

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 238_2338_C KB Home 238.2338.C

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I42970928 thru I42970957

My license renewal date for the state of North Carolina is December 31, 2020.

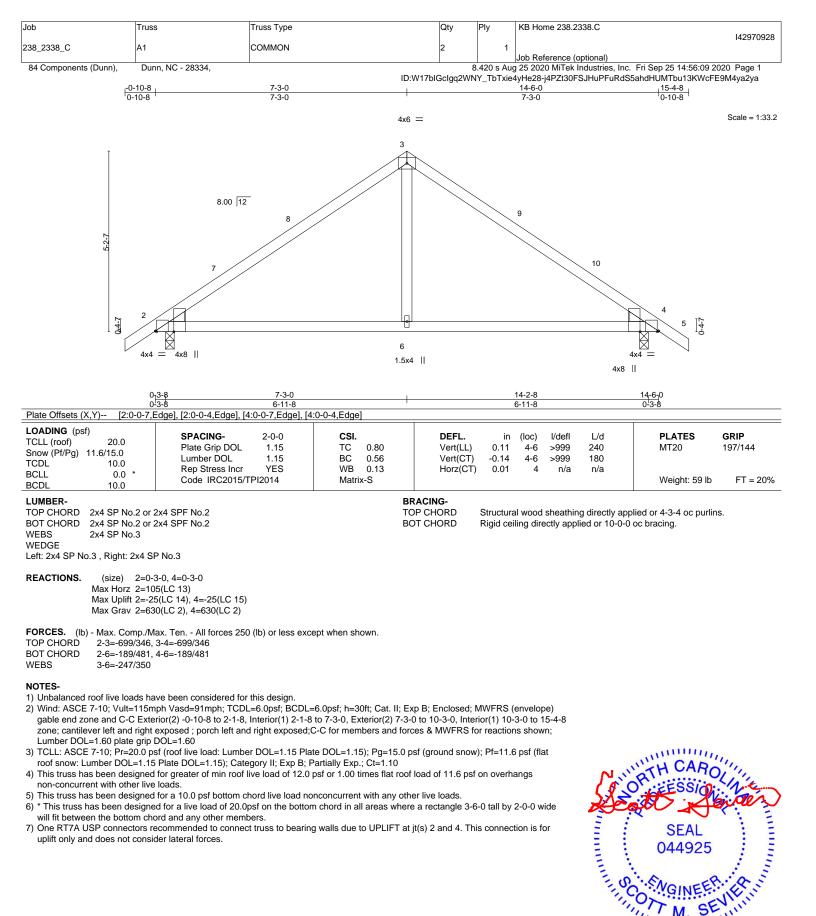
North Carolina COA: C-0844



September 28,2020

Sevier, Scott

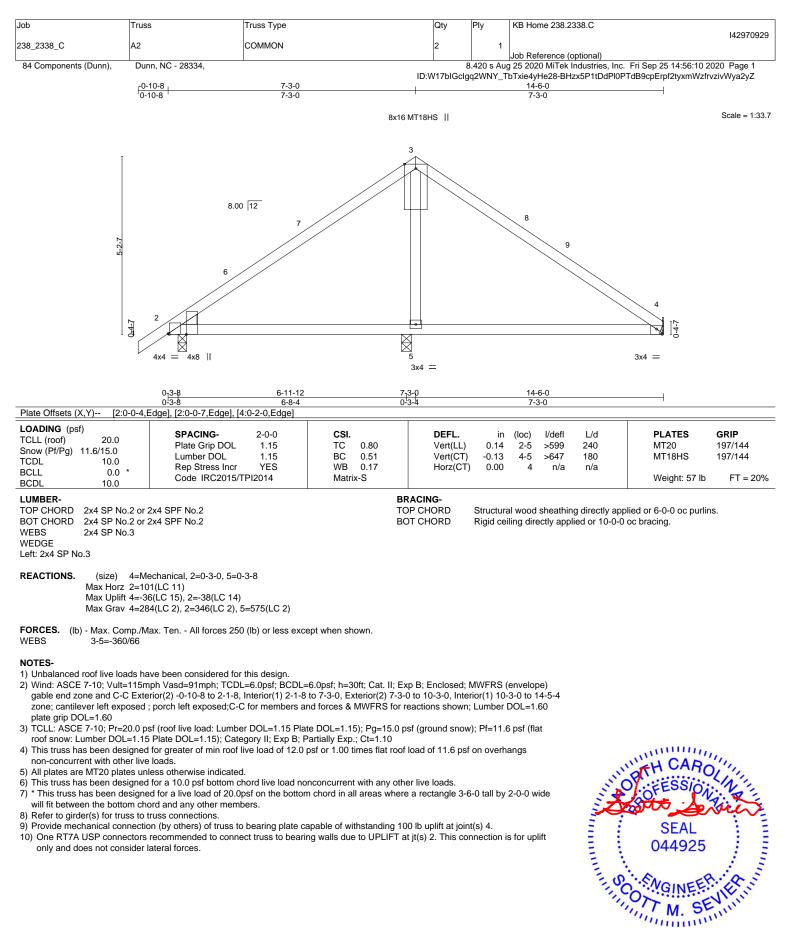
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



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818 Soundside Road Edenton, NC 27932

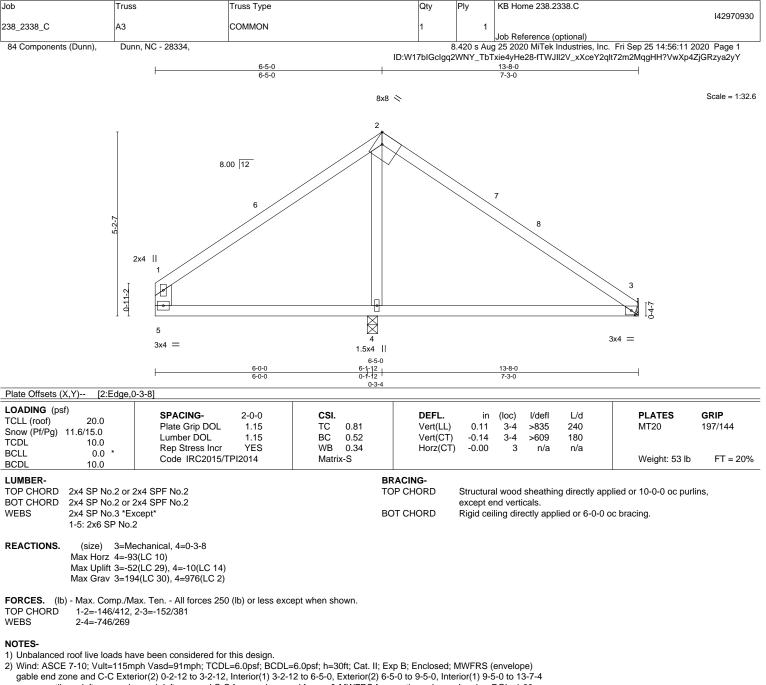
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2) Which Note 1 for, for the transportation of the transport of transp

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

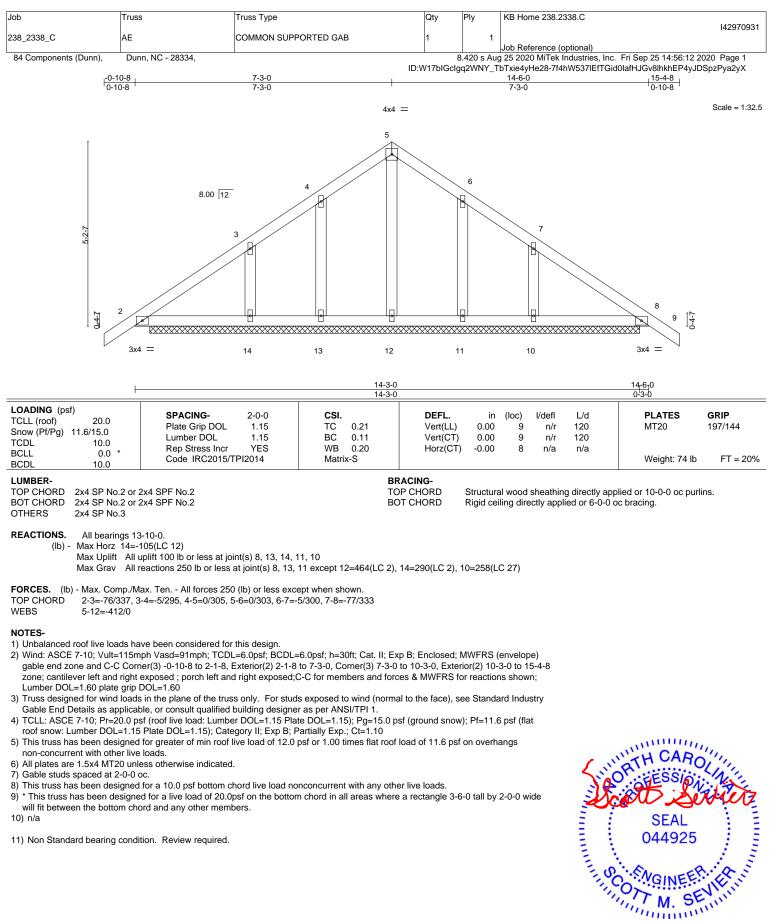
 One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This connection is for uplift only and does not consider lateral forces.



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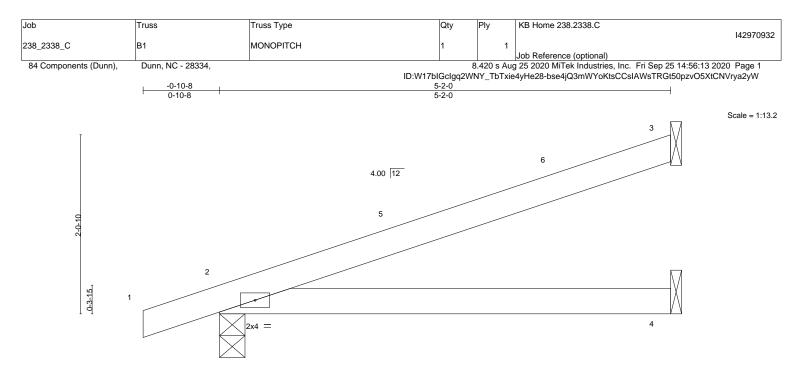
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			5-2-0 5-2-0						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.31 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.04 -0.07 -0.00	(loc) 2-4 2-4 3	l/defl >999 >821 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 17 lb	GRIP 197/144 FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2 or 2x4 SPF No.2BOT CHORD2x4 SP No.2 or 2x4 SPF No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-2-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=63(LC 12) Max Uplift 3=-53(LC 16), 2=-38(LC 12)

Max Grav 3=142(LC 2), 2=266(LC 2), 4=99(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

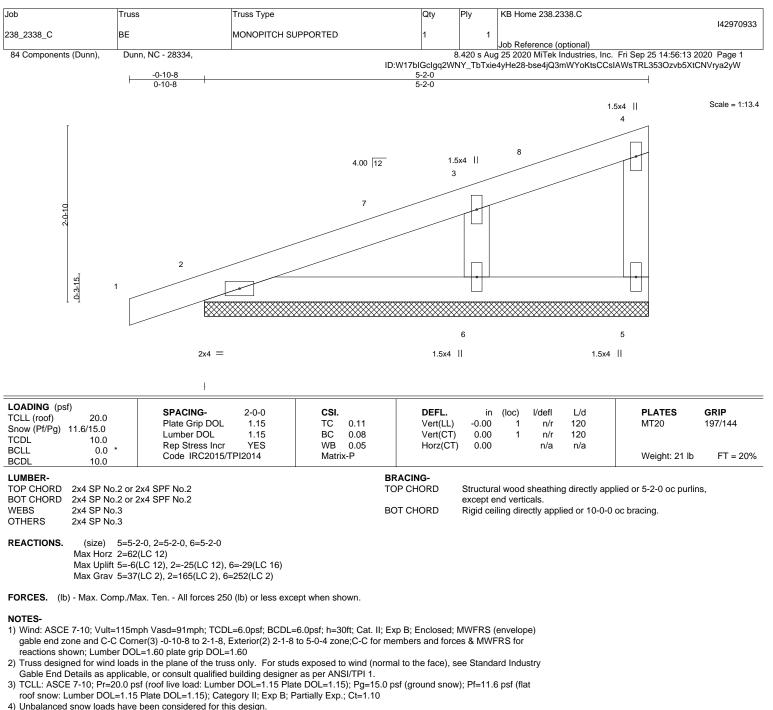
NOTES-

- Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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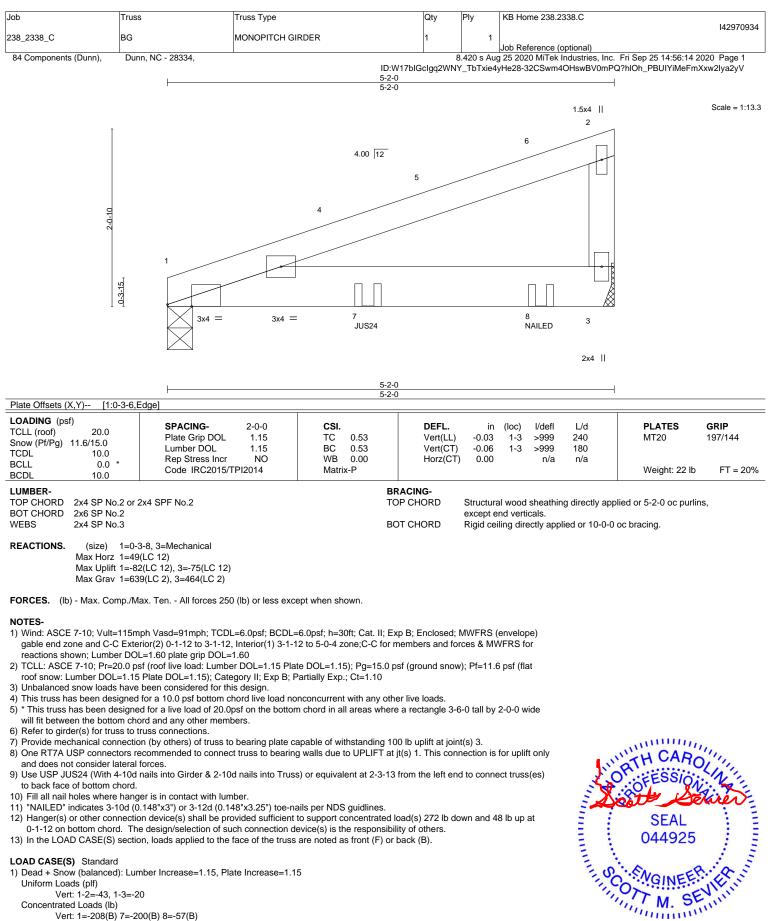
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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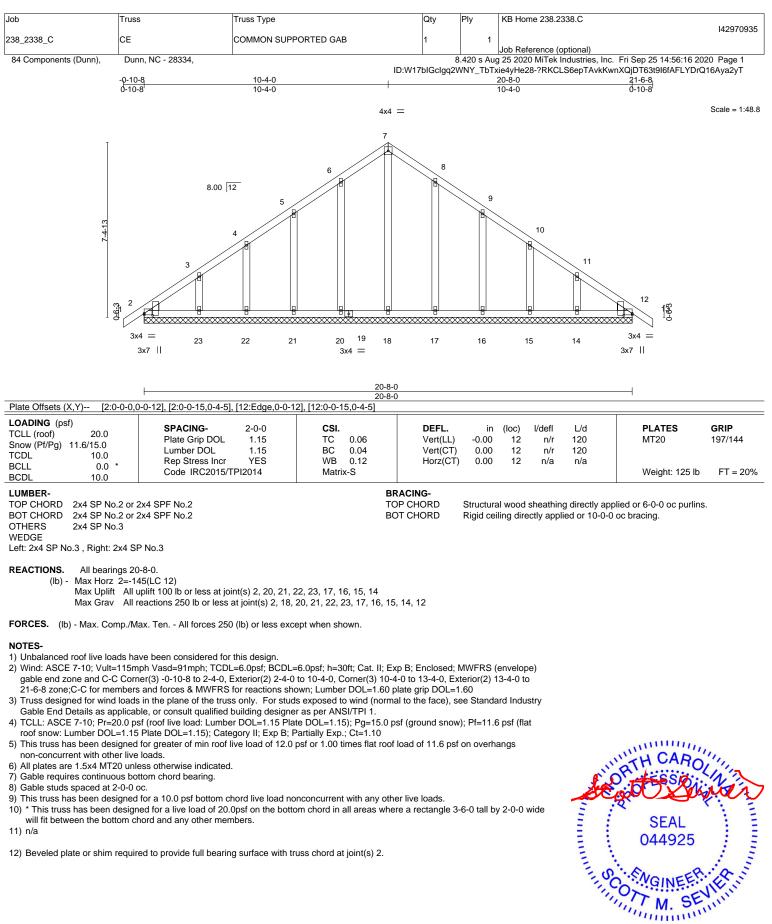


Vert: 1=-208(B) 7=-200(B) 8=-57(B)

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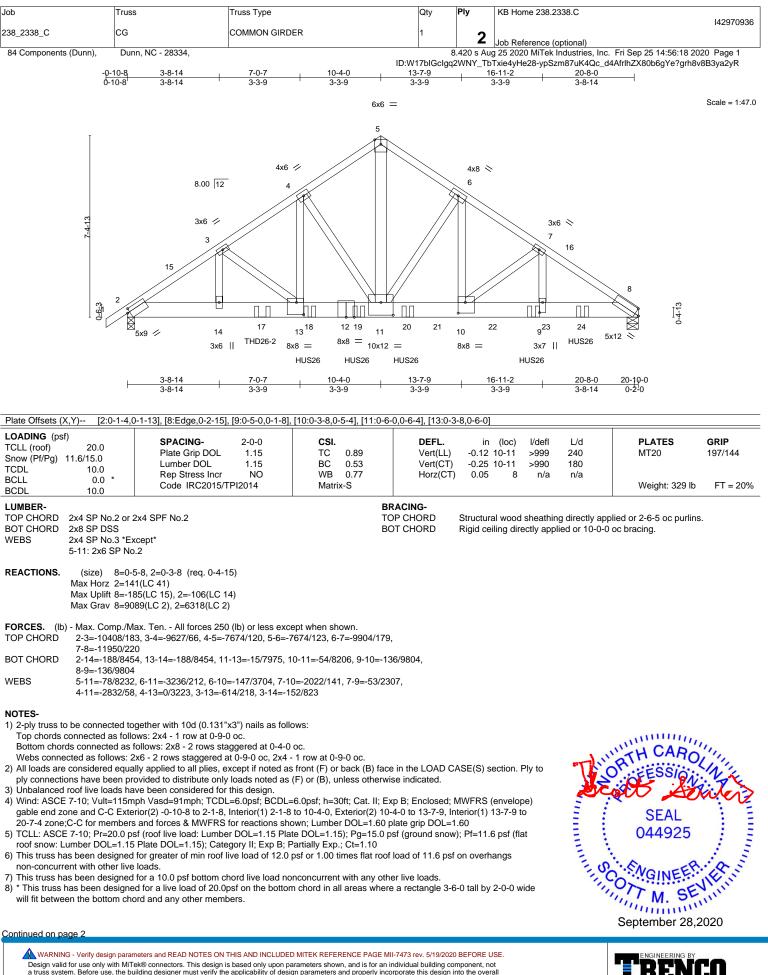
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a truss system and to use the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Job	Truss	Truss Type	Qty	Ply	KB Home 238.2338.C
					142970936
238_2338_C	CG	COMMON GIRDER	1	2	
				~	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8	.420 s Aug	25 2020 MiTek Industries, Inc. Fri Sep 25 14:56:18 2020 Page 2
		ID:W1	17blGclgq2	2WNY_Tb	Txie4yHe28-ypSzm87uK4Qc_d4AfrlhZX80b6gYe?grh8v8B3ya2yR

NOTES-

9) WARNING: Required bearing size at joint(s) 2 greater than input bearing size.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=185, 2=106.

- 11) Use USP THD26-2 (With 18-16d nails into Girder & 12-10d nails into Truss) or equivalent at 5-6-0 from the left end to connect truss(es) to back face of bottom chord.
- 12) Use USP HUS26 (With 14-16d nails into Girder & 6-16d nails into Truss) or equivalent spaced at 5-1-8 oc max. starting at 7-5-4 from the left end to 18-6-12 to connect truss(es) to back face of bottom chord.

13) Fill all nail holes where hanger is in contact with lumber.

14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1451 lb down and 53 lb up at 12-8-4, and 1414 lb down and 58 lb up

at 14-11-4, and 1424 lb down and 49 lb up at 20-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

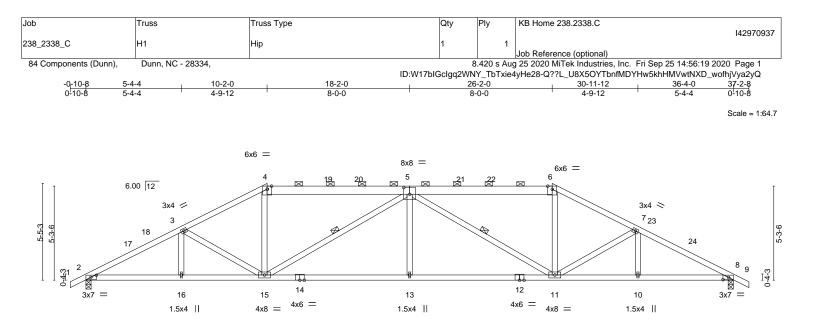
Uniform Loads (plf)

Vert: 1-5=-43, 5-8=-43, 2-8=-20 Concentrated Loads (lb)

Vert: 8=-1121(B) 17=-1988(B) 18=-1303(B) 19=-1381(B) 20=-1424(B) 21=-1451(B) 22=-1111(B) 23=-1111(B) 24=-1111(B)

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5-4-4	10-2-0	18-2-0	26-2-0	30-11-12	36-4-0	1
5-4-4	4-9-12	8-0-0	8-0-0	4-9-12	5-4-4	1
Plate Offsets (X,Y) [2:0-3-1	2,0-1-8], [5:0-4-0,0-4-8], [8:0-3-1	2,0-1-8]				
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0- Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr YE- Code IRC2015/TPI2014	5 TC 0.51 5 BC 0.90 S WB 0.43	DEFL. in (loc) Vert(LL) -0.19 13 Vert(CT) -0.42 13-15 Horz(CT) 0.15 8	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 199 lb	GRIP 197/144 FT = 20%
4-5,5-6: 2x6 SI BOT CHORD 2x4 SP No.2 o	r 2x4 SPF No.2 *Except* P No.2 r 2x4 SPF No.2	Т	except 2-0-0 oc purlins	I sheathing directly app	·	IS,
)-3-8 8=0-3-8	-	VEBS 1 Row at midpt	ectly applied or 10-0-0 5-15, 5-11		

10 2 0

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-71(LC 21) Max Grav 2=1503(LC 2), 8=1503(LC 2)

- 4 4

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-2777/156, 3-4=-2410/162, 4-5=-2100/172, 5-6=-2100/172, 6-7=-2410/162,

10 0 0

- 7-8=-2777/156
- BOT CHORD 2-16=-85/2404, 15-16=-85/2404, 13-15=-43/2786, 11-13=-43/2786, 10-11=-88/2404, 8-10=-88/2404
- WEBS 3-15=-458/103, 4-15=0/695, 5-15=-901/102, 5-13=0/344, 5-11=-901/102, 6-11=0/695, 7-11=-458/104

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-9-2, Interior(1) 2-9-2 to 10-2-0, Exterior(2) 10-2-0 to 15-3-11, Interior(1) 15-3-11 to 26-2-0, Exterior(2) 26-2-0 to 31-3-11, Interior(1) 31-3-11 to 37-2-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



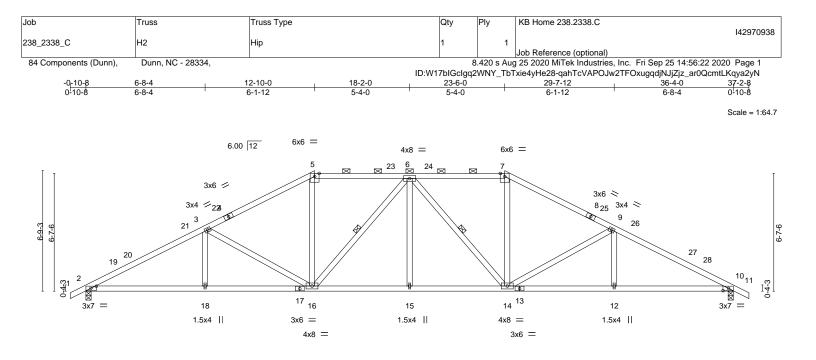
20 44 42

20 4 0

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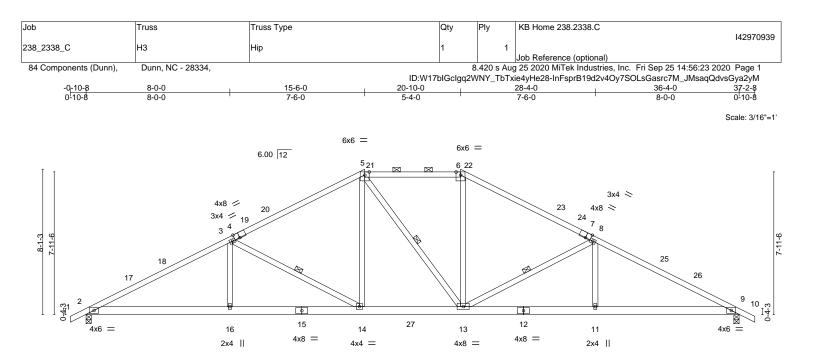
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⊢	6-8-4	12-10-0	18-2-0	23-6-0		29-7-12		36-4-0	
Plate Offsets (X,Y)	<u>6-8-4</u> [2:0-3-12	<u> </u>	5-4-0	5-4-0		6-1-12	•	6-8-4	
LOADING (psf) TCLL (roof) 2 Snow (Pf/Pg) 16.5/1 TCDL	20.0 5.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.76 BC 0.81 WB 0.62	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (-0.16 -0.32 15 0.13	(loc) l/defl 15 >999 5-16 >999 10 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCLL BCDL 1	0.0 * 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 196 lb	FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S REACTIONS. (si Max Max	SP No.2 or 2 SP No.2 or 2 SP No.3 ize) 2=0-3 Horz 2=89 Uplift 2=-21	2x4 SPF No.2	тс	Racing- Racing- DP Chord DT Chord EBS	2-0-0 oc p	urlins (3-11-10 ng directly appl	max.): 5-7.	ied or 2-2-0 oc purlins oc bracing.	, except
TOP CHORD 2-3 9-1 BOT CHORD 2-1 10- WEBS 3-1	=-2737/143 0=-2737/143 8=-65/2361 12=-67/236 8=0/276, 3-	16-18=-65/2361, 15-16=-1/2081, 14-15	367/181, 7-9=-2184/167 =-1/2081, 12-14=-67/23	61,					
 Wind: ASCE 7-10; gable end zone ar 23-6-0, Exterior(2) exposed;C-C for n TCLL: ASCE 7-10 roof snow: Lumbel governs. Rain sur Unbalanced snow This truss has bee non-concurrent wii Provide adequate This truss has bee will fit between the One RT7A USP coupling of the structure 	Vult=115m ad C-C Exter 23-6-0 to 2 (Fr=20.0 ps r DOL=1.15 rcharge appl loads have an designed th other live drainage to en designed be bottom cho connectors re- s not conside	prevent water ponding. for a 10.0 psf bottom chord live load nor d for a live load of 20.0psf on the bottom rd and any other members. commended to connect truss to bearing	o 12-10-0, Exterior(2) 1: e; cantilever left and righ Lumber DOL=1.60 platt a DOL=1.15); Pg=15.0 p artially Exp.; Ct=1.10, Lu ess than 0.500/12 in acc sf or 1.00 times flat roof aconcurrent with any oth chord in all areas where walls due to UPLIFT at	2-10-0 to 18-2-0, tt exposed ; end e grip DOL=1.60 usf (ground snow i=50-0-0; Min. fla cordance with IBC load of 11.6 psf er live loads. e a rectangle 3-6 jt(s) 2 and 10. Th	Interior(1) vertical left troof snow C 1608.3.4. on overhan -0 tall by 2- his connecti	18-2-0 to and right obsf (flat r load ngs -0-0 wide ion is for		SEAL 044925 MGINEE September 28,2	

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8	-0-0 15-6	0 20-1	0-0	28-4-0	36-	4-0	
	-0-0 7-6-	0 5-4	-0	7-6-0	8-0)-0	1
Plate Offsets (X,Y) [4:0-3-7	,Edge], [7:0-3-7,Edge]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.94 BC 0.64 WB 0.32 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.13 14 >999 -0.26 14-16 >999 0.09 9 n/a	240 M 180 n/a	LATES IT20 /eight: 217 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 4-5,6-7: 2x4 SP BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3	2x4 SPF No.2 *Except* P No.1	TC	DT CHORD	Structural wood sheathing 2-0-0 oc purlins (4-0-14 n Rigid ceiling directly appli 1 Row at midpt	nax.): 5-6.		
REACTIONS. (size) 2=0	-3-8. 9=0-3-8						

REACTIONS. (size) 2=0-3-8, 9=0-3-8 Max Horz 2=106(LC 16) Max Uplift 2=-39(LC 16), 9=-39(LC 17) Max Grav 2=1503(LC 2), 9=1503(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2818/130, 3-5=-2053/164, 5-6=-1706/186, 6-8=-2055/163, 8-9=-2817/130

BOT CHORD 2-16=-56/2452, 14-16=-56/2452, 13-14=0/1704, 11-13=-44/2451, 9-11=-44/2451

WEBS 3-16=0/346, 3-14=-837/143, 5-14=0/530, 6-13=0/524, 8-13=-835/144, 8-11=0/344

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-9-2, Interior(1) 2-9-2 to 15-6-0, Exterior(2) 15-6-0 to 25-11-11, Interior(1) 25-11-11 to 37-2-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

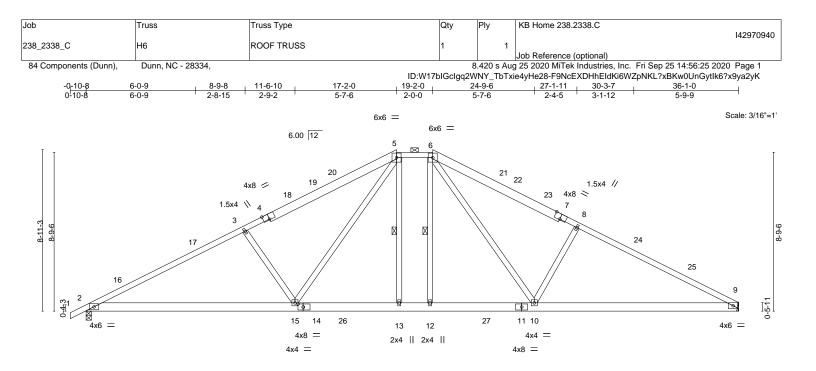
9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	8-9-8 11-6-10 8-9-8 2-9-2	17-2-0 5-7-6	19-2-0 2-0-0	24-9-6 5-7-6	27-6-7	<u>36-1-0</u> 8-6-9	<u> </u>
Plate Offsets (X,Y) [4:0-4-0,	Edge], [7:0-4-0,Edge], [14:0-3-6,0-2-0]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.92 BC 0.76 WB 0.35 Matrix-S	DEFL. Vert(LL) Vert(CT Horz(C) -0.38 2-1	15 >999 240	PLATES MT20 Weight: 227 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 *E: 5-6: 2x4 SP No. BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3	xcept* 2, 1-4: 2x4 SP No.1, 7-9: 2x4 SP DSS		BRACING- TOP CHORD BOT CHORD WEBS	2-0-0 oc purli	bod sheathing directly a lins (4-1-9 max.): 5-6. directly applied or 10-0 dpt 5-13, 6-)-0 oc bracing.	

REACTIONS. (size) 2=0-3-8, 9=Mechanical Max Horz 2=123(LC 16) Max Uplift 2=-49(LC 16), 9=-33(LC 17) Max Grav 2=1514(LC 40), 9=1471(LC 40)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2782/115, 3-5=-2517/141, 5-6=-1665/161, 6-8=-2516/157, 8-9=-2745/120
- BOT CHORD 2-15=-104/2418, 13-15=0/1665, 12-13=0/1665, 10-12=0/1664, 9-10=-26/2375
- WEBS 3-15=-535/212, 5-15=-82/831, 6-12=-143/263, 6-10=-96/838, 8-10=-527/221

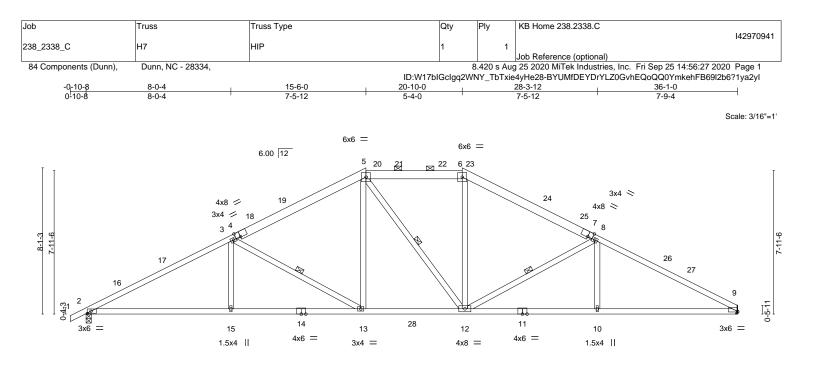
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 17-2-0, Exterior(2) 17-2-0 to 23-4-15, Interior(1) 23-4-15 to 36-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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	0-4 15-6-0 0-4 7-5-12		20-10-0	28-3-12		<u>36-1-0</u> 7-9-4			
	0-4 /-5-12)-1-8], [4:0-3-2,Edge], [7:0-3-2,Edge], [9	:0-0-0,0-0-6]	5-4-0	7-5-12		7-9-4			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.86 BC 0.91 WB 0.30 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.13 13 -0.30 2-15	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 203 lb	GRIP 197/144 FT = 20%		
BRACING- TOP CHORD 2x6 SP No.2 *Except* 1-4: 2x4 SP No.1, 7-9: 2x4 SP DSS BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3 REACTIONS. (size) (size) 2=0-3-8, 9=Mechanical Max Horz Max Horz 2=110(LC 16)									
Max Uplift 2=-39(LC 16), 9=-23(LC 17) Max Grav 2=1497(LC 2), 9=1444(LC 39) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2758/108, 3-5=-2030/151, 5-6=-1691/170, 6-8=-2026/153, 8-9=-2716/110 BOT CHORD 2-15=-59/2389, 13-15=-59/2389, 12-13=0/1694, 10-12=-30/2357, 9-10=-30/2357 WEBS 3-15=0/343, 3-13=-784/140, 5-13=0/512, 6-12=0/491, 8-12=-750/142, 8-10=0/339									
 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 15-6-0, Exterior(2) 15-6-0 to 19-8-15, Interior(1) 19-8-15 to 20-10-0, Exterior(2) 20-10-0 to 25-0-15, Interior(1) 25-0-15 to 36-0-4 zone; C-C for members and forces & MWFRS for reactions 									

shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9.

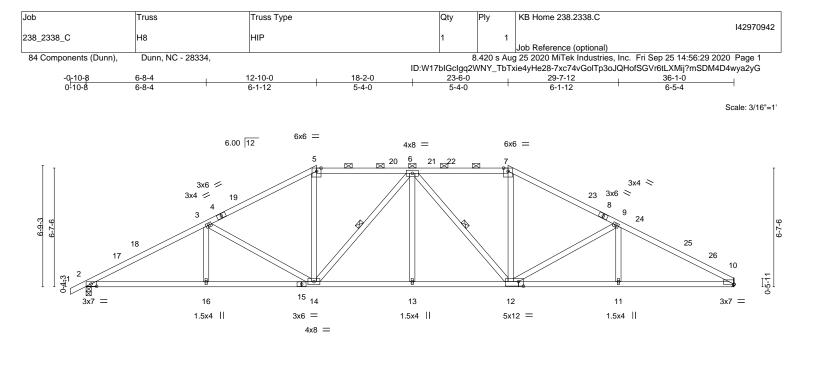
 One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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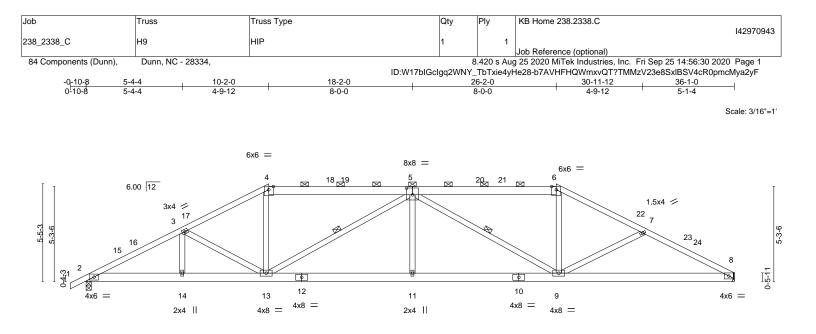




L	6-8-4	12-10-0	18-2-0	23-6-0			29-7-12		36-1-0	
Plate Offsets (X,Y	6-8-4		5-4-0	5-4-0	1		6-1-12		6-5-4	
] [2.0-3-12,0	<u>5-1-8], [10.0-0-0,0-0-2], [12.0-3-4,0-</u>	5-0]							
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 16. TCDL BCLL	10.0 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.92 BC 0.81 WB 0.61 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.15 -0.31 0.13	(loc) 13 13-14 10	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 193 lb	GRIP 197/144 FT = 20%
BCDL	10.0									
REACTIONS.	x4 SP No.2 or 2x x4 SP No.3 (size) 2=0-3- //ax Horz 2=94(L	x4 SPF No.2 -8, 10=Mechanical LC 16)	T ⁱ B	OT CHORD	2-0-0 oc	purlins ling dire	(3-11-13 ectly appl	max.): 5-7.	0 oc bracing.	
Ν	Max Grav 2=149	(LC 16), 10=-5(LC 17) /7(LC 2), 10=1434(LC 2) x. Ten All forces 250 (lb) or less 6	except when shown							
TOP CHORD		3-5=-2172/154, 5-6=-1855/169, 6-7		١,						
BOT CHORD		14-16=-63/2351, 13-14=-6/2065, 12	2-13=-6/2065, 11-12=-56/23	311,						
WEBS		4=-679/120, 5-14=0/630, 6-14=-44	6/69, 6-12=-459/67, 7-12=0	/625,						
 Wind: ASCE 7- gable end zone 23-6-0, Exterio Lumber DOL=1 TCLL: ASCE 7- roof snow: Lum governs. Rain Unbalanced sn This truss has I non-concurrent Provide adequa This truss has I swill fit between Refer to girder(Provide mech One RT7A US only and does 	-10; Vult=115mp e and C-C Exterior (r(2) 23-6-0 to 27 1.60 plate grip Di -10; Pr=20.0 psf hober DOL=1.15 F surcharge applie tow loads have b been designed fr t with other live le ate drainage to p been designed fr s been designed fr s been designed the bottom chorr (s) for truss to tru- nanical connection SP connectors re s not consider lat	i (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp I ed to all exposed surfaces with slop been considered for this design. or greater of min roof live load of 12 oads. or event water ponding. or a 10.0 psf bottom chord live load of for a live load of 20.0psf on the bo rd and any other members. uss connections. on (by others) of truss to bearing pla ecommended to connect truss to be	1-8 to 12-10-0, Exterior(2) 1 zone;C-C for members and Plate DOL=1.15); Pg=15.0 j 3; Partially Exp.; Ct=1.10, Li es less than 0.500/12 in ac 2.0 psf or 1.00 times flat roo nonconcurrent with any oth tom chord in all areas when the capable of withstanding aring walls due to UPLIFT a	2-10-0 to 17-0-15 I forces & MWFRS psf (ground snow) u=50-0-0; Min. fla cordance with IBC f load of 11.6 psf her live loads. re a rectangle 3-6 100 lb uplift at join at jt(s) 2. This con	5, Interiori 5 for reac 5 for reac 5 for reac 6 for reac 6 for reac 6 for reac 7 for reac 6 for reac 9 for rea	(1) 17- titions sh 5 psf (fla w load 4. angs 2-0-0 w s for up	15 to nown; at	and a superior	September 28,2	

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5-4-4	10-2-0	18-2-0		2-0		36-1-0	
5-4-4	4-9-12	8-0-0	8-0	0-0	·	9-11-0	·
Plate Offsets (X,Y) [5:0-4-	-0,0-4-8]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.51 BC 0.60 WB 0.42 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.15 11 :	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 222 lb	GRIP 244/190 FT = 20%
LUMBER-			ACING-				
TOP CHORD 2x4 SP No.2		TC	P CHORD S	Structural wood sh	heathing directly a	pplied or 3-1-15 oc purlir	ıs,
4-5,5-6: 2x6 \$	SP No.2		6	except			
BOT CHORD 2x6 SP No.2				2-0-0 oc purlins (4	-6-8 max.): 4-6.		
WEBS 2x4 SP No.3		BC	T CHORD	Riaid ceilina direc	tly applied or 10-0	-0 oc bracing.	
		W		1 Row at midpt	5-13, 5-		
REACTIONS. (size) 8=	Mechanical, 2=0-3-8						
Max Horz 2=	=77(LC 20)						
	1424(1 C 2) 2 1407(1 C 2)						

26.2.0

19-2-0

Max Grav 8=1434(LC 2), 2=1497(LC 2)

5-1-1

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-2801/140, 3-4=-2440/150, 4-5=-2127/161, 5-6=-2119/148, 6-7=-2435/134,

10.2.0

7-8=-2699/180

2-14=-88/2439, 13-14=-88/2439, 11-13=-55/2807, 9-11=-55/2807, 8-9=-119/2353 BOT CHORD WEBS 3-13=-465/104, 4-13=0/709, 5-13=-889/107, 5-11=0/304, 5-9=-898/118, 6-9=0/703, 7-9=-370/157

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 10-2-0, Exterior(2) 10-2-0 to 14-4-15, Interior(1) 14-4-15 to 26-2-0, Exterior(2) 26-2-0 to 30-4-15, Interior(1) 30-4-15 to 36-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Refer to girder(s) for truss to truss connections.

- 10) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

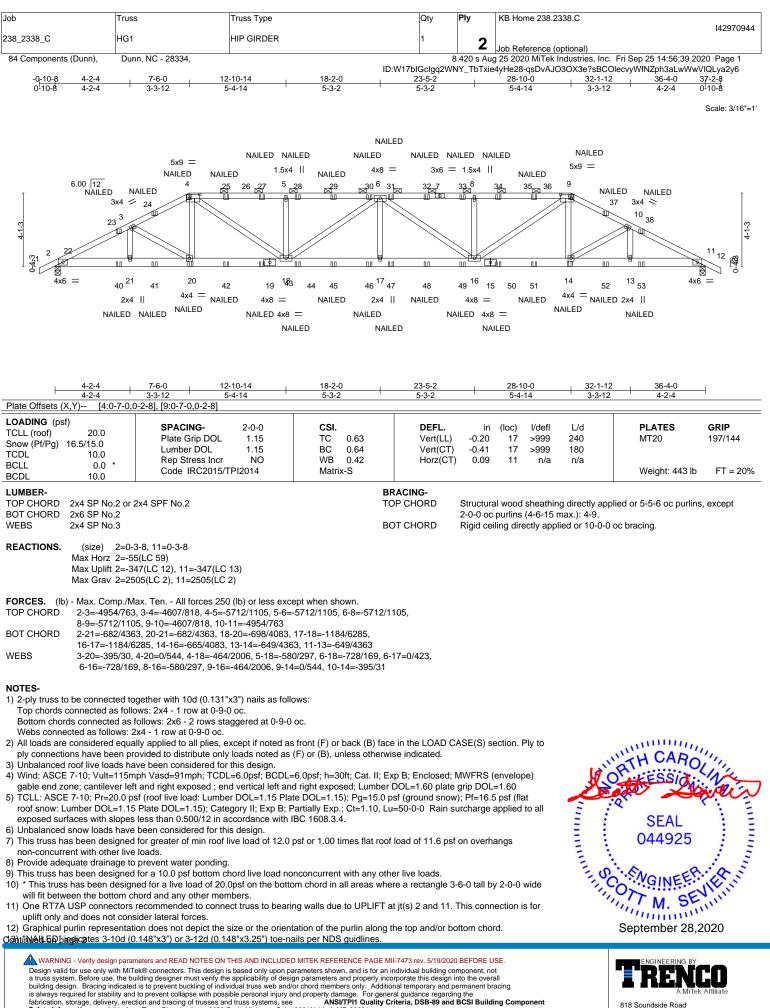


26-1-0

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 Satisfies
 Ansi/TPH Qu

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 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Qu** Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	KB Home 238.2338.C
					142970944
238_2338_C	HG1	HIP GIRDER	1	2	
				2	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8	3.420 s Aug	g 25 2020 MiTek Industries, Inc. Fri Sep 25 14:56:39 2020 Page 2

ID:W17blGclgq2WNY_TbTxie4yHe28-qsDvAJO3OX3e?sBCOlecvyWfNZph3aLwWwVlQLya2y6

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

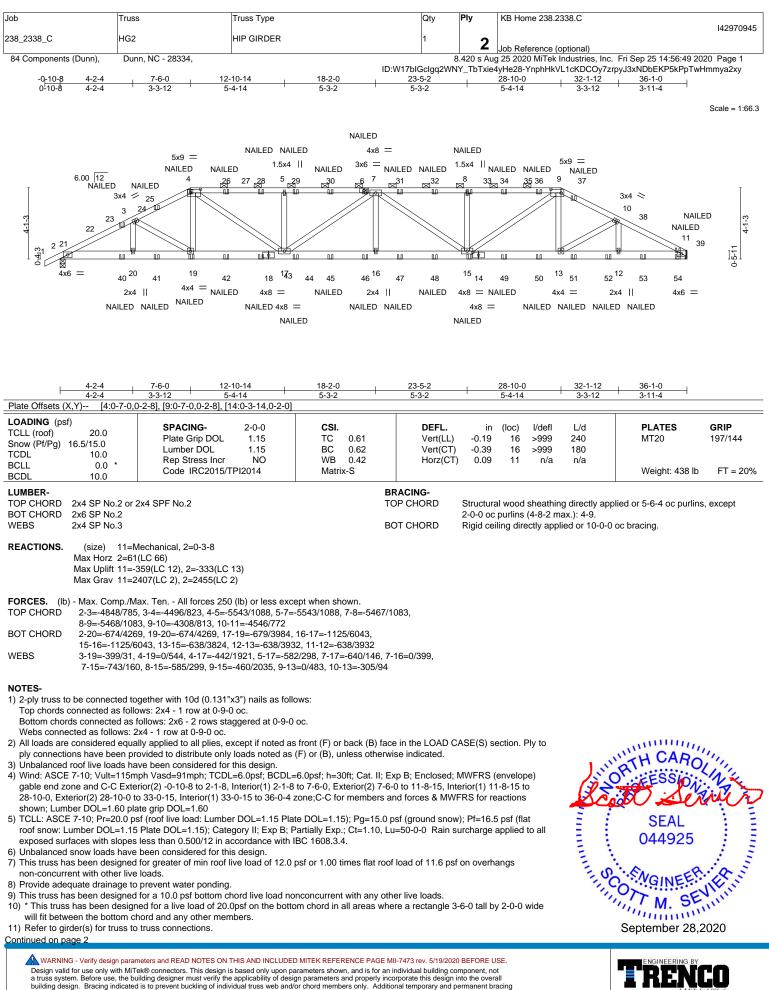
Uniform Loads (plf) Vert: 1-4=-43, 4-9=-53, 9-12=-43, 2-11=-20

Concentrated Loads (lb)

Vert: 4=-59(B) 9=-59(B) 20=-32(B) 14=-32(B) 23=-78(B) 24=-69(B) 25=-54(B) 27=-54(B) 28=-54(B) 29=-54(B) 30=-54(B) 31=-54(B) 32=-54(B) 33=-54(B) 33

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balling denige water to date only water the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	KB Home 238.2338.C
					142970945
238_2338_C	HG2	HIP GIRDER	1	2	
				_	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			.420 s Aug	25 2020 MiTek Industries, Inc. Fri Sep 25 14:56:49 2020 Page 2
		ID:W17bl0	Gelgq2WN	Y_TbTxie4	yHe28-YnphHkVL1cKDCOy7zrpyJ3xNDbEKP5kPpTwHmmya2xy

NOTES-

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=359.
 One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces. (14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 (15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

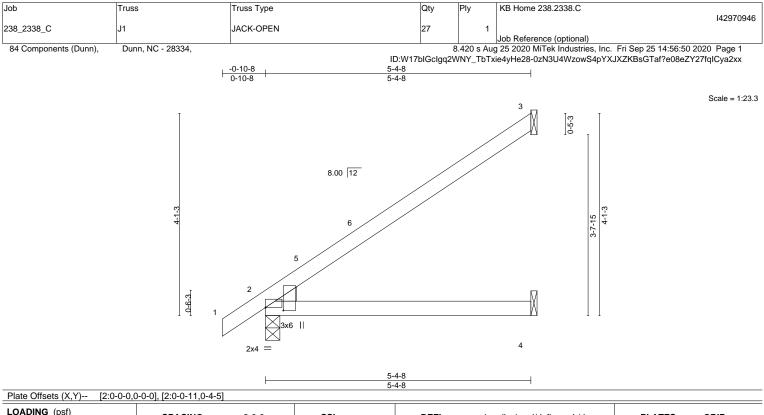
- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-4=-43, 4-9=-53, 9-11=-43, 2-11=-20

Concentrated Loads (lb)

Vert: 4=-59(F) 6=-54(F) 19=-32(F) 8=-54(F) 15=-32(F) 24=-78(F) 25=-69(F) 26=-54(F) 28=-54(F) 29=-54(F) 30=-54(F) 31=-54(F) 32=-54(F) 34=-54(F) 36=-54(F) 36=-56(F) 36= 37=-63(F) 39=-76(F) 40=-69(F) 41=-39(F) 42=-32(F) 43=-32(F) 44=-32(F) 45=-32(F) 46=-32(F) 47=-32(F) 48=-32(F) 48=-32(F) 50=-32(F) 51=-32(F) 52=-32(F) 52=-32 53=-32(F) 54=-37(F)

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LUMBER-		BR	ACING-					
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-P	Horz(CT)	-0.00 3	n/a	n/a	Weight: 20 lb	FT = 2
Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	Lumber DOL 1.15	BC 0.33	Vert(CT)	-0.09 2-4	>725	180		
TCLL (roof) 20.0 Snow (Pf/Pa) 11.6/15.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.50	DEFL. Vert(LL)	in (loc) -0.04 2-4	l/defl >999	L/d 240	PLATES MT20	GRIP 197/144

TOP CHORD

BOT CHORD

LL

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEDGE

Left: 2x4 SP No.3

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=123(LC 14) Max Uplift 3=-86(LC 14)

Max Grav 3=156(LC 26), 2=274(LC 2), 4=103(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-3-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 5-4-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

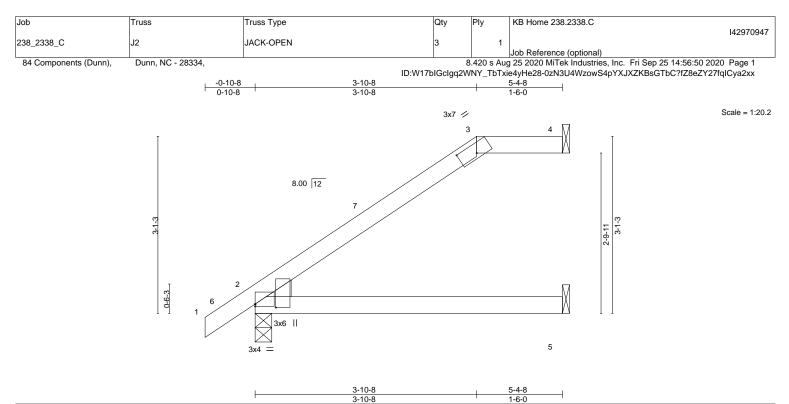
= 20%

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not beigh valid bit de only with with with exercising to be detuins the design is been only door particular is the transmission of the design in the overall building design en with versify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component**
 Satisfies
 Ansi/TPH Qu

 Safety Information
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Edenton, NC 27932



TCDL 10.0 Lumber DOL 1.15 BC 0.30 Vert(C1) -0.07 2-5 >832 180 BCLL 0.0 * Rep Stress Incr YES WB 0.00 Horz(CT) 0.07 4 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-P Weight: 20 lb FT = 209	Plate Offsets (X,Y) [2:0-0-11] LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 16.5/15.0	0-4-5], [2:Edge,0-0-8], [3:0-3-12,0-1-15 SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.40	DEFL. Vert(LL)	in -0.03	(loc) 2-5	l/defl >999	L/d 240	PLATES MT20	GRIP 197/144
BCDL 10.0 Construction and the second s	TCDL 10.0			Vert(CT) Horz(CT)	-0.07 0.07	2-5 4	>832 n/a	180 n/a		
	BCDL 10.0	Code IRC2015/1PI2014		ACING-					Weight: 20 lb	F1 = 20%

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEDGE
 WEDGE

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-4-8 oc purlins, except 2-0-0 oc purlins: 3-4. Rigid ceiling directly applied or 10-0-0 oc bracing.

Left: 2x4 SP No.3

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=95(LC 14) Max Uplift 4=-39(LC 14), 2=-6(LC 14) Max Grav 4=137(LC 2), 2=274(LC 2), 5=98(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-10-8, Exterior(2) 3-10-8 to 5-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

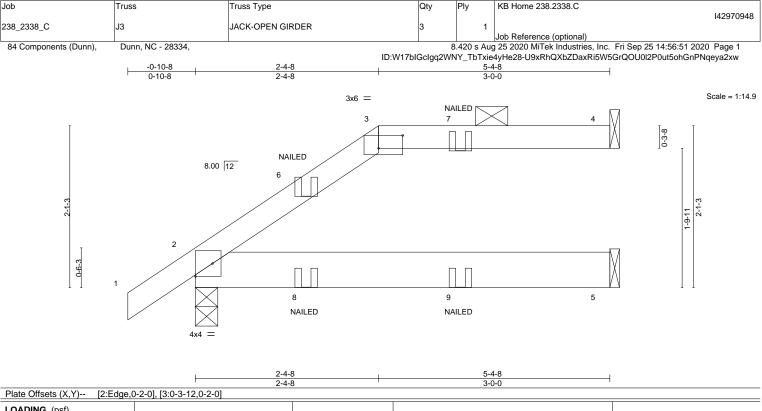
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 10) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.46 BC 0.23 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)		c) l/defl -5 >999 -5 >999 4 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 23 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 2 BOT CHORD 2x6 SP No.2	2x4 SPF No.2			Structural wo 2-0-0 oc purl		g directly ap	plied or 5-4-8 oc purlir	ns, except

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical Max Horz 2=64(LC 43) Max Uplift 4=-49(LC 11), 2=-20(LC 14) Max Grav 4=145(LC 2), 2=321(LC 2), 5=131(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 3-4=-53, 2-5=-20 Concentrated Loads (lb)

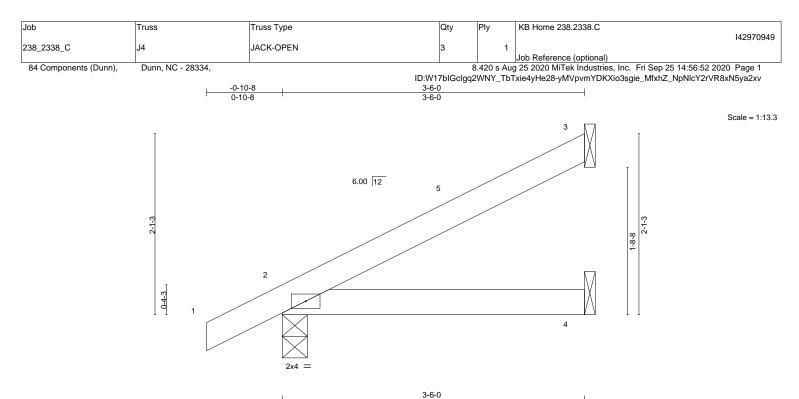
Vert: 6=-44(F) 7=-11(F) 8=-16(F) 9=-13(F)





Edenton, NC 27932

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			3-6-0				7		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.16 BC 0.13 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.01 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 13 lb	GRIP 197/144 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=63(LC 16)

Max Uplift 3=-41(LC 16), 2=-13(LC 16)

Max Grav 3=89(LC 2), 2=202(LC 2), 4=66(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

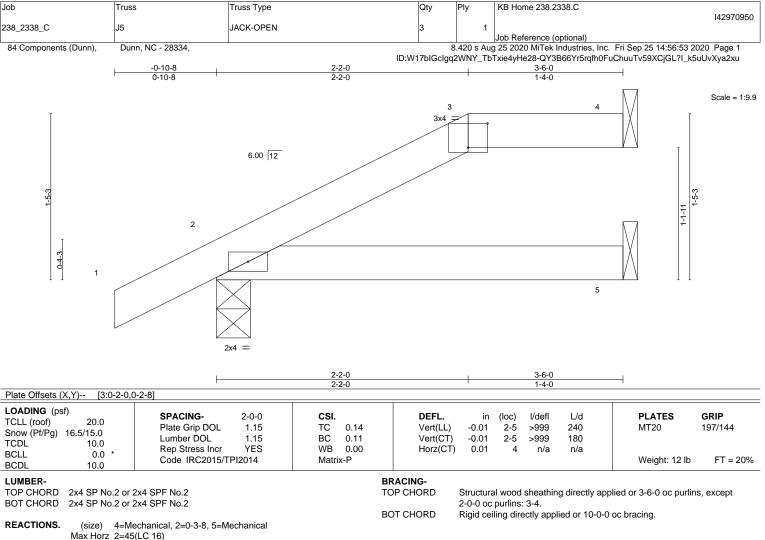
NOTES-

- Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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Max Horz 2=45(LC 16) Max Uplift 4=-21(LC 13), 2=-18(LC 16) Max Grav 4=87(LC 35), 2=235(LC 36), 5=61(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Refer to girder(s) for truss to truss connections.

- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 11) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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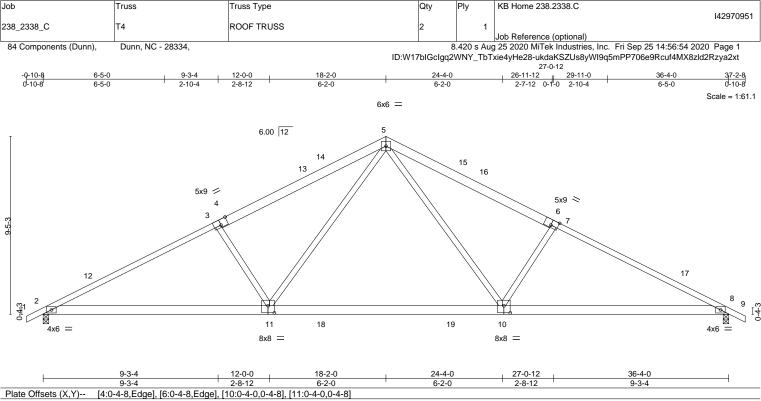


Plate Offsets (X,Y) [4:0-4-8,	Edge], [6:0-4-8,Edge], [10:0-4-0,0-4-8],	[11:0-4-0,0-4-8]				
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.90 BC 0.80 WB 0.39	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d -0.32 10-11 >999 240 -0.47 10-11 >915 180 0.07 8 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S			Weight: 212 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 *E:	•	тс		Structural wood sheathing directly		
1-4,6-9: 2x4 SP BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3	No.1	BC	OT CHORD	Rigid ceiling directly applied or 10-	0-0 oc bracing.	
REACTIONS. (size) 2=0-	3-8, 8=0-3-8					

Max Horz 2=0-3-6, 8=0-3-6Max Horz 2=-123(LC 21)Max Uplift 2=-54(LC 16), 8=-54(LC 17)Max Grav 2=1503(LC 2), 8=1503(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2602/94, 3-5=-2313/128, 5-7=-2313/128, 7-8=-2602/94

BOT CHORD 2-11=-107/2227, 10-11=0/1463, 8-10=0/2227

WEBS 5-10=-57/931, 7-10=-527/226, 5-11=-57/931, 3-11=-527/226

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-9-2, Interior(1) 2-9-2 to 18-2-0, Exterior(2) 18-2-0 to 21-9-10, Interior(1) 21-9-10 to 37-2-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.

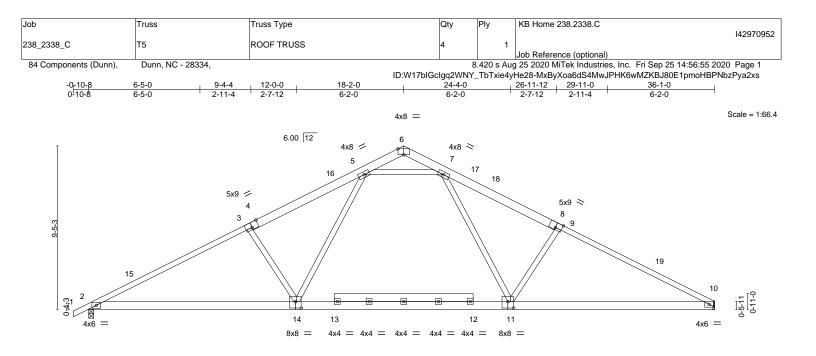
9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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TRENCO A MITek Affiliate 818 Soundside Road

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		10.0.0		00.44.40			
	9-4-4 12-0-0	18-2-0 6-2-0	<u>24-4-0</u> 6-2-0	26-11-12	<u>36-1-0</u> 9-1-4		
Plate Offsets (X,Y) [4:0-4-	8,Edge], [6:0-4-0,Edge], [8:0-4-8,Edge], [2.1.12	0.11		
LOADING (psf)							
TCLL (roof) 20.0	SPACING- 2-0-0	CSI.	DEFL.	in (loc) l/defl	L/d	PLATES	GRIP
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	TC 0.90	Vert(LL)	-0.20 2-14 >999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.79	Vert(CT)	-0.43 2-14 >999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.58	Horz(CT)	0.07 10 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 228 lb	FT = 20%
LUMBER-	Tueset*		RACING-		م مانده مدارد مصحانه ما		
TOP CHORD 2x6 SP No.2 *	'Except" lo.1, 8-10: 2x4 SP DSS			Structural wood sheathin Rigid ceiling directly appl			
BOT CHORD 2x6 SP No.2	0.1, 0-10. 284 35 033	D		rigiu cening unectly appr		racing.	
WEBS 2x4 SP No.3							
WEBS 224 51 110.5							
REACTIONS. (size) 2=	0-3-8, 10=Mechanical						
Max Horz 2=							
	-54(LC 16), 10=-38(LC 17)						
	1497(LC 2), 10=1434(LC 2)						
	/Max. Ten All forces 250 (lb) or less exe						
	6, 3-5=-2295/118, 7-9=-2280/133, 9-10=-	2562/111					
	206, 11-14=0/1631, 10-11=-12/2187						
WEBS 7-11=-46/82	3, 9-11=-523/233, 5-14=-44/844, 3-14=-5	32/226, 5-7=-1482/135					
NOTES-							
	have been considered for this design						
	have been considered for this design. 5mph Vasd=91mph; TCDL=6.0psf; BCDL	6 Opof: h 20ft: Cot II: E	n D. Englogod: M				
	xterior(2) -0-10-8 to 2-8-13, Interior(1) 2-8						
	ers and forces & MWFRS for reactions sh						
) psf (roof live load: Lumber DOL=1.15 Pla						
	15 Plate DOL=1.15); Category II; Exp B;		, (ground briott),				
	ve been considered for this design.					annin	
	ed for greater of min roof live load of 12.0	psf or 1.00 times flat roof	load of 11.6 psf o	n overhangs		IL CAD	11,
non-concurrent with other l			•	0	1	(H UARO	141
6) This truss has been design	ed for a 10.0 psf bottom chord live load n	onconcurrent with any oth	er live loads.		Ja or	ESSION	Alla .
	ned for a live load of 20.0psf on the botto		e a rectangle 3-6-0) tall by 2-0-0 wide			
will fit between the bottom	chord and any other members, with BCDI	. = 10.0psf.	-		SHE S		MAN
9) Defer to girder(a) for trues t	a truca connectiona					0	· · ·

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10.

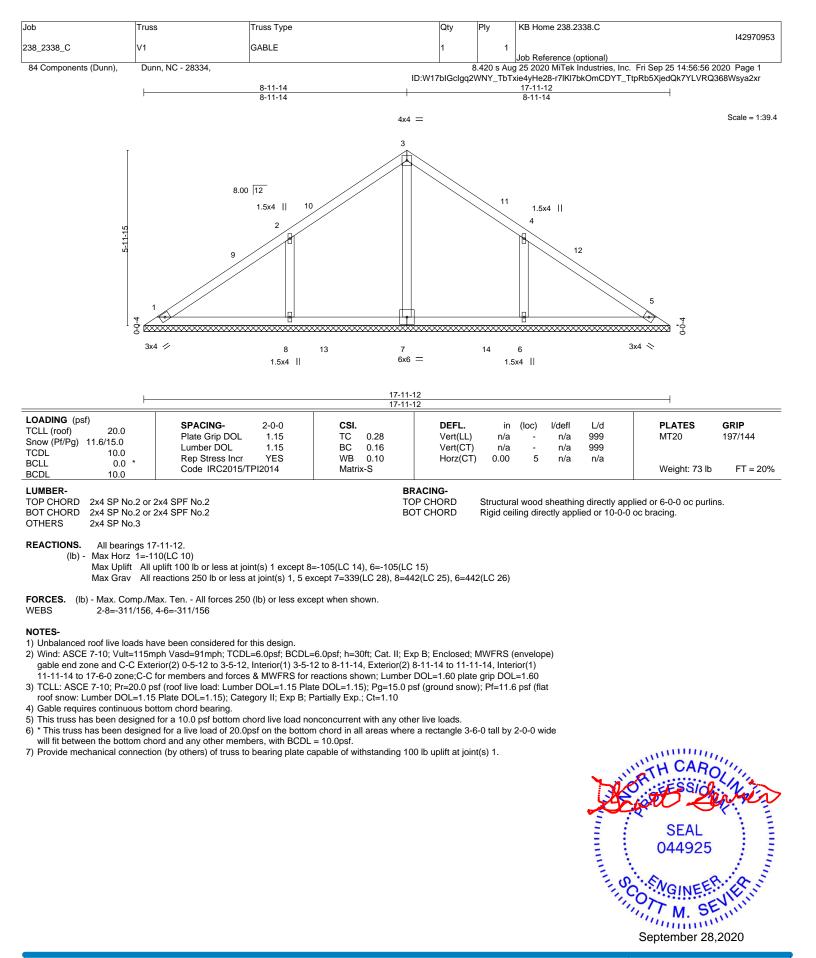
 One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

11) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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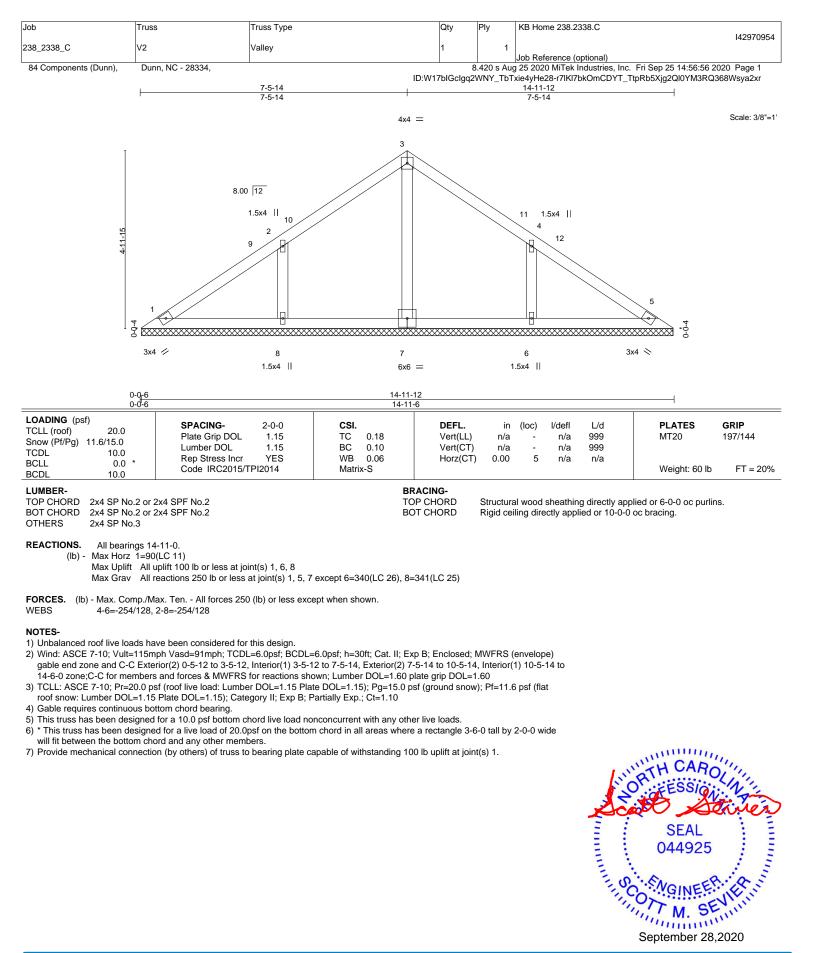
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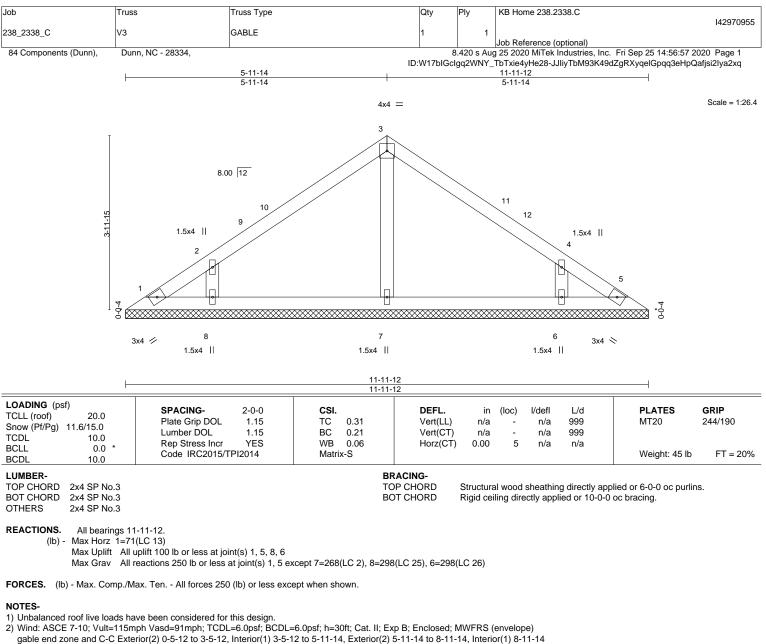


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- to 11-5-15 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

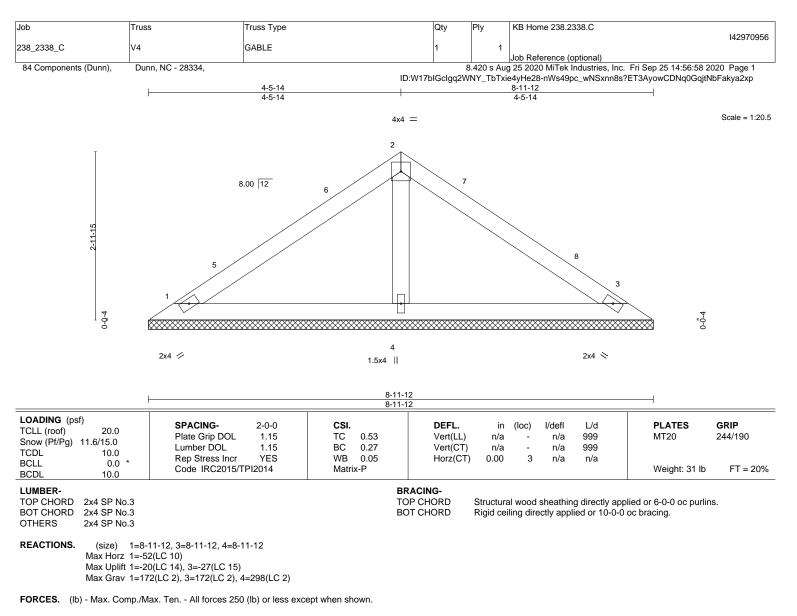
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-5-14, Exterior(2) 4-5-14 to 7-5-14, Interior(1) 7-5-14 to 8-5-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

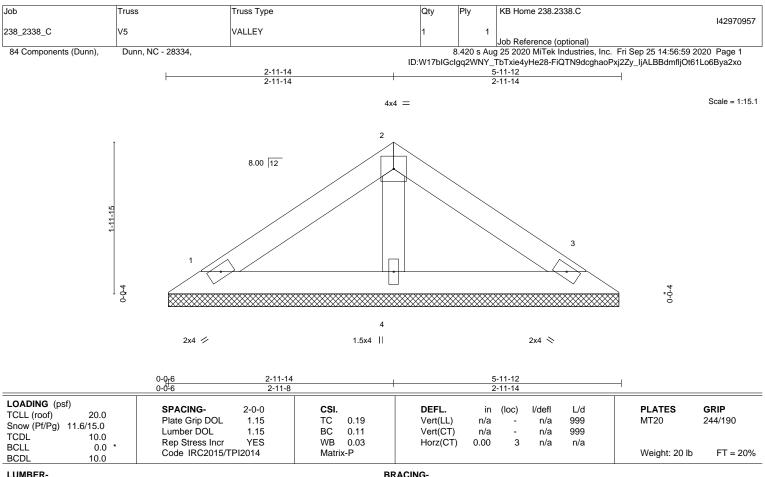
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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LUMBER-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-11-12 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 1=5-11-0, 3=5-11-0, 4=5-11-0 (size) Max Horz 1=32(LC 11) Max Uplift 1=-12(LC 14), 3=-17(LC 15) Max Grav 1=107(LC 2), 3=107(LC 2), 4=186(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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 Satisfies
 Ansi/TPH Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD WEBS 2x4 SP No.3

