

truss delivery package or online @ sbcindustry.co



Trenco RE: J0822-4072 818 Soundside Rd Lot 46 Liberty Meadows Edenton, NC 27932 Site Information: Customer: Benjamin Stout Real Estate Project Name: J0822-4072 Lot/Block: 46 Model: Ashville Address: 24 Melvill Lane Subdivision: Liberty Meadows State: NC City: General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4 Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf This package includes 21 individual, dated Truss Design Drawings and 0 Additional Drawings. No. Seal# Truss Name Date No. Seal# Truss Name Date E16391025 11/9/2021 A1 21 E16391045 V5 11/9/2021 1 2 E16391026 A1GE 11/9/2021 3 E16391027 A2 11/9/2021 4 E16391028 A3 11/9/2021 5 E16391029 A3GE 11/9/2021 6 E16391030 B1 11/9/2021

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The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

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My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

B1-GR

B1GE

C1-GR

C1GE

D1GE

M1GE

C1

D1

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PΒ

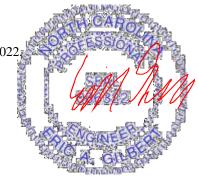
V1

V2

V3

V4

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



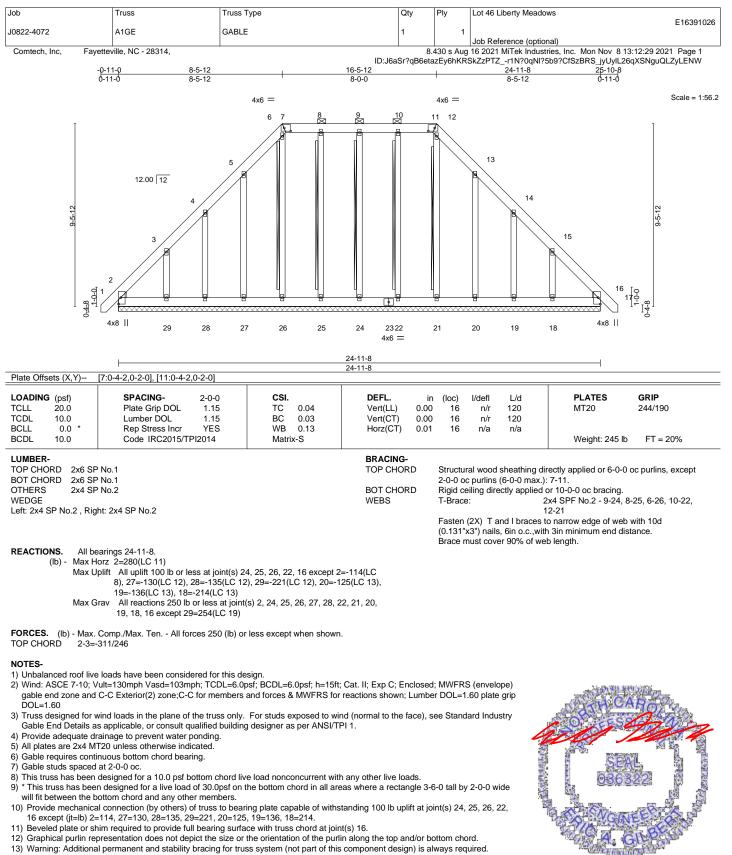
Gilbert, Eric

November 09, 2021

Job	Truss	Truss Type	Qty	Ply	Lot 46 Liberty Meadows	3
J0822-4072	A1	PIGGYBACK BASE	4	1		E163910
Comtech, Inc, Fayette	eville, NC - 28314,					s, Inc. Mon Nov 8 13:12:28 2021 Page 1
- <u>0-1</u> 0-1 ⁻	1-0 8-5-12 1-0 8-5-12		ID:J6aSr?qB6etazEy6 16-5-12 8-0-0	hKRSkZzł	PTZMqpdoUM7EoTIN2- 24-11-8 8-5-12	4GPTwDRWQD5Mu8NN4J809sp6yLENX <u>25-10-</u> 8 0-11-0
0-1	1-0 0-3-12	ee —	8-0-0	o	6-5-12	5-11-0 Scale = 1:
		6x6 = ³ 1 2 ⊠	⊠ ⊠ ³	6x6 =		
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		2x4	420 —	2x4		
	8-5-12 8-5-12		16-5-12 8-0-0	-	24-11-8 8-5-12	
	0-3-8,0-3-0], [4:0-3-8,0-3-0]					
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	5 TC 0.43	DEFL. ir Vert(LL) -0.15	5-7	l/defl L/d >999 360	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	WB 0.15	Vert(CT) -0.20 Horz(CT) 0.02	5	>999 240 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.19	2-9	>999 240	Weight: 167 lb FT = 20%
TOP CHORD 2x6 SP No BOT CHORD 2x6 SP No			TOP CHORD		ral wood sheathing direc c purlins (6-0-0 max.): 3	ctly applied or 6-0-0 oc purlins, except
WEBS 2x4 SP No WEDGE			BOT CHORD		eiling directly applied or	
Left: 2x4 SP No.2 , Right:	2x4 SP No.2					
	2=0-3-8, 5=0-3-8 2 2=223(LC 11)					
Max Uplif	t 2=-35(LC 12), 5=-35(LC 13) / 2=1309(LC 2), 5=1309(LC 2)					
		-/ 0 (lb) or less except when shown				
TOP CHORD 2-3=-14	65/308, 3-4=-908/350, 4-5=-1 /930, 7-9=-33/938, 5-7=-33/92	465/308				
WEBS 3-9=0/65	53, 4-7=0/653					
	ads have been considered for					
		DL=6.0psf; BCDL=6.0psf; h=15ft; to 8-5-12, Exterior(2) 8-5-12 to 14				
16-5-12 to 22-8-7, Inter plate grip DOL=1.60	rior(1) 22-8-7 to 25-8-14 zone	C-C for members and forces & N	MWFRS for reactions sho	own; Lum	ber DOL=1.60	. Marcelle (1999) (1999) (1999) (1999) (1999)
4) This truss has been dea		hord live load nonconcurrent with				CAR
will fit between the botto	om chord and any other mem		· ·			o secondo
		b bearing plate capable of withsta ize or the orientation of the purlin				
						SEAL
						036322
						A Dimension of the second
					1	210 A STREET
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						November 9,2021

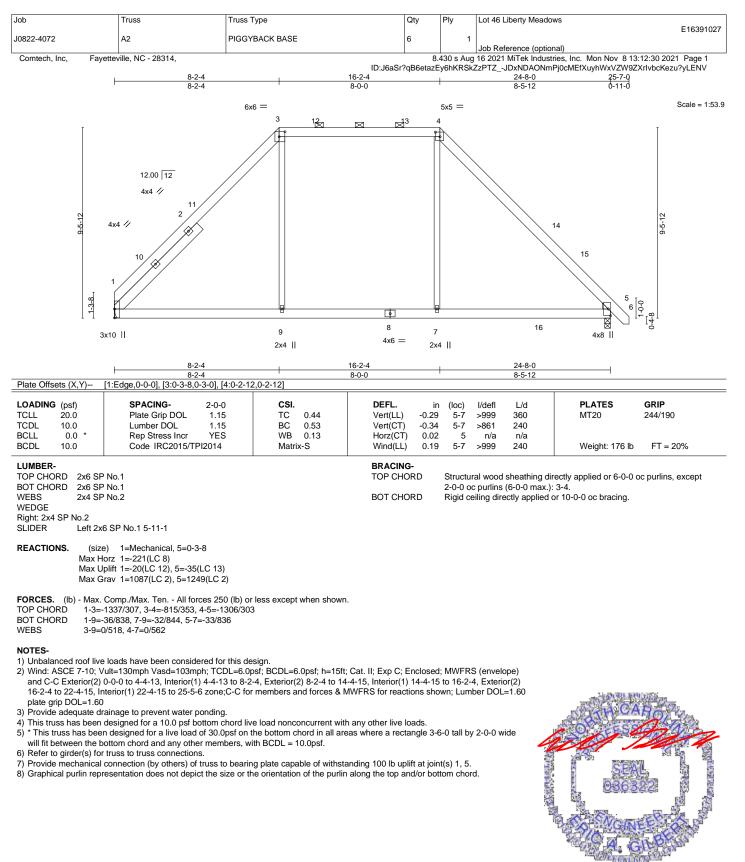
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidat truss eval and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





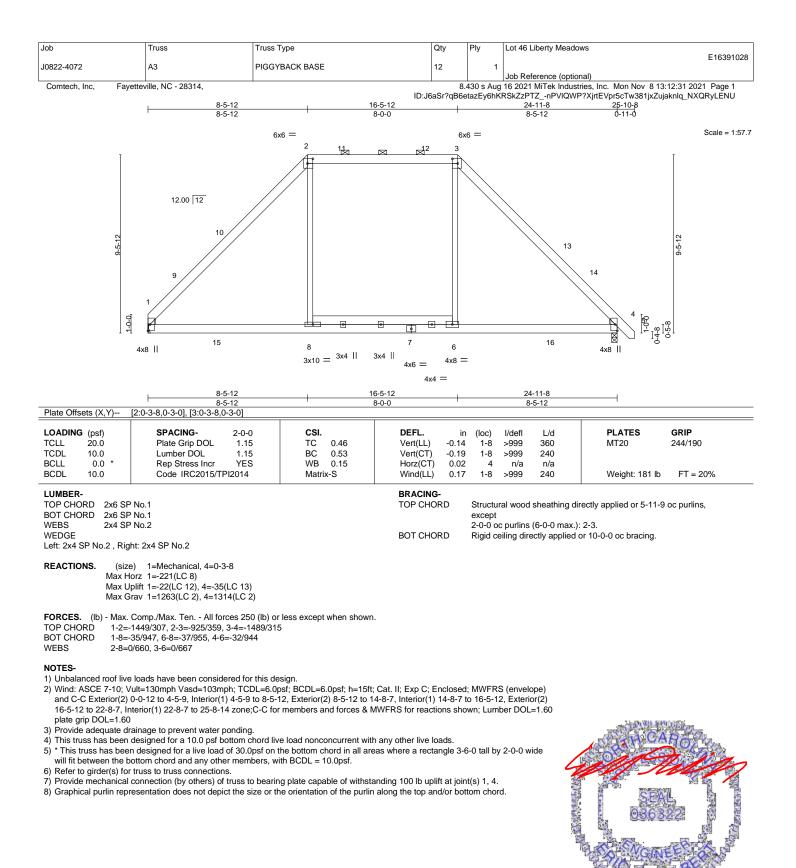


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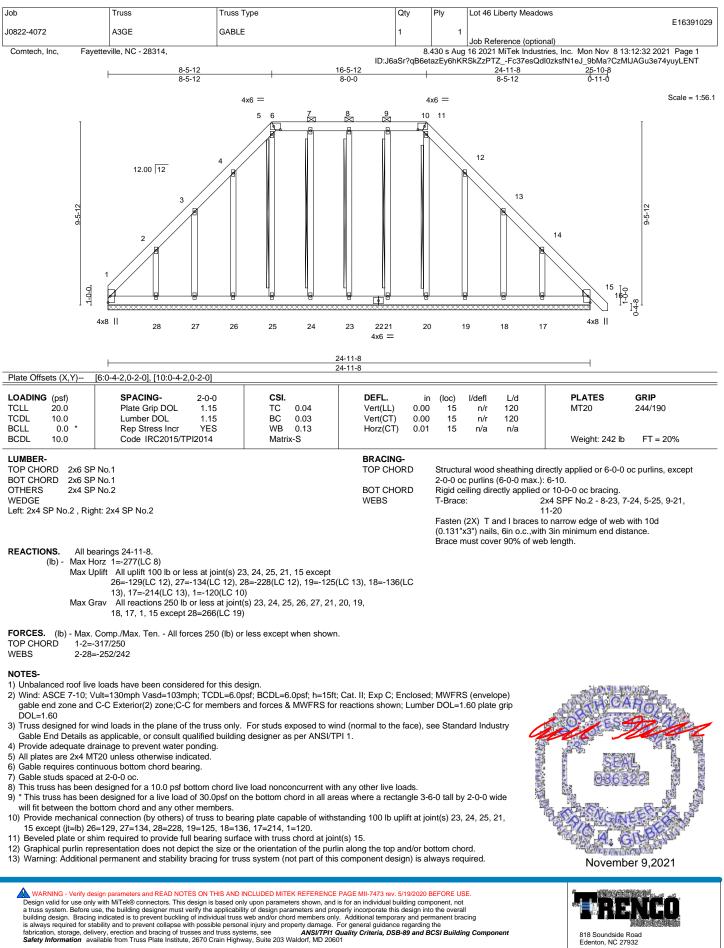
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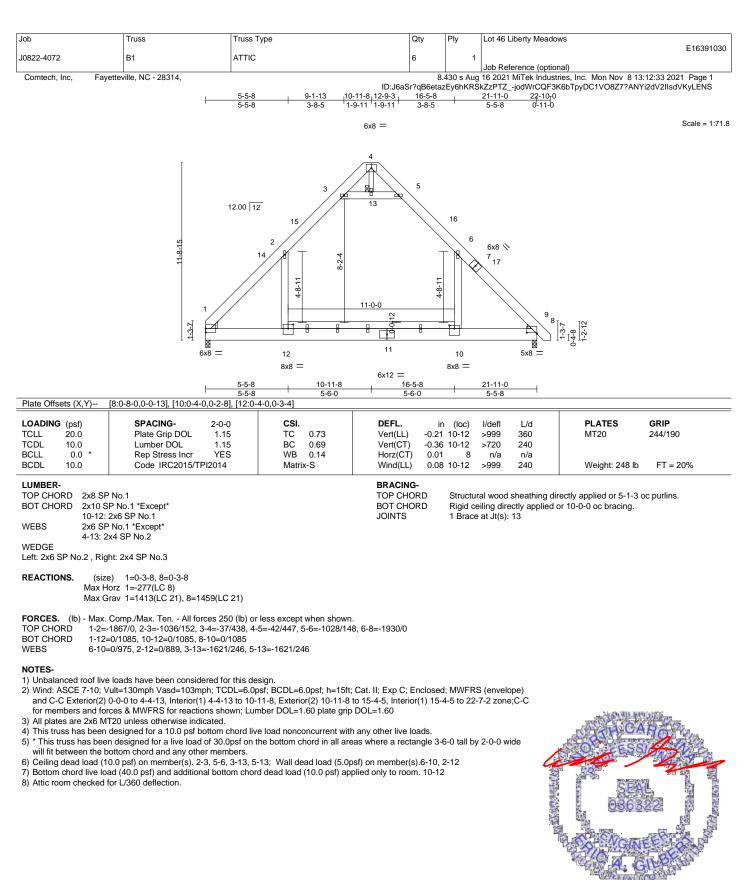


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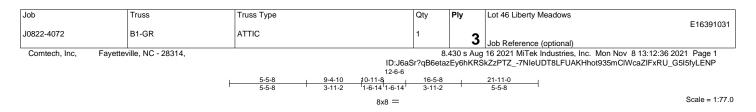


Edenton, NC 27932



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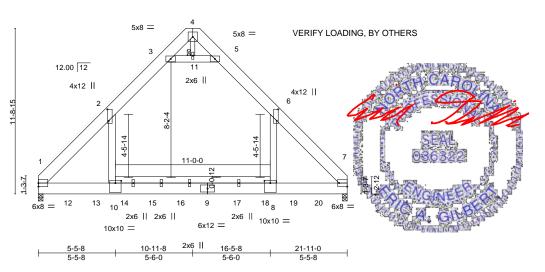


Plate Offsets (X,Y)	[2:0-9-12,0-1-4], [6:0-9-12,0-1-4], [7:Edg	e,0-3-0], [8:0-5-0,0-2-0], [10:0-5-0,0-2-0]						
LOADING (psf) ITCLL 20.0 ITCDL 10.0 SCLL 0.0 SCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.72 BC 0.76 WB 0.38 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.29	(loc) 8-10 8-10 7 10	l/defl >905 >666 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 801 lb	GRIP 244/190 FT = 20%
30T CHORD 2x10 S 8-10: 2 NEBS 2x6 SP 4-11: 2	P 2400F 2.0E P No.1 *Except* x6 SP No.1 P No.1 *Except* x4 SP No.2		BRACING- TOP CHOF BOT CHOF JOINTS	D D	Rigid c		ectly applied o	ectly applied or 6-0-0 o r 10-0-0 oc bracing.	oc purlins.
Max H	e) 1=0-4-0, 7=0-4-0 orz 1=271(LC 5) rav 1=9588(LC 14), 7=9573(LC 14)								
OP CHORD 1-2=- 30T CHORD 1-10=	Comp./Max. Ten All forces 250 (lb) or 10113/0, 2-3=-4213/35, 3-4=-19/3638, 4 =0/5656, 8-10=0/5720, 7-8=0/5656)/8207, 2-10=0/8177, 3-11=-11296/4, 5-	-5=-20/3649, 5-6=-4203/3	5, 6-7=-10124/0						
Top chords connect Bottom chords conn Webs connected as All loads are conside ply connections have) Unbalanced roof live) Wind: ASCE 7-10; V Lumber DOL=1.60 p) Concentrated loads MWFRS Wind (Pos. Left; #7 Dead + 0.61 MWFRS Wind (Pos. (Pos. Internal) 4th P- 2nd Parallel; #20 De Live (bal.) + 0.75 Att 0.75(0.6 MWFRS W Int) 2nd Parallel);	nected together with 10d (0.131"x3") na ed as follows: 2x10 - 2 rows staggered a ected as follows: 2x10 - 5 rows staggered follows: 2x6 - 2 rows staggered at 0-9-0 ered equally applied to all plies, except if e been provided to distribute only loads i e loads have been considered for this de fult=130mph Vasd=103mph; TCDL=6.0p ilate grip DOL=1.60 from layout are not present in Load Cas Internal) Left; #5 Dead + 0.6 MWFRS V MWFRS Wind (Neg. Internal) Right; #8 Internal) 2nd Parallel; #10 Dead + 0.61 WFRS Wind (Neg. Internal) Right; #8 Internal) 2nd Parallel; #10 Dead + 0.75 Roof Live (bal.) + 0.75 Attic F ic Floor + 0.75(0.6 MWFRS Wind (Neg. ind (Neg. Int) 1st Parallel); #23 Dead + 0 designed for a 10.0 psf bottom chord live	t 0-9-0 oc. d at 0-4-0 oc. oc, 2x4 - 1 row at 0-9-0 o noted as front (F) or back noted as (F) or (B), unless sign. sf; BCDL=6.0psf; h=15ft; e(s): #3 Dead + Uninhabit /ind (Pos. Internal) Right; Dead + 0.6 MWFRS Wind WFRS Wind (Pos. Intern leg. Internal) 1st Parallel; oor + 0.75(0.6 MWFRS W Int) Right); #22 Dead + 0. .75 Roof Live (bal.) + 0.75	(B) face in the L otherwise indica Cat. II; Exp C; Er able Attic Withou #6 Dead + 0.6 M (Pos. Internal) 1s al) 3rd Parallel; # #13 Dead + 0.6 M ind (Neg. Int) Le 75 Root Live (bal 5 Attic Floor + 0.7	ted. closed; storage WFRS W t Paralle 11 Dead WWFRS ' t); #21 E .) + 0.75 5(0.6 M ¹	MWFR e; #4 D Vind (N el; #9 D I + 0.6 I Wind (I Dead + 5 Attic F	S (envelo ead + 0.6 leg. Inter Dead + 0.1 MWFRS Neg. Inte 0.75 Roo Toor +	ope); 5 nal) 6 Wind mal) of	Novemb	ər 9,2021
WARNING - Verify de Design valid for use only a truss system. Before u building design. Bracing is always required for sta fabrication, storage, deli	sign parameters and READ NOTES ON THIS AND I with MITek® connectors. This design is based onl se, the building designer must verify the applicabili indicated is to prevent buckling of individual truss ability and to prevent collapse with possible person very, erection and bracing of trusses and truss syst alable from Truss Plate Institute, 2670 Crain Highw	NCLUDED MITEK REFERENCE F y upon parameters shown, and is y of design parameters and prop web and/or chord members only. al injury and property damage. F ems, see ANS//TPI1 Q	AGE MII-7473 rev. 5// for an individual buik erly incorporate this d Additional temporany or general guidance r ality Criteria, DSB-{	9/2020 BE ling compo esign into t and perma egarding th	onent, no the overa anent bra ne	t all acing	nent	818 Soundside F Edenton, NC 275	

Job		Truss	Truss Type	Qty	Ply	Lot 46 Liberty Meadows			
J0822-4072		B1-GR	ATTIC	1	2	E163910	331		
					3	Job Reference (optional)			
Comtech, Inc,	Fayettev	/ille, NC - 28314,		. 8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:36 2021 Page 2			
			ID:J6aSr?qB6etazEy6hKRSkZzPTZ -7NIeUDT8LFUAKHhot935mClWcaZIFxRU G5l5fyLENP						

NOTES-

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-11, 5-11; Wall dead load (5.0psf) on member(s).6-8, 2-10
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-10
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1726 lb down at 1-11-12, 1726 lb down at 3-11-12, 3826 lb down at 5-2-12, 376 lb down and 34 lb up at 5-11-12, 376 lb down and 34 lb up at 7-11-12, 376 lb down and 34 lb up at 9-11-12, 376 lb down and 34 lb up at 11-11-12, 376 lb down and 34 lb up at 13-11-12, 376 lb down and 34 lb up at 15-11-12, 3826 lb down at 16-8-4, and 1726 lb down at 17-11-12, and 1726 lb down at 19-9-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 11) Attic room checked for L/360 deflection.

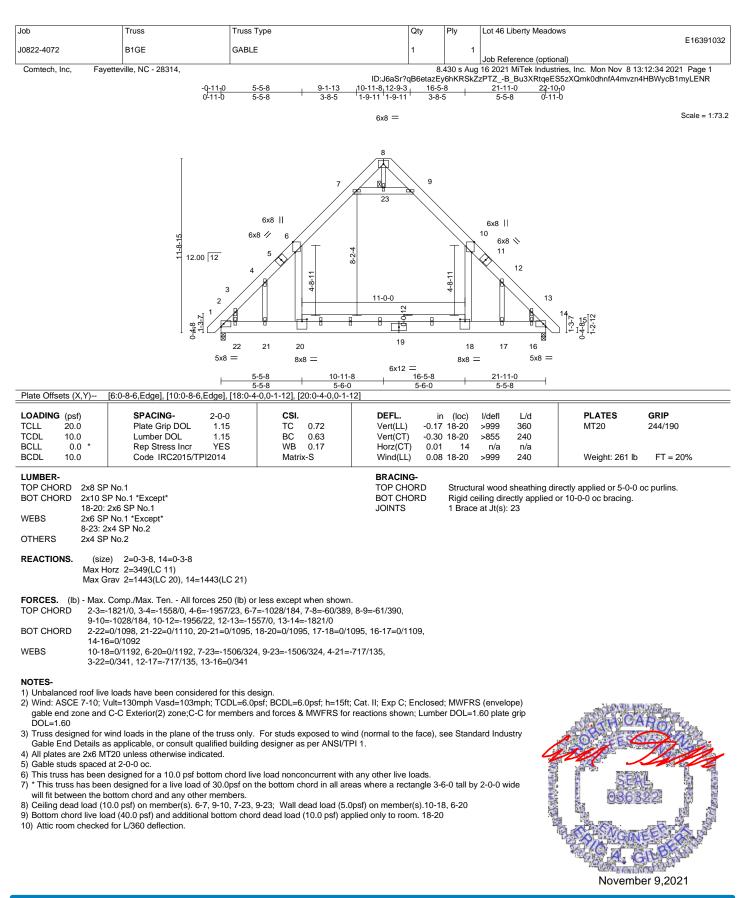
LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-2=-60, 2-3=-80, 3-4=-60, 4-5=-60, 5-6=-80, 6-7=-60, 1-10=-20, 8-10=-40, 7-8=-20, 3-5=-20
- Drag: 6-8=-10, 2-10=-10 Concentrated Loads (lb)

Vert: 9=-62(B) 8=-1029(B) 10=-1029(B) 12=-430(B) 13=-430(B) 14=-62(B) 15=-62(B) 16=-62(B) 17=-62(B) 18=-62(B) 19=-430(B) 20=-430(B) 10=-1029(B) 12=-430(B) 12=-430(B)

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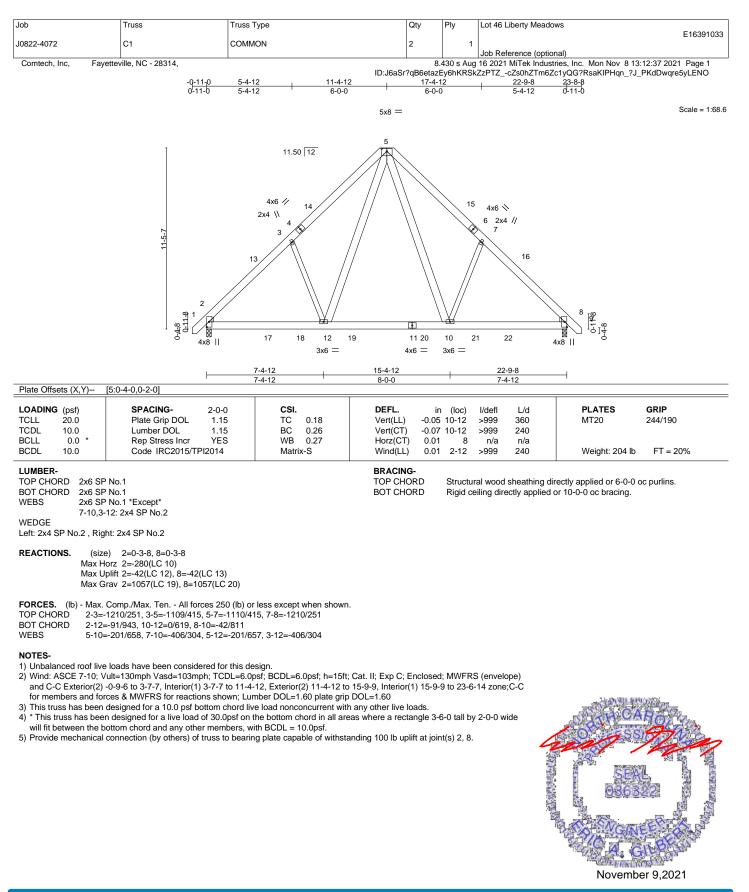




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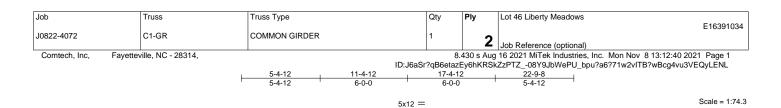
818 Soundside Road Edenton, NC 27932



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Edenton, NC 27932



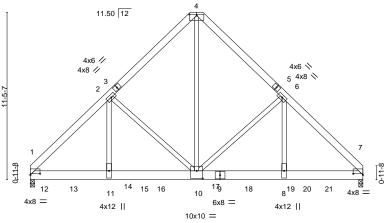


Plate Offsets (X,Y) [1:0-8-0,0-0-15], [4:0-6-0,0-1-0], [7:0-8-0,0-0-15], [10:0-5-0,0-6-4]										
		5-4-12	6-0-0	6-0-0	5-4-12					
		5-4-12	11-4-12	17-4-12	22-9-8					

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.10 8-10	>999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.40	Vert(CT) -0.17 8-10	>999 240	
BCLL 0.0 *	Rep Stress Incr NO	WB 0.94	Horz(CT) 0.04 7	n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.05 8-10	>999 240	Weight: 396 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x8 SP 2400F 2.0E
WEBS	2x4 SP No 2

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-270(LC 23) Max Uplift 1=-225(LC 9), 7=-237(LC 8) Max Grav 1=7977(LC 2), 7=8527(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-8698/296, 2-4=-5745/316, 4-6=-5744/316, 6-7=-8803/299

1-11=-233/5962, 10-11=-233/5971, 8-10=-139/6041, 7-8=-139/6032 BOT CHORD

4-10=-299/7498, 6-10=-2640/260, 6-8=-42/3902, 2-10=-2545/257, 2-11=-37/3758 WEBS

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-6-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=225, 7=237.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1246 lb down and 39 lb up at 0-10-4, 1243 lb down and 42 lb up at 2-10-4, 1243 lb down and 42 lb up at 4-10-4, 1243 lb down and 42 lb up at 6-10-4, 1243 lb down and 42 lb up at 8-10-4, 1243 lb down and 42 lb up at 10-10-4, 1243 lb down and 42 lb up at 12-10-4, 1243 lb down and 42 lb up at 14-10-4, 1243 lb down and 42 lb up at 16-10-4, 1243 lb down and 42 lb up at 18-10-4, and 1243 lb down and 42 lb up at 20-4-4, and 1250 lb down and 35 lb up at 22-7-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

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Structural wood sheathing directly applied or 5-5-14 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE

Construints - Strange, delivery, erection and bracing of trusses and truss even injury and property damage. For general guidance regarding the fashication strategy and bracing of the strategy and brack and bracing of the strategy and brack and

[Job	Truss	Truss Type	Qty	Ply	Lot 46 Liberty Meadows	
	10000 1070					E16391034	
	J0822-4072	C1-GR	COMMON GIRDER	1	2	Job Reference (optional)	
l							
	Comtech, Inc, Fayettev	ille, NC - 28314,		8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:40 202			

ID:J6aSr?qB6etazEy6hKRSkZzPTZ_08Y9JbWePU_bpu?a6?71w2vITB?wBcg4vu3VEQyLENL

LOAD CASE(S) Standard

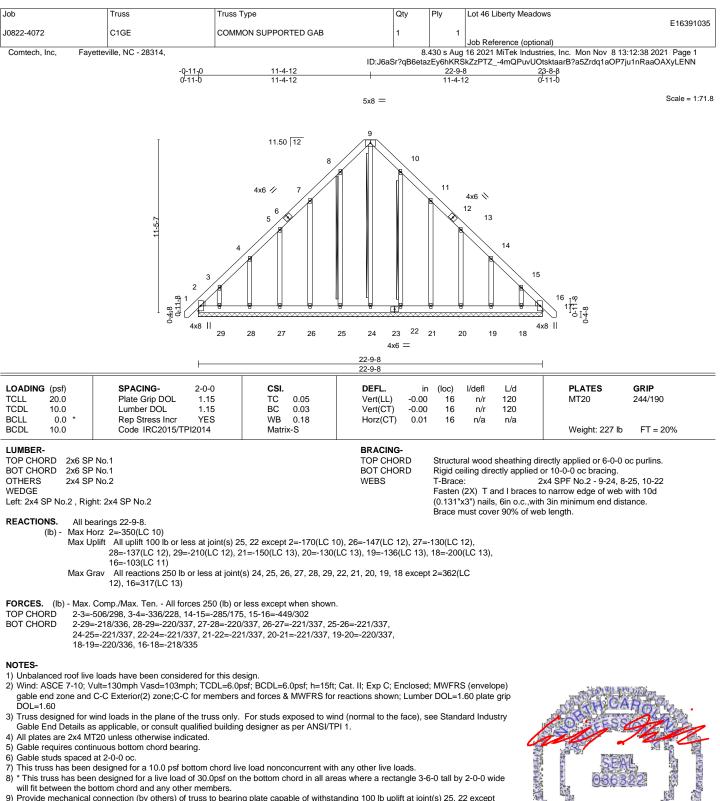
Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (Ib)

Vert: 9=-969(B) 7=-976(B) 12=-972(B) 13=-969(B) 14=-969(B) 15=-969(B) 16=-969(B) 17=-969(B) 18=-969(B) 19=-969(B) 20=-969(B) 21=-969(B) 20=-969(B) 20=-960

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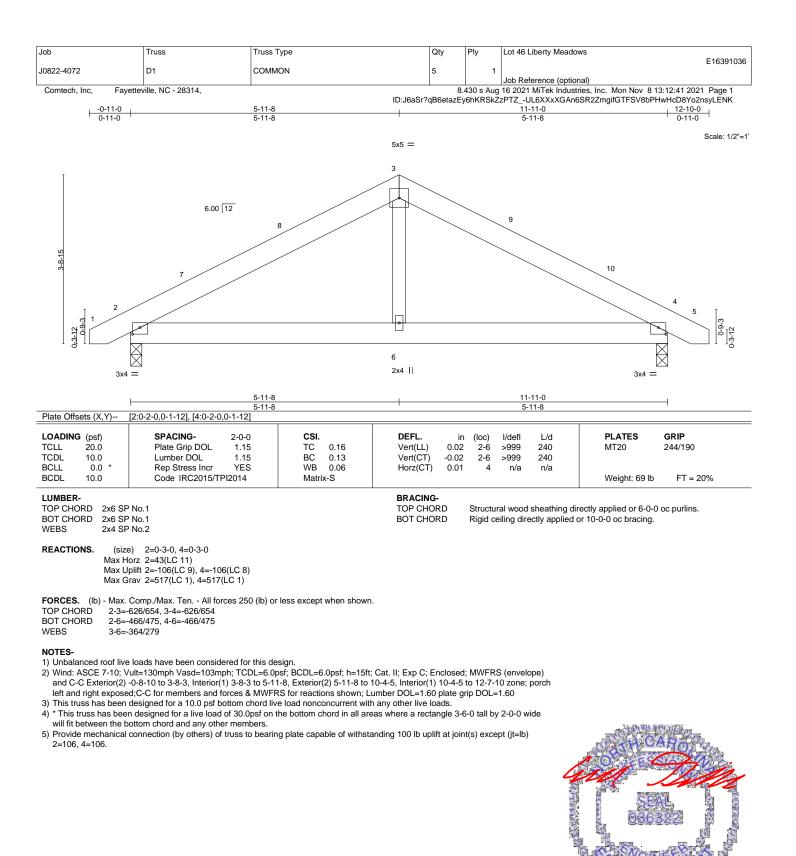


- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 22 except (jt=lb) 2=170, 26=147, 27=130, 28=137, 29=210, 21=150, 20=130, 19=136, 18=200, 16=103.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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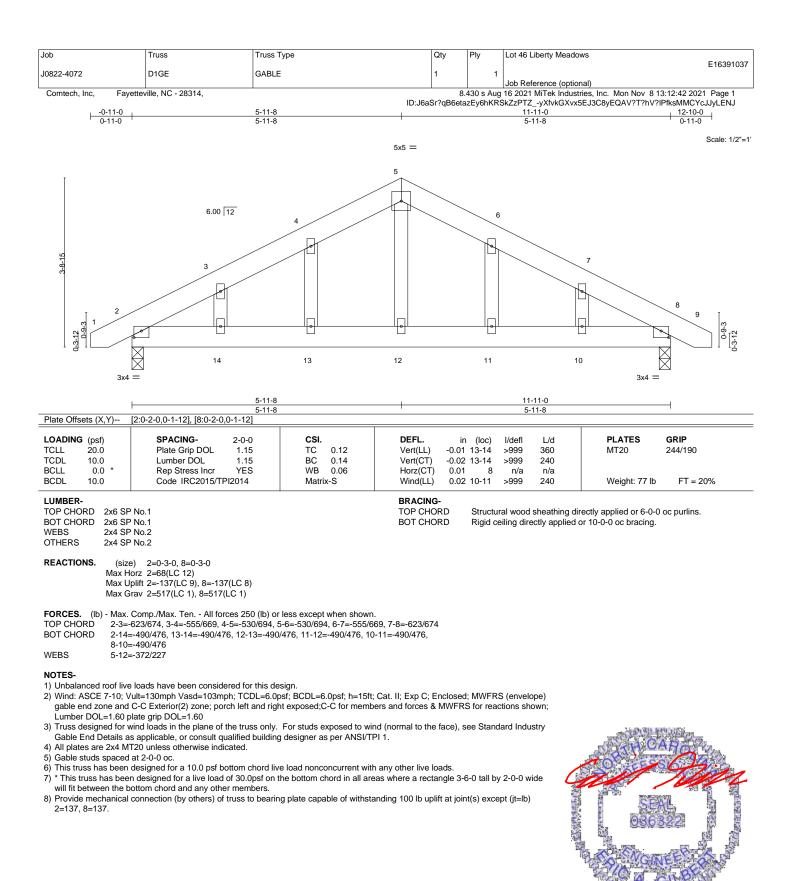
🗥 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange, delivery, erection and bracing of trusses and truss even injury and property damage. For general guidance regarding the fashication strategy and bracing of the strategy and brack and bracing of the strategy and brack and



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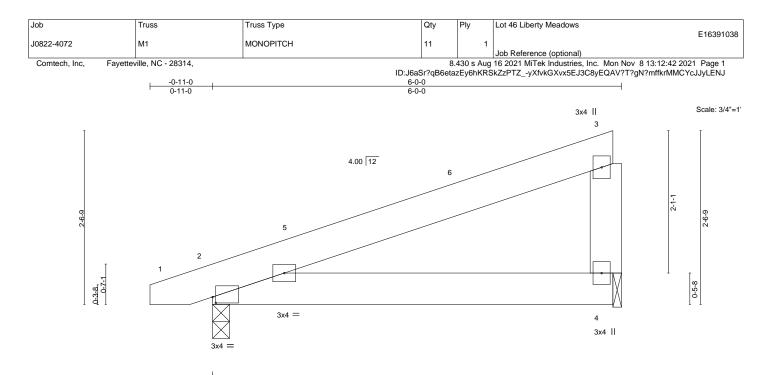


November 9,2021



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LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.19	Vert(LL) -0.01 2-4	>999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.12	Vert(CT) -0.03 2-4	>999 240	
3CLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00	n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.03 2-4	>999 240	Weight: 34 lb FT = 20%

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

TOP CHORD Structural wood s except end vertic: BOT CHORD Binid ceiling direc

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=71(LC 8) Max Uplift 2=-104(LC 8), 4=-97(LC 8) Max Grav 2=274(LC 1), 4=223(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=104.



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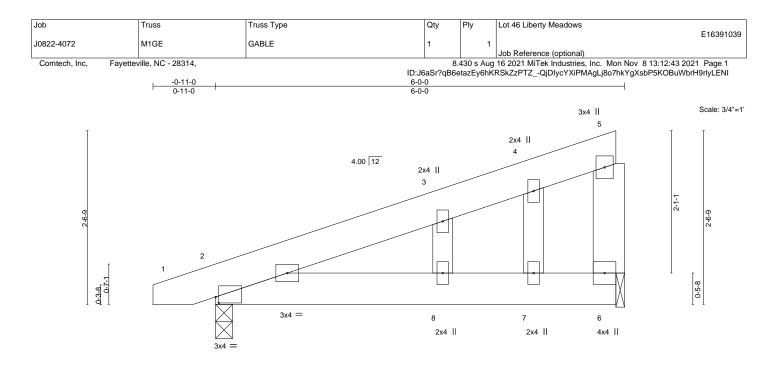


Plate Offs	ets (X,Y)	[2:0-0-9,0-1-1]		1		Т					1	
OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
ICLL	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	0.02	2-8	>999	240	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	-0.02	8	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.01	Horz(CT)	-0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 37 lb	FT = 20%
UMBER-						BRACING					·	
TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1					TOP CHOP	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc pu except end verticals.					oc purlins,	
VEBS DTHERS	2x6 SP 2x4 SP					BOT CHOP					or 10-0-0 oc bracing.	

REACTIONS. (size) 2=0-3-0, 6=0-1-8 Max Horz 2=101(LC 8) Max Uplift 2=-151(LC 8), 6=-142(LC 8) Max Grav 2=274(LC 1), 6=223(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

ł

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 1-4-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=151.6=142.



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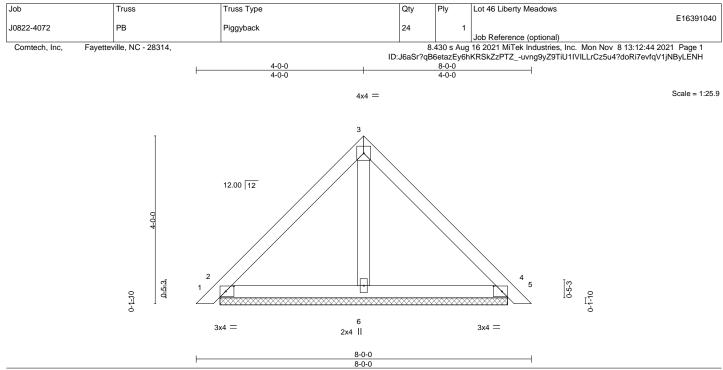


Plate Offsets (X,Y)	[2:0-2-6,0-1-8], [4:0-2-6,0-1-8]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.20 BC 0.09 WB 0.03 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) 0.01 5 n/r 120 Vert(CT) 0.01 5 n/r 120 Horz(CT) 0.00 4 n/a n/a Weight: 32 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF			BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No 2

REACTIONS. (size) 2=6-10-6, 4=6-10-6, 6=6-10-6

Max Horz 2=-114(LC 10)

Max Uplift 2=-65(LC 13), 4=-71(LC 13) Max Grav 2=190(LC 1), 4=190(LC 1), 6=214(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

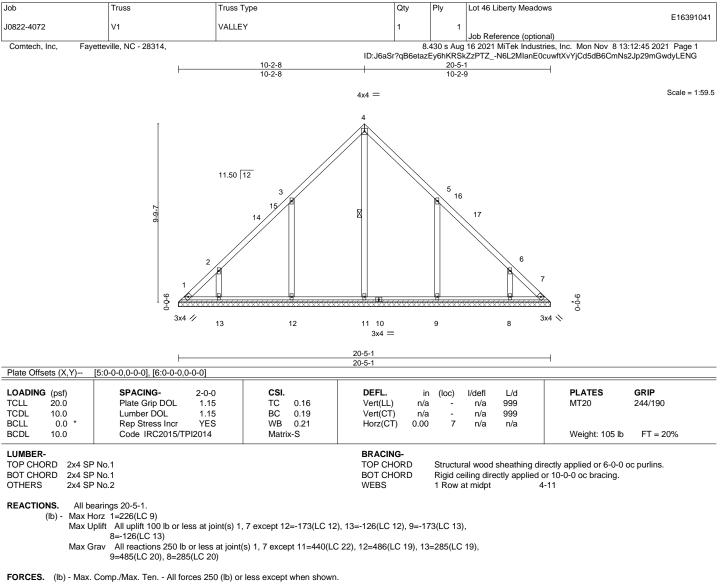
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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WEBS 3-12=-393/294, 2-13=-299/243, 5-9=-393/294, 6-8=-299/243

NOTES-

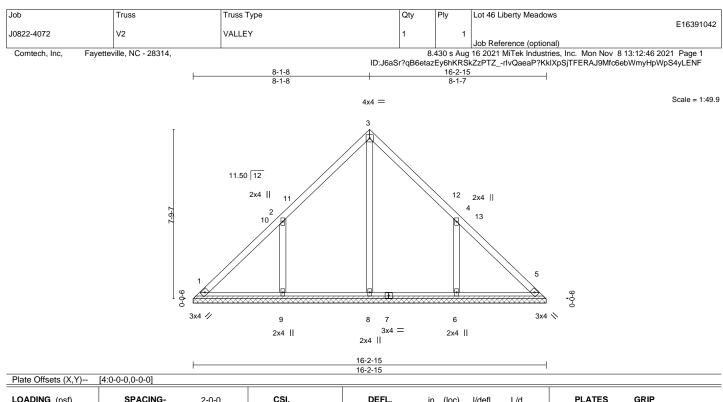
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-6 to 4-9-2, Interior(1) 4-9-2 to 10-2-8, Exterior(2) 10-2-8 to 14-7-5, Interior(1) 14-7-5 to 20-0-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 12=173, 13=126, 9=173, 8=126.



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.17 BC 0.19 WB 0.13 Matrix-S	Vert(CT)	in (n/a n/a).00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES GRIF MT20 244/ Weight: 77 lb F	
LUMBER- TOP CHORD 2x4 S	² No.1	BRACING- TOP CHORD	S	Structura	al wood	sheathing di	rectly applied or 6-0-0 oc pu	rlins.	

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD

2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. All bearings 16-2-15.

(lb) - Max Horz 1=-178(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-182(LC 12), 6=-182(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=418(LC 22), 9=503(LC 19), 6=503(LC 20)

WEBS 2-9=-406/302, 4-6=-406/302

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCF 7-10; Vult=130mph; TCDL=6.0ps; BCDL=6.0ps; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-6 to 4-9-2, Interior(1) 4-9-2 to 8-1-8, Exterior(2) 8-1-8 to 12-6-4, Interior(1) 12-6-4 to 15-10-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

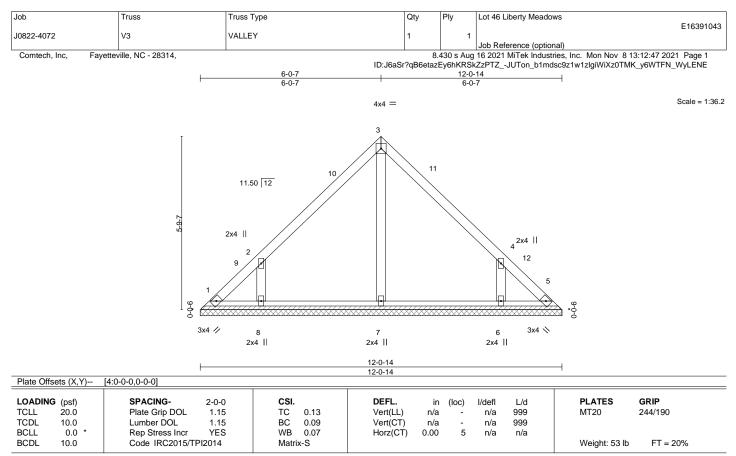
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=182, 6=182,



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 12-0-14.

(lb) - Max Horz 1=-130(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-150(LC 12), 6=-150(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=334(LC 19), 6=334(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS 2-8=-344/282, 4-6=-344/282

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-6 to 4-9-2, Interior(1) 4-9-2 to 6-0-7, Exterior(2) 6-0-7 to 10-5-4, Interior(1) 10-5-4 to 11-8-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=150, 6=150.



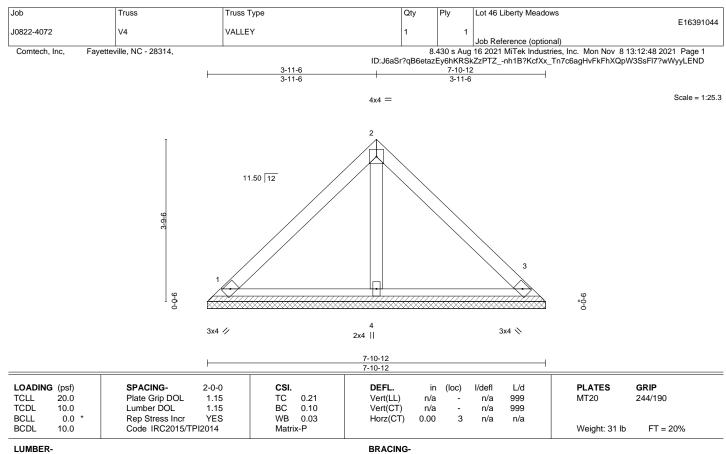
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

November 9,20

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TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=7-10-12, 3=7-10-12, 4=7-10-12

Max Horz 1=-82(LC 8)

Max Uplift 1=-30(LC 13), 3=-31(LC 13)

Max Grav 1=173(LC 1), 3=173(LC 1), 4=228(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

- and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



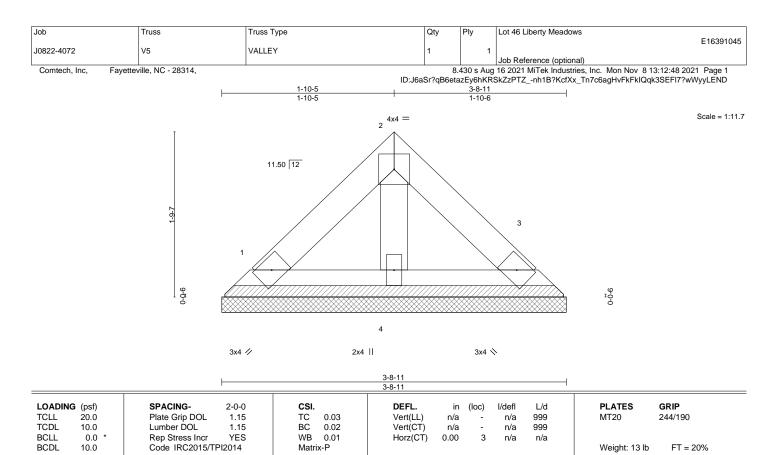
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=3-8-11, 3=3-8-11, 4=3-8-11 Max Horz 1=34(LC 9)

Max Uplift 1=-12(LC 13), 3=-13(LC 13)

Max Grav 1=72(LC 1), 3=72(LC 1), 4=95(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

- and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



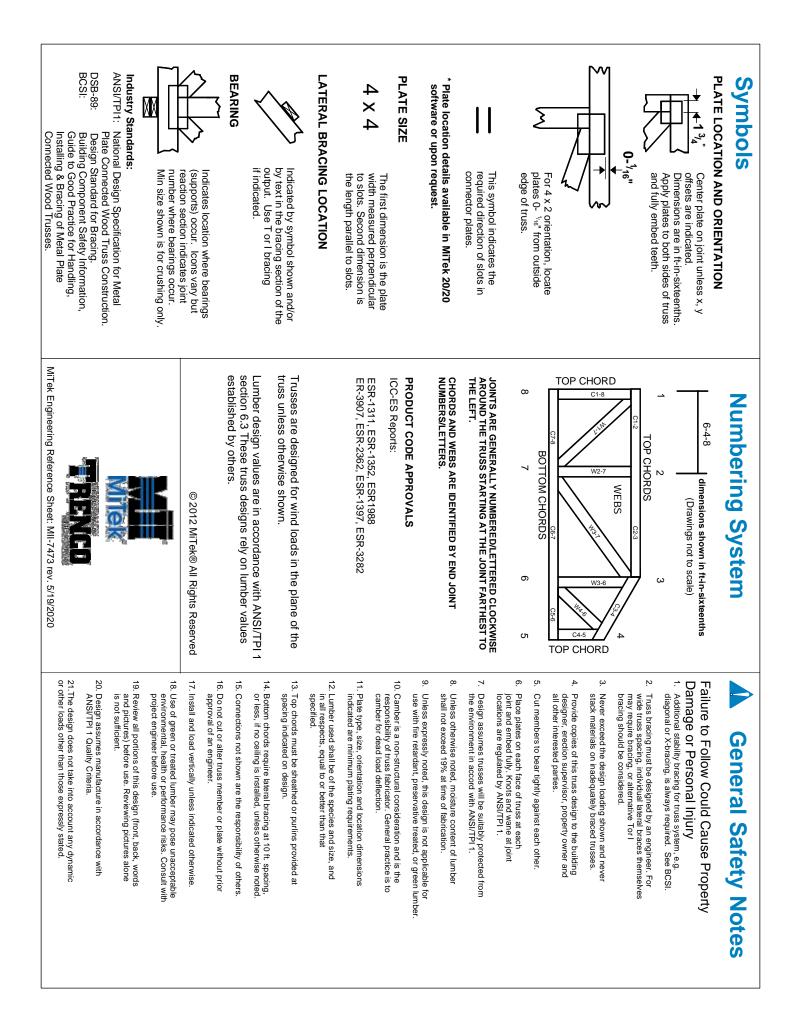
Structural wood sheathing directly applied or 3-8-11 oc purlins.

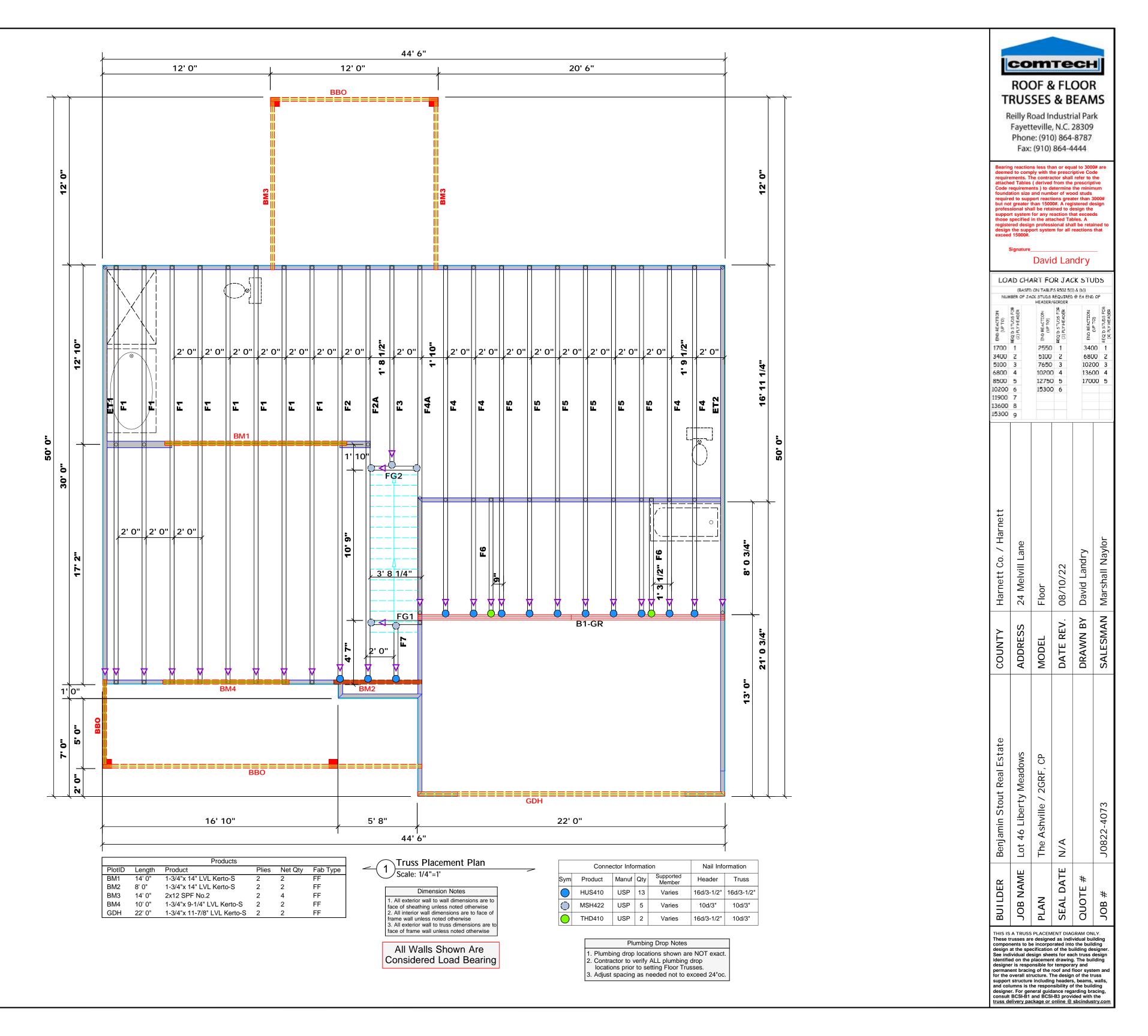
Rigid ceiling directly applied or 10-0-0 oc bracing.

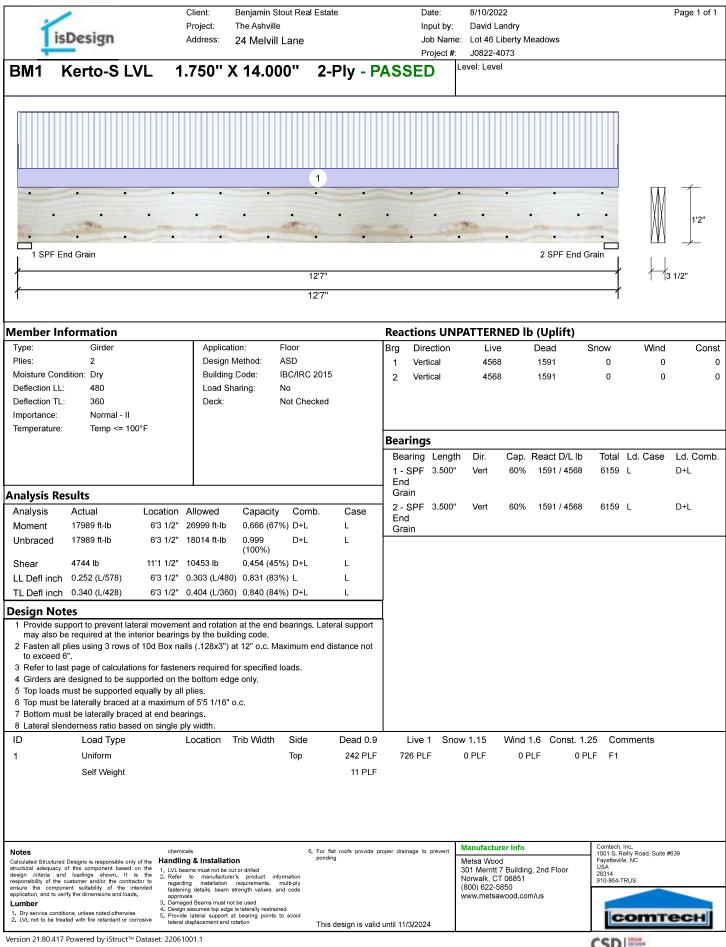


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





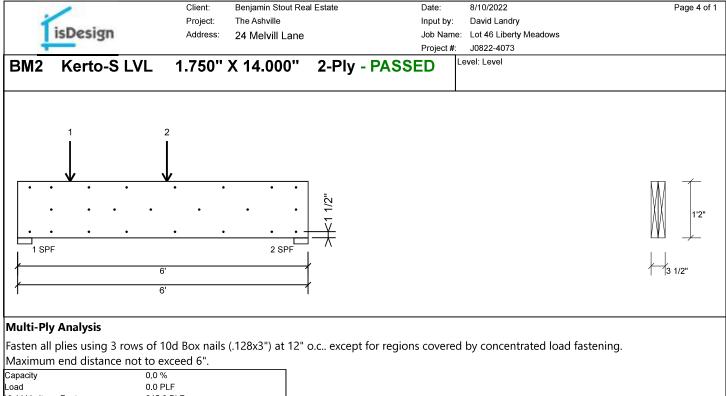




	/		Client:	Benjamin Stout Re	eal Estate		Date:	8/10/2022	Page 2 of 1
1	isDesign		Project: Address:	The Ashville 24 Melvill Lane				e: Lot 46 Liberty Meadows	
BM1	Kerto-S	LVL	1.750"	X 14.000"	2-Plv	- PASSI	Project #	: J0822-4073 Level: Level	
					_ · · · y				
	•	•	• •	•	•	• •		• • •	
.	•	•	•		•	•	•		
.	•	•		•	•	• •			· · · · · · · · · · · · · · · · · · ·
1 SPF	End Grain							2 SPF End	
					12'7"				3 1/2"
1					12'7"				1
Multi-Ply	Analysis								
Fasten all	-		d Box nails	s (.128x3") at 12"	o.c Maxim	num end dis	stance n	ot to exceed 6".	
Capacity Load		0.0 % 0.0 PL	F						
Yield Limit per Yield Limit per		245.6 I 81.9 lb							
Yield Mode Edge Distance		IV 1 1/2"							
Min. End Dista	ance	3"							
Load Combina Duration Facto		1.00							
Notes			hemicals		 For flat roofs pr ponding 	rovide proper drainage	e to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
structural adequad design criteria	red Designs is responsible cy of this component bas and loadings shown ne customer and/or the co	sed on the 1.L It is the 2.R	Ndling & Install VL beams must not b Refer to manufac	e cut or drilled turer's product information	nun8			Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	Fayetteville, NC USA 28314 910-864-TRUS
ensure the com	ponent suitability of the verify the dimensions and I	e intended fa loads. a	egarding installatic astening details, bea pprovals)amaged Beams mus	on requirements, multi-ply am strength values, and code at not be used				(800) 622-5850 www.metsawood.com/us	
1. Dry service cor	nditions, unless noted other treated with fire retardant o	rwise 5. P	esign assumes top e	edge is laterally restrained ort at bearing points to avoid	This design i	s valid until 11/3/2	2024		соттесн
	17 Poworod by iStrue							•	STATE AND A

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

1	Davier	Client: Project:	The Ashvi		ate		Date Inpu	t by:	8/10/202 David La	ndry				Page 3 of
19	Design	Address:	24 Melvi	ll Lane					Lot 46 Lil	•	adows			
	Karta C IVI	4 7501	V 4 4 0	0011 0				ect #:	evel: Level	173				
BM2	Kerto-S LVI	_ 1.750	X 14.0	00" 2	-Piy - P	A2:	SED							
1 SPF				• • • • • • • • • • • • • • • • • • •										1'2" 3 1/2"
	<i>•</i> ··										<u></u>			
	formation Girder	Appli	cation:	Floor		1					· · /	Snow	Wind	000
Type: Plies:	2		n Method:	ASD		Brg 1	Direct Vertica		Live 678	L	Dead 709	Snow 0	vvina 0	Coi
Moisture Cor		-	ng Code:	IBC/IRC 201	5	2	Vertica		189		546	0	0	
Deflection LL		Load	Sharing:	No										
Deflection TL		Deck:		Not Checked	I									
Importance: Temperature	Normal - II Temp <= 100°F													
remperature	100 F					Bea	rings							
							aring L	enath	Dir.	Cap. R	eact D/L II	o Total	Ld. Case	Ld. Con
							SPF 3		Vert	27%	709 / 67			D+L
						2-	SPF 3	.500"	Vert	14%	546 / 18	9 734	L	D+L
Analysis Re														
Analysis		ocation Allowed	Capacit	-	Case									
Moment		2'8 7/16" 26999 ft-lt		%) D+L	L									
Unbraced	1305 ft-lb 2 1165 lb	2'8 7/16" 17702 ft-lt 1'5 1/2" 10453 lb		%) D+L 1%) D+L	L									
Shear LL Defl inch		2'7 3/8" 0.139 (L/4		,	L									
LL Den inch	(L/21799)	213/8 0.139 (L/4	0.022 (2	70) L	L									
TL Defl inch	0.008 (L/8727) 2'	10 1/16" 0.185 (L/3	60) 0.041 (4	%) D+L	L									
Design No	tes					1								
 may also b 2 Fasten all to exceed 3 Refer to la 4 Concentra present. 5 Girders and 6 Top loads 7 Top must b 	pport to prevent lateral be required at the interiv plies using 3 rows of 10 6". st page of calculations ted load fastener speci e designed to be suppor- must be supported equ be laterally braced at er ust be laterally braced at	or bearings by the build Box nails (.128x3) for fasteners require fication is in addition rted on the bottom e ally by all plies. nd bearings.	uilding code. ") at 12" o.c. I d for specifie to hanger fas	Maximum end d loads.	distance not									
	nderness ratio based o							~				. or -		
ID	Load Type	Location	Trib Width		Dead 0.9		Live 1	Snov			6 Const.		mments	
1	Uniform			Тор	150 PLF		0 PLF		0 PLF	0 PLF			all Above	
2	Point	1-1-0		Far Face	238 lb		714 lb		0 lb	0 11		0 lb F2	A	
3	Point	3-1-0		Far Face	51 lb		153 lb		0 lb	0 18	C	0 lb F7		
	Self Weight				11 PLF									
structural adequacy design criteria an esponsibility of the ensure the compo- application, and to ve	d Designs is responsible only of th of this component based on th d loadings shown. It is th customer and/or the contractor nent suitability of the intende rifly the dimensions and loads.	 LVL beams must not be Refer to manufactor regarding installation fastening details, beat approvals Damaged Beams must 	e cut or drilled turer's product i n requirements, m strength values, t not be used	pond nformation multi-ply and code	flat roofs provide p ling	proper dra	inage to pre		Manufacture Metsä Wood 301 Merritt 7 Norwalk, CT (800) 622-58 www.metsaw	Building, 2 06851 50		Comtech 1001 S. Fayettev USA 28314 910-864	Reilly Road, Suite a ille, NC	#639
Lumber 1. Dry service cond 2. LVL not to be tre	itions, unless noted otherwise ated with fire retardant or corrosiv	 Design assumes top e Provide lateral support lateral displacement at 	dge is laterally restr rt at bearing point	s to avoid	s design is valid		0.0000					le	OMT	есн



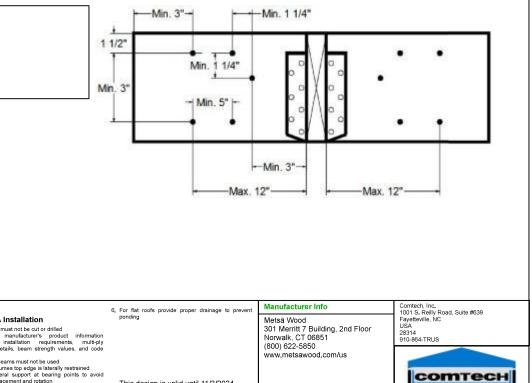
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	
		_

Concentrated Load

Fasten at concentrated side load at 1-1-0 with a minimum of (6) – 10d Box nails (.128x3") in the ببيرهماء مسمعهم

pattern snown.		
Capacity Load	96.9 %	
Load	476.0lb.	
Total Yield Limit	491.0 lb.	
Cg	0.9998	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Load Combination	D+L	
Duration Factor	1.00	

Min/Max fastener distances for Concentrated Side Loads



			_
Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Ν
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.	Handling & Installation 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals	ponding	N S M () V
Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	 Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation 	This design is valid until 11/3/2024	

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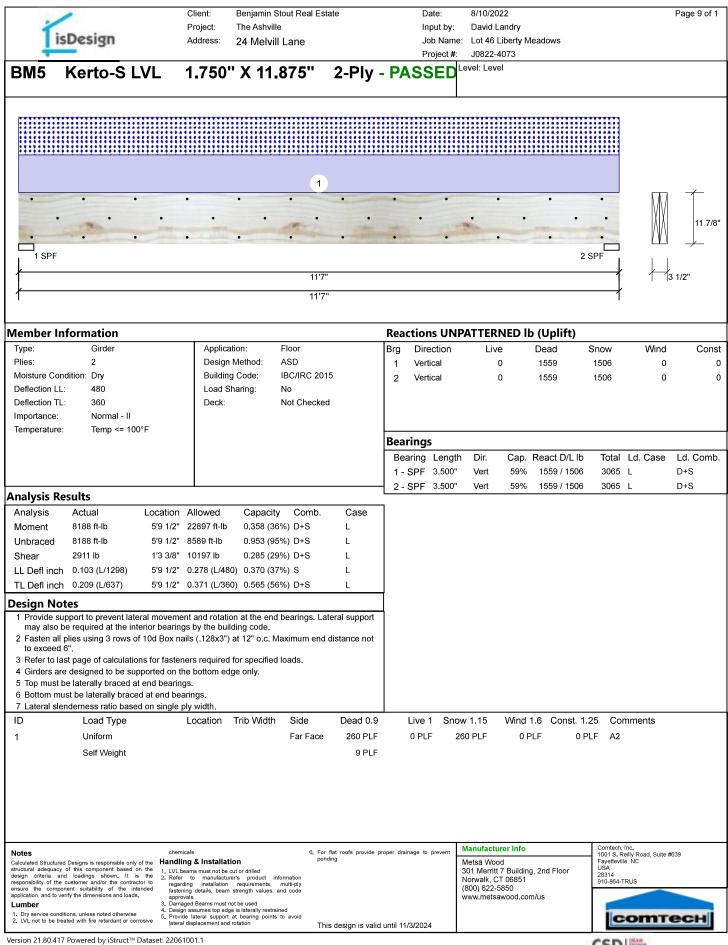
			Client:	Benjamin Sto	ut Real Estate	е		Date:	8/10/20	22				Page 5 of 1
Tie	Design			The Ashville				Input by:	David L		adove			
15	resign		Address:	24 Melvill L	ane			Job Name Project #:	Lot 46		eauows			
BM3	S-P-F #2	2.0	00" X ⁻	12.000	" 2-P	ly - P	ASSE	D	_evel: Lev	əl				
					1									
•	• •	•	•	•	•	•	-		•	•	•		M	$\overline{1}$
	CARE	-	-	-	at 17		-	YUT	W1.			1000	XIX	11 1/4"
•	•		•	•	Fortant •		1000	St. Has	•	•	•		<u> </u>	<u> </u>
1 SPF Er	nd Grain										2 SPF End	Grain		
					12'3 1/2"								<u>[</u> 3'	1
1					12'3 1/2"	1						1		
Member In							1				(Uplift)			
Type: Plies:	Girder 2		Applicati Design N		loor \SD		Ŭ	rection ertical	Liv	e 0	Dead 799	Snow 799	Wind 0	Const 0
Moisture Cond			Building		BC/IRC 2015			ertical		0	799	799	0	0
Deflection LL:	480 360		Load Sha Deck:	-	lo Ist Checked									
Deflection TL: Importance:	360 Normal - II		Deck:	N	lot Checked									
Temperature:	Temp <= 100	°F					. .							
							Bearing	gs g Length	n Dir.	Can	React D/L lb	Total	Ld. Case	Ld. Comb.
								= 3.500"	Vert	36%	799 / 799			D+S
Analysis Bo	culto						End Grain							
Analysis Re Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2 - SPF	3.500"	Vert	36%	799 / 799	1598	L	D+S
Moment	4551 ft-lb	6'1 3/4"	5306 ft-lb	0.858 (86%		L	End Grain							
Unbraced	4551 ft-lb	6'1 3/4"	4552 ft-lb	1.000 (100%)	D+S	L								
Shear	1278 lb	1'2 3/4"	3493 lb	0.366 (37%) D+S	L								
LL Defl inch	0.115 (L/1234)		0.296 (L/480)			L								
	0.230 (L/617)	6'1 3/4"	0.394 (L/360)) 0.584 (58%) D+S	L	-							
Design Not	port to prevent late	ral moveme	ent and rotation	at the end b	earings Later	al support	4							
may also b	e required at the inte lies using 2 rows of	erior bearing	gs by the build	ing code.		••								
to exceed 6	5".					stance not								
	t page of calculation designed to be sup		•	•	bads.									
	nust be supported e e laterally braced at		-	c										
7 Bottom mu	st be laterally brace	d at end bea	arings.											
8 Lateral sler	iderness ratio based Load Type	t on single p	-	Frib Width	Side	Dead 0.9	L ive	e 1 Snov	w 1.15	Wind 1	.6 Const. 1	25 Cor	nments	
1	Uniform		Loodion		Тор	130 PLF	0 P		30 PLF	0 P		PLF D1		
									Manufactu	irer Info		Comtech,	Inc. eilly Road, Suite #	630
								Γ				Fayettevill USA	e, NC	
												28314 910-864-T	RUS	
					This c	design is valio	l until 11/3/20	24				C	OMT	есн
Version 21.80.417	Powered by iStruct™ [Dataset: 2206	1001.1									CSD	DRAW	
												200	1 MALLO	

Í	isDesign	Pr	oject: The	amin Stout Re Ashville /Ielvill Lane	al Estate			8/10/2022 David Landry e: Lot 46 Liberty Meadows	Page 6 of 1
BM3	S-P-F #	¢2 2.00	0" X 12.	000"	2-Ply	- PASSE	Project #:	J0822-4073 Level: Level	
	•	• •	•	•	•	•	•	• • •	
	•		•	•	•	•	•		
	F End Grain			-				2 SPF End G	
					2'3 1/2" 2'3 1/2"				
				Ι.	231/2				I
Multi-Ply			ila (120	211 - (121					
Capacity	plies using 2	0.0 % 0.0 PLF	x nalis (.128	x3) at 12	o.c Maxir	num end dis	stance no	ot to exceed 6".	
Load Yield Limit pe		157.4 PLF							
Yield Limit pe Yield Mode		78.7 lb. IV							
Edge Distand Min. End Dis		1 1/2" 3"							
Load Combin Duration Fac	nation	1.00							
							[Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road. Suite #639
									Fayetteville, NC USA 28314 910-864-TRUS
					This design	is valid until 11/3/	2024		соттесн

Address: 24 Melvill Lane Lob Name: Lot 46 Liberty Meadows Project #: J0822-4073 BM4 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED Level: Level	9 ·
4 3 3 1 SPF End Grain 8'7"	////
2 3 1 1 1 1 1 SPF End Grain 2 SPF End Grain 8'7''	////
1 SPF End Grain 2 SPF End Grain 8'7"	////
1 SPF End Grain 8'7"	////
1 SPF End Grain 2 SPF End Grain 8'7"	////
1 SPF End Grain 2 SPF End Grain 8'7"	////
8'7"	
8'7"	. 3 1/2"
łł	1 10 1/2
	•
Member Information Reactions UNPATTERNED Ib (Uplift) Type: Girder Application: Floor Brg Direction Live Dead Snow	Wind Con
Plies: 2 Design Method: ASD 1 Vertical 1330 2005 240	0
Moisture Condition: Dry Building Code: IBC/IRC 2015 2 Vertical 1330 2005 240 Deflection LL: 480 Load Sharing: No 100 <td>0</td>	0
Deflection TL: 360 Deck: Not Checked	
Importance: Normal - II	
Temperature: Temp <= 100°F	
Bearings	
	Ld. Case Ld. Coml
1 - SPF 3.500" Vert 32% 2005 / 1330 3335 End	L D+L
Analysis Results Grain	
Analysis Actual Location Allowed Capacity Comb. Case 2 - SPF 3.500" Vert 32% 2005 / 1330 3335 End	L D+L
Moment 6413 ft-lb 4'3 1/2" 12542 ft-lb 0.511 (51%) D+L L Grain	
Unbraced 6413 ft-lb 4'3 1/2" 8468 ft-lb 0.757 (76%) D+L L	
Shear 2515 lb 1' 3/4" 6907 lb 0.364 (36%) D+L L	
LL Defl inch 0.075 (L/1301) 4'3 9/16" 0.203 (L/480) 0.369 (37%) L L	
TL Defl inch 0.188 (L/519) 4'3 9/16" 0.271 (L/360) 0.694 (69%) D+L L	
Design Notes	
 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". 	
3 Refer to last page of calculations for fasteners required for specified loads.	
4 Girders are designed to be supported on the bottom edge only.	
5 Top loads must be supported equally by all plies. 6 Top must be laterally braced at end bearings.	
7 Bottom must be laterally braced at end bearings.	
8 Lateral slenderness ratio based on single ply width.	mmonto
	omments
1 Uniform Top 104 PLF 310 PLF 0 PLF 0 PLF 0 PLF F1	
2 Uniform Top 56 PLF 0 PLF 0 PLF 0 PLF 0 PLF M1 2 Uniform Top 56 PLF 0 PLF	
	all Above
4 Uniform Top 180 PLF 0 PLF 0 PLF 0 PLF 0 PLF 0 PLF C1G	JE
Self Weight 7 PLF	
Manufacturer Info Comtech, I	Inc.
Notes chemicais contentiation ponding proper drainage to prevent Calculated Structured Designs is responsible only of the Handling & Installation ponding ponding variety of the Handling & Installation ponding ponding proper drainage to prevent for the Handling & Installation ponding po	Reilly Road, Suite #639
structural adequacy of this component based on the 1, LVL beams must not be cut or drilled 301 Merritt 7 Building, 2nd Floor USA 28314 28314 28314	TDUE
responsibility of the customer and/or the contractor to regarding installation requirements, multi-by (800) 622-5550 910-304-11	60/1
Lumber 3. Damaged Beams must not be used 4. Defining accurate the definition of the	
	OMTECH
fersion 21.80.417 Powered by iStruct™ Dataset: 22061001.1	

			Client:	Benjamin Stout Re	eal Estate		Date:	8/10/2022	Page 8 of 1
2			Project:	The Ashville			Input by:	David Landry	
1	isDesign		Address:	24 Melvill Lane				Lot 46 Liberty Meadows	
-							Project #:	J0822-4073	
BM4	Kerto-S	LVL	1.750'	' X 9.250"	2-Ply	- PAS	SED I	_evel: Level	
				/	_ · · ,				
							I		
	•	•		•	•	•	•	•	• • •
	•	•		•	•	•	•	•	• <u> </u>
									9 1/4
•	•	•		•	•	•	•	•	• <u>+</u> <u>¥</u> <u>¥</u> <u>¥</u>
	'F End Grain							2 SPF End Gr	
					01711				
					8'7"				1 1/2"
1					8'7"				1
Multi-Ply	Analysis								
_	-	we of 10d	Boy nails ((128v3") at 12"	o.c. Maximi	ım end di	stance no	ot to exceed 6".	
Capacity	plies using 2 re	0.0 %			0.0 เพื่องการ	in chu ui	stance ne		
Load		0.0 PLF							
Yield Limit per		163.7 PLF	=						
Yield Limit pe Yield Mode	r Fastener	81.9 lb. IV							
Edge Distance	e	1 1/2"							
Min. End Dist		3"							
Load Combina Duration Fact		1.00							
Duration r dot	.01	1.00							
Notes		chem	icals		6. For flat roofs provi	ide proper drainaç	ge to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structu	ured Designs is responsible o icy of this component base	only of the Handli			ponding		Γ	Metsä Wood 301 Merritt 7 Building, 2nd Eloor	Fayetteville, NC USA
design criteria responsibility of th	and loadings shown. It he customer and/or the cor	is the 2 Refer	to manufacture	cut or drilled er's product information requirements, multi-ply				301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
ensure the com application, and to	ponent suitability of the verify the dimensions and lo	intended faster ads. appro	ning details, beam vals	strength values, and code				(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service col	nditions, unless noted otherw	3. Dama 4. Desig vise 5. Provid	iged Beams must n in assumes top edg	e is laterally restrained					
2. LVL not to be	treated with fire retardant or		de lateral support I displacement and	at bearing points to avoid rotation	This design is v	/alid until 11/3/	2024		соттесн
Version 21.80.4	17 Powered by istruct	M Datacet: 2206	1001 1						CONTRACTOR AND

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1



	1		Client:	Benjamin Stout R	eal Estate	Date:	8/10/2022	Page 10 of 1
Í	isDesign		Project: Address:	The Ashville 24 Melvill Lane	9		ne: Lot 46 Liberty Meadows	
BM5	Kerto-S	LVL	1.750'	' X 11.87	5" 2-Plv	Project #	#: J0822-4073 Level: Level	
				// 1110/ 4	,			
	•	•	•	• •	•	• •	• •	
				•••••••••••••••••••••••••••••••••••••••	• •		• •	· · · · · · · · · · · · · · · · · · ·
	:							
					11'7"			3 1/2"
1					11'7"			ł
Multi-Ply	Analysis							
-	-	ws of 10d	Box nails	(.128x3") at 12'	" o.c Maximun	n end distance r	not to exceed 6".	
Capacity Load		92.1 % 260.0 PLI	=					
Yield Limit pe Yield Limit pe		282.4 PLI 94.1 lb.	=					
Yield Mode		IV						
Edge Distanc Min. End Dist		1 1/2" 3"						
Load Combin Duration Fact		D+S 1.15						
Notes		chem		ion	6. For flat roofs provide ponding	proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
structural adequa design criteria responsibility of t	ured Designs is responsible on acy of this component based and loadings shown. It the customer and/or the contr	on the 1. LVL b is the 2. Refer actor to regar	eams must not be to manufactur				Metsä Wood 301 Merritt 7 Building, 2nd Floo Norwalk, CT 06851	Favetteville, NC USA 28314 910-864-TRUS
ensure the con application, and to Lumber	nponent suitability of the i o verify the dimensions and load	ntended faster ds appro 3 Dama	ning details, beam wals aged Beams must r	strength values, and code tot be used			(800) 622-5850 www.metsawood.com/us	
2. LVL not to be	onditions, unless noted otherwis treated with fire retardant or c	orrosive latera	de lateral support I displacement and	e is laterally restrained at bearing points to avoid rotation	This design is vali	id until 11/3/2024		соттесн
Vorcion 21.80 /	117 Powered by iStruct™	Unstacet: 2206	10011					

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

CSD

is	Design	F	Client: Project: Address:	The Ashville 24 Melvill	Lane		Job Pro	ut by: Name: bject #:	J0822-40	ndry berty Meadow 073	/S			Page 11 o
GDH I	Kerto-S L\	/L 1.	750" 2	X 11.87	75" 2-	Ply - P	ASSED)	evel: Level					
		2							3					
					1		• •			•	•		M	1
1 SPF End	d Grain		.西		ANT DE		10			23	• SPF End (Grain		11 7/8"
					16'10" 16'10"								1]3	1/2"
lember Inf	formation						Reaction	s UNP	ATTERN	IED lb (Up	lift)			
Туре:	Girder		Applicat		Floor		Brg Dire	ction	Live	Dead	a :	Snow	Wind	Cor
Plies: Moisture Conc Deflection LL: Deflection TL:	480		Design I Building Load Sh Deck:	Code:	ASD IBC/IRC 2015 No Not Checked		1 Vertio		0 0	2266 2266		168 168	0 0	
Importance:	Normal - II													
Temperature:	Temp <= 100	°F					Boarings							
							Bearings Bearing 1 - SPF End	-	Dir. Vert	Cap. Reac 24% 220	t D/L lb 36 / 168	Total 2434	Ld. Case L	Ld. Con D+S
nalysis Re	sults		•				Grain 2 - SPF	3 500"	Vert	24% 220	66 / 168	2434	I	D+S
Analysis Moment	Actual 9024 ft-lb	Location A 8'5" 1	Allowed 17919 ft-lb	Capacity 0.504 (50		Case Uniform	End	0.000	Vort	2470 220	507 100	2404	L	0.0
Unbraced	9694 ft-lb		9704 ft-lb	0.999	D+S	L	Grain							
Shear	1930 lb	15'6 5/8" 7	7980 lb	(100%) 0.242 (24	%) D	Uniform								
	0.035 (L/5617)			0.242 (24		L								
	0.506 (L/388)) 0.927 (93		L								
esign Not	es						1							
	port to prevent late e required at the inte				bearings. Later	ral support								
2 Fasten all p to exceed 6	lies using 2 rows of	10d Box nail	s (.128x3") a	at 12" o.c. M	aximum end di	stance not								
3 Refer to las4 Girders are	t page of calculatior designed to be sup nust be supported e	ported on the	bottom edg	•	loads.									
6 Top must be	e laterally braced at	a maximum o	of 9'6 3/4" o	.C.										
	st be laterally braced derness ratio based		0											
ID	Load Type	L	ocation	Trib Width	Side	Dead 0.9	Live 1		w 1.15	Wind 1.6 C			mments	
1	Uniform				Тор	180 PLF	0 PLF		0 PLF	0 PLF	0 PI			
2	Uniform Tio In	0.0.0.1	16 10 0	100	Тор Тор	60 PLF	0 PLF 0 PSF		0 PLF 20 PSF	0 PLF 0 PSF	0 PI		ll Above	
3	Tie-In Self Weight	0-0-0 10	16-10-0	1-0-0	Тор	20 PSF 9 PLF	UFSF	2	.v F 0F	U F OF	0 P\$	or ⊓00	of Load	
Notes		chemica	ls				roper drainage to p	revent	Manufactur	er Info		Comtech, 1001 S F	Inc. eilly Road, Suite #	1639
Calculated Structured tructural adequacy of lesign criteria and esponsibility of the of nsure the compon- pplication, and to veri _umber	Designs is responsible only o of this component based or loadings shown. It is ustomer and/or the contract ent suitability of the inte fy the dimensions and loads.	of the Handling 1. LVL bea the 1. LVL bea 2. Refer regardin fastening approva 3. Damage 4. Design a	g & Installations ms must not be cu to manufacture g installation g details, beam s ls d Beams must no assumes top edge	it or drilled r's product int requirements, strength values, a t be used is laterally restrai	ponding formation multi-ply ind code ned				Norwalk, CT (800) 622-58	Building, 2nd F 06851	loor	Fayettevil USA 28314 910-864-	le, NC	
 Dry service conditie 	ons, unless noted otherwise ted with fire retardant or corn	5. Provide	lateral support a isplacement and r	t bearing points	to avoid	design is valid						C	OMT	есн

Ť	isDesign		Client: Project: Address:	Benjamin Stout Re The Ashville 24 Melvill Lane		Date: Input by Job Nan	ne: Lot 46 Liberty Meadows	Page 12 of 1
GDH	Kerto-S	LVL	1.750"	X 11.875"	2-Ply -	Project #	⊭: J0822-4073 Level: Level	
·	•••	•	•••	•••	•••	• •		
	End Grain				16'10"		2 SPF End	Grain 7
 					16'10"			1
Capacity Load Yield Limit pe Yield Mode Edge Distanc Min. End Dist Load Combin Duration Fact	plies using 2 r r Foot r Fastener e ance ation	0.0 % 0.0 F	6 PLF 7 PLF Ib.	(.128x3") at 12"	6. For flat roofs prov	um end distance r	not to exceed 6".	Contech. Inc.
structural adequa design criteria responsibility of t ensure the con application, and to Lumber 1. Dry service co 2. LVL not to be	red Designs is responsible cy of this component ba and loadings shown, he customer and/or the o opponent suitability of th verify the dimensions and nditions, unless noted othe treated with fire retardant 1 17 Desugrad by sitter.	sed on the 1 It is the 2 ontractor to loads. 7 wise 5 or corrosive	Andling & Installa LVL beams must not be Refer to manufacturegarding installatior fastening details, bear approvals Damaged Beams must Design assumes top ec Provide lateral suppor lateral displacement an	cut or drilled urer's product information requirements, multi-ply n strength values, and code not be used lge is laterally restrained t at bearing points to avoid	ponding	valid until 11/3/2024	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	1001 S. Relly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

CSD



RE: J0822-4073 Lot 46 Liberty Meadows Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Address: 24 Melvill Lane

City:

Customer: Benjamin Stout Real Estate Project Name: J0822-4073 Lot/Block: 46 Model: Ashville Subdivision: Liberty Meadows State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 13 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E16391095	ET1	11/9/2021
2	E16391095	ET2	11/9/2021
3	E16391097	F1	11/9/2021
4	E16391098	F2	11/9/2021
5	E16391099	F2A	11/9/2021
6	E16391100	F3	11/9/2021
7	E16391101	F4	11/9/2021
8	E16391102	F4A	11/9/2021
9	E16391103	F5	11/9/2021
10	E16391104	F6	11/9/2021
11	E16391105	F7	11/9/2021
12	E16391106	FG1	11/9/2021
13	E16391107	FG2	11/9/2021

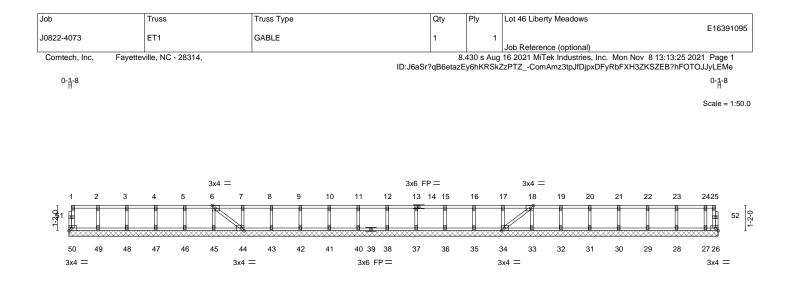
The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



November 09, 2021



29-11-0 + 1-4-0 + 2-8-0 + 4-0-0 + 5-4-0 + 6-8-0 + 8-0-0 + 9-4-0 + 10-8-0 + 12-0-0 + 13-4-0 + 14-8-0 + 16-0-0 + 17-4-0 + 18-8-0 + 20-0-0 + 21-4-0 + 22-8-0 + 24-0-0 + 25-4-0 + 28-0-0 + 29-4-0 + 1-4-0

Flate Olisets (X, I)	[0.0-1-0,Euge], [10.0-1-0,Euge], [34.0-1	1-0,Eugej, [44.0-1-0,Euge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. i Vert(LL) n/i Vert(CT) n/i Horz(CT) -0.00	a - n/a 999	PLATES MT20 Weight: 128 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.1 (flat) P No.1 (flat) P No.3 (flat) P No.3 (flat)	individ O	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o 10-0-0 oc bracing: 49-50,48-4	ectly applied or 6-0-0 or 6-0-0 c bracing, E	oc purlins, xcept:

REACTIONS. All bearings 29-11-0.

(lb) - Max Uplift All uplift 100 lb or less at joint(s) 26

Max Grav All reactions 250 lb or less at joint(s) 50, 49, 48, 47, 46, 45, 44, 43, 42, 41, 40, 38, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 27

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26.

7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



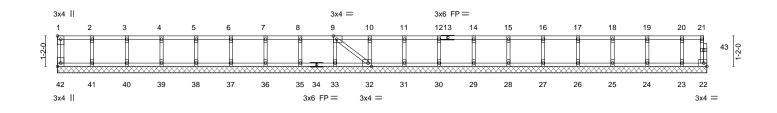
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 46 Liberty Meadows
J0822-4073	ET2	GABLE	1	1	E16391096
30822-4073		GABLE	1	· ·	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:26 2021 Page 1

ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-g?KYzJ3Vacn4KyWPpfyqnlpEHkoozeErU2CxrlyLEMd 0-1-8

Scale = 1:41.7



 1-4-0
 2-8-0
 4-0-0
 5-4-0
 6-8-0
 9-4-0
 10-8-0
 12-0-0
 13-4-0
 14-8-0
 16-0-0
 17-4-0
 18-8-0
 20-0-0
 21-4-0
 22-8-0
 24-0-0
 24-11-8

 1-4-0
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LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S		in (loc) /a - /a - 00 22	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 106 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 Si BOT CHORD 2x4 Si WEBS 2x4 Si	BRACING- TOP CHORD BOT CHORD	except	t end vert	icals.	irectly applied or 6-0-0 or 10-0-0 or 10-0-0 or bracing.	oc purlins,		

2x4 SP No.3(flat) WFBS 2x4 SP No.3(flat) OTHERS

REACTIONS.

 All bearings 24-11-8.
 (lb) - Max Grav All reactions 250 lb or less at joint(s) 42, 22, 41, 40, 39, 38, 37, 36, 35, 33, 32, 31, 30, 29, 28, 27, 26, 25, 24, 23

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

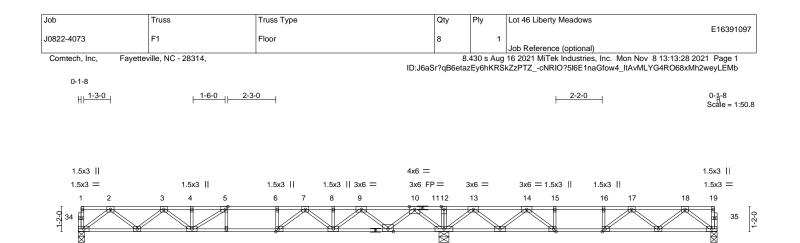
7) CAUTION, Do not erect truss backwards.



November 9,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling in trusses and truss systems, see **AVSUPPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





27 26

4x6 =

3x6 FP =

25

3x10 =

24

3x6 =

23

22

21

20

3x6 =

	<u> </u>						29-1 12-9		
Plate Offsets (X,Y)	[5:0-1-8,Edge], [22:0-1-8,Edge], [23:0-1	-8,Edge], [29:0-1-8,Edge]							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.86 BC 0.85 WB 0.58 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.20 -0.28 0.05	(loc) 30 30 20	l/defl >999 >735 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 149 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1 (flat) P No.1 (flat) P No.3 (flat)		BRACING- TOP CHOR BOT CHOR	D	except	end verti	cals.	rectly applied or 6-0-0 o or 6-0-0 oc bracing.	oc purlins,
REACTIONS. (siz	e) 33=0-3-8, 25=0-5-8, 20=0-3-8								

Max Grav 33=826(LC 3), 25=1934(LC 1), 20=608(LC 4)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-1695/0, 3-4=-2732/0, 4-5=-2732/0, 5-6=-2963/0, 6-7=-2963/0, 7-8=-2067/0, 8-9=-2067/0, 9-10=-539/303, 10-12=0/2152, 12-13=0/2152, 13-14=-551/972, 14-15=-1575/269, 15-16=-1575/269, 16-17=-1575/269, 17-18=-1162/0 BOT CHORD

32-33=0/1030, 31-32=0/2330, 30-31=0/2963, 29-30=0/2963, 28-29=0/2561, 26-28=-37/1417, 25-26=-833/0, 24-25=-1263/0, 23-24=-662/1135, 22-23=-269/1575, 21-22=-24/1515, 20-21=0/747 WEBS 2-33=-1290/0, 2-32=0/866, 3-32=-826/0, 3-31=0/514, 10-25=-1655/0, 10-26=0/1228, 9-26=-1188/0, 9-28=0/878, 7-28=-685/0, 7-29=0/807, 6-29=-359/0, 5-31=-475/153, 13-25=-1321/0, 13-24=0/882, 14-24=-926/0, 14-23=0/942, 18-20=-935/0, 18-21=0/540, 17-21=-460/99, 17-22=-345/76, 15-23=-417/0

NOTES-

33

3x6 =

32

31

30

3x6 = 1.5x3 ||

29

28

3x6 =

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

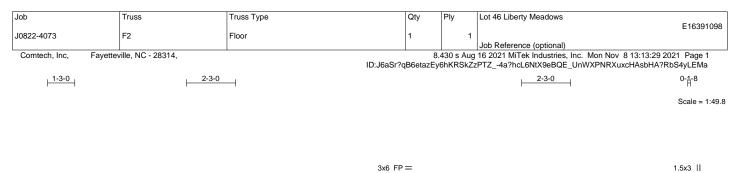
Strongbacks to be attached to walls at their outer ends or restrained by other means. 5) CAUTION, Do not erect truss backwards.



November 9,2021

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This does not have a seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSETPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





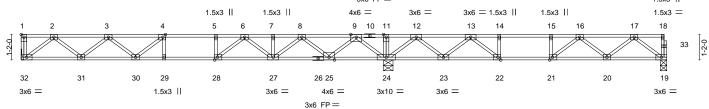


Plate Offsets (X,Y)	16-7-8 16-7-8 [1:Edge,0-1-8], [4:0-1-8,Edge], [21:0-1-	8,Edge], [22:0-1-8,Edge],	16 ₁ 9-0 0-1-8 [28:0-1-8,Edge]			29-7-8 12-10-8		
LOADING(psf)TCLL40.0TCDL10.0BCLL0.0BCDL5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.87 BC 0.85 WB 0.57 Matrix-S	Vert(CT) -0	in (loc)).18 28-29).25 28-29).05 19	l/defl >999 >810 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 146 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 S WEBS 2x4 S REACTIONS. (s	SP No.1(flat) SP No.1(flat) SP No.3(flat) ze) 32=Mechanical, 19=0-3-8, 24=0-5- Grav 32=814(LC 3), 19=615(LC 4), 24=1		BRACING- TOP CHORD BOT CHORD	except	end vert	icals.	ttly applied or 6-0-0 o	oc purlins,
FORCES. (lb) - Mai TOP CHORD 2-3 8-9 14- BOT CHORD 31-	c. Comp./Max. Ten All forces 250 (lb) o =-1658/0, 3-4=-2582/0, 4-5=-2848/0, 5-6= =-563/296, 9-11=0/2082, 11-12=0/2082, 15=-1611/221, 15-16=-1611/221, 16-17=: 32=0/1002, 30-31=0/2282, 29-30=0/2848	less except when shown -2848/0, 6-7=-2034/0, 7-8 12-13=-578/905, 13-14=-1 1179/0 , 28-29=0/2848, 27-28=0/	3=-2034/0, 611/221, 2500,					
20- WEBS 2-3 9-2 17-	27=-36/1414, 24-25=-810/0, 23-24=-1186 21=0/1540, 19-20=0/756 2=-1257/0, 2-31=0/854, 3-31=-813/0, 3-3 5=0/1194, 8-25=-1155/0, 8-27=0/843, 6-2 19=-946/0, 17-20=0/550, 16-20=-470/84, 23=0/876, 13-23=-920/0, 13-22=0/936, 1-	0=0/392, 4-30=-431/27, 9- 7=-652/0, 6-28=0/750, 5-2 16-21=-317/90, 12-24=-13	24=-1619/0, 28=-323/0,					

NOTES-

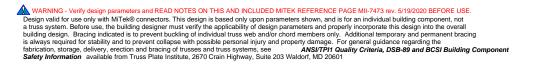
1) Unbalanced floor live loads have been considered for this design.

- All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

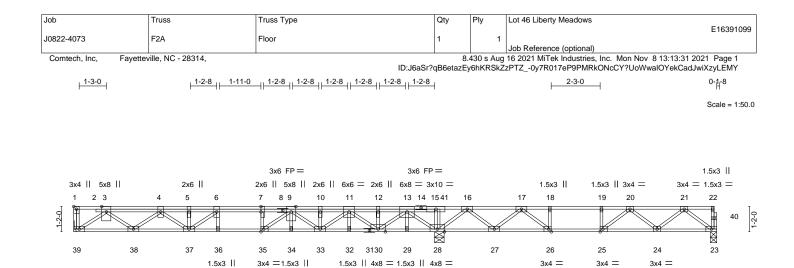
6) CAUTION, Do not erect truss backwards.



November 9,2021







3x6 FP =

16₁9-0

29-7-8

	16-7-8		0-1-8		12-10-		
Plate Offsets (X,Y)-		8,Edge], [26:0-1-8,Edge],			12 10	0	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.66 BC 0.48 WB 0.69 Matrix-S	Vert(LL) -0.15) 35-36 >993	L/d 480 360 n/a	PLATES MT20 Weight: 173 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 WEBS 2x4 REACTIONS.	4 SP 2400F 2.0E(flat) 4 SP 2400F 2.0E(flat) 4 SP No.3(flat) (size) 39=Mechanical, 28=0-5-4, 23=0-3- ax Grav 39=951(LC 3), 28=2436(LC 1), 23=		BRACING- TOP CHORD BOT CHORD	except end verti	cals.	ectly applied or 6-0-0 c	oc purlins,
FORCES. (Ib) - M TOP CHORD 3- 10	lax. Comp./Max. Ten All forces 250 (lb) or -4=-2099/0, 4-5=-3320/0, 5-6=-3320/0, 6-7= 0-11=-2750/0, 11-12=-1201/0, 12-13=-1201 6-17=-224/1085, 17-18=-1394/330, 18-19=	eless except when shown -3521/0, 7-9=-3521/0, 9- /0, 13-15=0/2484, 15-16=	10=-2750/0, =0/2500,				
BOT CHORD 33	8-39=0/1255, 37-38=0/2907, 36-37=0/3521 32-33=0/2041, 30-32=0/2041, 29-30=-501/7 6-27=-753/865, 25-26=-330/1394, 24-25=-5	, 35-36=0/3521, 34-35=0/ 7, 28-29=-501/77, 27-28=	/3180, 33-34=0/3180,				
WEBS 3- 1: 1:	-39=-1541/0, 3-38=0/1072, 4-38=-1027/0, 4 3-30=0/1449, 11-30=-1115/0, 11-33=0/936, 6-28=-1479/0, 6-37=-372/209, 16-27=0/905 21-23=-888/0, 21-24=-6/501, 20-24=-405/11	-37=0/515, 5-37=-259/12 9-33=-593/0, 9-35=0/799 , 17-27=-970/0, 17-26=0/	9, 7-35=-397/0,				
NOTES-	r live loads have been considered for this d	ocian					

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x6 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

16-7-8

6) CAUTION. Do not erect truss backwards.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 222 lb down at 4-1-4, and 576 Ib down at 15-1-12 on top chord. The design/selection of such connection device(s) is the responsibility of others. 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

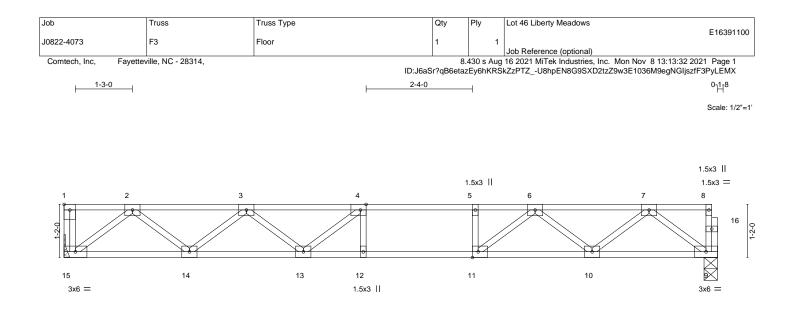
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 23-39=-10, 1-22=-100

Concentrated Loads (lb) Vert: 4=-142(F) 13=-496(F)



818 Soundside Road Edenton, NC 27932

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L			14-4-0			
			14-4-0			1
Plate Offsets (X,Y)	[1:Edge,0-1-8], [4:0-1-8,Edge], [11:0-1-	8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.60 BC 0.87 WB 0.38 Matrix-S	Vert(LL) -0.19	n (loc) I/defi L/d 9 12-13 >906 480 5 12-13 >687 360 4 9 n/a n/a	PLATES MT20 Weight: 71 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)	1	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	<i>,</i> , , , , , , , , , , , , , , , , , ,	oc purlins,
REACTIONS. (size	e) 15=Mechanical, 9=0-3-8					

Max Grav 15=775(LC 1), 9=768(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1559/0, 3-4=-2384/0, 4-5=-2550/0, 5-6=-2550/0, 6-7=-1538/0

 $14\text{-}15\text{=}0/947, \, 13\text{-}14\text{=}0/2143, \, 12\text{-}13\text{=}0/2550, \, 11\text{-}12\text{=}0/2550, \, 10\text{-}11\text{=}0/2118, \, 9\text{-}10\text{=}0/954$ BOT CHORD WEBS 2-15=-1188/0, 2-14=0/797, 3-14=-761/0, 3-13=0/398, 7-9=-1194/0, 7-10=0/760, 6-10=-755/0, 6-11=0/740, 5-11=-317/0, 4-13=-437/18

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

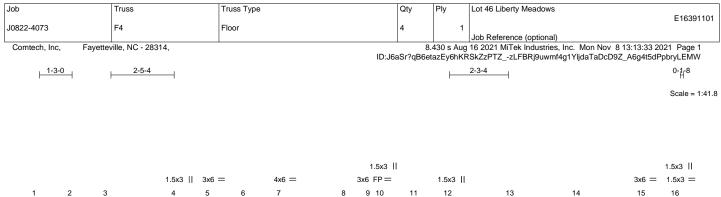
6) CAUTION, Do not erect truss backwards.



November 9,2021

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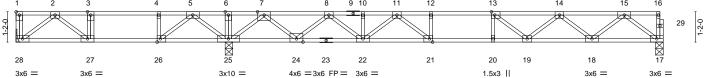


Plate Offsets (X,Y)		- <u>2-8</u> -0-4 1-8.Edge], [26:0-1-8.Edge]		24-11-8 16-9-0			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.85 BC 0.86 WB 0.54 Matrix-S	DEFL. ir Vert(LL) -0.21	n (loc) l/defl 1 21-22 >964 3 21-22 >719 5 17 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 125 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 S	SP No.1(flat) SP No.1(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	except end vert	icals.	ectly applied or 6-0-0 or 6-0-0 or 6-0-0 oc bracing.	oc purlins,
FORCES. (Ib) - Ma TOP CHORD 1-2 7-8 13- BOT CHORD 27- 21- WEBS 2-2 15-	 ize) 28=Mechanical, 25=0-3-8, 17=0-3 Grav 28=1746(LC 3), 25=1571(LC 1), 1 x. Comp./Max. Ten All forces 250 (lb) of 8=-1403/0, 2-3=-629/297, 3-4=-629/297 =-1123/0, 8-10=-2493/0, 10-11=-2493/0 14=-2793/0, 14-15=-1763/0 28=-55/424, 26-27=-297/629, 25-26=-73 22=0/2904, 20-21=0/3157, 19-20=0/315 8=-532/70, 2-27=-303/257, 5-25=-823/0 18=0/917, 14-18=-871/0, 14-19=0/501, 1 2=0/753, 11-22=-559/0, 11-21=0/624, 12 	7=851(LC 7) or less except when shown ,4-5=-629/297, 5-6=0/1215 ,11-12=-3157/0, 12-13=-31 30/183, 24-25=-35/277, 22-: 7, 18-19=0/2432, 17-18=0/ ,5-26=0/878, 4-26=-429/0, 7-25=-1524/0, 7-24=0/1128	9, 6-7=0/1219, 57/0, 24=0/1926, 1059 15-17=-1326/0,				
 2) All plates are 3x4 3) Plates checked for 4) Refer to girder(s) f 5) Recommend 2x6 s Strongbacks to be 	ive loads have been considered for this MT20 unless otherwise indicated. r a plus or minus 1 degree rotation about or truss to truss connections. strongbacks, on edge, spaced at 10-0-0 attached to walls at their outer ends or n erect truss backwards.	its center. oc and fastened to each tr	uss with 3-10d (0.131" X	(3") nails.		S IN CA	ROL

LOAD CASE(S) Standard

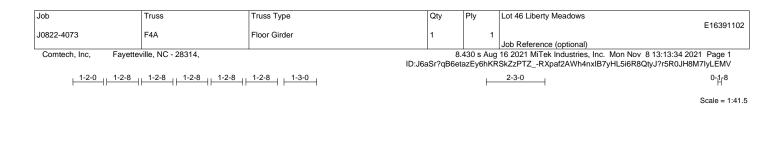
- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
- Uniform Loads (plf)
 - Vert: 17-28=-10, 1-16=-100
- Concentrated Loads (lb)
 - Vert: 1=-1350

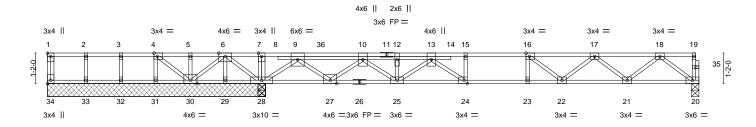


November 9,2021



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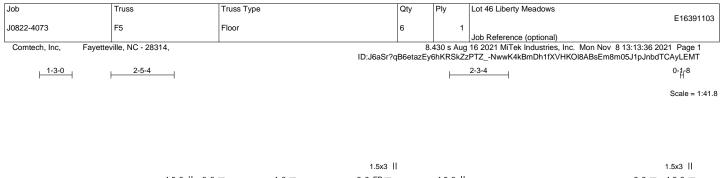
	8-2-8	8-4-4		24-11-8			
Plate Offsets (X,Y)	8-2-8 ([1:Edge,0-1-8], [4:0-1-8,Edge], [16:0-1-	8-4-4)-1-12 8 Edgo] [24:0 1 8 Edgo]	[24:Edgo 0 1 9]	16-7-4			1
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.72 BC 0.88 WB 0.66 Matrix-S	DEFL. in Vert(LL) -0.18	n (loc) l/defl 8 22-23 >999 4 22-23 >825 3 20 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 134 lb	GRIP 244/190 FT = 20%F, 11%E
REACTIONS. All be (lb) - Max L		xcept 29=-516(LC 4), 30=		except end vertion Rigid ceiling dire 6-0-0 oc bracing	cals. ctly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing, 9,27-28.	•
TOP CHORD 4-5= 12-1: BOT CHORD 29-3i 23-2: WEBS 6-28 9-27:	Comp./Max. Ten All forces 250 (ib) o 0/372, 5-6=0/372, 6-7=0/2805, 7-9=0/28 3=-1747/0, 13-15=-2588/0, 15-16=-258 0=-1225/0, 28-29=-1225/0, 27-28=-1194 4=0/2589, 22-23=0/2589, 21-22=0/2158 =-1983/0, 6-29=0/505, 6-30=0/1101, 4 =0/1383, 10-27=-1347/0, 10-25=0/837, 2 =0/375, 16-22=-401/0, 13-25=-579/0, 1	06, 9-10=-255/233, 10-12 \0, 16-17=-2406/0, 17-18 \0, 25-27=0/1076, 24-25= , 20-21=0/952 0=-472/0, 4-31=0/251, 9- 18-20=-1191/0, 18-21=0/8	2=-1747/0, =-1570/0 =0/2211, 28=-2137/0, 304, 17-21=-766/0,				
NOTES- 1) Unbalanced floor liv 2) All plates are 1.5x3 3) Plates checked for a 4) Provide mechanical joint 30 and 239 lb u 5) Recommend 2x6 st Strongbacks to be a 6) CAUTION, Do not e 7) Hanger(s) or other of chord. The design/	re loads have been considered for this d MT20 unless otherwise indicated. a plus or minus 1 degree rotation about i connection (by others) of truss to bearin uplift at joint 31. rongbacks, on edge, spaced at 10-0-0 o attached to walls at their outer ends or re	esign. ts center. Ig plate capable of withsta oc and fastened to each tr istrained by other means. ufficient to support concer the responsibility of othe	anding 516 lb uplift at joi russ with 3-10d (0.131" > ntrated load(s) 407 lb do rs.	(3") nails.			
Uniform Loads (plf)	balanced): Lumber Increase=1.00, Plate =-10, 1-19=-100	Increase=1.00				Con NGN	

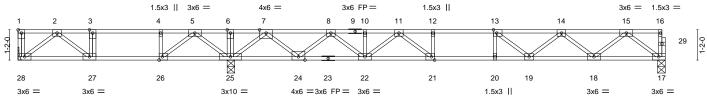
- Vert: 20-34=-10, 1-19=-100
- Concentrated Loads (lb)
- Vert: 36=-327(B)





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	8-2-4	8-2-8				24-11-8			
Plate Offsets (X,Y	8-2-4 [1:Edge,0-1-8], [13:0-1-8,E	0-0-4 [dge], [21:0-1-8,Edge]	, [26:0-1-8,Edge]			16-9-0			<u>`</u>
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI	1.00 Tr 1.00 B YES W		DEFL. Vert(LL) Vert(CT) Horz(CT)	in (l -0.21 21- -0.28 21- 0.05	-22 >964	L/d 480 360 n/a	PLATES MT20 Weight: 125 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x	TOP CHORD 2x4 SP No.1(flat) TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.						oc purlins,		
	REACTIONS. (size) 28=Mechanical, 25=0-3-8, 17=0-3-8 Max Uplift 28=-14(LC 4) Max Grav 28=396(LC 3), 25=1571(LC 1), 17=851(LC 7)								
TOP CHORD	8-10=-2493/0, 10-11=-2493/0, 11-12=-3157/0, 12-13=-3157/0, 13-14=-2793/0, 14-15=-1763/0								
WEBS	OT CHORD 27-28=-56/423, 26-27=-297/629, 25-26=-730/184, 24-25=-34/277, 22-24=0/1927, 21-22=0/2904, 20-21=0/3157, 19-20=0/3157, 18-19=0/2432, 17-18=0/1059 VEBS 2-28=-531/70, 2-27=-302/259, 5-25=-823/0, 5-26=0/878, 4-26=-429/0, 15-17=-1326/0, 15-18=0/917, 14-18=-871/0, 14-19=0/501, 7-25=-1524/0, 7-24=0/1128, 8-24=-1076/0, 8-22=0/753, 11-22=-559/0, 11-21=0/624, 12-21=-277/0, 13-19=-603/0								

NOTES-

1) Unbalanced floor live loads have been considered for this design.

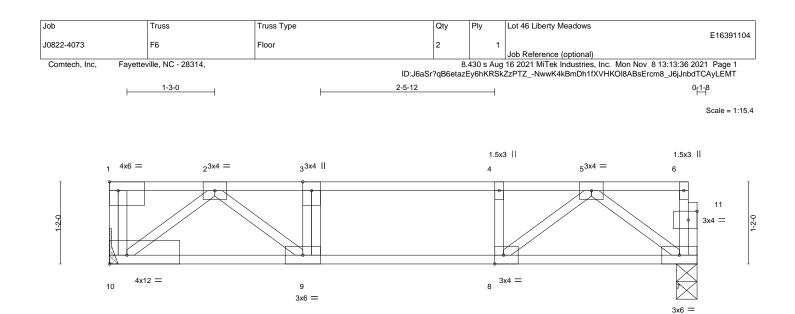
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 28.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.



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			8-4-4			
			8-4-4			1
Plate Offsets (X,Y)	[1:Edge,0-1-8], [8:0-1-8,Edge], [10:Edge	e,0-1-8], [11:0-1-8,0-1-8]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.39 BC 0.27 WB 0.22 Matrix-S	DEFL. in Vert(LL) -0.04 Vert(CT) -0.05 Horz(CT) 0.07	5 9-10 >999 360	PLATES MT20 Weight: 43 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	, ,,) oc purlins,
REACTIONS. (size Max G	e) 10=Mechanical, 7=0-3-8 Grav 10=3846(LC 1), 7=440(LC 1)					

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-10=-3459/0, 2-3=-821/0, 3-4=-821/0, 4-5=-821/0

BOT CHORD 9-10=0/493, 8-9=0/821, 7-8=0/489

WEBS 2-10=-619/0, 2-9=0/460, 5-7=-609/0, 5-8=0/469

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

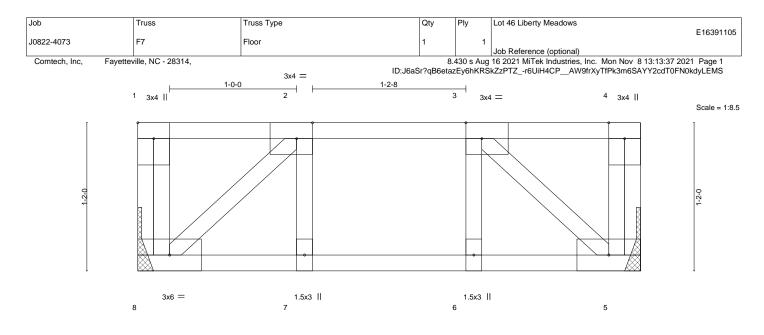
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 7-10=-10, 1-6=-100 Concentrated Loads (lb) Vert: 1=-3400



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3x6 =

			3-11-0				
	I		3-11-8			I	
Plate Offsets (X,Y)	[1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,	Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.06 WB 0.05 Matrix-S	DEFL. ir Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00	7 >999 7 >999	L/d 480 360 n/a	PLATES MT20 Weight: 24 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP	No.1(flat) No.1(flat) No.3(flat)	Ματηχ-5	BRACING- TOP CHORD BOT CHORD	except end vert	icals.	ectly applied or 3-11- r 10-0-0 oc bracing.	

3-11-8

REACTIONS. (size) 8=Mechanical, 5=Mechanical Max Grav 8=204(LC 1), 5=204(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

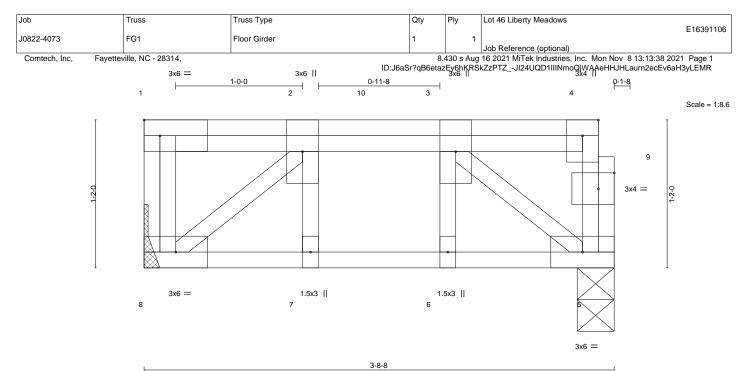
a) Refer to girder(s) for truss to truss connections.
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



November 9,2021







			3-8-8			
Plate Offsets (X,Y)	[9:0-1-8,0-1-8]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.05 BC 0.06 WB 0.07 Matrix-S	DEFL. ir Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00	7 >999 480 7 >999 360	PLATES MT20 Weight: 27 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- BRACING- TOP CHORD 2x4 SP No.1(flat) TOP CHORD Structural wood sheathing directly appli except end verticals.					ectly applied or 3-8-8	oc purlins,

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 8=Mechanical, 5=0-3-8

2x4 SP No.3(flat)

Max Grav 8=242(LC 1), 5=236(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-8=-294/0, 3-5=-291/0

NOTES-

WFBS

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 122 lb down at 1-10-4 on top

chord. The design/selection of such connection device(s) is the responsibility of others.

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100

Concentrated Loads (lb)

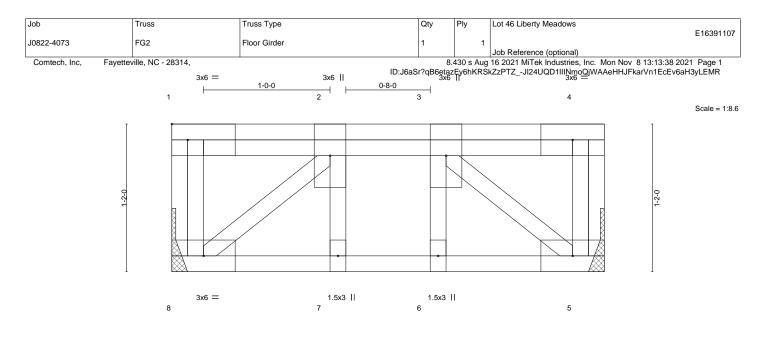
Vert: 10=-104(F)





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling in trusses and truss systems, see **AVSUPPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3x6 =

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

				<u>3-5-0</u> <u>3-5-0</u>	I
LOADING TCLL TCDL BCLL	(psf) 40.0 10.0 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO	CSI. TC 0.16 BC 0.21 WB 0.16	DEFL. in (loc) l/defl L/d Vert(LL) -0.01 7 >999 480 Vert(CT) -0.01 7 >999 360 Horz(CT) 0.00 5 n/a n/a	PLATES GRIP MT20 244/190
BCDL	5.0	Code IRC2015/TPI2014	Matrix-S		Weight: 26 lb FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat)				BRACING- TOP CHORD Structural wood sheathing dire	ectly applied or 3-5-0 oc purlins,

BOT CHORD

TOP CHORD BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat) 2x4 SP No.3(flat) WEBS

REACTIONS. (size) 8=Mechanical, 5=Mechanical Max Grav 8=596(LC 1), 5=427(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-528/0

BOT CHORD 7-8=0/528, 6-7=0/528, 5-6=0/528

WEBS 2-8=-684/0, 3-5=-684/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 705 lb down at 1-6-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb) Vert: 2=-675(B)



November 9,2021

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