



Herring Homes The Hayes - B

McMillan Design
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SIDE LOAD ELEVATIONS

REVI	SIONS
NUMBER	DATE
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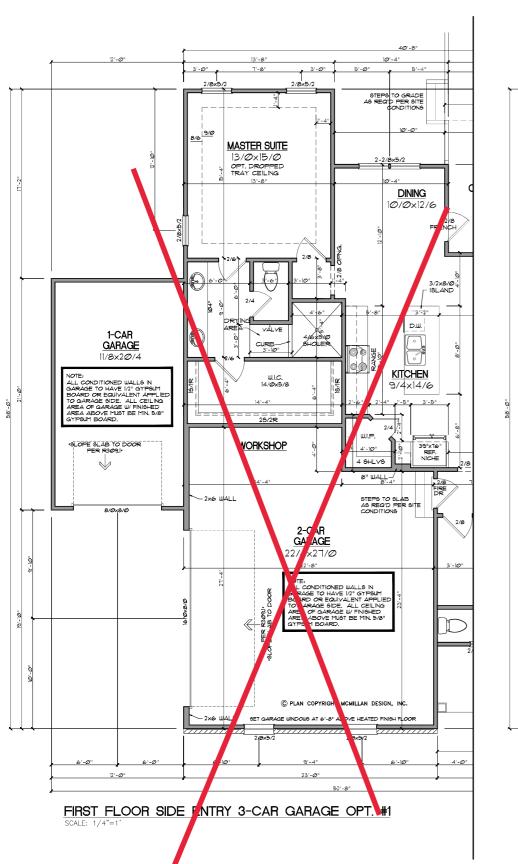
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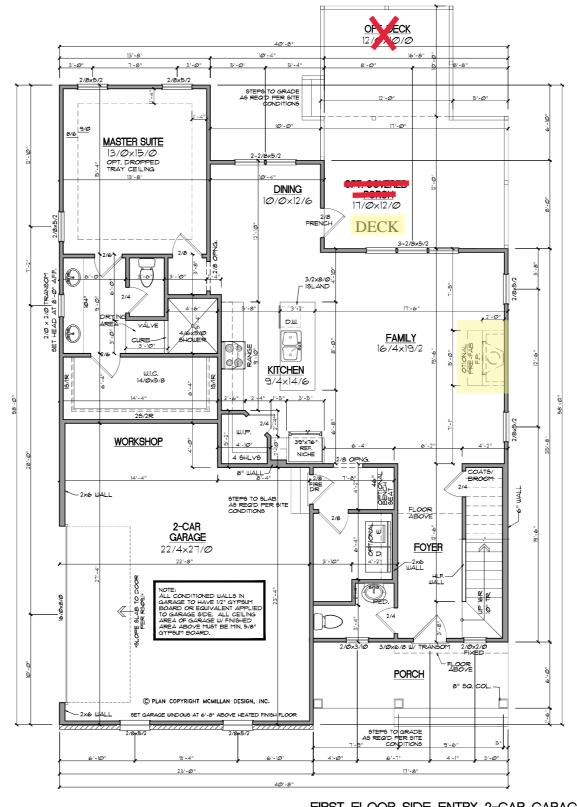
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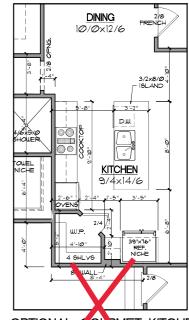
Date: 04/12/22

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FIRST FLOOR SIDE ENTRY 2-CAR GARAGE



SOURMET KITCHEN OPTIONAL

AL NOTES

WALLS:
ALL WALLS ARE DRAWN 4"
THICK UNO.
ANGLED WALL ARE DRAWN
645' UNO. SMOKE DETECTORS:

GENE

LOCATION AND NUMBER OF DETECTORS SHALL CONFORM TO NEC. ECRESS:

EGRESS:

ALL BEDPROM'S MUST HAVE
AT LEST ONE UNDOW UNICH
COMPACTS OR R-30 OF THE
NC BLDG CODE IT IS THE
CONTRACTOR'S RESPONSIBILIT
TO VERFY CHOSEN UNDOWN
MEET EGRESS REQUIRENTS
AS MANUFATURERS VARY.
ATTIC ACCESS:
MIN. ATTIC ACCESS SHALL BE
PROVIDED BY BUILDER AND
LOCATED ON SITE.

WALL/CEILING HGT.

WALL AD EILING HEGT, WALL AND DEILING HEIGHT NOTES ARE BASED ON NOMINAL WALL SIZE IN THE WA

Heated Square Footage	
First Floor	1,319.6
Second Floor	1,256.5
	2,576.2
Heated Square Footage	
First Floor	1,319.6
Opt. 3-Car Second Floor	1,403.2
	2,722.8
Jnheated Square Footage	
-Car Garage Front Load	264.0
-Car Garage Side Load	252.0
2-Car Garage Front/Side Load	588.7
3-Car Garage Side Load Opt. #2	818.7
Covered Porch	201.3
Deck	120.0
ront Porch	106.0

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Herring Homes The Hayes - B SIDE ENTRY **GARAGE PLAN**

REVISIONS NUMBER DATE

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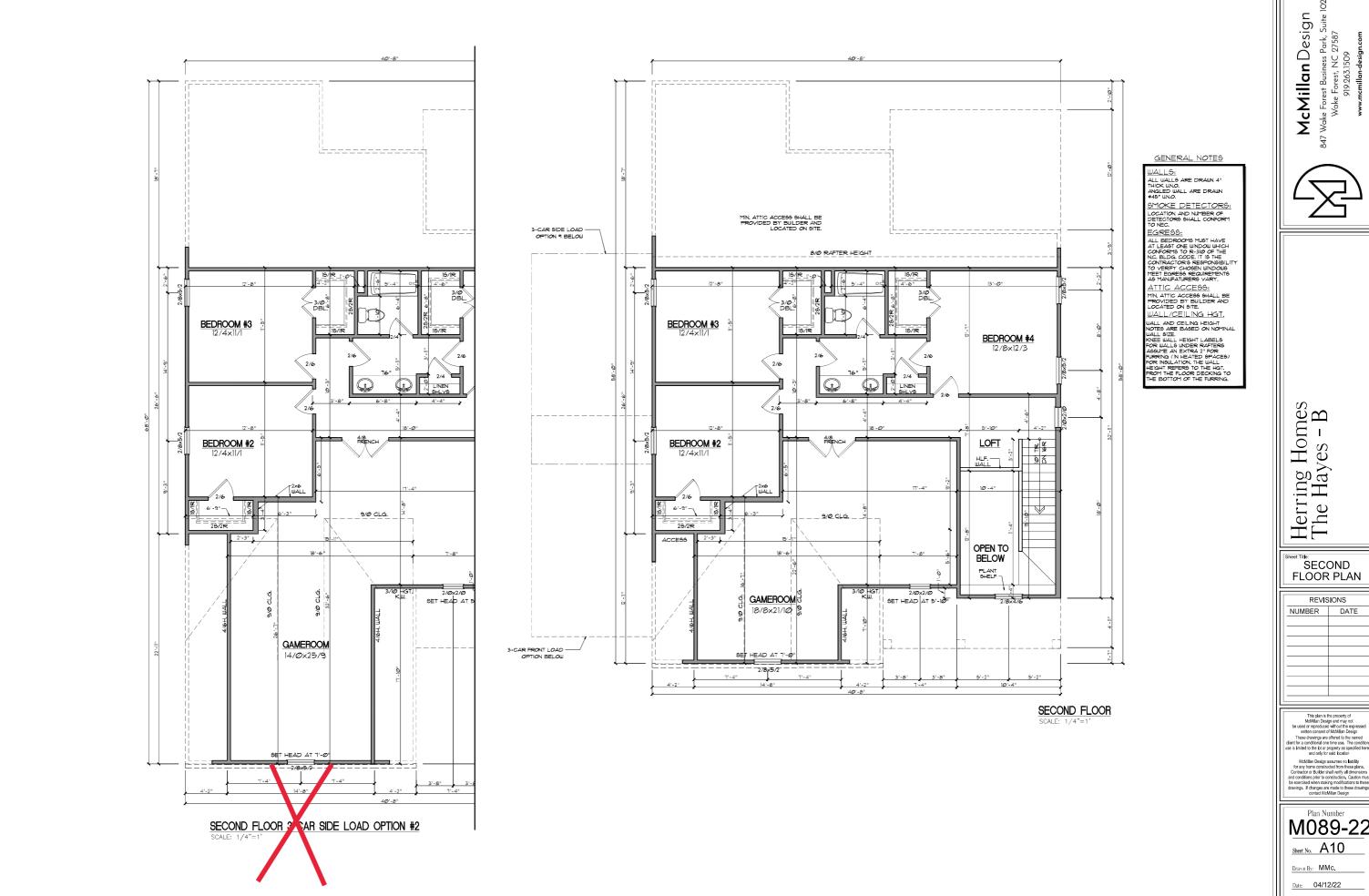
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Herring Homes The Hayes - B

SECOND FLOOR PLAN

REVIS	SIONS
NUMBER	DATE

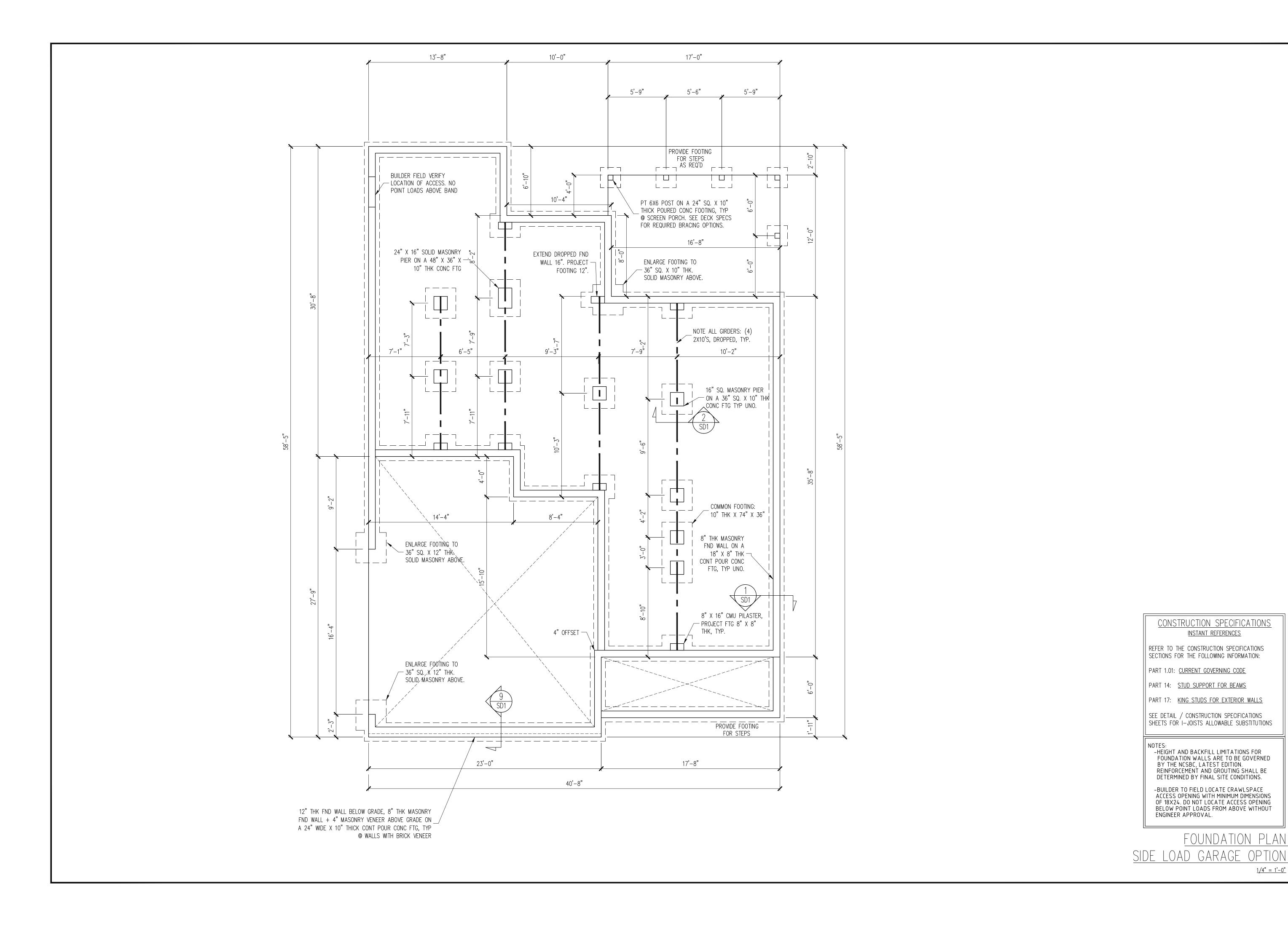
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SCOPE STRUCTURAL ADDENDUM

LOT #: 5 GRIFFON POINTE ENG NBG

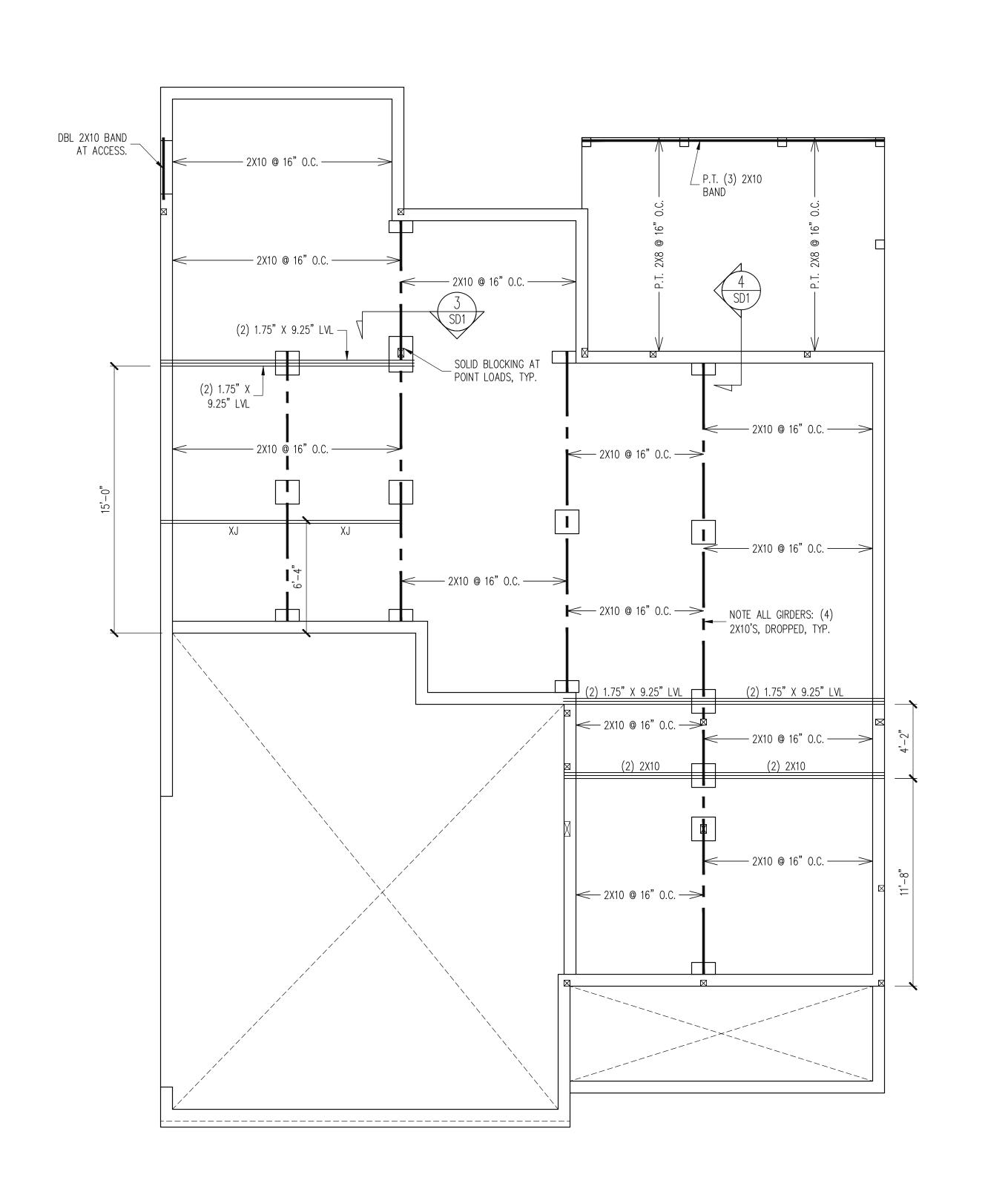
PLAN: THE HAYES B REV: DATE 7/22/202

PLAN NO. M089-22B

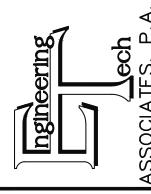
PROJECT NO. 22-21-144

SHEET NO.

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PLAN NO. M089-22B

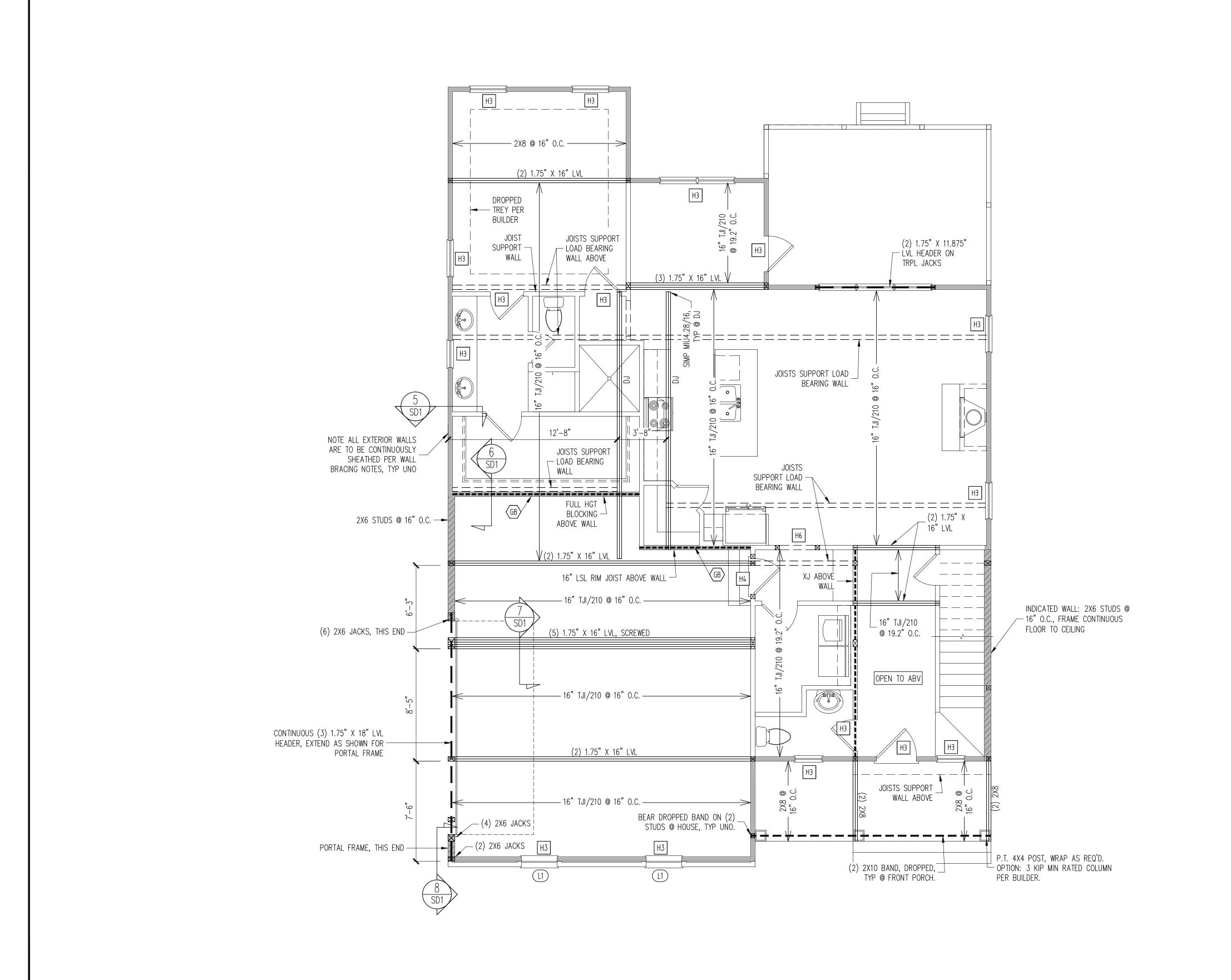
PROJECT NO. 22-21-144

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CRAWL SPACE FRAMING PLAN
SIDE LOAD OPTION

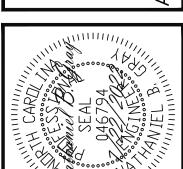
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 $\frac{1/4" = 1'-0"}{1}$



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LINTEL SCHEDULE

1ST FLOOR ONLY

L1 L 3 1/2 X 3 1/2 X 1/4 TYP UNO

L2 L 5 X 3 1/2 X 5/16

L3 L 6 X 4 X 5/16 ATTACHED TO HEADER (2)— 1/2" DIA. X 3" LONG LAG SCREWS AT 16" O.C.

L4 16 GAUGE FLEX LINTEL PER BUILDER

WALL BRACING

SHADED WALLS:

ALL EXTERIOR STUD WALLS, EXTERIOR SIDE, ARE TO BE CONTINUOUSLY SHEATHED WITH 7/16 APA RATED OSB NAILED TO STUDS WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD.

- WSP ONE SIDE OF INTERIOR WALL OR INSIDE OF EXTERIOR WALL WITH 3/8" MIN. THICKNESS WOOD STRUCTURAL PANELING. ATTACH WSP TO STUD WALL WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD.
- GB INTERIOR BRACED WALL. 1/2" GB SECURED
 PER TABLE R602.10.2 OF THE 2018 NCRBC.
 (FASTENERS @ 7" O.C.) BOTH SIDES OF WALL,
 OR (FASTENERS @ 4" O.C.) ONE SIDE OF
 WALL AT STAIRS (BUILDER PERMITTED TO
 SUBSTITUTE "WSP" FOR ANY "GB" WALL)

NOTES:

PROVIDED CONTINUOUS SHEATHING = 200' MIN.

REFERENCE PART 16.02 OF CONSTRUCTION SPECIFICATIONS FOR GENERAL WIND BRACING INFORMATION.

HEADER SCHEDULE

H1 SINGLE 2X4 TURNED FLAT (A)

H2 (2) 2X4'S ON SINGLE JACKS (B)

H3 (2) 2X10'S ON SINGLE JACKS (C)

H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS

H5 (3) 2X10'S ON SINGLE JACKS

H6 (2) 2X10'S ON DBL JACKS

(A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.

(B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.

(C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.

NOTES:

-HEADERS IN NON LOAD BEARING INTERIOR

WALLS ARE NOT LABELED.

1ST FLOOR FRAMING PLAN SIDE LOAD GARAGE OPTION

WALLS AND CEILING $\frac{1/4"}{1} = 1'-0"$

S3
3 of 7

PLAN NO.

M089-22B

PROJECT NO.

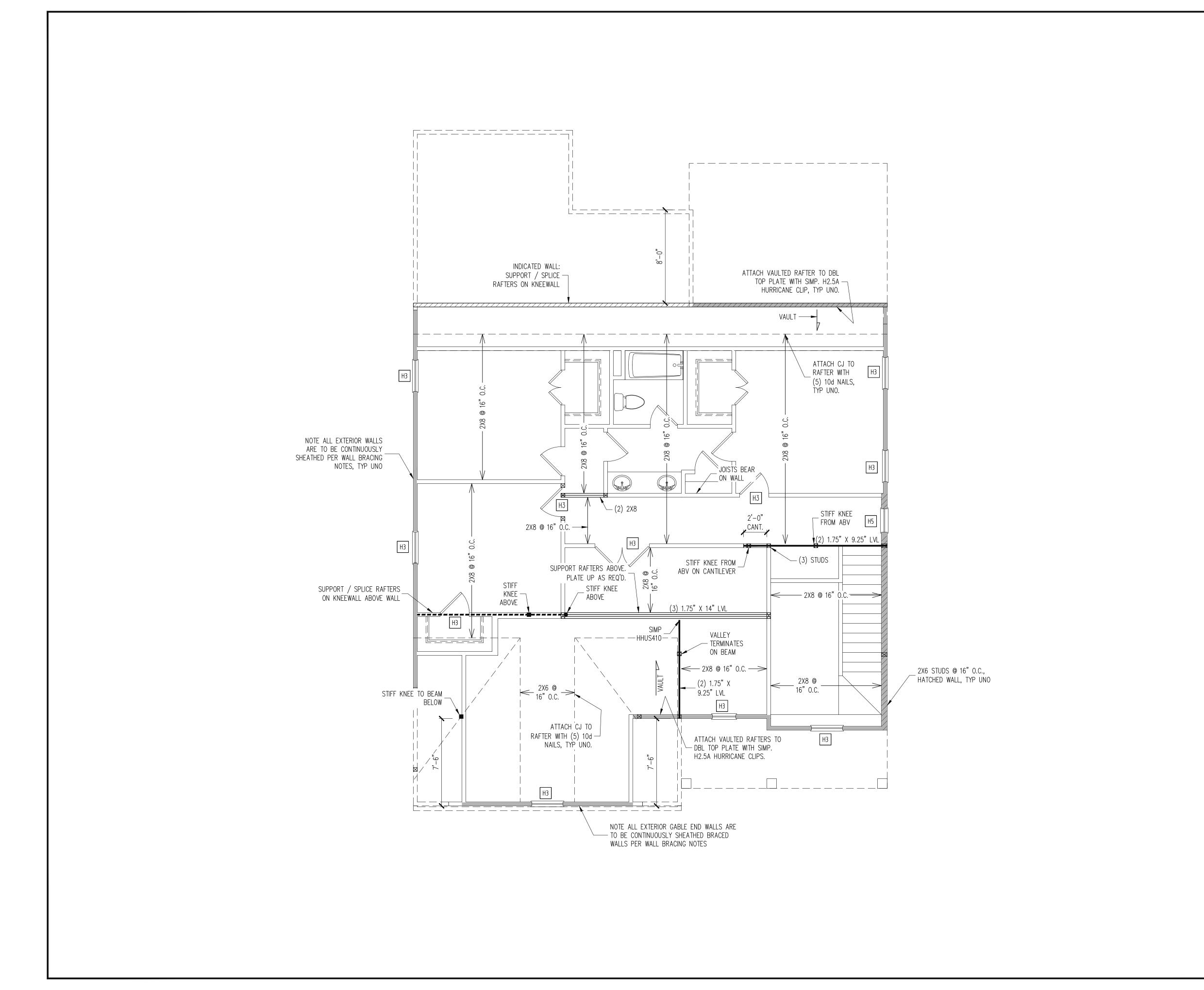
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CLIENT:	HERRING HOMES		
SCOPE	STRUCTURAL ADDENDUM	MO	
LOT #:	5 GRIFFON POINTE	ENG	NBC
PLAN:	THE HAYES B	REV:	
		DATE 7/22/2	7/22/2



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WALL BRACING

SHADED WALLS:

ALL EXTERIOR STUD WALLS, EXTERIOR SIDE, ARE TO BE CONTINUOUSLY SHEATHED WITH 7/16 APA RATED OSB NAILED TO STUDS WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD.

NOTES:

PROVIDED CONTINUOUS SHEATHING = 114' MIN.

REFERENCE PART 16.02 OF CONSTRUCTION SPECIFICATIONS FOR GENERAL WIND BRACING INFORMATION.

HEADER SCHEDULE

- H1 SINGLE 2X4 TURNED FLAT (A)
- H2 (2) 2X4'S ON SINGLE JACKS (B)
- H3 (2) 2X10'S ON SINGLE JACKS (C)
- H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS
- H5 (3) 2X10'S ON SINGLE JACKS
- (A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.
- (B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.
- (C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.

OTES:

-HEADERS IN NON LOAD BEARING INTERIOR
WALLS ARE NOT LABELED.

2ND FLOOR FRAMING PLAN

WALLS AND CEILING $\frac{1/4"}{1} = 1'-0"$

PROJECT NO. 22-21-144

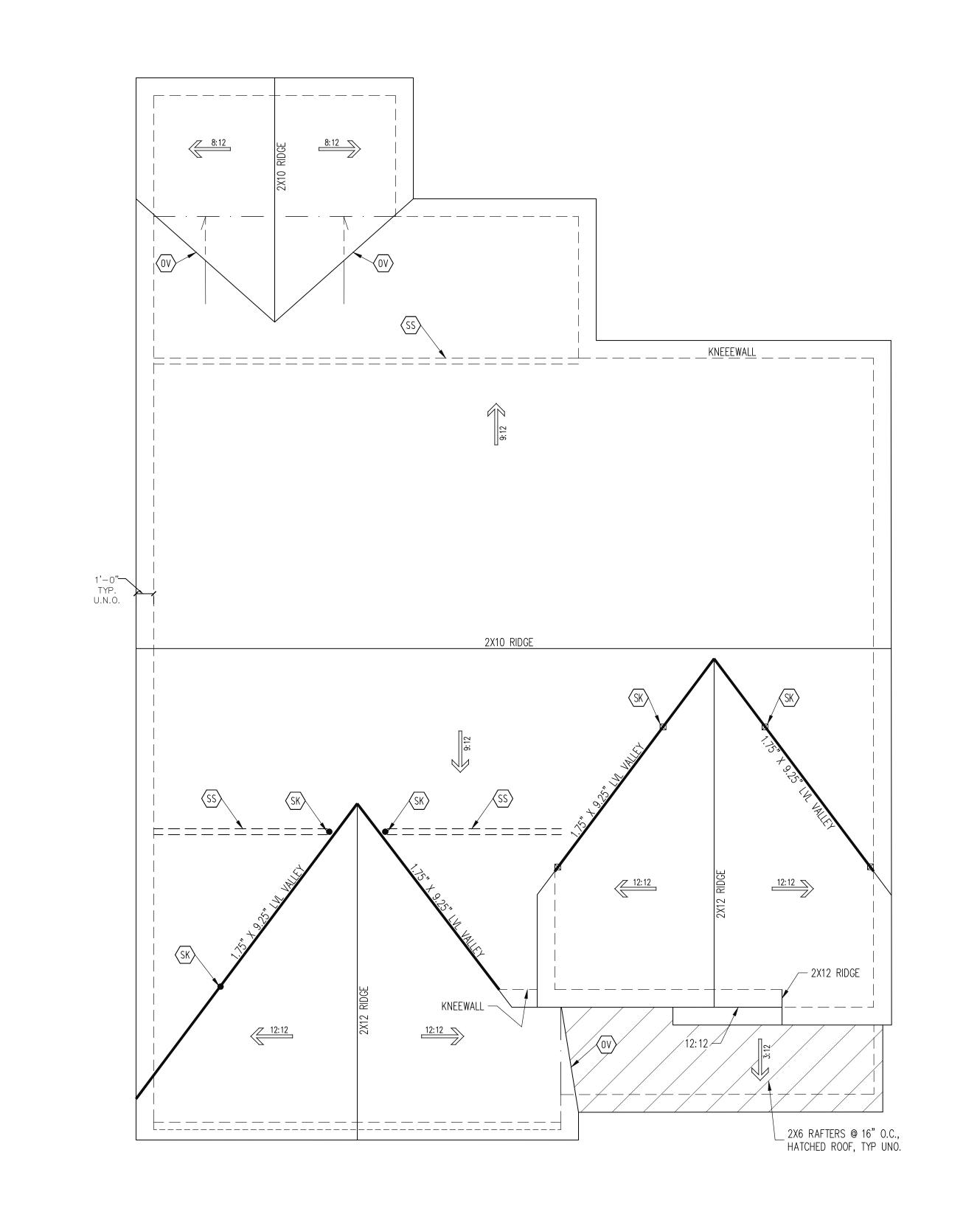
CLIENT: SCOPE: LOT #:

PLAN NO.

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SCOPE	STRUCTURAL ADDENDUM	MNC	
LOT #:	5 GRIFFON POINTE	ENG	NBG
PLAN:	THE HAYES B	REV:	
		DATE	DATE: 7/22/202

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FRAMING NOTES

ROOF ONLY -COMMON RAFTERS 2X8 @ 16" O.C. TYP U.N.O. -COLLAR TIES 2X4 EVERY 3RD SET OF RAFTERS TYP U.N.O. -ROOF PITCHES 12:12 TYP U.N.O.

-VERIFY ROOF PITCHES, OVERHANG LENGTHS, AND KNEEWALL FRAMING HGTS WITH ARCHITECTURAL DRAWINGS, TYPICAL.

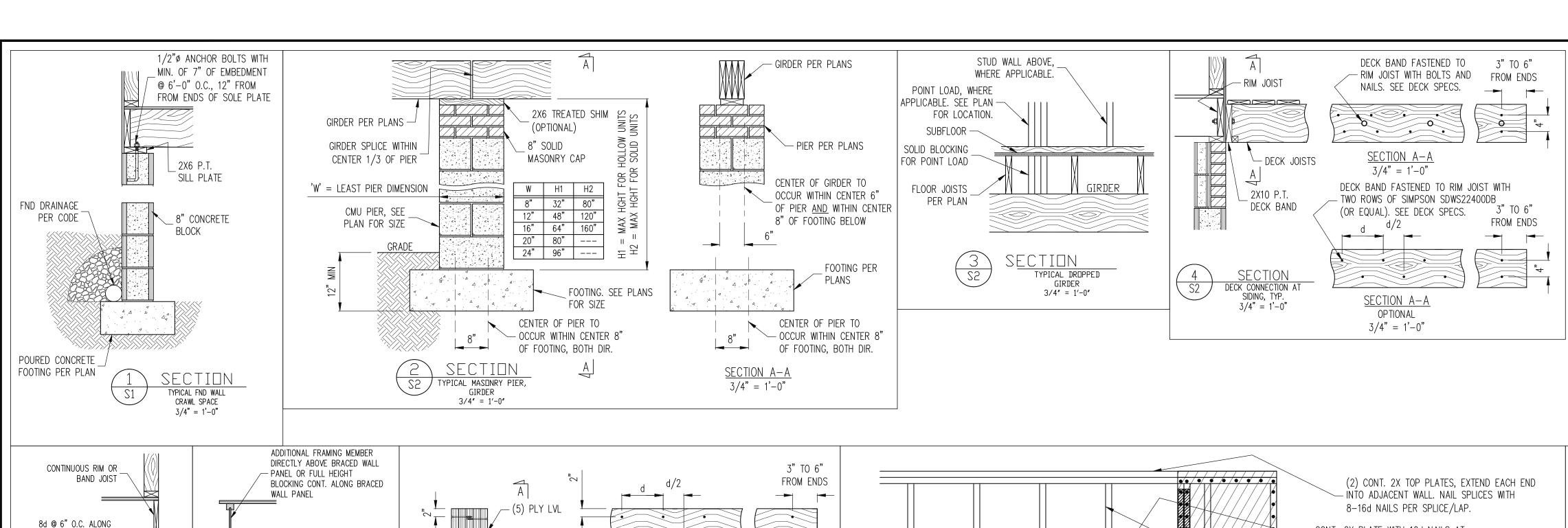
FRAMING SCHEDULE ROOF ONLY

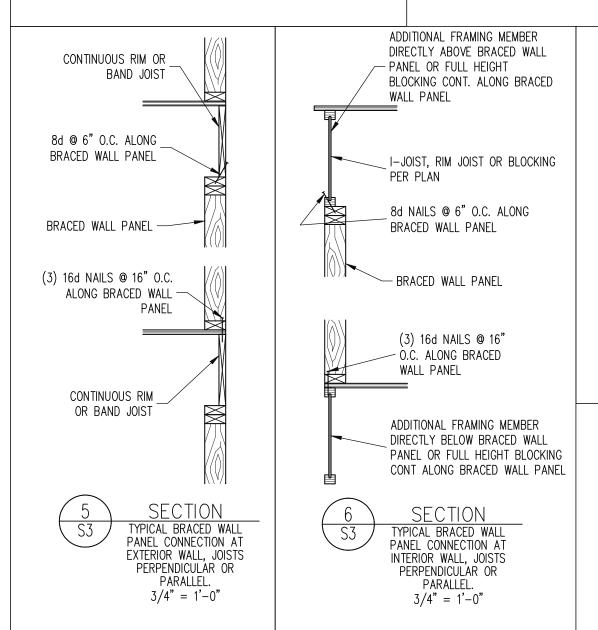
OV OVERFRAME VALLEY (2X10 SLEEPER)

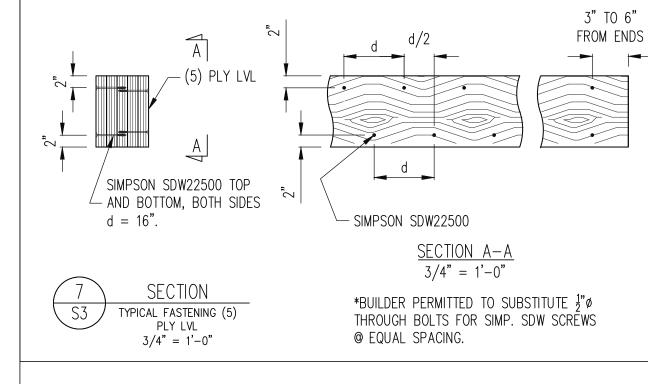
SK DBL 2X4 STIFF KNEE

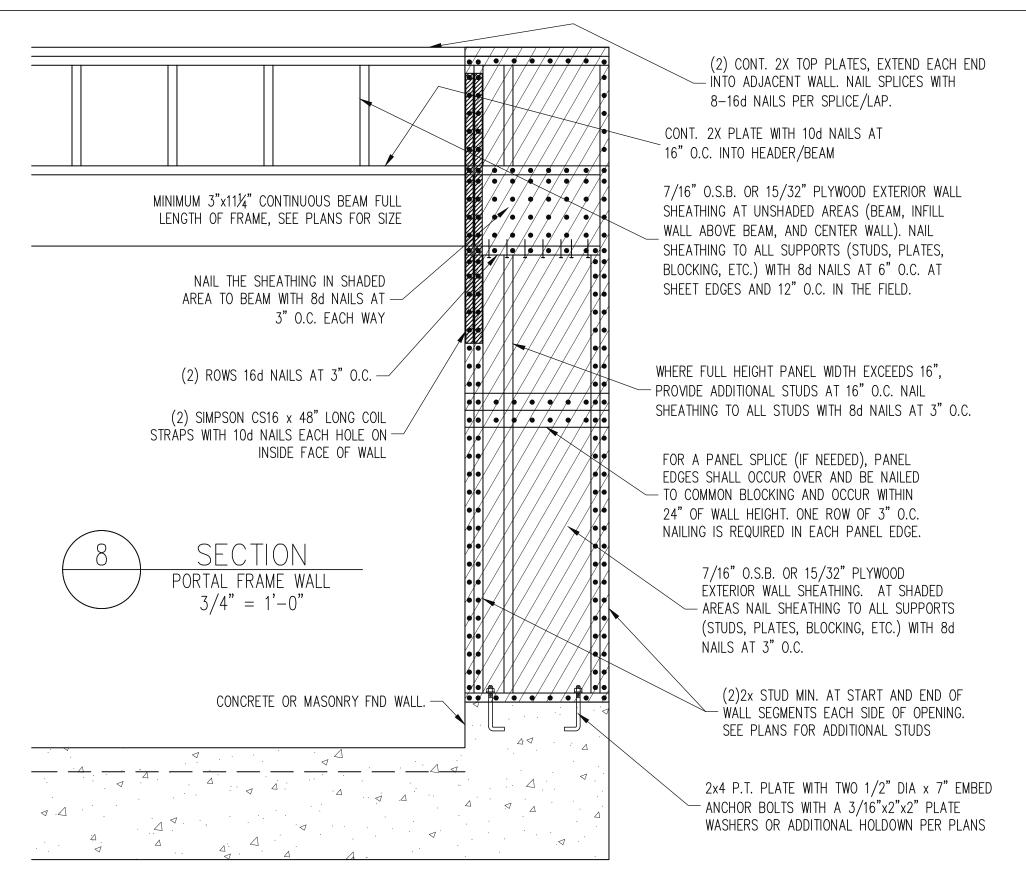
SS SUPPORT/SPLICE RAFTERS ON KNEEWALL BELOW

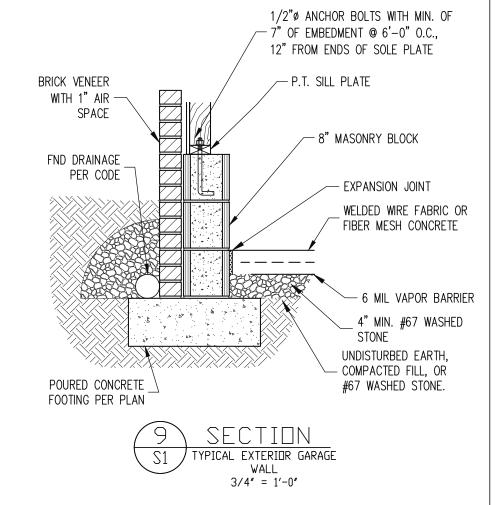
ROOF FRAMING PLAN







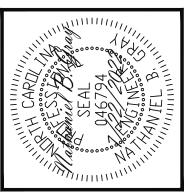




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CLIENT: HERRING HOMES	M ENG NBG REV: DATE 7/22/2022	DATE	5 GRIFFON POINTE THE HAYES B	SCOPE LOT #: PLAN:
HERRING HOMES STRUCTURAL ADDENDUM 5 GRIFFON POINTE ENG		REV:	THE HAYES B	PLAN:
	NBG	ENG	5 GRIFFON POINTE	# 1O1
		MNC	STRUCTURAL ADDENI	SCOPE
			HERRING HOMES	CLIENT:

PLAN NO. M089-22B

PROJECT NO. 22-21-144

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SHEET NO.

	CONSTRUCTION	S
	PART 1: GENERAL	
1.01	CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.	11
1.02	DIMENSIONS SHOWN SHALL GOVERN OVER SCALE ON THESE DRAWINGS.	12
1.05	METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR, WHO SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.	''
	PART 2: DESIGN LOADS	
2.01	DESIGN LOADS SHALL CONFORM WITH THE TABLE BELOW:	1.
	USE LIVE LOAD (PSF) DEAD LOAD (PSF)	
	BALCONIES, DECKS, ATTICS WITH FIXED STAIR ACCESS, DWELLING UNITS INCLUDING ATTICS WITH FIXED STAIR ACCESS, STAIRS, FIRE ESCAPES 40 10	
	GARAGES (PASSENGER CARS ONLY) 50	14
	ATTICS (NO STORAGE, LESS THAN 5' HEADROOM) 10 10 10 ATTICS (WITH STORAGE) 20 10	
	ROOF 20 10 (15 FOR VAULTS)	
NOTES 2.02	S: - INDIVIDUAL STAIR TREADS ARE TO BE DESIGNED FOR THE UNIFORMLY DISTRIBUTED LIVE LOAD OF 40 PSF OR A 300 LB. CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQ. WHICHEVER PRODUCES THE GREATER STRESS. - BUILDER TO VERIFY DEAD LOAD DOES NOT EXCEED 10 PSF WHEN HEAVY FLOOR OR ROOF FINISHES SUCH AS TILE OR SLATE ARE UTILIZED. NOTIFY ENGINEERING UNDER THESE CONDITIONS INTERIOR WALLS: 5 PSF LATERAL.	
2.03	BASIC WIND DESIGN VELOCITY OF 120 MPH.	1.
2.04	SOIL BEARING CAPACITY 2000 PSF (PRESUMPTIVE).	
	PART 3: STRUCTURAL STEEL	
3.01	WIDE FLANGE BEAMS AND TEE SECTIONS SHALL CONFORM TO ASTM A992 MINIMUM GRADE	
3.02	SQUARE AND RECTANGULAR TUBING SHALL CONFORM TO ASTM A500 GRADE B MINIMUM GRADE.	1,
3.03	STEEL PIPE SHALL CONFORM TO ASTM A53 GRADE B, TYPE S, MINIMUM GRADE	
3.04	ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 MINIMUM GRADE	14
3.05	STRUCTURAL STEEL CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.	
4.01	PART 4: WELDING WELDING ELECTRODES SHALL BE E70XX AND ALL WELDING SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER	
	PART 5: CONCRETE AND SLABS ON GRADE	15
5.01	CAST IN PLACE CONCRETE SHALL BE OF NORMAL WEIGHT, 6% AIR ENTRAINMENT, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS TYP UNO. ALL CONCRETE, INCLUDING CONCRETE FOR FOOTINGS, IS TO BE CAST IN PLACE, TYP UNO.	1,1
5.02	REINFORCED CAST IN PLACE CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED IN ACCORDANCE WITH THE SPECIFICATIONS OF ACI 318, LATEST EDITION.	15
5.03	SLABS ON GRADE, IF ANY, SHALL CONTAIN SYNTHETIC POLYPROPYLENE FIBRILLATED MICRO FIBERS, FIBER LENGTH 1 1/2", DOSAGE RATE 1 1/2 LBS/CU YD. SLAB TO BE PLACED ON A 6 MIL VAPOR BARRIER ON 2" MIN GRANULAR FILL ON SOIL WITH 90% MIN STANDARD PROCTOR DENSITY. VAPOR BARRIER MAY BE OMITTED FOR SLABS NOT IN ENCLOSED AREAS	16
	PART 6: REBAR AND WIRE REINFORCEMENT	
6.01	REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60 TYP UNO	
6.02	LAP SPLICES SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UNO	
6.03	WIRE REINFORCEMENT SHALL BE 9 GA AND SHALL CONFORM TO ASTM A1064. PART 7: MASONRY	4
7.01	CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT, f'M = 1,500 PSI MIN	16
7.02	CLAY MASONRY UNITS SHALL CONFORM TO ASTM C62-17 GRADE SW	
7.03	MORTAR SHALL BE TYPE S. MORTAR AND GROUT SHALL CONFORM TO ASTM C476, MIN COMPRESSIVE STRENGTH OF 2000 PSI.	
7.04	MASONRY CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS OF ACI 530	
7.05	LADDER WIRE REINFORCEMENT SHALL CONFORM TO ASTM A951. 6" MIN LAPS FOR CONTINUOUS WALL APPLICATIONS	
	PART 8: BOLTS AND LAG SCREWS	
8.01	BOLTS SHALL CONFORM TO ASTM A307 MINIMUM GRADE TYP UNO. INSTALL STANDARD STEEL WASHERS (ASTM F844-07a) FOR THE NUT $/$ BOLT HEAD WHEN BOLTING WOOD MEMBERS	17
8.02	LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1—1981. PILOT HOLES SHALL BE USED FOR LAG SCREW INSTALLATION AND SHALL BE BORED ACCORDING TO NDS SPECIFICATIONS. INSTALL STANDARD STEEL WASHERS (ASTM F844—07a) FOR SCREW HEAD	
8.03	ANCHOR RODS AND BOLTS SHALL CONFORM TO ASTM F1554-15 GRADE 36 UNO. BENT ANCHOR BOLTS SHALL HAVE A 2" MIN HOOK UNO	1
	DADT Q. DDIVEN EASTENEDS	1 10

PART 9: DRIVEN FASTENERS

PART 10: DIMENSIONAL LUMBER

FOR JOISTS, RAFTERS, GIRDERS, BEAMS, STUDS, ETC.

11.01 LVL OR PSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS: E= 1.9 X 10E6 PSI, Fb = 2600 PSI, Fv = 285 PSI, Fc = 750 PSI LSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS:

E= 1.3 X 10E6 PSI, Fb = 1700 PSI, Fv = 400 PSI, Fc = 680 PSI

COMMÓN WIRE OR BOX

PART 11: ENGINEERED LUMBER

9.01 NAILS, SPIKES AND STAPLES SHALL CONFORM TO ASTM F 1667- 05. NAILS ARE TO BE

10.01 SOLID SAWN WOOD FRAMING DESIGN IS BASED ON NO. 2 SPRUCE PINE FIR OR SYP #2

11.02 LVL OR PSL MEMBERS MAY BE RIPPED FROM DEEPER MEMBERS TO MATCH THE MEMBER | SHALL IMMEDIATELY CONTACT THE ENGINEER OF RECORD (EOR) BEFORE PROCEEDING IF THE IREMENTS OF THE NORTH CAROLINA RESIDENTIAL DEPTH SPECIFIED IN THE PLANS PART 12: PRESSURE TREATED LUMBER

LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL OTHER EXPOSED LUMBER SHALL BE TREATED IN ACCORDANCE WITH AWPA STANDARD C-2 OR BY ANY METHOD ES OF CONSTRUCTION ARE THE RESPONSIBILITY OF NECESSARY PRECAUTIONS TO MAINTAIN AND JRE AT ALL STAGES OF CONSTRUCTION. DECAY RESISTANT WOOD PER SECTION 19-6(A)

PART 13: STEEL FLITCH PLATE BEAMS 13.01 FLITCH PLATE BEAMS SHALL CONSIST OF A CONTINUOUS STEEL PLATE BOLTED BETWEEN CALCULATIONS THAT ARE NOT DIRECTLY RELATED TO STRUCTURAL ENGINEERING. LIVE LOAD (PSF) DEAD LOAD (PSF) TWO PIECES OF CONTINUOUS LUMBER AS SIZED ON THE PLANS. BOLT PIECES TOGETHER MAINTAIN A 2" EDGE DISTANCE. PLACE TWO BOLTS, ONE ABOVE THE OTHER, 6" ± 2" | TRUSS DRAWING SHOULD BE SUBMITTED TO THE EOR FOR REVIEW

PART 14: STUD SUPPORTS FOR BEAMS

COLUMN TYP UNO.

FROM EACH END OF THE BEAM.

CONSTRUCTION SPECIFICATIONS

14.01 STEEL, ENGINEERED LUMBER, AND FLITCH PLATE BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:

-WHEN THE BEAM IS PERPENDICULAR TO. OR SKEWED RELATIVE TO THE WALL. THE BEAM SHALL BEAR <u>FULL WIDTH</u> ON THE SUPPORTING WALL INDICATED AND SHALL BE SUPPORTED BY A MINIMUM OF THREE GANGED STUDS, OR A GANGED STUD COLUMN WITH A NUMBER OF STUDS SUCH THAT THE STUD COLUMN IS AT LEAST AS WIDE AS THE TRUE WIDTH OF THE BEAM BEING SUPPORTED, WHICHEVER IS GREATER, TYP UNO. FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON | -BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 4 1/2" ONTO THE WALL AND BE SUPPORTED BY A TRPL STUD GANGED

14.02 DIMENSIONAL LUMBER BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:

1-WHEN THE BEAM IS PERPENDICULAR TO, OR SKEWED RELATIVE TO THE WALL, THE BEAM SHALL BEAR FULL WIDTH ON THE SUPPORTING WALL INDICATED (LESS 1 1/2" TO ALLOW FOR A CONTINUOUS RIM JOIST WHERE APPLICABLE) AND SHALL BE SUPPORTED BY A GANGED STUD COLUMN THE SAME WIDTH AS THE BEAM TYP UNO. (E.G. A TRIPLE 2X10 IS TO BE SUPPORTED BY (3) STUDS). FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM 2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 3" ONTO THE WALL AND BE SUPPORTED BY A DBL STUD GANGED COLUMN

14.03 EXTRA JOISTS BEARING ON A STUD WALL PERPENDICULAR TO OR SKEWED RELATIVE TO THE BEAM SHALL BE SUPPORTED BY ONE ADDITIONAL STUD.

14.04 STUDS THAT ARE GANGED TO FORM A COLUMN SHALL HAVE ADJACENT STUDS WITHIN THE COLUMN NAILED TOGETHER WITH ONE ROW OF 10d NAILS AT 8" O.C. (TWO ROWS OF 10d NAILS @ 8" O.C., 3" APART, FOR 2X8 OR 2X10 STUDS) ALL COLUMNS SHALL BE CONTINUOUS DOWN TO THE FOUNDATION OR OTHER PROPERLY DESIGNED STRUCTURAL ELEMENT SUCH AS A BEAM. COLUMNS TRANSFERRING LOADS THROUGH FLOOR LEVELS SHALL BE SOLIDLY BLOCKED FOR THE FULL WIDTH OF THE STUD COLUMN WITHIN THE CAVITY FORMED BY THE

PART 15: NAILING OF MULTI PLY WOOD BEAMS

15.01 SOLID SAWN LUMBER JOISTS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM NAILED TOGETHER WITH THREE ROWS OF 10d NAILS @ 16" O.C. FOR 2X10 OR LARGER, TWO ROWS OF 10d NAILS @ 16" O.C. FOR 2X8, ONE ROW OF 10d NAILS @ 16" O.C. FOR 2X6 OR SMALLER. STAGGER ROWS 5" MIN.

15.02 LVL MEMBERS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM FASTENED TOGETHER PER MANUFACTURERS RECOMMENDATIONS, TYP

PART 16: WALL FRAMING AND BRACING

STUD WALLS SHALL CONSIST OF 2X4 STUDS SPACED AT 16" O.C. UNO. STUDS SHALL E CONTINUOUS FROM SOLE PLATE AT FLOOR TO DOUBLE TOP PLATE AT THE CEILING R ROOF. NO INTERMEDIATE BANDS OR PLATES SHALL CAUSE DISCONTINUITIES IN A STUD WALL EXCEPT AS REQUIRED FOR DOOR OR WINDOW OPENINGS. THE KING STUDS FOR SLICH OPENINGS SHALL BE CONTINUOUS TYP UNO MAX ALLOWABLE WALL HEIGHTS FOR EXTÉRIOR STUD WALLS. INCLUSIVE OF SOLE PLATE AND DBL TOP PLATE AND 7/16" OSB EXTERIOR BRACING AND ROW OF 2X4 2X6 PURLINS AT 8' HEIGHT (AND AT 16' HEIGHT FOR TALL WALLS), TYP UNO: 2X4 @ 16" O.C.: 11'-1 1/2" 2X6 @ 16" O.C.: 17'-0" 2X4 @ 12" O.C.: 12'-1 1/2" 2X6 @ 12" O.C.: 18'-8" DBL 2X4 @ 16" O.C.: 13'-4" DBL 2X6 @ 16" O.C.: 21'-0"

16.02 FOR WALL BRACING THE FOLLOWING SHALL APPLY: -BLOCKING AT UNSUPPORTED PANEL EDGES IS REQUIRED TYP UNO.

-WALL BRACING IS BY ENGINEERED DESIGN AND NOT PRESCRIPTIVE PER SECTION 602.10 OF THE 2018 NCRC. CONTINUOUS SHEATHING HAS BEEN PROVIDED, ALONG WITH ALTERNATIVE METHODS TO INSURE THE MINIMUM INTENT OF SECTION 602.10 OF THE 2018 NCRC HAS BEEN MET AND EXCEEDED. -BRACED WALL PANELS SHALL BE FASTENED IN ACCORDANCE WITH TABLE 602.3(1) TO PROVIDE CONTINUOUS PANEL UPLIFT RESISTANCE AND COMPLIANCE WITH NCRBC ' R602.3.5 AND R802.11 UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS. -MAY SUBSTITUTE WSP FOR GB -SINGLE JOIST, CONTINUOUS RIM JOIST, OR BLOCKING OF EQUAL DEPTH IS REQUIRED ABOVE AND BELOW ALL BRACED WALLS. NAIL BLOCKING ABOVE WALL TO TOP PLATE WITH 16d TOE NAILS @ 6" O.C. NAIL SOLE PLATE OF BRACED WALL TO BLOCKING BELOW WITH (3) 16d NAILS @ 16" O.C. BLOCKING AT HORIZONTAL JOINTS IN BRACED WALL LINES ONLY REQUIRED AT SHADED WALLS, UNO.

PART 17: KING STUDS

17.01 KING STUDS FOR OPENINGS IN EXTERIOR WALLS SHALL BE AS FOLLOWS: NUMBER OF KING STUDS

MAX OPENING WIDTH 5'-0" 9'-0" 13'-0" 17'-0" 21'-0"

PART 18: SUBSTITUTIONS

18.01 MATERIAL OR MEMBER SIZE SUBSTITUTIONS OR PLAN DEVIATIONS REQUIRE THE WRITTEN AUTHORIZATION OF THE DESIGNERS. UNAUTHORIZED DEVIATIONS ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

PART 19: OWNERSHIP OF STRUCTURAL DESIGN

19.01 THE STRUCTURAL DESIGN OF THIS PLAN IS THE PROPERTY OF ENGINEERING TECH ASSOCIATES (ETA). THESE PLANS ARE FOR THE ONE TIME USE AT THE LOCATION INDICATED AND FOR THE CLIENT LISTED. ETA ASSUMES NO LIABILITY FOR THESE PLANS IF THEY ARE REPRODUCED, IN WHOLE OR IN PART, FOR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT WRITTEN PERMISSION FROM ETÁ

ABBREVIATIONS

ABV ABOVE

B. BOTH

BTWN BETWEEN

CONC CONCRETE

DIA DIAMETER

DBL DOUBLE

EQ EQUAL

B.E. BOTH ENDS

CIP CAST IN PLACE

DJ DOUBLE JOIST

DSP DBL STUD POCKET

CS CONTINUOUS SHEATHING

FOLLOWING CONDITIONS ARE NOTED BEFORE OR DURING CONSTRUCTION:) THE WORKING PLANS DO NOT BEAR THE SEAL OF THE EOR 2) THE PLANS CONTAIN DISCREPANT OR INCOMPLETE INFORMATION

ANY ERRORS DUE TO A FAILURE TO FOLLOW THE ABOVE PROCEDURES SHALL NOT BE THE RESPONSIBILITY OF THE EOR. FURTHERMORE, IT IS THE RESPONSIBILITY OF THE BUILDER TO GIVING EQUAL PROTECTION. THE BUILDING CODE OFFICE MAY ALSO APPROVE A NATURAL ENSURE THAN ANY REVISIONS ISSUED BY THE EOR ARE PROMPLY DISTRIBUTED TO THE

THE BUILDER IS RESPONSIBLE FOR REVIEWING PLANS PRIOR TO CONSTRUCTION. THE BUILDER

THE EOR DOES NOT PERFORM FENESTRATION OR VENTING CALCULATIONS OR ANY OTHER

EA EACH FLG FLANGE USING 1/2" Ø BOLTS SPACED AT 24" O.C. STAGGERED TOP TO BOTTOM OF THE BEAM. | ROOF AND FLOOR TRUSSES TO BE DESIGNED BY AN ENGINEER REGISTERED BY THE STATE. FINAL | FL PL FLITCH PLATE FLR FLOOR

DECK SPECIFICATIONS

A DECK IS AN EXPOSED EXTERIOR WOOD FLOOR STRUCTURE WHICH MAY BE ATTACHED TO JOIST SPAN DECKING A STRUCTURE OR BE FREE STANDING. ROOFED PORCHES, OPEN OR SCREENED IN, MAY BE CONSTRUCTED USING THESE PROVISIONS. 12" O.C. 1" S4S 1" T&G SUPPORT POSTS SHALL BE SUPPORTED BY A FOOTING. 24" O.C. 1 1/4" S4S

WHEN ATTACHED TO A STRUCTURE, THE STRUCTURE TO WHICH ATTACHED SHALL HAVE A TREATED WOOD BAND FOR THE LENGTH OF THE DECK. OR CORROSION RESISTANT FLASHING 9. SHALL BE USED TO PREVENT MOISTURE FROM COMING IN CONTACT WITH THE UNTREATED FRAMING OF THE STRUCTURE. THE DECK BAND AND THE STRUCTURE BAND SHALL BE CONSTRUCTED IN CONTACT WITH EACH OTHER EXCEPT AT BRICK VENEER AND WHERE PLYWOOD SHEATHING IS REQUIRED AND PROPERLY FLASHED. SIDING SHALL NOT BE INSTALLED BETWEEN THE STRUCTURE AND THE DECK BAND. IF ATTACHED TO A BRICK STRUCTURE, NEITHER FLASHING NOR A TREATED BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND SHALL BE CONSTRUCTED IN CONTACT

WHEN THE DECK IS SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE STRUCTURE, THE FOLLOWING ATTACHMENT SCHEDULES SHALL APPLY FOR ATTACHING THE DECK BAND TO THE STRUCTURE:

A. ALL STRUCTURES EXCEPT BRICK STRUCTURES

n. ALL O	The street executive and street a				
	JOIST I	LENGTH			
	UP TO 8' MAX.	UP TO 16' MAX.			
REQUIRED FASTENERS	(2) ROWS OF 12d NAILS @ 8" O.C. OR	ONE- 5/8" Ø BOLT @ 20" O.C. AND (3) ROWS OF 12d NAILS @ 6" O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 16" O.C. STAGGERED			
A BRICK VE	NEER STRUCTURES				

l		@ d = 32" O.C. STAGGERED	@ d = 16" O.C. STAGGERED
	A . BRICK VE	NEER STRUCTURES	
		JOIST I	ENGTH
		UP TO 8' MAX.	UP TO 16' MAX.
	REQUIRED FASTENERS	ONE- 5/8" Ø BOLT @ 28" O.C.	ONE- 5/8" Ø BOLT @ 16" O.C.

IF THE DECK BAND IS SUPPORTED BY A 1/2" MINIMUM MASONRY LEDGE ALONG THE FOUNDATION WALL, 5/8" Ø BOLTS SPACED @ 48" O.C. MAY BE USED FOR SUPPORT.

OTHER MEANS OF SUPPORT, SUCH AS JOIST HANGERS, MAY BE USED TO CONNECT DECK JOISTS TO A TREATED STRUCTURE BAND

GIRDERS SHALL BEAR DIRECTLY ON POSTS OR BE BE CONNECTED TO THE SIDES OF POSTS WITH 2- 5/8" Ø BOLTS

FLOOR DECKING SHALL BE NO. 2 GRADE TREATED SOUTHERN PINE OR EQUIVALENT. THE MINIMUM FLOOR DECKING THICKNESS SHALL BE AS FOLLOWS:

FND FOUNDATION

HDG HOT DIPPED

GALVANIZED

LVL LAMINATED VENEER

PSL PARALLEL STRAND

PT PRESSURE TREATED

LUMBFR

NTS NOT TO SCALE

).C. ON CENTER

LUMRFR

OJ QUAD JOIST

SQ SQUARE

SP STUD POCKET

FTG FOOTING

HGR HANGER

TJ TRIPLE JOIST

UNO UNLESS NOTED

XJ EXTRA JOIST

OTHERWISE

TSP TRIPLE STUD POCKET

TYP TYPICAL

	32 U.C.	2 545
MAX	KIMUM HEIGHT OF DECK SUPPORT POSTS	S IS AS FOLLOWS:
	POST SIZE	MAX POST HEIGHT
	4X4	8'
	6X6	20'
	ENGINEERED	20' +

NOTES: 1) THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. 2) THIS TABLE IS BASED ON A MAXIMUM TRIBUTARY AREA OF 128 SQ. FT. 3) POST HEIGHT IS FROM TOP OF FOOTING TO BOTTOM OF GIRDER.

10. DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THE FOLLOWING

A. WHEN THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION 4, LATERAL BRACING IS NOT REQUIRED.

B. 4X4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE ATTACHED AT THE ENDS TO THE GIRDER AND THE POST WITH ONE - 5/8" BOLT

C. FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN CONCRETE IN ACCORDANCE WITH THE FOLLOWING:

POST SIZE	TRIBUT. AREA	POST HEIGHT	EMB. DEPTH	CONC. DIAM.
4X4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
6X6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

D. 2X6 DIAGONAL VERTICAL CROSS BRACING SHALL BE PROVIDED IN TWO PERPENDICULAR DIRECTIONS FOR FREE STANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE BRACES SHALL BE ATTACHED TO THE POSTS WITH ONE -5/8" ϕ BOLT AT EACH END OF THE BRACE.

NOTES: 1) ALL NAILS AND BOLTS ARE TO BE HOT DIPPED GALVANIZED. 2) MINIMUM EDGE DISTANCE FOR BOLTS IS 2 1/2". 3) NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MINIMUM OF 1 1/2".

ALLOWABLE I-JOIST SUBSTITUTION NOTE: MAINTAIN JOIST DEPTH, DIRECTION, AND SPACING SPECIFIED ON PLANS. CIMPCON EVCE CIMPCON TO

MANUFACTURER	DEPTH	SERIES	MOUNT HGR	SIMPSON TOP FLANGE HGR
BLUELINX	16"	BLI 40	IUS2.56/16	ITS2.56/16
BLUELINX	16"	BLI 60	IUS2.56/16	ITS2.56/16
BOISE CASCADE	16"	BCI 5000s	IUS2.06/16	ITS2.06/16
BOISE CASCADE	16"	BCI 6000S	IUS2.37/16	ITS2.37/16
INTERNATIONAL	16"	IB 600	IUS2.56/16	ITS2.56/16
BEAMS			,	•
LP CORP	16"	LPI 20+	IUS2.56/16	ITS2.56/16
NORDIC	16"	NI 40X	IUS2.56/16	ITS2.56/16
ROSEBURG	16"	RFPI 60S	IUS2.56/16	ITS2.56/16
WEYERHAEUSER	16"	TJI 210	IUS2.06/16	ITS2.06/16
JOISTS NOT LISTED	IN THE	AROVE TARIL	F MAY RE LISED P	ROVIDED THEY

JOISTS NOT LISTED IN THE ABOVE TABLE MAY BE USED PROVIDED THEY MEET OR EXCEED THE PROPERTIES OF THOSE LISTED. SUBSTITUTE USP BRAND HANGERS WITH EQUIVALENT VALUES AS DESIRED.

0070504

PROJECT NO.

SPECS

PLAN NO. M089-22B

SCOPE.
LOT #:
PLAN:

22-21-144

SHEET NO.

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