		Sharing: No		2 vortiour	0000	1200	0	0
Deflection TL: 360	Deck:	Not Checke	ed					
Importance: Normal - II Temperature: Temp <= 100°	F							
				Bearings				
				Bearing Leng	-	Cap. React D/L lb		
				1 - SPF 3.500 2 - SPF 3.500		88% 1200/3389 88% 1200/3389		D+L D+L
Analysis Results				2-377 3.300	y ven	12007 3303	4303 L	DTL
•	Location Allowed	Capacity Comb.						
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Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457)	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotai	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L	L L L	-				
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotat rior bearings by the bu	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La	L L L ateral support					
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of 4 to exceed 6".	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotation for bearings by the bu 10d Box nails (.128x3) s for fasteners require	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La iilding code. ') at 12" o.c. Maximum end d for specified loads.	L L L ateral support	-				
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of 1 to exceed 6". 3 Refer to last page of calculations 4 Girders are designed to be supp	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotati rior bearings by the bu 10d Box nails (.128x3' s for fasteners require ported on the bottom e	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La iilding code. ') at 12" o.c. Maximum end d for specified loads.	L L L ateral support					
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Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of 4 to exceed 6". 3 Refer to last page of calculations 4 Girders are designed to be supp 5 Top must be laterally braced at e 6 Bottom must be laterally braced	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotai rior bearings by the bu 10d Box nails (.128x3' s for fasteners require borted on the bottom e end bearings. at end bearings.	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La iilding code. ') at 12" o.c. Maximum end d for specified loads.	L L ateral support d distance not	Live 1 Sr 267 PLF	now 1.15 \ 0 PLF	Wind 1.6 Const. 1 0 PLF 0	.25 Comments PLF F4	3
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of to exceed 6". 3 Refer to last page of calculations 4 Girders are designed to be support to prevent based of Lateral slenderness ratio based Top must be laterally braced at each of Lateral slenderness ratio based ID Load Type	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotation for bearings by the bu 10d Box nails (.128x3) s for fasteners require ported on the bottom e end bearings. at end bearings. on single ply width.	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La iiding code. ') at 12" o.c. Maximum end d for specified loads. dge only.	L L ateral support d distance not			0 PLF 0		3

Ti	sDesign	-	agnolia-II Elev. C Iagnolia-II Elev. C			Christine Shivy Magnolia-II Ele				
GDH	Kerto-S LVL	1.750" X	11.875" 2-	-Ply - P	Project #:	Level: Level				
	nd Grain	2	1	đ	•	- -	2 SPF Enc	· · ·		11 7/8"
/			16'10'					\longrightarrow	1 1-	
	nformation	A 12 - 12			Reactions UN			0	1477 -	
Type: Plies: Moisture Con Deflection Ll Deflection TI mportance:		Application Design Me Building C Load Shar Deck:	ethod: ASD ode: IBC/IRC 201		Brg Direction 1 Vertical 2 Vertical	Live 0 0	Dead 2098 2098	Snow 337 337	Wind 0 0	Cor
emperature	:: Temp <= 100°F				Bearings					
					Bearing Lengt 1 - SPF 3.500" End	h Dir. Cap Vert 24%	React D/L lb 2098 / 337		Ld. Case L	Ld. Com D+S
nalysis R			Oran a site and a sate	0	Grain 2 - SPF 3.500"	Vert 24%	2098 / 337	2434	L	D+S
Analysis Moment Jnbraced Shear LL Defl inch	8354 ft-lb 9694 ft-lb 1788 lb 1 10.070 (L/2809) 8'5	8'5" 17919 ft-lb 8'5" 9704 ft-lb	. ,	Case Uniform L Uniform L	End Grain					
esign No				_	ſ					
1 Provide si may also 2 Fasten all to exceed 3 Refer to la 4 Girders au 5 Top loads 6 Top must 7 Bottom m	upport to prevent lateral m be required at the interior plies using 2 rows of 10d	bearings by the buildin Box nails (.128x3") at r fasteners required for ed on the bottom edge ly by all plies. aximum of 9'6 3/4" o.c. end bearings.	g code. 12" o.c. Maximum end c specified loads.							
D	Load Type	Location Tr	ib Width Side	Dead 0.9			1.6 Const. 1		mments	
2	Uniform Uniform		Тор Тор	200 PLF 40 PLF	0 PLF 0 PLF				erior Loads)" Gable End	
	Self Weight			9 PLF						
uctural adequacy sign criteria a sponsibility of the sure the comp plication, and to v umber	ed Designs is responsible only of the y of this component based on the nd loadings shown. It is the customer and/or the contractor to onent suitability of the intended erify the dimensions and loads. ditions, unless noted otherwise eated with fire retardant or corrosive	chemicals Handling & Installation 1. LVL beams must not be cut on 2. Refer to manufacturers regarding installation re fastening details, beam stre approvals 3. Damaged Beams must not be 4. Design assumes top edge is: 5. Provide lateral support at b	pond drilled product information quirements, multi-ply ngth values, and code used aterally restrained	flat roofs provide pi ing	oper drainage to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Buildin Norwalk, CT 06851 (800) 622-5850 www.metsawood.co	-	Fayettevil USA 28314 910-864-1	teilly Road, Suite # le, NC	