

Trenco
818 Soundside Rd
Edenton, NC 27932

Re: 150_1910_D_Vo
KB Home 150.1910.D Vo2

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I45987461 thru I45987501

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



May 6, 2021

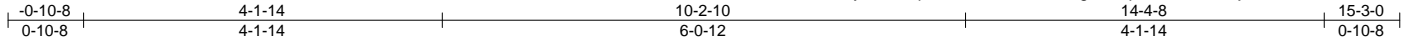
Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

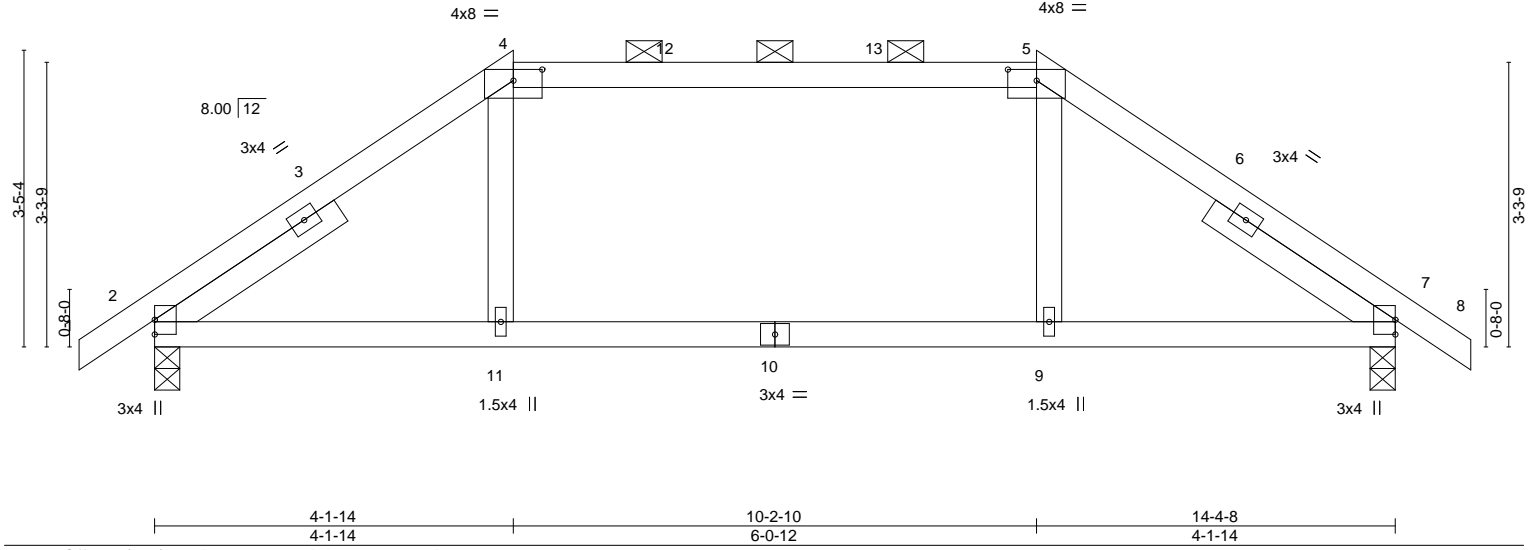
Job 150_1910_D_VO	Truss A1	Truss Type Hip	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987461
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:04 2021 Page 1
ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-fe40TkgTAI2tpskVuokPkt15jXlnfQmdP5FZIHzzJTub



Scale = 1:26.7



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.62	Vert(LL) -0.07	9	>999	240	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15		BC 0.35	Vert(CT) -0.09	9-11	>999	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.09	Horz(CT) 0.01	7	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S					Weight: 65 lb	FT = 20%
BCDL 10.0									

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	2-0-0 oc purlins (5-10-1 max.): 4-5.
WEBS 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
SLIDER Left 2x4 SP or SPF No.2 -t 2-6-2, Right 2x4 SP or SPF No.2 -t 2-6-2	

REACTIONS. (size) 2=0-3-8, 7=0-3-8
 Max Horz 2=66(LC 13)
 Max Uplift 2=-12(LC 14), 7=-12(LC 15)
 Max Grav 2=628(LC 2), 7=627(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-4=-788/64, 4-5=-587/81, 5-7=-788/64
 BOT CHORD 2-11=0/593, 9-11=0/587, 7-9=0/593

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-1-14, Exterior(2R) 4-1-14 to 8-4-13, Interior(1) 8-4-13 to 10-2-10, Exterior(2R) 10-2-10 to 14-4-8, Interior(1) 14-4-8 to 15-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



May 6, 2021

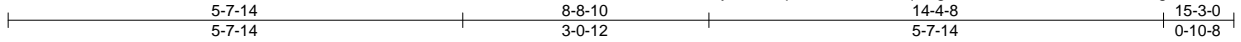
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss A2	Truss Type Hip	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987462
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:05 2021 Page 1
ID:VMD62rz1yihD_OqRtbnrIFztQ8K-7qeOgHh6wBAkR0JhSVsfG5ZJSwg7Otmel?6qkzJTua



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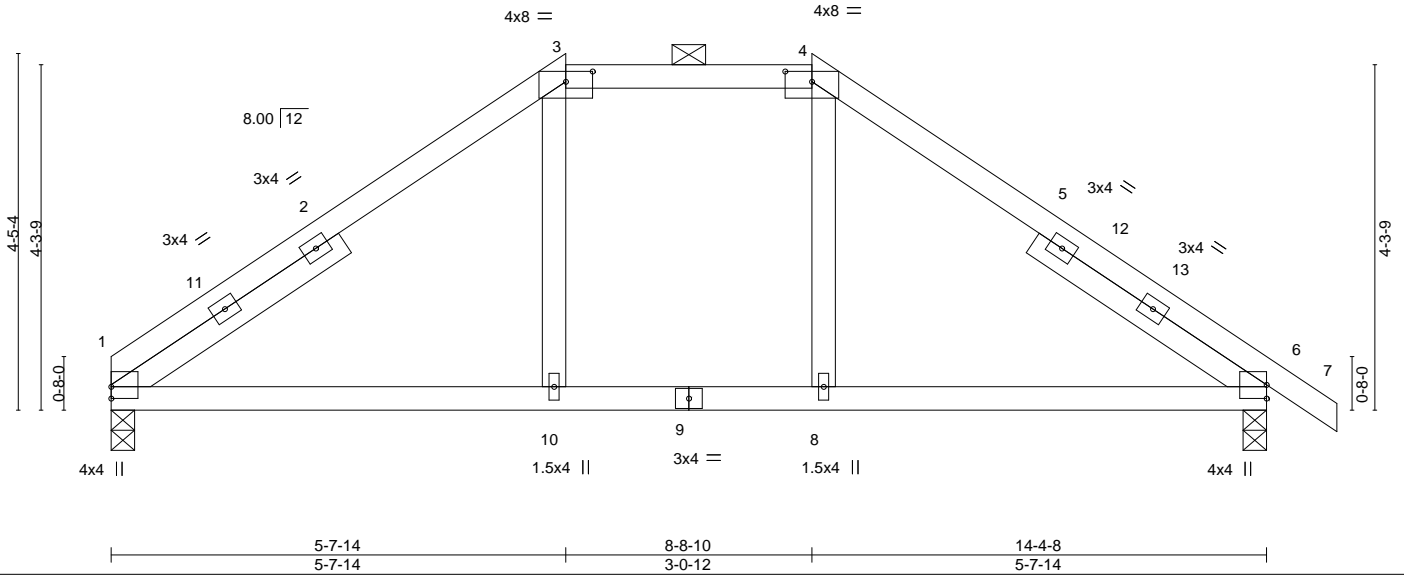


Plate Offsets (X,Y)-- [3:0-4-0,0-1-9], [4:0-4-0,0-1-9]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.43	Vert(LL) -0.07	1-10	>999	240	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15		BC 0.41	Vert(CT) -0.10	1-10	>999	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.09	Horz(CT) 0.01	6	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S					Weight: 70 lb	FT = 20%
BCDL 10.0									

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP or SPF No.2 -t 3-4-15, Right 2x4 SP or SPF No.2 -t 3-4-15

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 3-4.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=0-3-8, 6=0-3-8
 Max Horz 1=-84(LC 12)
 Max Uplift 1=-9(LC 14), 6=-22(LC 15)
 Max Grav 1=635(LC 3), 6=683(LC 27)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-782/67, 3-4=-577/95, 4-6=-783/76
 BOT CHORD 1-10=0/584, 8-10=0/577, 6-8=0/584

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 5-7-14, Exterior(2E) 5-7-14 to 8-8-10, Exterior(2R) 8-8-10 to 12-11-9, Interior(1) 12-11-9 to 15-3-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 6. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



May 6, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component

Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss A3	Truss Type Common	Qty 2	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987463
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84 Components (Dunn), Dunn, NC - 28334, 8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:06 2021 Page 1
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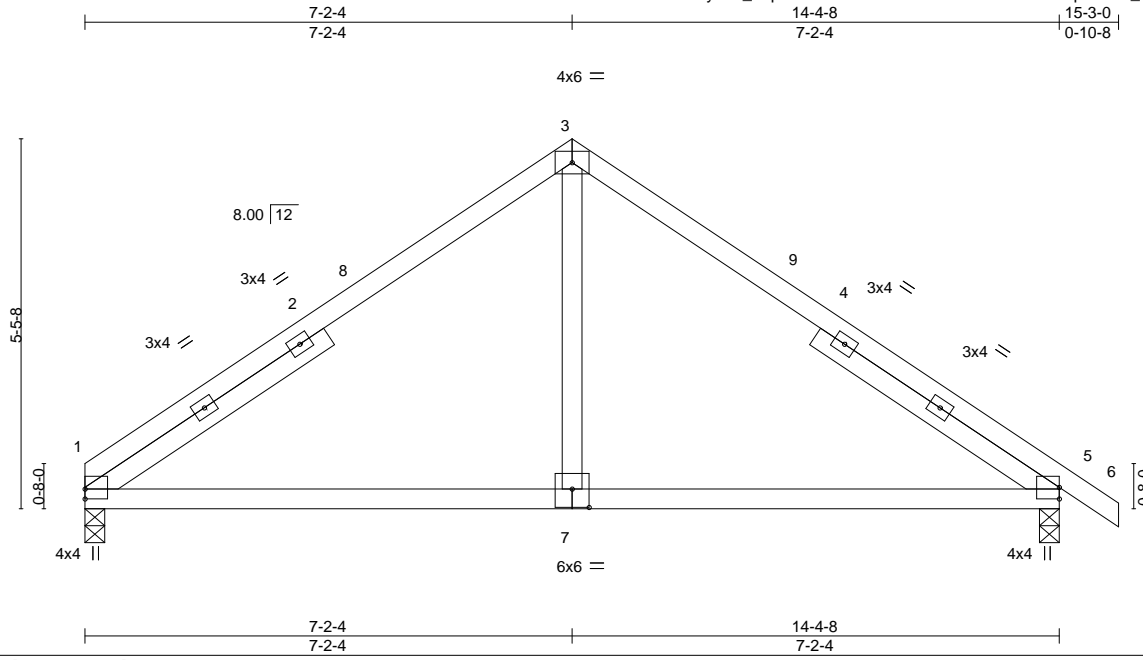


Plate Offsets (X,Y)-- [7:0-3-0,0-3-4]					
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.82	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.53	Vert(LL) -0.06 1-7 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.13	Vert(CT) -0.12 1-7 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 5 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 69 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -t 4-2-15, Right 2x4 SP No.3 -t 4-2-15

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 3-7-9 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=0-3-8, 5=0-3-8
 Max Horz 1=-106(LC 12)
 Max Uplift 1=-15(LC 14), 5=-29(LC 15)
 Max Grav 1=573(LC 2), 5=629(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-3=-658/74, 3-5=-659/74
 BOT CHORD 1-7=0/457, 5-7=0/457
 WEBS 3-7=0/337

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 7-2-4, Exterior(2R) 7-2-4 to 10-2-4, Interior(1) 10-2-4 to 15-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 5. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



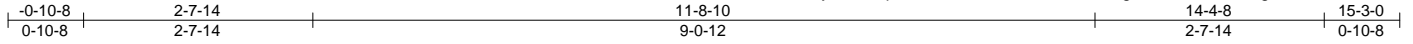
May 6, 2021

Job 150_1910_D_VO	Truss AE	Truss Type GABLE	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987464
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:07 2021 Page 1

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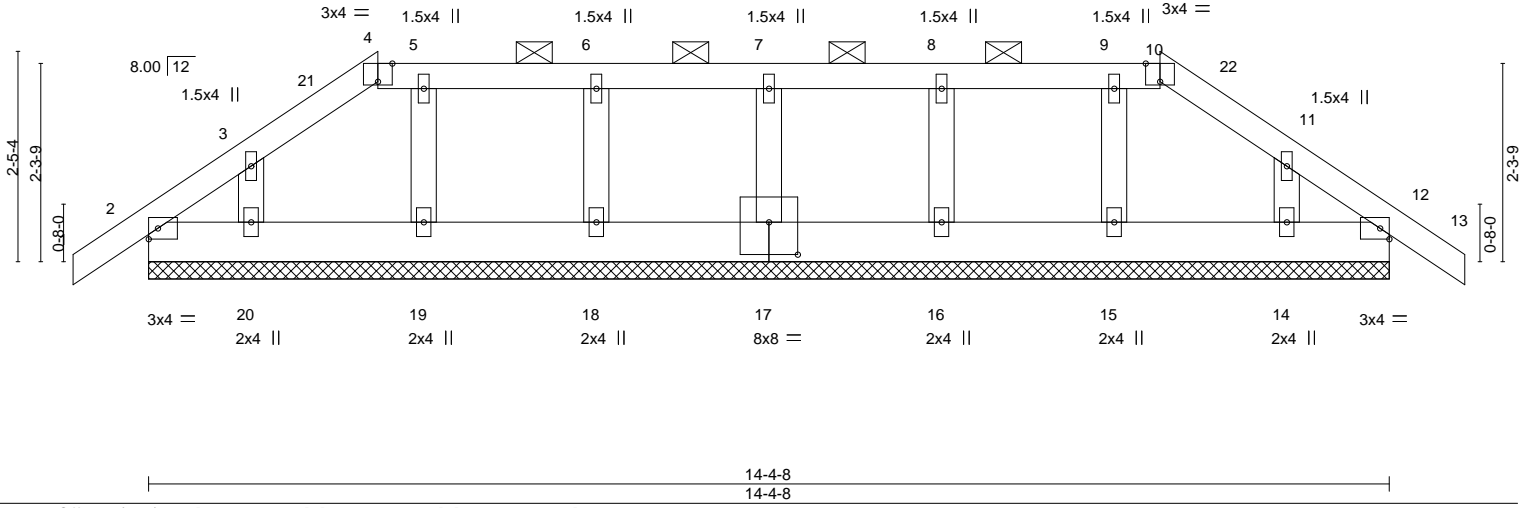


Plate Offsets (X,Y)-- [4:0-2-0,Edge], [10:0-2-0,Edge], [17:0-4-0,0-4-8]					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) -0.00 12 n/r 120	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15	BC 0.02	Vert(CT) -0.00 13 n/r 120		
TCDL 10.0	Rep Stress Incr YES	WB 0.03	Horz(CT) 0.00 12 n/a n/a		
BCLL 0.0 *	Code IRC2018/TPI2014	Matrix-S		Weight: 74 lb	FT = 20%
BCDL 10.0					

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-10.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.3	

REACTIONS. All bearings 14-4-8.
 (lb) - Max Horz 2--46(LC 12)
 Max Uplift All uplift 100 lb or less at joint(s) 12, 17, 18, 19, 20, 16, 15, 14, 2
 Max Grav All reactions 250 lb or less at joint(s) 12, 17, 18, 19, 20, 16, 15, 14, 2

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 2-7-14, Corner(3R) 2-7-14 to 5-7-14, Exterior(2N) 5-7-14 to 11-8-10, Corner(3R) 11-8-10 to 14-8-10, Exterior(2N) 14-8-10 to 15-3-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - N/A
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Job 150_1910_D_VO	Truss AG	Truss Type COMMON GIRDER	Qty 1	Ply 2	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987465
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84 Components (Dunn), Dunn, NC - 28334, 8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:10 2021 Page 1
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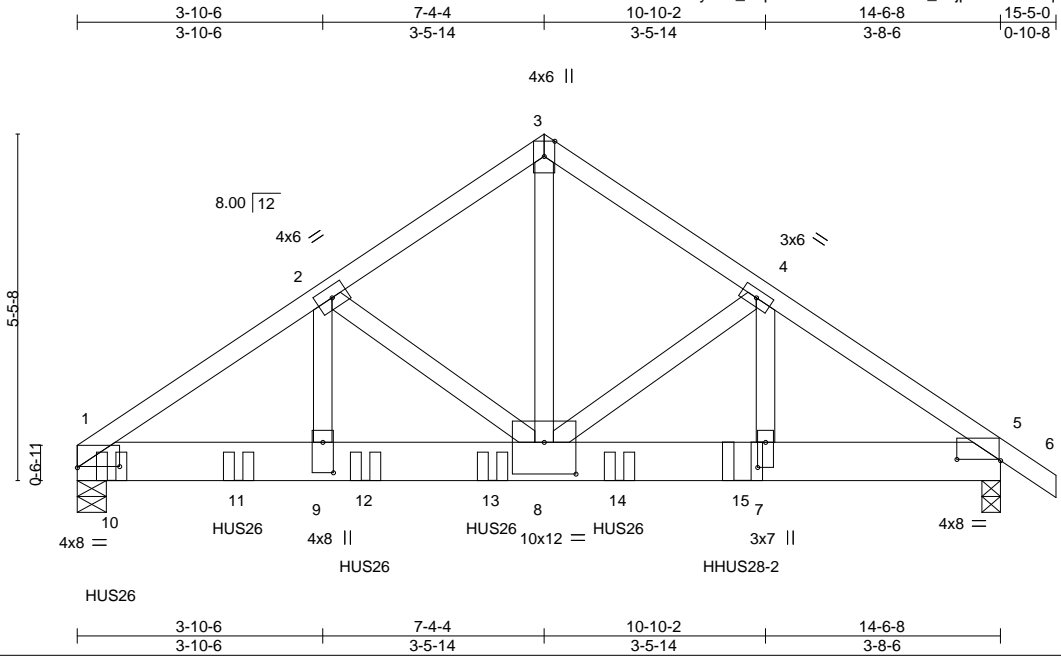


Plate Offsets (X,Y)--	[1:0-8-0,0-0-3], [5:0-8-4,0-0-4], [7:0-4-12,0-1-8], [8:0-6-0,0-6-0], [9:0-5-12,0-2-0]				
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.52	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.43	Vert(LL) -0.06 8-9 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.74	Vert(CT) -0.12 7-8 >999 180		
BCLL 0.0 *	Rep Stress Incr NO	Matrix-S	Horz(CT) 0.03 5 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 196 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
 BOT CHORD 2x8 SP DSS
 WEBS 2x4 SP No.3 *Except*
 3-8: 2x4 SP No.2 or 2x4 SPF No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 3-11-2 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=(0-5-8 + LGT2 Simpson Strong-Tie) (req. 0-5-12), 5=(0-3-8 + TBE4 Simpson Strong-Tie) (req. 0-3-12)
 Max Horz 1=105(LC 34)
 Max Uplift 5=384(LC 11)
 Max Grav 1=7338(LC 3), 5=4817(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-8242/0, 2-3=-5727/320, 3-4=-5725/321, 4-5=-7524/597
 BOT CHORD 1-9=0/6624, 8-9=0/6624, 7-8=-434/5968, 5-7=-434/5968
 WEBS 3-8=-297/6062, 4-8=-1566/384, 4-7=-353/2083, 2-8=-2460/0, 2-9=0/3052

- NOTES-**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-2-0 oc.
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
 - TBE4 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 0-6-8 from the left end to 8-6-8 to connect truss(es) to back face of bottom chord.



May 6, 2021

Job 150_1910_D_VO	Truss AG	Truss Type COMMON GIRDER	Qty 1	Ply 2	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987465
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:10 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-UoSHk_ElIjp0XnBfF2Rqz8H7rxN_3z_Wo1itVxzJTuv

NOTES-

- 13) Use Simpson Strong-Tie HHUS28-2 (22-10d Girder, 4-10d Truss) or equivalent at 10-5-12 from the left end to connect truss(es) to back face of bottom chord.
- 14) Fill all nail holes where hanger is in contact with lumber.
- 15) LGT2 Hurricane ties must have two studs in line below the truss.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf)
 - Vert: 1-3=-43, 3-6=-43, 1-5=-20
 - Concentrated Loads (lb)
 - Vert: 10=-1618(B) 11=-1544(B) 12=-1541(B) 13=-1608(B) 14=-1657(B) 15=-2133(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

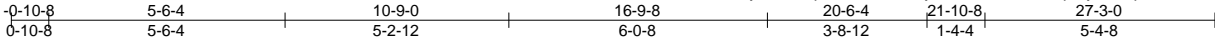
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss B1	Truss Type HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987466
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84 Components (Dunn), Dunn, NC - 28334, 8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:11 2021 Page 1
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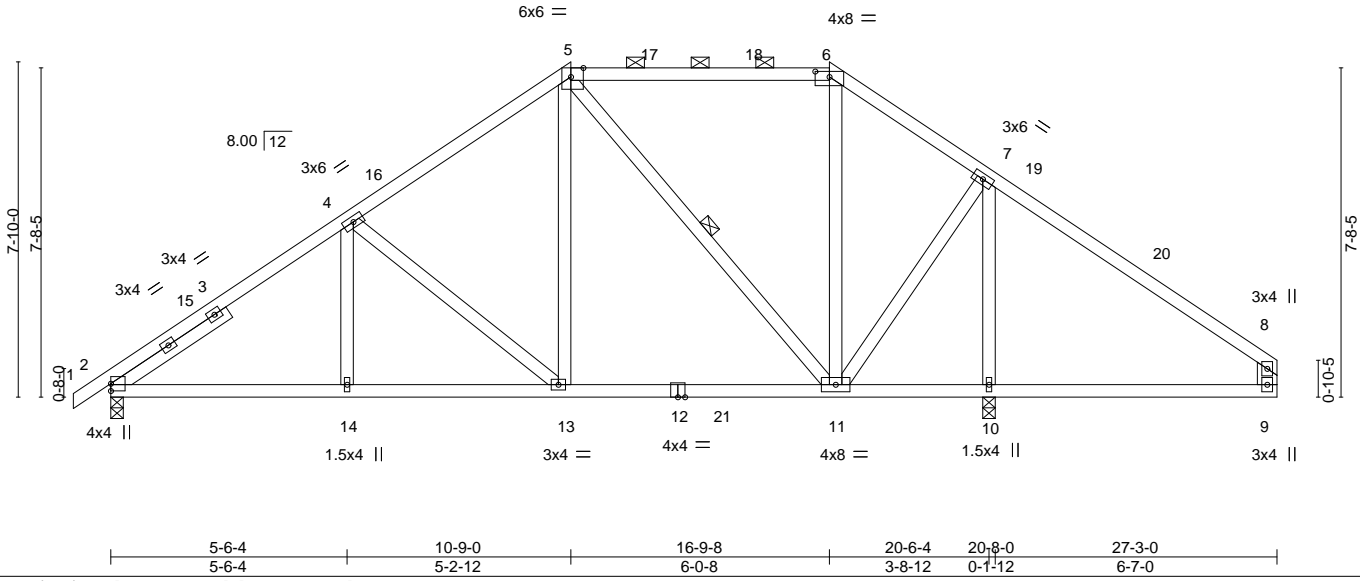


Plate Offsets (X,Y)--	[5:0-3-8,Edge], [6:0-4-0,0-1-9]									
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.72	Vert(LL)	-0.07 11-13 >999	240	MT20	197/144
Snow (Pf/Pg)	16.5/15.0	Lumber DOL	1.15	BC	0.52	Vert(CT)	-0.12 11-13 >999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(CT)	0.02 10 n/a n/a			
BCLL	0.0 *	Code IRC2018/TPI2014		Matrix-S						
BCDL	10.0								Weight: 161 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD	TOP CHORD
BOT CHORD	BOT CHORD
WEBS	WEBS
SLIDER	

REACTIONS. (size) 2=0-3-8, 10=0-3-8
 Max Horz 2=151(LC 11)
 Max Uplift 2=44(LC 14), 10=18(LC 15)
 Max Grav 2=870(LC 26), 10=1538(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-4=-1101/57, 4-5=-746/85, 6-7=-299/95, 7-8=-28/467
 BOT CHORD 2-14=-82/911, 13-14=-82/911, 11-13=-66/566, 10-11=-307/66, 9-10=-307/66
 WEBS 4-13=-436/126, 5-13=-5/525, 5-11=-572/38, 7-11=0/769, 7-10=-1272/64

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 10-9-0, Exterior(2R) 10-9-0 to 14-11-15, Interior(1) 14-11-15 to 16-9-8, Exterior(2R) 16-9-8 to 21-0-7, Interior(1) 21-0-7 to 27-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



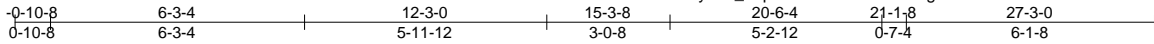
May 6, 2021

Job 150_1910_D_VO	Truss B2	Truss Type Hip	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987467
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:12 2021 Page 1

ID:VMD62rz1yihD_OqRtbnrIFztQ8K-QAZ18gmVHK3km5L2MTUI2ZMQrI3EXvRoFLB_aqzJTUT



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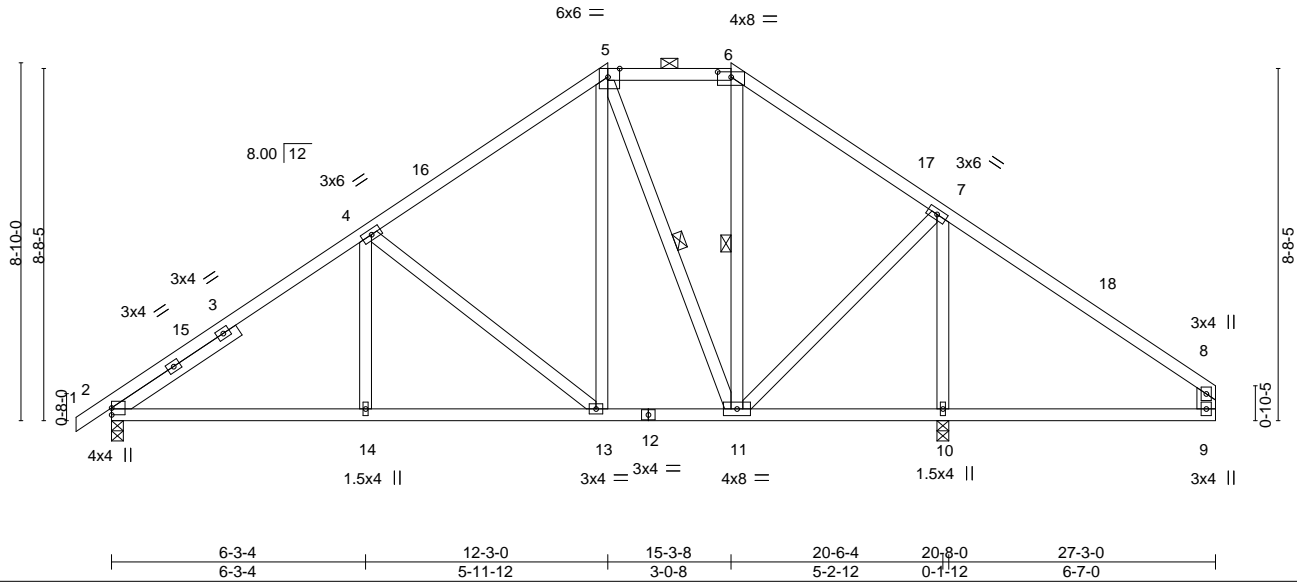


Plate Offsets (X,Y)-- [5:0-3-8,Edge], [6:0-4-0,0-1-9]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.74	Vert(LL) -0.04	2-14	>999	240	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15		BC 0.44	Vert(CT) -0.09	2-14	>999	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.62	Horz(CT) 0.02	10	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S						
BCDL 10.0								Weight: 168 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
 WEBS 2x4 SP No.3 *Except*
 8-9: 2x6 SP No.2
 SLIDER Left 2x4 SP or SPF No.2 -t 3-8-5

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midt 5-11, 6-11

REACTIONS.

(size) 2=0-3-8, 10=0-3-8
 Max Horz 2=171(LC 11)
 Max Uplift 2=-48(LC 14), 10=-29(LC 15)
 Max Grav 2=807(LC 32), 10=1422(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1000/62, 4-5=-586/98, 5-6=-250/104, 6-7=-381/107, 7-8=-24/433
 BOT CHORD 2-14=-91/761, 13-14=-91/761, 11-13=-44/392, 10-11=-254/65, 9-10=-254/65
 WEBS 4-14=0/267, 4-13=-469/138, 5-13=-28/390, 5-11=-418/53, 7-11=0/662, 7-10=-1228/92

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 12-3-0, Exterior(2E) 12-3-0 to 15-3-8, Exterior(2R) 15-3-8 to 19-6-7, Interior(1) 19-6-7 to 27-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



May 6, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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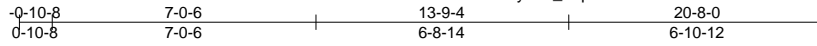
Job 150_1910_D_VO	Truss B3	Truss Type Common	Qty 3	Ply 1	KB Home 150.1910.D Vo2	I45987468
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84 Components (Dunn),

Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:13 2021 Page 1

ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-uM7PM0n72eBbOFwEwA?Xbnvby9OmGSByU?xY6GzJTUS



4x6 =

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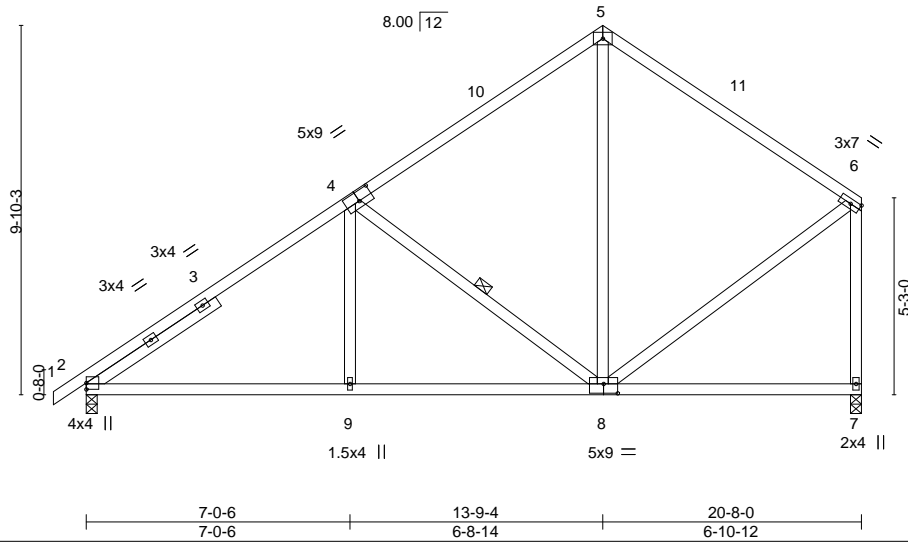


Plate Offsets (X,Y)-- [4:0-4-8,0-3-0], [8:0-4-8,0-3-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.72	Vert(LL) -0.06	2-9	>999	240	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Lumber DOL 1.15		BC 0.48	Vert(CT) -0.13	2-9	>999	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.20	Horz(CT) 0.02	7	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S						
BCDL 10.0								Weight: 128 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -t 4-1-13

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-7-5 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 4-8

REACTIONS.

(size) 2=0-3-8, 7=0-3-8
 Max Horz 2=224(LC 14)
 Max Uplift 2=-32(LC 14), 7=-39(LC 14)
 Max Grav 2=874(LC 2), 7=820(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1087/36, 4-5=-616/91, 5-6=-604/95, 6-7=-759/73
 BOT CHORD 2-9=-152/832, 8-9=-152/832
 WEBS 4-9=0/289, 4-8=-531/161, 5-8=0/286, 6-8=-27/483

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 13-9-4, Exterior(2R) 13-9-4 to 16-9-4, Interior(1) 16-9-4 to 20-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



May 6, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component

Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



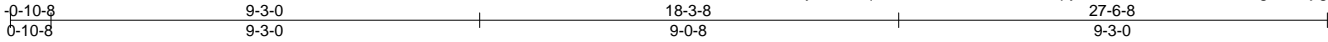
818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss BE	Truss Type GABLE	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987469
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:14 2021 Page 1

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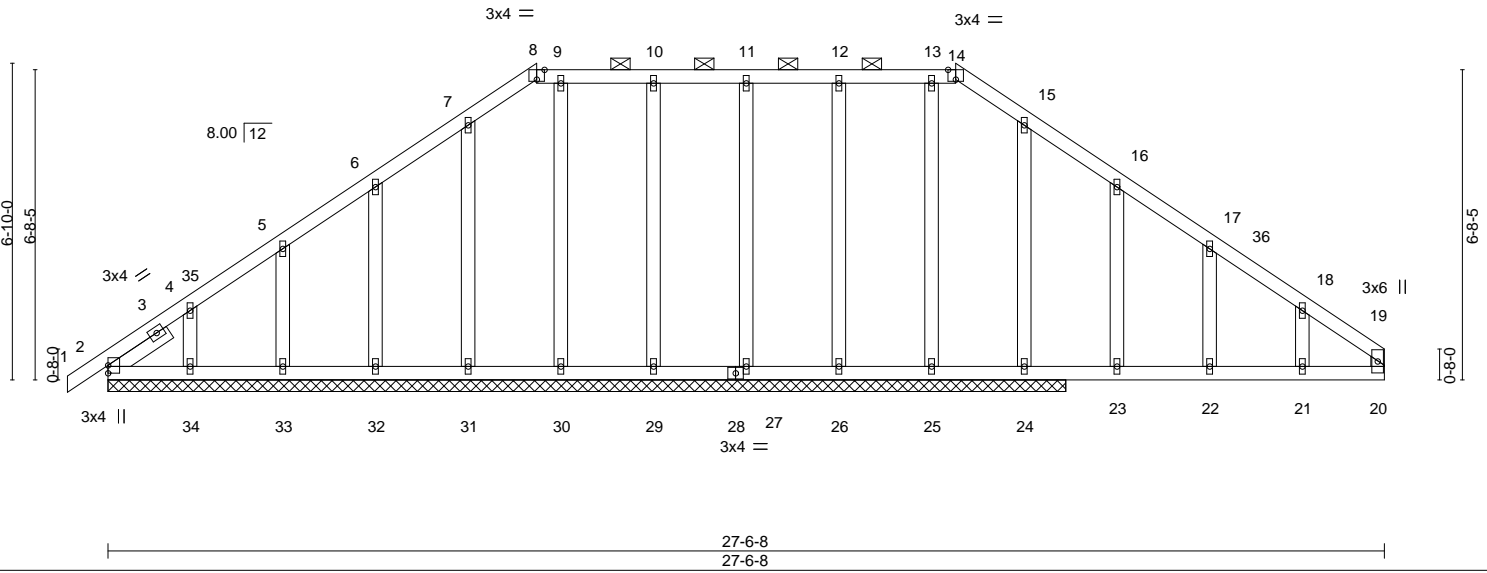


Plate Offsets (X,Y)-- [8:0-2-0,Edge], [14:0-2-0,Edge]					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15	TC 0.75	Vert(LL) 0.00 1 n/r 120	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15	BC 0.72	Vert(CT) 0.00 1 n/r 120		
TCDL 10.0	Rep Stress Incr YES	WB 0.29	Horz(CT) -0.01 24 n/a n/a		
BCLL 0.0 *	Code IRC2018/TPI2014	Matrix-S			
BCDL 10.0				Weight: 177 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 8-14.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3	
OTHERS 2x4 SP No.3	
SLIDER Left 2x4 SP or SPF No.2 -t 1-6-5	

REACTIONS. All bearings 20-8-0.
 (lb) - Max Horz 2=131(LC 11)
 Max Uplift All uplift 100 lb or less at joint(s) 27, 29, 31, 32, 33, 34, 26 except 2=226(LC 33), 25=168(LC 27), 24=198(LC 15)
 Max Grav All reactions 250 lb or less at joint(s) 2, 27, 29, 31, 32, 33, 34, 26, 25 except 30=333(LC 2), 24=898(LC 27)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-4=-109/449, 4-5=-47/410, 5-6=0/410, 6-7=0/418, 7-8=0/361, 8-9=0/301, 9-10=0/314, 10-11=0/314, 11-12=0/314, 12-13=0/314, 13-14=0/311, 14-15=0/282, 15-16=0/492, 16-17=-3/446, 17-18=-41/404, 18-19=-78/371
 BOT CHORD 2-34=-324/85, 33-34=-324/85, 32-33=-324/85, 31-32=-324/85, 30-31=-324/85, 29-30=-324/85, 27-29=-324/85, 26-27=-324/85, 25-26=-324/85, 24-25=-324/85, 23-24=-324/85, 22-23=-324/85, 21-22=-324/85, 20-21=-324/85
 WEBS 9-30=-295/0, 15-24=-480/120

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 9-3-0, Corner(3R) 9-3-0 to 12-3-0, Exterior(2N) 12-3-0 to 18-3-8, Corner(3R) 18-3-8 to 21-3-8, Exterior(2N) 21-3-8 to 27-4-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Ct=1.0, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 1.5x4 MT20 unless otherwise indicated.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide is indicated on page 2.



Job 150_1910_D_VO	Truss BE	Truss Type GABLE	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987469
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:15 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-qlFAnipNaFRJdY4c2b1?gC_xxy?WkLNFxJQeB8zJTUQ

NOTES-

- 11) N/A
- 12) N/A
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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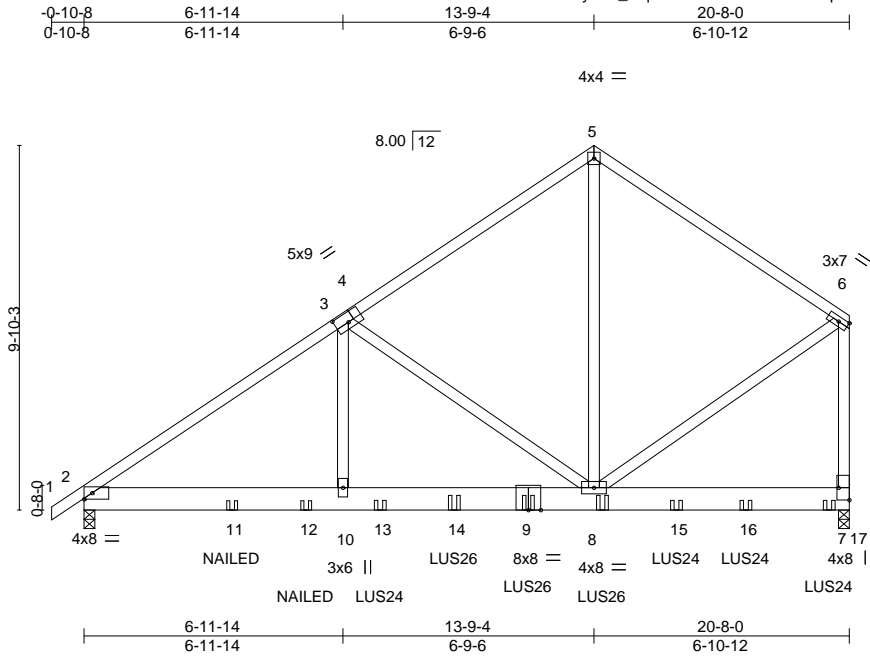
Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss BG	Truss Type Common Girder	Qty 1	Ply 2	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987470
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84 Components (Dunn), Dunn, NC - 28334, 8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:17 2021 Page 1
 ID:VMD62rz1yiHD_OqRtbnriFztQ8K-n8NwCNqd6th1tsD?903Tld3LGmo9CCzXPdvlF1zJTUO



Scale = 1:62.2

Plate Offsets (X,Y)-- [4:0-4-4,0-3-0], [7:Edge,0-3-8]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.50	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.26	Vert(LL) 0.04 8-10 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.47	Vert(CT) 0.06 8-10 >999 180		
BCLL 0.0 *	Rep Stress Incr NO	Matrix-S	Horz(CT) -0.01 7 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 305 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
 BOT CHORD 2x8 SP No.2
 WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

(size) 2=0-3-8, 7=0-3-8
 Max Horz 2=224(LC 45)
 Max Uplift 2=820(LC 38), 7=1323(LC 37)
 Max Grav 2=822(LC 55), 7=840(LC 55)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1047/1603, 3-5=-608/980, 5-6=-615/1053, 6-7=-743/940
 BOT CHORD 2-10=-1275/867, 8-10=-1275/867
 WEBS 3-10=-869/235, 3-8=-510/606, 5-8=-1318/379, 6-8=-990/535

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-9-0 oc.
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.

Continued on page 2



May 6, 2021

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818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss BG	Truss Type Common Girder	Qty 1	Ply 2	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987470
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:17 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnriFztQ8K-n8NwCNqd6th1tsD?903TId3LGmo9CCzXPdvlF1zJTUO

NOTES-

- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 8-0-0 oc max. starting at 8-0-0 from the left end to 20-1-8 to connect truss(es) to back face of bottom chord.
- 13) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 10-0-0 from the left end to 14-0-0 to connect truss(es) to back face of bottom chord.
- 14) Fill all nail holes where hanger is in contact with lumber.
- 15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- 16) LGT2 Hurricane ties must have two studs in line below the truss.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-5=-43, 5-6=-43, 2-7=-20
Concentrated Loads (lb)
Vert: 9=224(B) 8=171(B) 11=103(B) 12=126(B) 13=160(B) 14=200(B) 15=-130(B) 16=-130(B) 17=-136(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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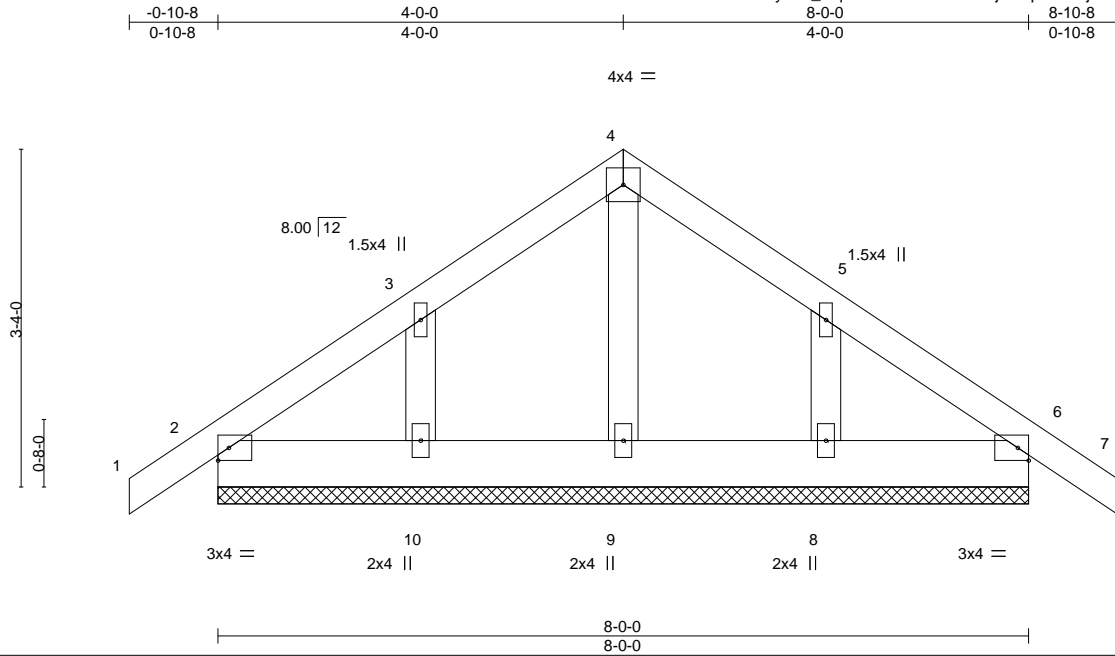


818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss CE	Truss Type COMMON SUPPORTED GAB	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987471
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84 Components (Dunn),

Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:18 2021 Page 1
ID:VMD62rz1yiHD_OqRtbnriFztQ8K-FKxIPrFtAptU0oBjkbilqcd5ACExmuhdHeJnTzJTuN

Scale = 1:22.7

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 2-0-0	TC 0.05	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Lumber DOL 1.15	BC 0.02	Vert(LL) -0.00 6 n/r 120		
TCDL 10.0	Rep Stress Incr YES	WB 0.04	Vert(CT) -0.00 7 n/r 120		
BCLL 0.0 *	Code IRC2018/TPI2014	Matrix-P	Horz(CT) 0.00 6 n/a n/a		
BCDL 10.0				Weight: 44 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
 BOT CHORD 2x6 SP No.2
 OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 8-0-0.
 (lb) - Max Horz 2=65(LC 13)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 10, 8
 Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 2-0-0, Exterior(2N) 2-0-0 to 4-0-0, Corner(3R) 4-0-0 to 7-0-0, Exterior(2N) 7-0-0 to 8-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); ls=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- N/A
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



May 6, 2021

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component

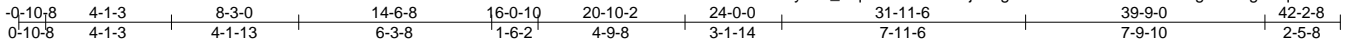
Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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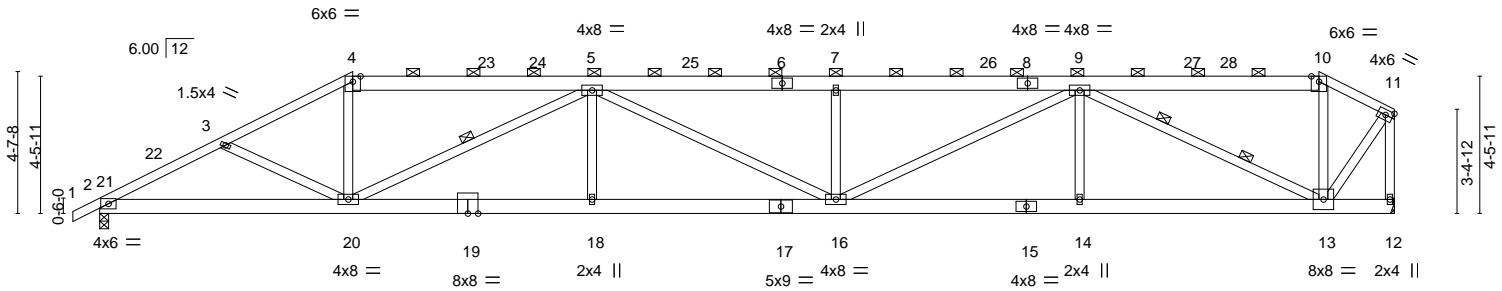
818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss H1	Truss Type Hip	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987472
84 Components (Dunn), Dunn, NC - 28334,					Job Reference (optional)	

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:19 2021 Page 1
 ID:VMD62rz1yIHd_OqRtbnrIFztQ8K-jWUgc3sudUxk6ANOHR6xr28gwaM9g?QqsxOskwzJTUM



Scale = 1:75.1



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.56	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15	BC 0.80	Vert(LL) -0.30 16-18 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.92	Vert(CT) -0.61 16-18 >826 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.15 12 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 285 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2 *Except* 1-4,10-11: 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-12 oc purlins, except end verticals, and 2-0-0 oc purlins (3-2-14 max.): 4-10.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 5-20 2 Rows at 1/3 pts 9-13

REACTIONS. (size) 2=0-3-8, 12=Mechanical
 Max Horz 2=119(LC 16)
 Max Uplift 2=62(LC 13), 12=108(LC 12)
 Max Grav 2=1739(LC 2), 12=1677(LC 40)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-3177/185, 3-4=-3044/187, 4-5=-2673/179, 5-7=-4362/362, 7-9=-4362/362,
 9-10=-921/73, 10-11=-999/69, 11-12=-1697/98
 BOT CHORD 2-20=200/2740, 18-20=-337/4225, 16-18=-337/4225, 14-16=-273/3360, 13-14=-273/3360
 WEBS 4-20=0/974, 7-16=-460/146, 9-14=0/317, 9-13=-2738/245, 11-13=-92/1589, 5-18=0/306,
 9-16=-92/1203, 5-20=-1815/213

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 8-3-0, Exterior(2R) 8-3-0 to 12-5-15, Interior(1) 12-5-15 to 39-9-0, Exterior(2E) 39-9-0 to 42-0-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=108.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



May 6, 2021

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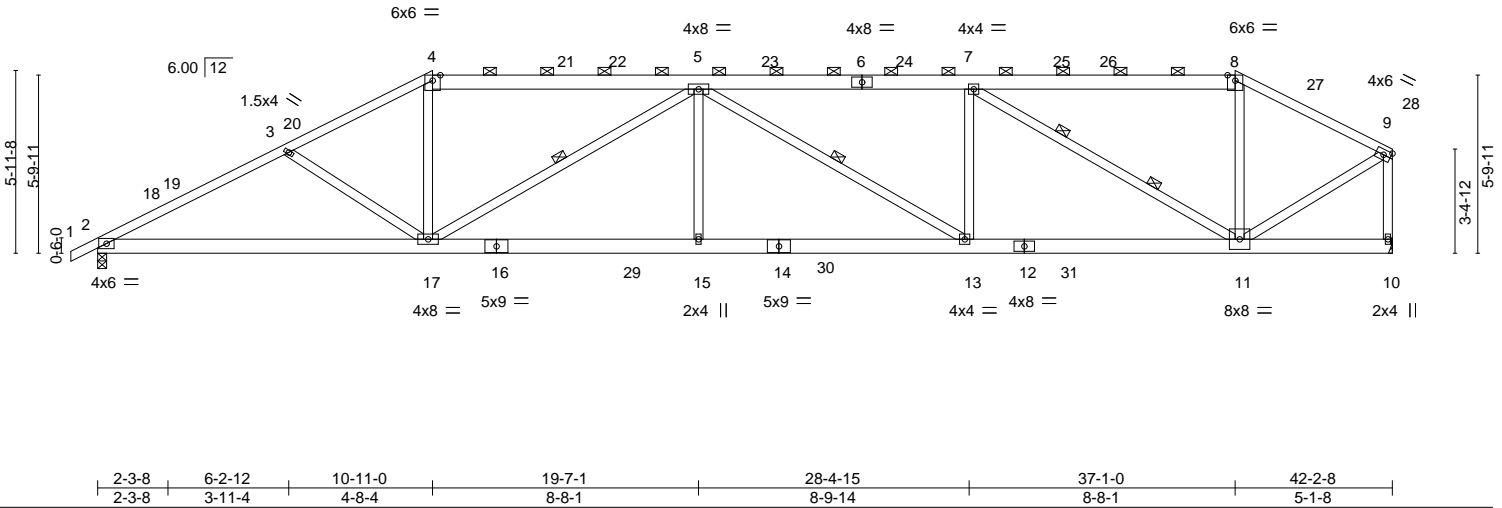
Job 150_1910_D_VO	Truss H2	Truss Type Hip	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987473
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:21 2021 Page 1
ID:VMD62r21yiHD_OqRtbrrfFztQ8K-fvcR1lt895BSLUXmOs8PwTEzLN?c8v?7KFtzOozJTUk



Scale = 1:75.1



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 2-0-0 1.15	TC 0.75	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15	BC 0.93	Vert(LL) -0.26 13-15 >999 240		
TCDL 10.0	Rep Stress Incr YES	WB 0.92	Vert(CT) -0.47 13-15 >999 180		
BCLL 0.0 *	Code IRC2018/TPI2014	Matrix-S	Horz(CT) 0.13 10 n/a n/a		
BCDL 10.0				Weight: 282 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 6-8,4-6: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-14 oc purlins, except end verticals, and 2-0-0 oc purlins (3-2-4 max.): 4-8.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 15-17.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 5-13, 5-17 2 Rows at 1/3 pts 7-11

REACTIONS. (size) 10=Mechanical, 2=0-3-8
Max Horz 2=137(LC 16)
Max Uplift 10=63(LC 12), 2=30(LC 13)
Max Grav 10=1837(LC 3), 2=1880(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3344/144, 3-4=-3146/143, 4-5=-2763/140, 5-7=-3388/220, 7-8=-1526/98,
8-9=-1725/93, 9-10=-1784/80
BOT CHORD 2-17=-163/2912, 15-17=-230/3788, 13-15=-230/3788, 11-13=-201/3388
WEBS 4-17=0/1065, 5-15=0/405, 5-13=-508/33, 7-13=0/617, 7-11=-2182/172, 8-11=0/462,
9-11=-65/1805, 5-17=-1250/175, 3-17=-356/163

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 10-11-0, Exterior(2R) 10-11-0 to 15-1-15, Interior(1) 15-1-15 to 37-1-0, Exterior(2R) 37-1-0 to 41-3-15, Interior(1) 41-3-15 to 42-0-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



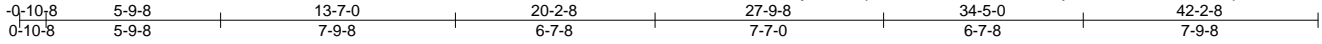
May 6, 2021

Job 150_1910_D_VO	Truss H3	Truss Type HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987474
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:23 2021 Page 1

ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-blkBSRvOhjRABnh9WHA?uJKsBiprcrQnZM3ThzJTul



Scale = 1:76.5

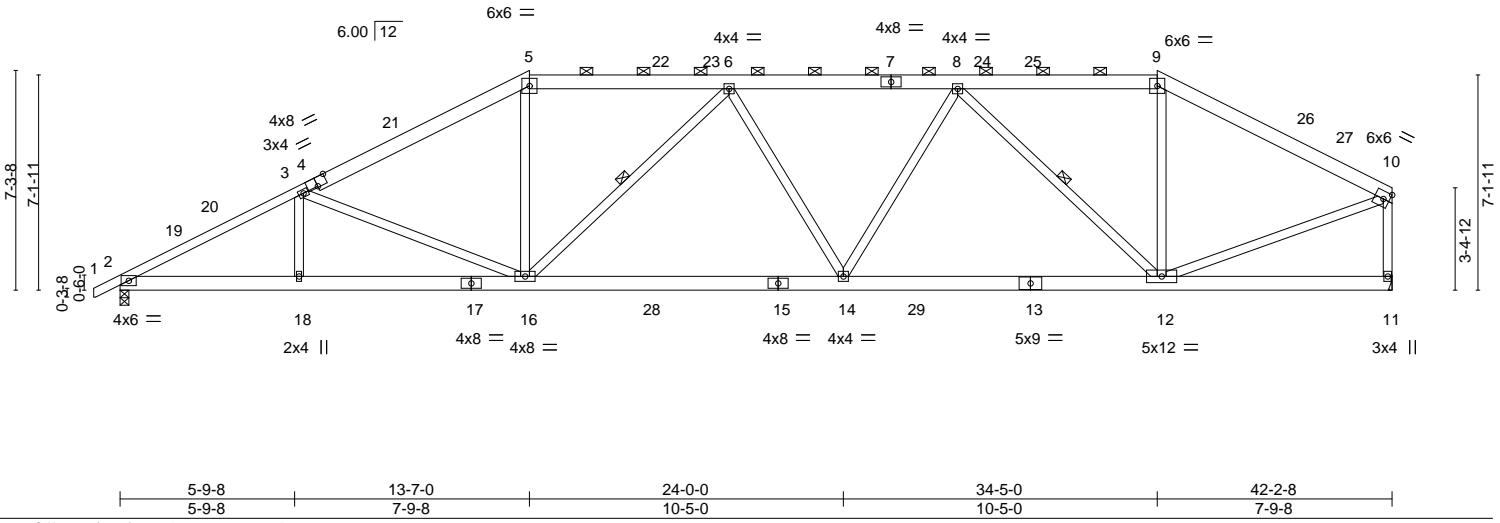


Plate Offsets (X,Y)-- [4:0-4-0,Edge]	
LOADING (psf)	SPACING-
TCLL (roof) 20.0	2-0-0
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15
TCDL 10.0	Lumber DOL 1.15
BCLL 0.0 *	Rep Stress Incr YES
BCDL 10.0	Code IRC2018/TPI2014
	CSI.
	TC 0.69
	BC 0.89
	WB 0.78
	Matrix-S
	DEFL.
	in (loc) l/defl L/d
	Vert(LL) -0.23 14-16 >999 240
	Vert(CT) -0.41 14-16 >999 180
	Horz(CT) 0.10 11 n/a n/a
	PLATES
	MT20
	GRIP
	197/144
	Weight: 297 lb FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2 *Except* 1-4: 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 2-8-7 oc purlins, except end verticals, and 2-0-0 oc purlins (4-0-3 max.): 5-9.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midt 6-16, 8-12

REACTIONS. (size) 2=0-3-8, 11=Mechanical
 Max Horz 2=152(LC 16)
 Max Uplift 2=-27(LC 16), 11=-17(LC 12)
 Max Grav 2=1885(LC 3), 11=1850(LC 3)

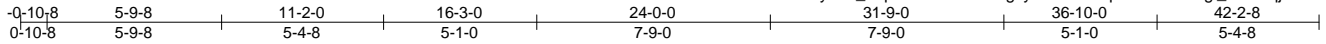
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-3499/80, 3-5=-2957/104, 5-6=-2571/128, 6-8=-3000/124, 8-9=-1774/106,
 9-10=-2067/82, 10-11=-1750/83
 BOT CHORD 2-18=-97/3034, 16-18=-97/3034, 14-16=-137/3002, 12-14=-119/2734
 WEBS 6-16=-701/148, 8-14=0/583, 8-12=-1352/142, 9-12=0/614, 10-12=-23/1887, 5-16=0/956,
 3-16=-626/148

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-1 to 2-1-15, Interior(1) 2-1-15 to 13-7-0, Exterior(2R) 13-7-0 to 17-9-15, Interior(1) 17-9-15 to 34-5-0, Exterior(2R) 34-5-0 to 38-7-15, Interior(1) 38-7-15 to 42-0-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Job 150_1910_D_VO	Truss H4	Truss Type Hip	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987475
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84 Components (Dunn), Dunn, NC - 28334, 8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:25 2021 Page 1
 ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-Ygsyt6weDKiuq5rXdiDL4JOGi_Pc4mqjEtrAXZzJTUG



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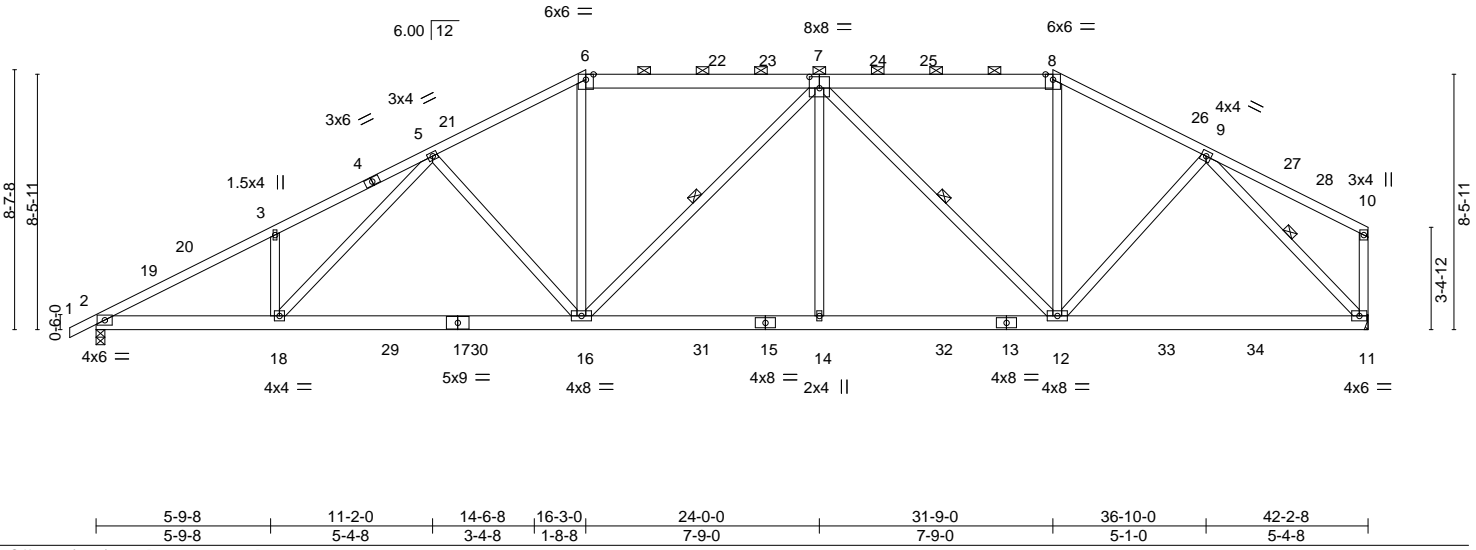


Plate Offsets (X,Y)-- [7:0-4-0,0-4-8]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.60	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15	BC 0.80	Vert(LL) -0.23 16-18 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.74	Vert(CT) -0.42 16-18 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.10 11 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 299 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 7-8,6-7: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-5-14 oc purlins, except end verticals, and 2-0-0 oc purlins (4-5-15 max.): 6-8.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midt 7-16, 7-12, 9-11

REACTIONS. (size) 2=0-3-8, 11=Mechanical
 Max Horz 2=173(LC 16)
 Max Uplift 2=47(LC 16)
 Max Grav 2=1923(LC 3), 11=1900(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-3625/58, 3-5=-3568/133, 5-6=-2703/132, 6-7=-2373/141, 7-8=-1870/122, 8-9=-2141/119
 BOT CHORD 2-18=-132/3141, 16-18=-82/2681, 14-16=-45/2518, 12-14=-45/2518, 11-12=-44/1505
 WEBS 6-16=0/890, 7-16=-375/122, 7-14=0/372, 7-12=-983/101, 8-12=0/640, 9-12=0/660, 9-11=-2141/72, 5-16=-567/160, 3-18=-253/153, 5-18=-74/695

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 16-3-0, Exterior(2R) 16-3-0 to 20-5-15, Interior(1) 20-5-15 to 31-9-0, Exterior(2R) 31-9-0 to 35-11-15, Interior(1) 35-11-15 to 42-0-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss connections.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

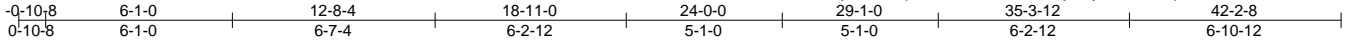


May 6, 2021

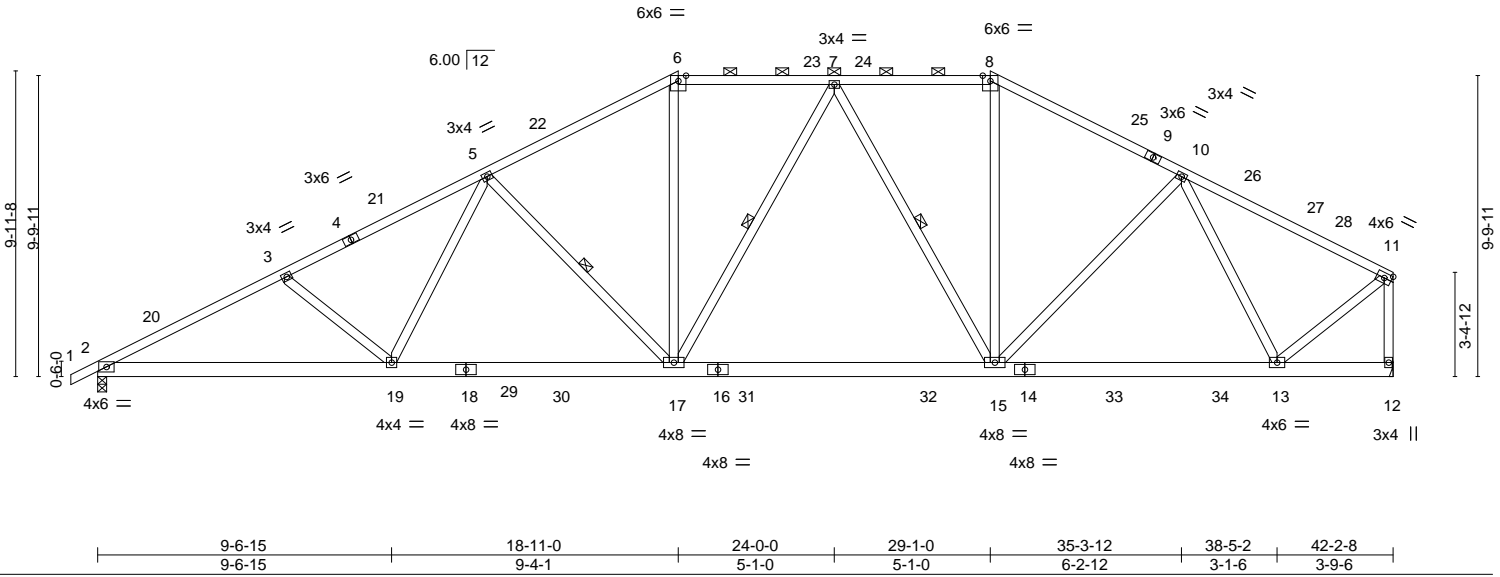
Job 150_1910_D_VO	Truss H5	Truss Type Hip	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987476
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:27 2021 Page 1
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-U3ziloyvixyc3P_wi7Fp9kU_To4eYeA0iAKHcSzJTUE



Scale = 1:75.1



LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.71	Vert(LL)	-0.22 15-17	>999	240	MT20	197/144
Snow (Pf/Pg)	16.5/15.0	Lumber DOL	1.15	BC	0.83	Vert(CT)	-0.38 15-17	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(CT)	0.09 12	n/a	n/a		
BCLL	0.0 *	Code IRC2018/TPI2014		Matrix-S						Weight: 290 lb	FT = 20%
BCDL	10.0										

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 2-7-5 oc purlins, except end verticals, and 2-0-0 oc purlins (3-9-12 max.): 6-8.
BOT CHORD	2x6 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3	WEBS	1 Row at midpt 5-17, 7-17, 7-15

REACTIONS.	(size) 2=0-3-8, 12=Mechanical
	Max Horz 2=192(LC 16)
	Max Uplift 2=65(LC 16), 12=6(LC 17)
	Max Grav 2=1916(LC 3), 12=1890(LC 3)

FORCES.	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-3=-3478/120, 3-5=-3256/104, 5-6=-2450/148, 6-7=-2121/158, 7-8=-1839/147, 8-10=-2136/142, 10-11=-1515/64, 11-12=-1864/54
BOT CHORD	2-19=-227/3019, 17-19=-108/2594, 15-17=-10/2067, 13-15=-36/1681
WEBS	3-19=-300/163, 5-19=0/588, 5-17=-734/172, 6-17=0/814, 7-17=-148/259, 7-15=-571/97, 8-15=0/677, 10-15=-12/378, 11-13=0/1652, 10-13=-888/88

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 18-11-0, Exterior(2R) 18-11-0 to 23-1-15, Interior(1) 23-1-15 to 29-1-0, Exterior(2R) 29-1-0 to 33-3-15, Interior(1) 33-3-15 to 42-0-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



May 6, 2021

Job 150_1910_D_VO	Truss HG1	Truss Type HALF HIP GIRDER	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987477
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:32 2021 Page 1
ID:VMD62r1yIHd_OqRtbnrIFztQ8K-r0nbLW01ZUauAAtuXgr_snBp?pnfDuEIrS12HfzJTU9



Scale: 3/16"=1'

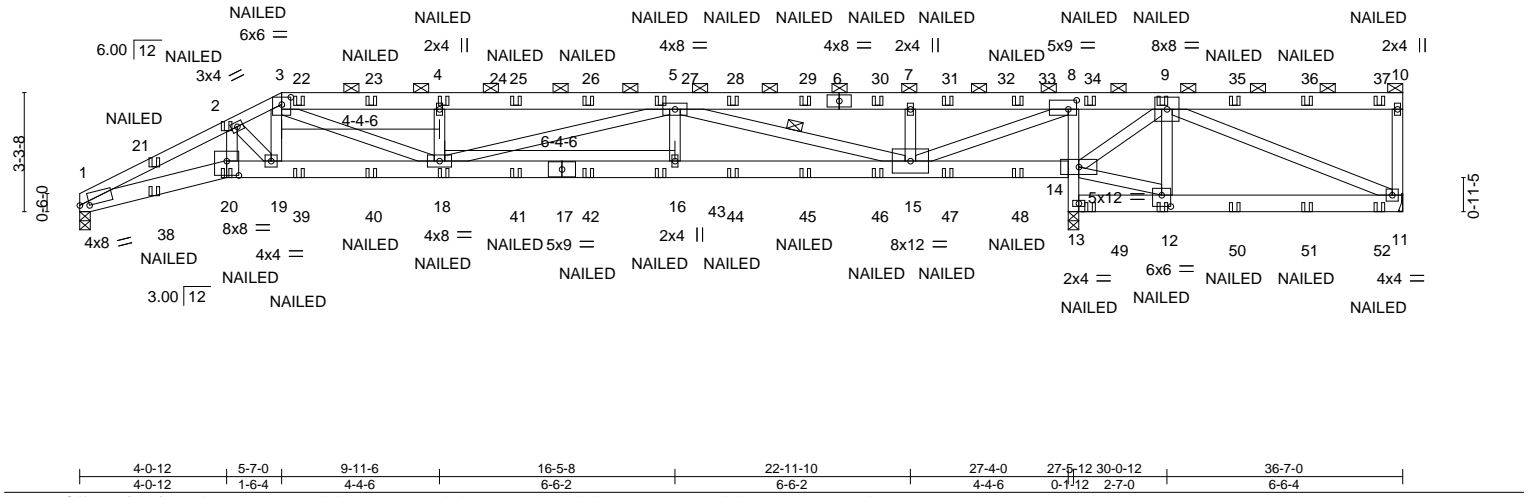


Plate Offsets (X,Y)--	[1:0-3-3,0-0-12], [3:0-3-0,0-2-7], [8:0-2-12,0-3-0], [12:0-3-0,0-3-12], [20:0-4-0,0-4-12]				
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15	TC 0.86	Vert(LL) -0.25 16-18 >999 240	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15	BC 0.84	Vert(CT) -0.52 16-18 >632 180		
TCDL 10.0	Rep Stress Incr NO	WB 0.88	Horz(CT) 0.14 13 n/a n/a		
BCLL 0.0 *	Code IRC2018/TPI2014	Matrix-S			
BCDL 10.0				Weight: 240 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2 *Except* 1-3: 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 2-3-9 oc purlins, except end verticals, and 2-0-0 oc purlins (3-4-7 max.): 3-10.
BOT CHORD 2x6 SP No.2 *Except* 8-13: 2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 2-9-13 oc bracing.
WEBS 2x4 SP No.3 *Except* 3-18,8-15: 2x4 SP No.2 or 2x4 SPF No.2	WEBS 1 Row at midpt 5-15

REACTIONS. (size) 1=0-3-8, 11=Mechanical, 13=(0-3-8 + H10A Simpson Strong-Tie) (req. 0-4-9)
 Max Horz 1=90(LC 54)
 Max Uplift 1=149(LC 9), 11=-198(LC 2), 13=-530(LC 9)
 Max Grav 1=1224(LC 2), 11=30(LC 9), 13=2910(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-3743/557, 2-3=-3069/500, 3-4=-3738/661, 4-5=-3737/661, 5-7=-298/70, 7-8=-298/70, 8-9=-546/3047
 BOT CHORD 1-20=-534/3310, 19-20=-507/3134, 18-19=-447/2711, 16-18=-584/3197, 15-16=-584/3197, 14-15=-3049/544, 13-14=-2939/533, 8-14=-1670/341, 12-13=-318/38, 11-12=-1146/201
 WEBS 2-20=-47/671, 2-19=-542/124, 3-19=-73/673, 3-18=-247/1158, 4-18=-500/218, 5-18=-85/651, 5-16=0/348, 5-15=-3041/543, 7-15=-421/212, 8-15=-657/3605, 12-14=-876/173, 9-14=-2378/432, 9-12=0/585, 9-11=-225/1279

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 1, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=198.
 - H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13. This connection is for uplift only and does not consider lateral forces.



May 6, 2021

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss HG1	Truss Type HALF HIP GIRDER	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987477
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:32 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-r0nbLW01ZUauAAtuXgr_snBp?pnfDuElrS12HfzJTU9

NOTES-

- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- 16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-43, 3-10=-53, 1-20=-20, 14-20=-20, 11-13=-20

Concentrated Loads (lb)

Vert: 20=-25(F) 2=-20(F) 18=-19(F) 4=-16(F) 12=-19(F) 9=-16(F) 21=-55(F) 22=-19(F) 23=-16(F) 25=-16(F) 26=-16(F) 27=-16(F) 28=-16(F) 29=-16(F) 30=-16(F) 31=-16(F) 33=-16(F) 34=-16(F) 35=-16(F) 36=-16(F) 37=-29(F) 38=-25(F) 39=-19(F) 40=-19(F) 41=-19(F) 42=-19(F) 43=-19(F) 44=-19(F) 45=-19(F) 46=-19(F) 47=-19(F) 48=-19(F) 49=-19(F) 50=-19(F) 51=-19(F) 52=-24(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



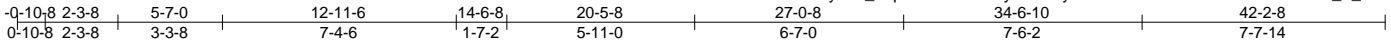
818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss HG2	Truss Type Half Hip Girder	Qty 1	Ply 2	KB Home 150.1910.D Vo2	145987478
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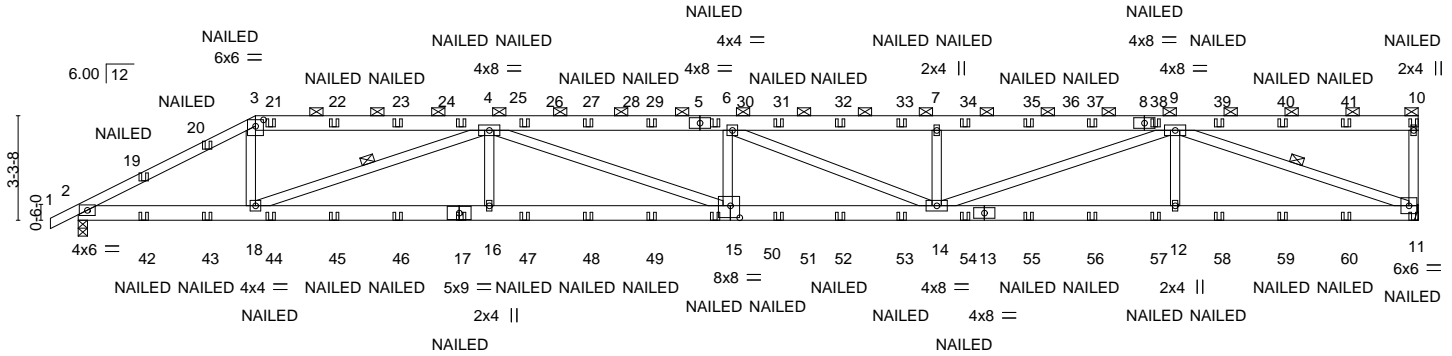
84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:45 2021 Page 1

ID:VMD62rz1yIHd_OqrtnrFzQ8K-yW3W4yABWTD2EAMNova1uXD8?2DXmo_fr_hEDPzJTty



Scale = 1:72.6



2-3-8	5-7-0	12-11-6	14-6-8	20-5-8	27-0-8	34-6-10	42-2-8
2-3-8	3-3-8	7-4-6	1-7-2	5-11-0	6-7-0	7-6-2	7-7-14

Plate Offsets (X,Y)-- [3:0-3-0,0-2-7], [15:0-3-8,0-4-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL	1.15	TC 0.39	Vert(LL)	-0.37	15	>999	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL	1.15	BC 0.85	Vert(CT)	-0.74	15	>676		
TCDL 10.0	Rep Stress Incr	NO	WB 0.79	Horz(CT)	0.13	11	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S						
BCDL 10.0								Weight: 543 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2 *Except* 1-3: 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 5-7-6 oc purlins, except end verticals, and 2-0-0 oc purlins (5-0-12 max.): 3-10.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midt 9-11, 4-18

REACTIONS. (size) 11=Mechanical, 2=0-3-8
 Max Horz 2=98(LC 55)
 Max Uplift 11=-431(LC 9), 2=-334(LC 9)
 Max Grav 11=2323(LC 2), 2=2309(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-4423/704, 3-4=-3753/634, 4-6=-8437/1539, 6-7=-7755/1420, 7-9=-7755/1420, 10-11=-307/142
 BOT CHORD 2-18=-641/3832, 16-18=-1303/7201, 15-16=-1303/7201, 14-15=-1533/8430, 12-14=-928/5056, 11-12=-928/5056
 WEBS 3-18=-151/1568, 4-16=0/408, 6-14=-734/124, 7-14=-522/249, 9-14=-526/2885, 9-12=0/434, 9-11=-5309/974, 4-18=-3718/730, 4-15=-258/1334

- NOTES-**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
 Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)



May 6, 2021

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPHI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss HG2	Truss Type Half Hip Girder	Qty 1	Ply 2	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987478
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:45 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnriFztQ8K-yW3W4yABWTD2EAMNova1uXD8?2DXmo_fr_hEDPzJTty

NOTES-

- 13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 16) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-43, 3-10=-53, 2-11=-20

Concentrated Loads (lb)

Vert: 10=-39(B) 11=-28(B) 17=-19(B) 19=-44(B) 20=-38(B) 21=-18(B) 22=-16(B) 23=-16(B) 25=-16(B) 26=-16(B) 27=-16(B) 29=-16(B) 30=-16(B) 31=-16(B) 32=-16(B) 33=-16(B) 34=-16(B) 35=-16(B) 37=-16(B) 38=-16(B) 39=-16(B) 40=-16(B) 41=-16(B) 42=-28(B) 43=-21(B) 44=-19(B) 45=-19(B) 46=-19(B) 47=-19(B) 48=-19(B) 49=-19(B) 50=-19(B) 51=-19(B) 52=-19(B) 53=-19(B) 54=-19(B) 55=-19(B) 56=-19(B) 57=-19(B) 58=-19(B) 59=-19(B) 60=-19(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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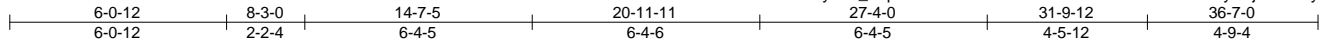


818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss HV1	Truss Type HALF HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987479
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:47 2021 Page 1
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-uvBGVeBS15TmTTWmwJcVzyJNjsseEhJyIIALiJzJTW



Scale: 3/16"=1'

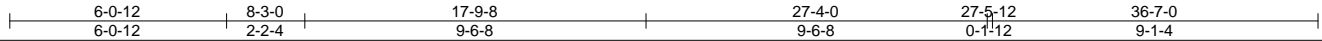
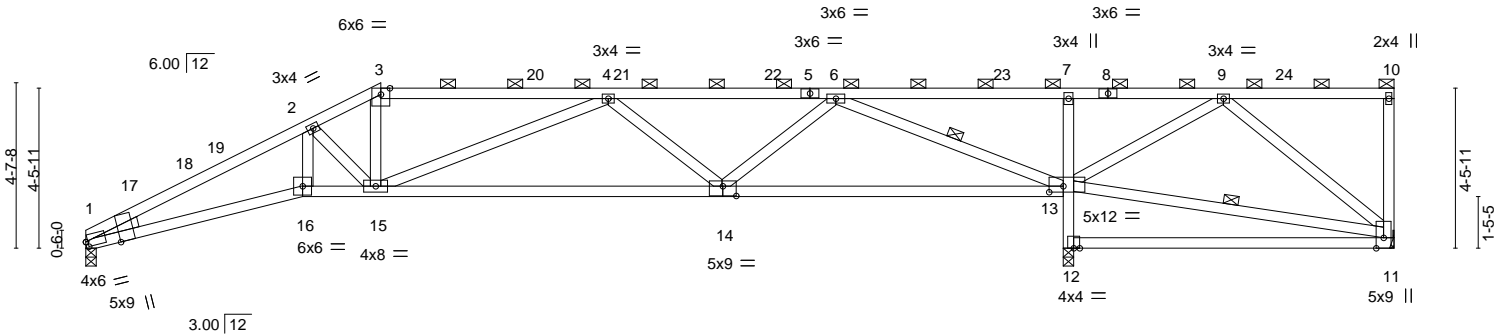


Plate Offsets (X,Y)-- [1:0-2-14,Edge], [1:0-0-11,0-1-12], [13:0-4-12,0-2-0], [14:0-4-8,0-3-4]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.89	Vert(LL) -0.22 11-12	>482	240		MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15		BC 1.00	Vert(CT) -0.48 14-15	>687	180			
TCDL 10.0	Rep Stress Incr YES		WB 0.80	Horz(CT) 0.13 12	n/a	n/a			
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S						
BCDL 10.0								Weight: 195 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (3-5-15 max.): 3-10.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 1-4-12 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midt 6-13, 11-13
WEDGE	
Left: 2x4 SP No.3	

REACTIONS. (size) 11=Mechanical, 1=0-3-8, 12=(0-3-8 + H10A Simpson Strong-Tie) (req. 0-3-9)
 Max Horz 1=133(LC 16)
 Max Uplift 11=-218(LC 2), 1=-1(LC 16), 12=-182(LC 13)
 Max Grav 11=18(LC 16), 1=904(LC 2), 12=2258(LC 36)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-2682/124, 2-3=-1993/93, 3-4=-1738/88, 4-6=-1344/51, 6-7=-138/1764, 7-9=-135/1788
 BOT CHORD 1-16=-182/2377, 15-16=-165/2246, 14-15=-166/1873, 13-14=-96/626, 12-13=-2167/237, 7-13=-373/107
 WEBS 2-16=-25/608, 2-15=-746/163, 3-15=0/650, 4-15=-217/250, 4-14=-698/152, 6-14=0/1013, 6-13=-2550/242, 11-13=-546/0, 9-13=-1508/158, 9-11=0/773

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 8-3-0, Exterior(2R) 8-3-0 to 12-5-15, Interior(1) 12-5-15 to 36-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=218.
 - H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12. This connection is for uplift only and does not consider lateral forces.

Continued on page 2



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
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ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss HV1	Truss Type HALF HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987479
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:47 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-uvBGVeBS15TmTTWmwJcVzyJNjsseEhJyIIALizJTw

- NOTES-**
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
 - 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

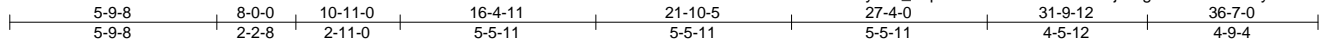


818 Soundside Road
Edenton, NC 27932

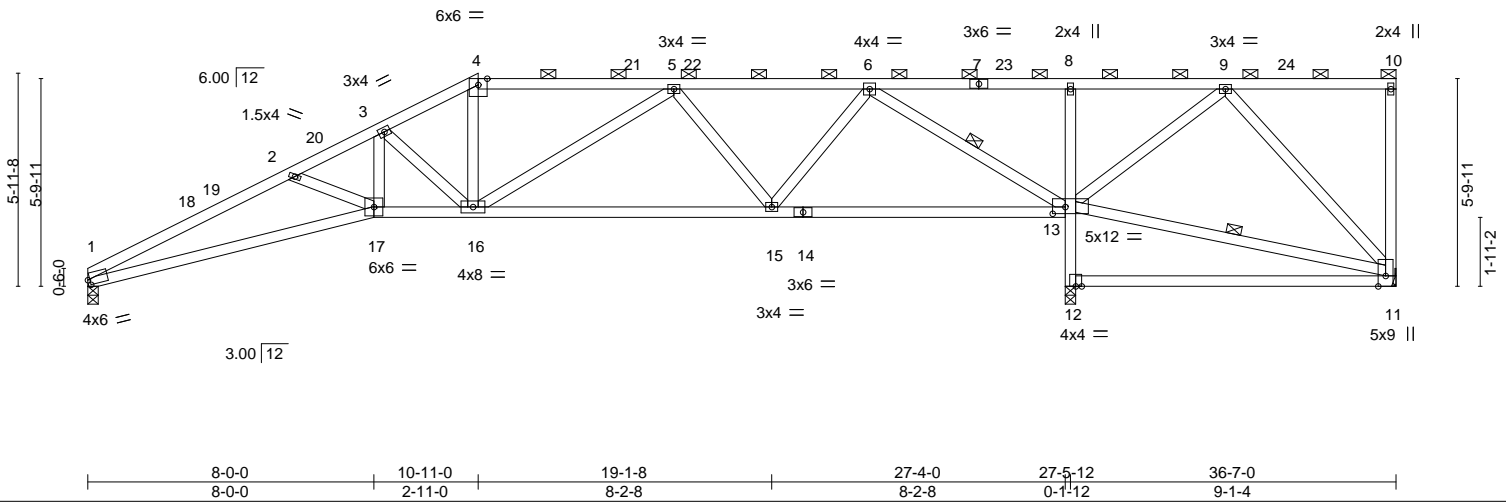
Job 150_1910_D_VO	Truss HV2	Truss Type HALF HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987480
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:49 2021 Page 1
ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-rlI0vKDizijUing91kez2NOhrfZyia6FmcfSMAzJTtu



Scale: 3/16"=1'



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.97	Vert(LL) -0.23	11-12	>466	240	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15		BC 0.95	Vert(CT) -0.46	11-12	>233	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.84	Horz(CT) 0.12	11	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S						
BCDL 10.0								Weight: 206 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (4-7-2 max.): 4-10.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 6-13, 11-13

REACTIONS. (size) 1=0-3-8, 11=Mechanical, 12=(0-3-8 + H10A Simpson Strong-Tie) (req. 0-3-10)
 Max Horz 1=174(LC 16)
 Max Uplift 1=8(LC 16), 11=320(LC 37), 12=169(LC 13)
 Max Grav 1=889(LC 37), 11=47(LC 16), 12=2324(LC 36)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-2550/158, 2-3=-2269/100, 3-4=-1551/66, 4-5=-1371/73, 5-6=-756/31, 6-8=-95/1548, 8-9=-91/1560
 BOT CHORD 1-17=-273/2271, 16-17=-132/1932, 15-16=-97/1169, 12-13=-2232/225, 8-13=-338/96
 WEBS 3-17=-67/757, 3-16=-799/110, 4-16=0/433, 5-16=-76/497, 5-15=-693/122, 6-15=0/921, 6-13=-1961/162, 11-13=-513/0, 9-13=-1387/126, 9-11=-6/791, 2-17=-322/147

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 10-11-0, Exterior(2R) 10-11-0 to 15-1-15, Interior(1) 15-1-15 to 36-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=320.
 - H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12. This connection is for uplift only and does not consider lateral forces.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and conforms to standard ANSI/TPI 1.



May 6, 2021

Job 150_1910_D_VO	Truss HV2	Truss Type HALF HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2 I45987480 Job Reference (optional)
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:49 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-rlI0vKDizijUing91kez2NOhrfZyia6FmcfSMAzJTtu

NOTES-

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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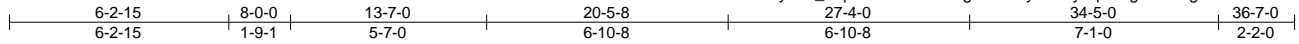


818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss HV3	Truss Type HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987481
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:51 2021 Page 1
ID:VMD62rz1yIHd_OqRtbnriFztQ8K-ngQnK?Ey5JzCy5qX99gR8oT2gTE4AWoYDv8YR3zJTts



Scale = 1:65.6

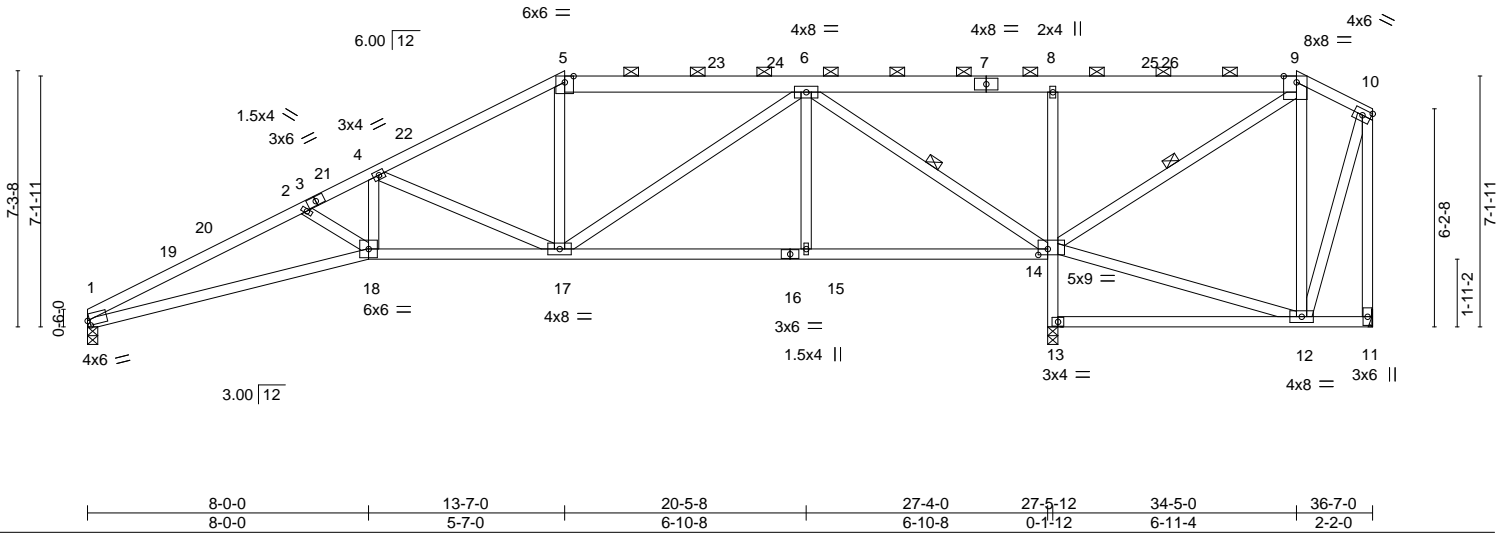


Plate Offsets (X,Y)-- [1:0-0-11,0-1-12], [9:0-4-6,Edge], [14:0-3-4,0-2-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.95	Vert(LL)	-0.19 1-18	>999	240	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15	BC 0.97	Vert(CT)	-0.43 1-18	>767	180		
TCDL 10.0	Lumber DOL 1.15	WB 0.77	Horz(CT)	0.12 13	n/a	n/a		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S						
BCDL 10.0	Code IRC2018/TPI2014						Weight: 242 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 5-7,7-9: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-9.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 6-14, 9-14

REACTIONS. (size) 1=0-3-8, 11=Mechanical, 13=(0-3-8 + H10A Simpson Strong-Tie) (req. 0-3-13)
Max Horz 1=200(LC 16)
Max Uplift 1=-12(LC 16), 11=-439(LC 55), 13=-137(LC 13)
Max Grav 1=856(LC 55), 11=77(LC 16), 13=2420(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2444/184, 2-4=-2202/141, 4-5=-1082/47, 5-6=-913/66, 6-8=-77/1269,
8-9=-74/1270, 10-11=-94/406
BOT CHORD 1-18=-317/2180, 17-18=-195/1888, 15-17=-58/410, 14-15=-58/410, 13-14=-2357/176,
8-14=-575/145
WEBS 4-18=-64/785, 4-17=-1185/182, 6-17=-122/904, 6-15=0/304, 6-14=-1834/91,
9-14=-1417/77, 9-12=-8/528, 10-12=-366/92

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 13-7-0, Exterior(2R) 13-7-0 to 17-9-15, Interior(1) 17-9-15 to 34-5-0, Exterior(2E) 34-5-0 to 36-5-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 1, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=439.
 - H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13. This connection is for uplift only and does not consider lateral forces.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.



May 6, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss HV3	Truss Type HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987481
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:51 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-ngQnK?Ey5JzCy5qX99gR8oT2gTE4AWoYDv8YR3zJTts

NOTES-

- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



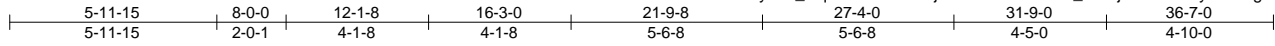
818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss HV4	Truss Type HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987482
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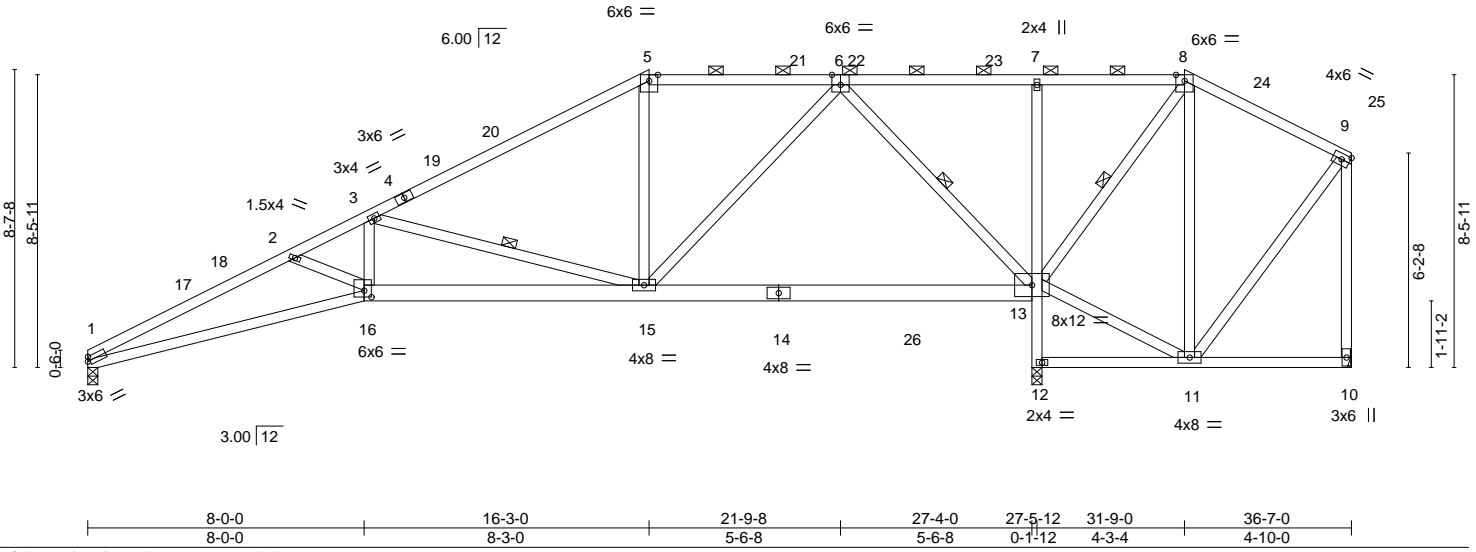
84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:53 2021 Page 1

ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-j3YXlHGcdxDwBO_wGajvDDZNVHy7eR6rgDdfWyzJTtq



Scale = 1:66.7



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 1.00	Vert(LL) -0.21	1-16	>999	240	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15		BC 0.80	Vert(CT) -0.42	1-16	>773	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.65	Horz(CT) 0.11	12	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S						
BCDL 10.0								Weight: 241 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-8.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 1-16: 2x4 SP No.1, 14-16,13-14: 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 3-0-8 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midt 3-15, 6-13, 8-13

REACTIONS. (size) 1=0-3-8, 10=Mechanical, 12=(0-3-8 + H10A Simpson Strong-Tie) (req. 0-4-4)
 Max Horz 1=218(LC 16)
 Max Uplift 1=-13(LC 16), 10=-549(LC 57), 12=-86(LC 16)
 Max Grav 1=902(LC 57), 10=108(LC 16), 12=2692(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-2498/200, 2-3=-2375/178, 3-5=-838/23, 5-6=-666/53, 6-7=-90/1146, 7-8=-87/1159, 8-9=-57/447, 9-10=-86/614
 BOT CHORD 1-16=-352/2247, 15-16=-272/2111, 13-15=-255/94, 12-13=-2631/146, 7-13=-391/104
 WEBS 3-16=-26/783, 3-15=-1565/293, 6-15=-76/1200, 6-13=-1527/110, 11-13=-338/87, 8-13=-1415/116, 8-11=-50/798, 9-11=-604/100

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 16-3-0, Exterior(2R) 16-3-0 to 20-5-15, Interior(1) 20-5-15 to 31-9-0, Exterior(2R) 31-9-0 to 35-11-15, Interior(1) 35-11-15 to 36-5-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=549.
 - H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12. This connection is for uplift only and does not consider lateral forces.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.



May 6, 2021

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss HV4	Truss Type HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987482
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:53 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-j3YXlhGCdxDwBO_wGajvDDZNVHy7eR6rgDdfWyzJTtq

NOTES-

- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

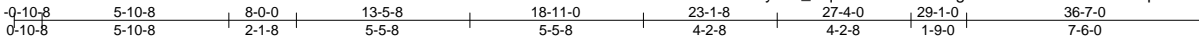


818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss HV5	Truss Type HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987483
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:55 2021 Page 1
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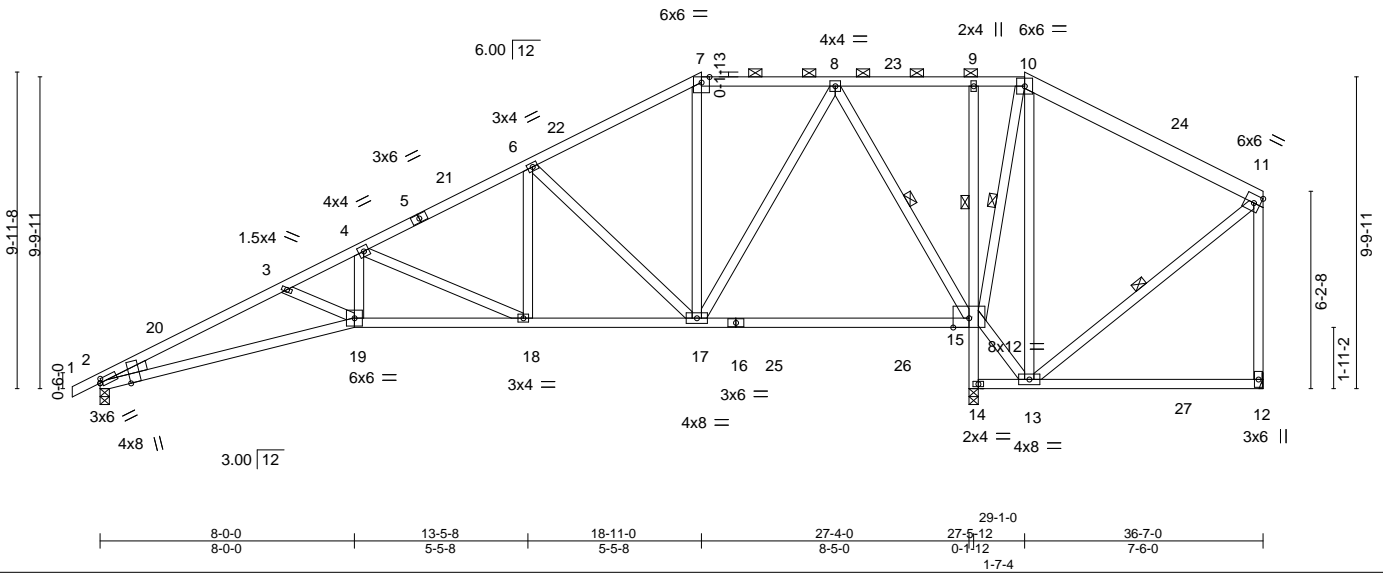


Plate Offsets (X,Y)-- [2:0-0-12,0-1-8], [2:0-2-14,Edge]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.60	Vert(LL) -0.27	15-17	>999	240	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15		BC 0.80	Vert(CT) -0.44	15-17	>741	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.94	Horz(CT) 0.10	14	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S						
BCDL 10.0								Weight: 257 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 10-11: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-10-2 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-10.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 2-19: 2x4 SP No.1	BOT CHORD Rigid ceiling directly applied or 3-1-3 oc bracing. Except: 1 Row at midpt 9-15
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 8-15, 10-15, 11-13
WEDGE Left: 2x4 SP No.3	

REACTIONS. (size) 2=0-3-8, 12=Mechanical, 14=(0-3-8 + H10A Simpson Strong-Tie) (req. 0-4-0)
Max Horz 2=241(LC 16)
Max Uplift 2=-33(LC 16), 12=-493(LC 56), 14=-125(LC 16)
Max Grav 2=988(LC 3), 12=117(LC 16), 14=2551(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2624/238, 3-4=-2423/182, 4-6=-1236/63, 6-7=-503/60, 7-8=-393/74, 8-9=-60/813, 9-10=-58/813, 10-11=-79/610, 11-12=-80/580
BOT CHORD 2-19=-404/2361, 18-19=-269/2086, 17-18=-107/1050, 14-15=-2628/95
WEBS 4-19=-67/872, 4-18=-1154/178, 6-18=-20/606, 6-17=-930/177, 8-17=-91/1122, 8-15=-1337/115, 13-15=-632/163, 10-15=-1619/157, 10-13=-91/1127, 11-13=-648/112

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 18-11-0, Exterior(2R) 18-11-0 to 23-1-8, Interior(1) 23-1-8 to 29-1-0, Exterior(2R) 29-1-0 to 33-3-15, Interior(1) 33-3-15 to 36-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 2, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)



May 6, 2021

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
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ENGINEERING BY TRESCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss HV5	Truss Type HIP	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987483
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:55 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnriFztQ8K-fSgHANIT9YTdQi7IO?INleepE4ee6H178X6maqzJTto

NOTES-

- 12) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.
- 13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

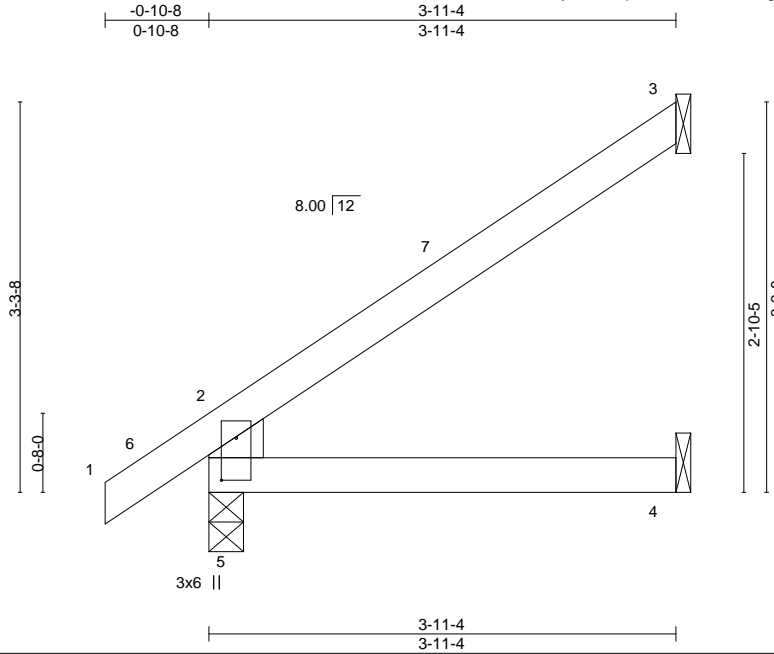


818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss J1	Truss Type Jack-Open	Qty 24	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987484
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:56 2021 Page 1
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Scale = 1:19.4

Plate Offsets (X,Y)-- [5:0-4-4,0-1-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.19	Vert(LL) -0.01	4-5	>999	240	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Lumber DOL 1.15		BC 0.15	Vert(CT) -0.02	4-5	>999	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.00	Horz(CT) 0.01	3	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-R					Weight: 16 lb	FT = 20%
BCDL 10.0									

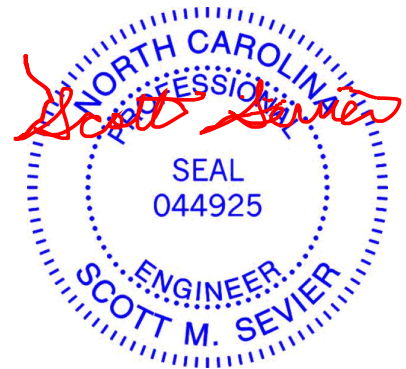
LUMBER-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
WEBS 2x6 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical
Max Horz 5=96(LC 14)
Max Uplift 3=54(LC 14)
Max Grav 5=222(LC 2), 3=99(LC 26), 4=68(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Refer to girder(s) for truss to truss connections.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
 - 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



May 6, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

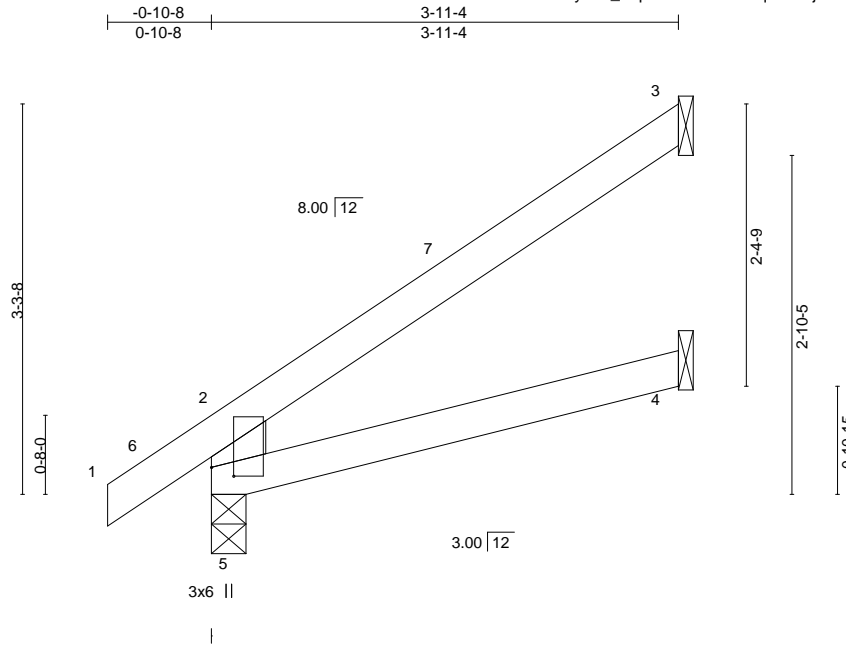


818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss J2	Truss Type Jack-Open	Qty 11	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987485
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:57 2021 Page 1
ID:VMD62rz1yiHD_OqRtnrIFztQ8K-cqn2b3Jjh9kLg0HhVQnrN3jF6uUHaPEQbrbtjzJTtm



Scale = 1:19.4

Plate Offsets (X,Y)-- [5:0-0-14,0-2-4]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.19	Vert(LL)	-0.01	4-5	>999	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.15	Vert(CT)	-0.02	4-5	>999		
TCDL 10.0	Lumber DOL 1.15	WB 0.00	Horz(CT)	0.01	3	n/a		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-R						
BCDL 10.0	Code IRC2018/TPI2014						Weight: 16 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x6 SP No.2	

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical
 Max Horz 5=96(LC 14)
 Max Uplift 3=54(LC 14)
 Max Grav 5=222(LC 2), 3=100(LC 26), 4=68(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Refer to girder(s) for truss to truss connections.
 - 7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
 - 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Job 150_1910_D_VO	Truss J3	Truss Type Jack-Open	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987486
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84 Components (Dunn), Dunn, NC - 28334, 8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:58 2021 Page 1
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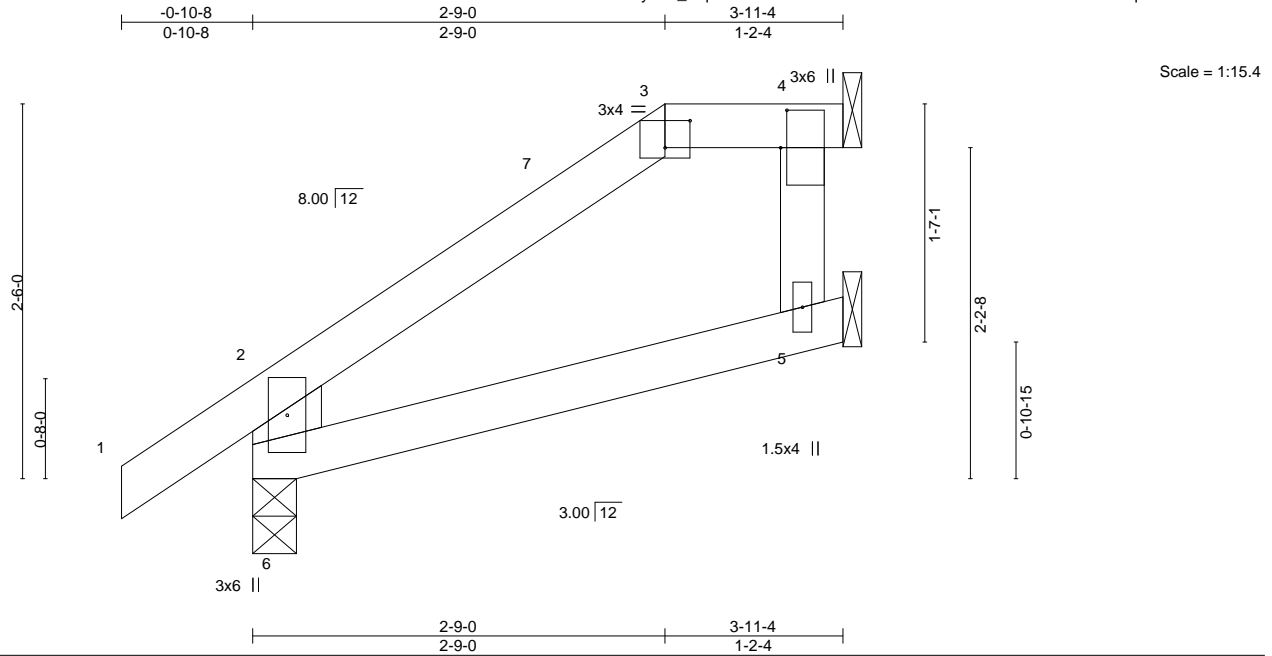


Plate Offsets (X,Y)-- [3:0-2-0,0-2-3], [4:0-3-0,0-0-8]

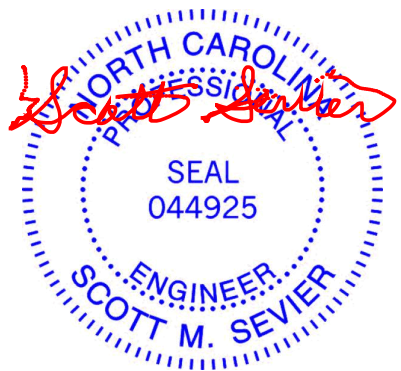
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.14	Vert(LL) -0.00	5-6	>999	240	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL 1.15		BC 0.09	Vert(CT) -0.01	5-6	>999	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.00	Horz(CT) -0.01	4	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-R						
BCDL 10.0								Weight: 17 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x6 SP No.2 *Except* 4-5: 2x4 SP No.3	

REACTIONS. (size) 6=0-3-8, 4=Mechanical, 5=Mechanical
 Max Horz 6=73(LC 14)
 Max Uplift 6=-8(LC 14), 4=-29(LC 14)
 Max Grav 6=214(LC 2), 4=79(LC 2), 5=76(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 2-9-0, Exterior(2E) 2-9-0 to 3-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces.
 - One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



May 6, 2021

Job 150_1910_D_VO	Truss J4	Truss Type Jack-Open Girder	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987487
84 Components (Dunn), Dunn, NC - 28334,					8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:59 2021 Page 1 ID:VMD62rz1yIHd_OqRtbnrlFztQ8K-YDvo0kLzDn_3vJR4drqJSUpa6h9u2Jk394zjbzJTtk	

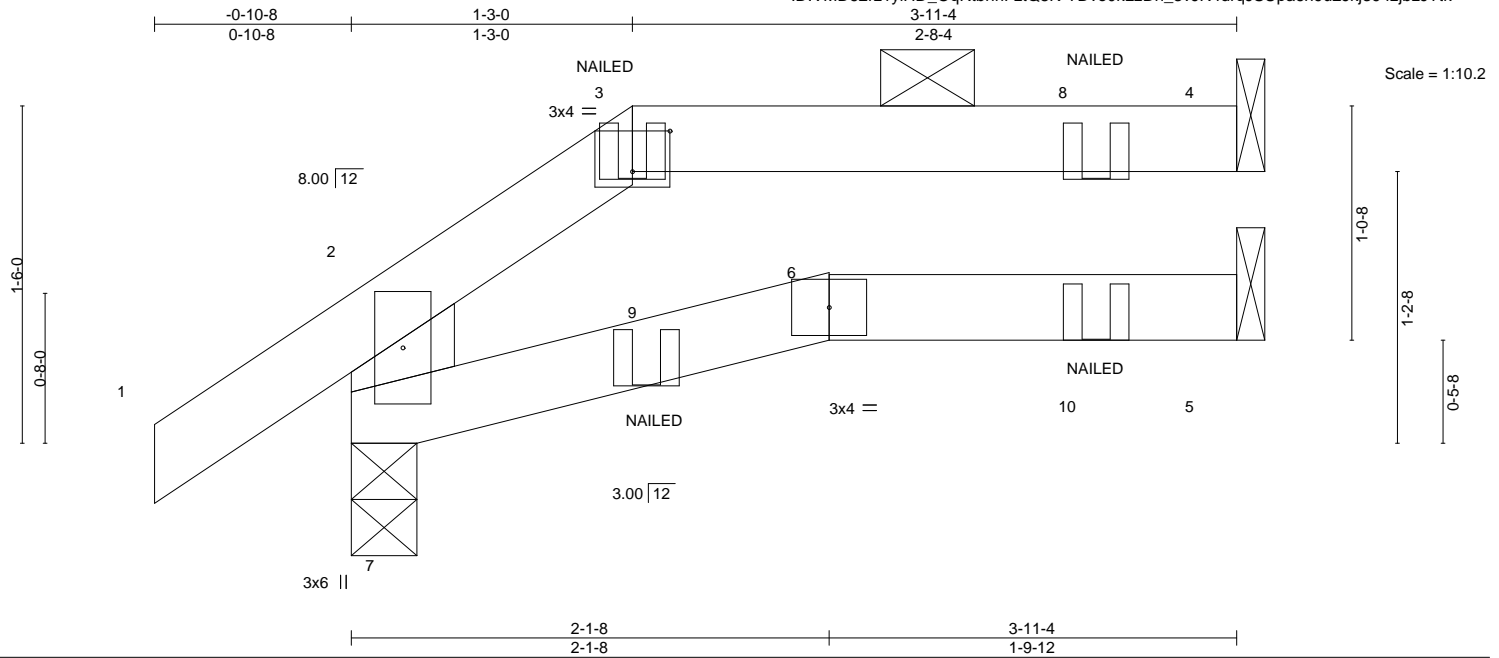


Plate Offsets (X,Y)--	[3:0-2-0,0-2-3]											
LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	-0.01	6	>999	240	MT20	197/144
Snow (Pf/Pg)	16.5/15.0	Lumber DOL	1.15	BC	0.14	Vert(CT)	-0.03	6	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.03	4	n/a	n/a		
BCLL	0.0 *	Code	IRC2018/TPI2014	Matrix-R							Weight: 15 lb	FT = 20%
BCDL	10.0											

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 3-11-4 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	2x6 SP No.2		

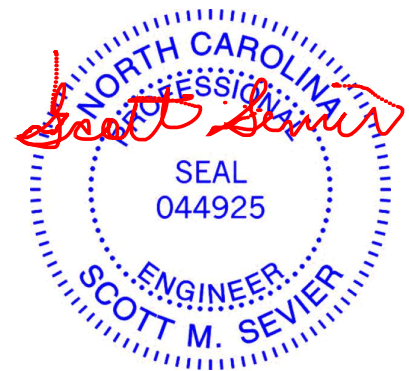
REACTIONS. (size) 7=0-3-8, 4=Mechanical, 5=Mechanical
 Max Horz 7=42(LC 10)
 Max Uplift 7=24(LC 10), 4=-44(LC 7)
 Max Grav 7=225(LC 2), 4=107(LC 30), 5=75(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.
 - One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2



May 6, 2021

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601</p>	<p>ENGINEERING BY TRENCO A MiTek Affiliate</p> <p>818 Soundside Road Edenton, NC 27932</p>
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Job 150_1910_D_VO	Truss J4	Truss Type Jack-Open Girder	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987487
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:51:59 2021 Page 2
ID:VMD62rz1yIHd_OqRtbnrlFztQ8K-YDvo0kLzDn_3vJR4drqJSUpa6h9u2Jkj394zjbzJTtk

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-43, 2-3=-43, 3-4=-53, 6-7=-20, 5-6=-20

Concentrated Loads (lb)

Vert: 3=-3(B) 8=-16(B) 9=-1(B) 10=-4(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss J5	Truss Type Jack-Open Girder	Qty 1	Ply 1	KB Home 150.1910.D Vo2	145987488
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84 Components (Dunn), Dunn, NC - 28334, 8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:00 2021 Page 1
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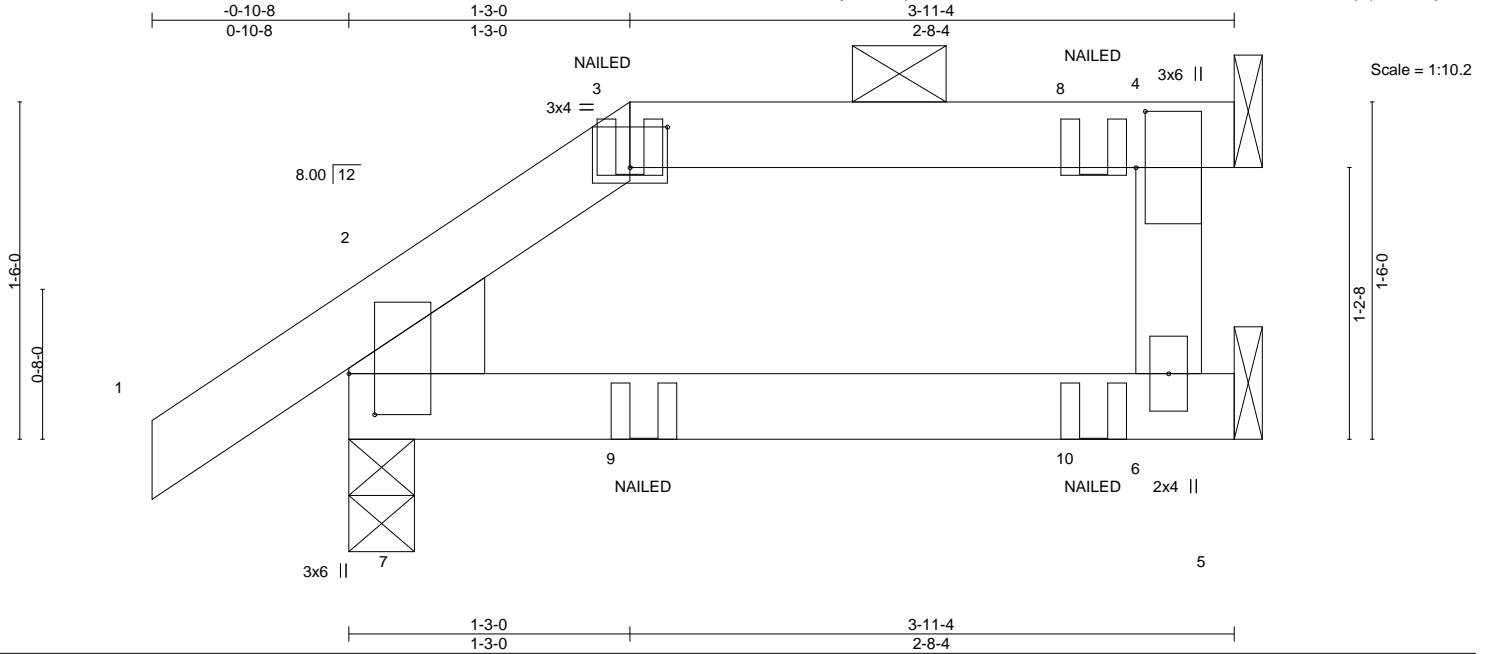


Plate Offsets (X,Y)-- [3:0-2-0,0-2-3], [4:0-3-0,0-0-8], [7:0-2-3,0-1-6]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL	1.15	TC 0.17	Vert(LL)	-0.01	6-7	>999	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Lumber DOL	1.15	BC 0.13	Vert(CT)	-0.01	6-7	>999		
TCDL 10.0	Rep Stress Incr	NO	WB 0.00	Horz(CT)	0.02	4	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-S					Weight: 17 lb	FT = 20%
BCDL 10.0									

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x8 SP No.2 *Except* 4-6: 2x4 SP No.3	

REACTIONS. (size) 7=0-3-8, 4=Mechanical, 6=Mechanical
 Max Horz 7=42(LC 10)
 Max Uplift 7=-25(LC 10), 4=-42(LC 7)
 Max Grav 7=218(LC 2), 4=93(LC 30), 6=82(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.
 - One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
 - "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2



May 6, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
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ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss J5	Truss Type Jack-Open Girder	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987488
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:00 2021 Page 2
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LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-43, 2-3=-43, 3-4=-53, 5-7=-20

Concentrated Loads (lb)

Vert: 3=-4(F) 8=-12(F) 10=-3(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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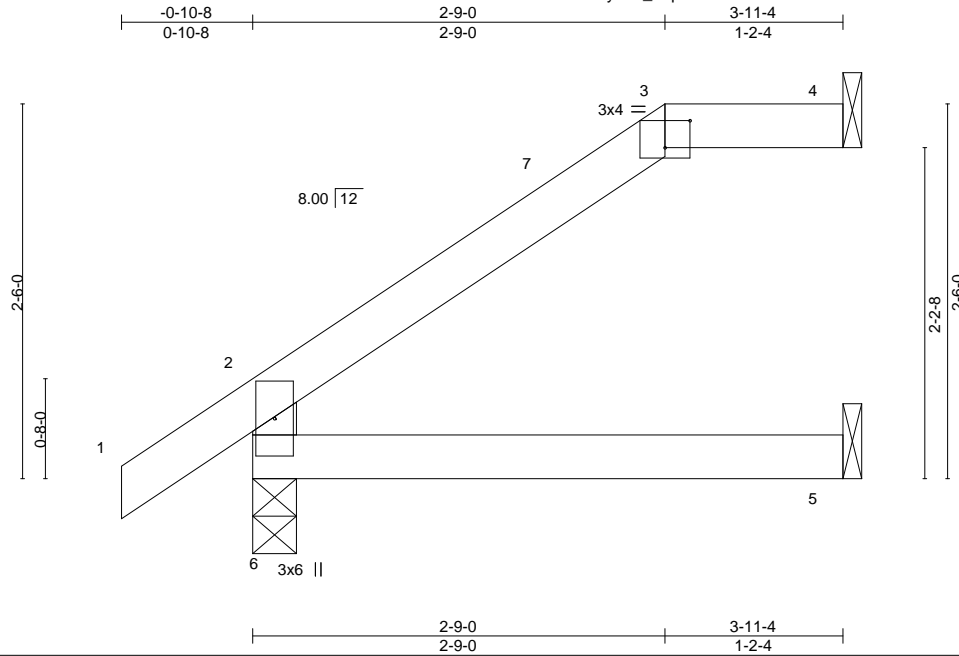
Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss J8	Truss Type Jack-Open	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987489
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84 Components (Dunn), Dunn, NC - 28334, 8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:01 2021 Page 1
 ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-Ub1ZRQMEkOEn8dbSkGsnYvuw1VrGWDD0WTZ4oUzJTti



Scale = 1:15.4

Plate Offsets (X,Y)--	[3:0-2-0,0-2-3]				
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.19	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15	BC 0.15	Vert(LL) -0.01 5-6 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.00	Vert(CT) -0.02 5-6 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-R	Horz(CT) 0.02 4 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 15 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.2 or 2x4 SPF No.2	

REACTIONS. (size) 6=0-3-8, 4=Mechanical, 5=Mechanical
 Max Horz 6=73(LC 14)
 Max Uplift 6=8(LC 14), 4=30(LC 14)
 Max Grav 6=219(LC 2), 4=98(LC 2), 5=70(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 2-9-0, Exterior(2E) 2-9-0 to 3-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces.
- One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



May 6, 2021

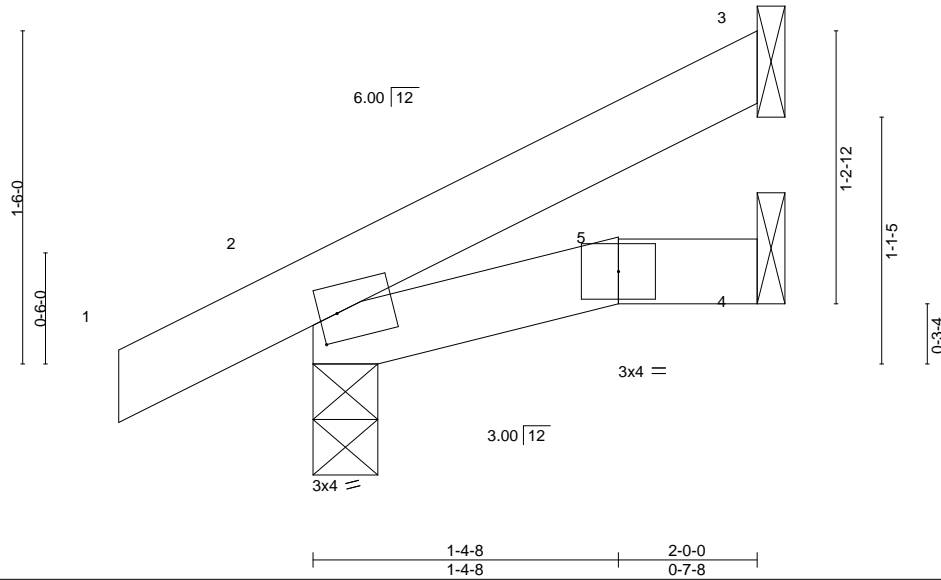
Job 150_1910_D_VO	Truss JA	Truss Type Jack-Open	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987490
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:02 2021 Page 1
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-yobxemNsViMemnAfizN046Q7jvDJFgT9I7JeKwzJTth



Scale = 1:10.4



LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL (roof)	20.0	Plate Grip DOL	2-0-0	TC	0.07	in (loc)	l/defl	L/d	MT20	197/144	
Snow (Pf/Pg)	11.6/15.0	Lumber DOL	1.15	BC	0.03	Vert(LL)	-0.00 2 >999	240			
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Vert(CT)	-0.00 5 >999	180			
BCLL	0.0 *	Code	IRC2018/TPI2014	Matrix-P		Horz(CT)	0.00 4 n/a	n/a			
BCDL	10.0									Weight: 8 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical
 Max Horz 2=44(LC 16)
 Max Uplift 3=-24(LC 16), 2=-11(LC 16)
 Max Grav 3=49(LC 23), 2=148(LC 23), 4=35(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - 3) Unbalanced snow loads have been considered for this design.
 - 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 7) Refer to girder(s) for truss to truss connections.
 - 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
 - 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

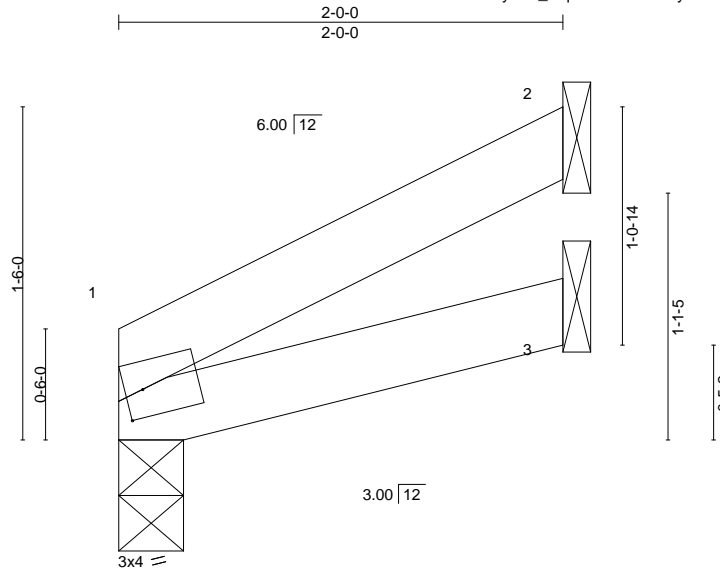


May 6, 2021

Job 150_1910_D_VO	Truss JB	Truss Type Jack-Open	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987491
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:02 2021 Page 1
ID:VMD62rz1yiHD_OqRtbnriFztQ8K-yobxemNsviMemnAfizN046Q7mvD8FgT9I7JeKwzJTth



Scale = 1:10.4

Plate Offsets (X,Y)-- [1:0-0-15,0-1-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.07	Vert(LL) -0.00	1	>999	240	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Lumber DOL 1.15		BC 0.04	Vert(CT) -0.00	1-3	>999	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.00	Horz(CT) -0.00	2	n/a	n/a		
BCLL 0.0 *	Code IRC2018/TPI2014		Matrix-P					Weight: 7 lb	FT = 20%
BCDL 10.0									

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

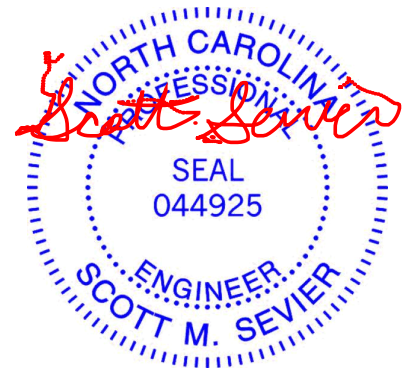
REACTIONS.

(size) 1=0-3-8, 2=Mechanical, 3=Mechanical
Max Horz 1=39(LC 16)
Max Uplift 2=-31(LC 16)
Max Grav 1=78(LC 2), 2=59(LC 2), 3=39(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



May 6,2021

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component

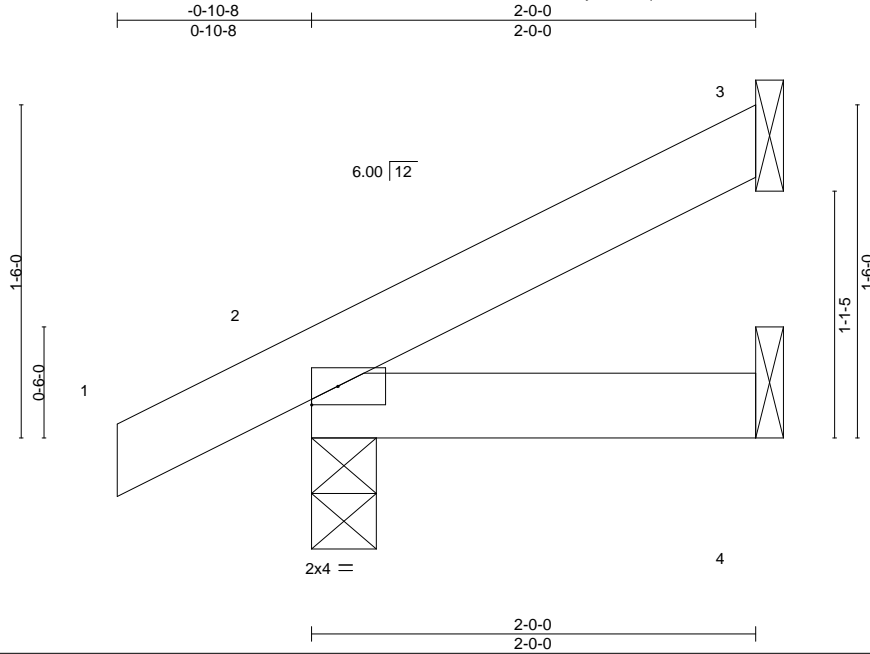


818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss JC	Truss Type Jack-Open	Qty 2	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987492
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:03 2021 Page 1
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-Q_9Jr6OUG?UVOxkrshuFdKzJgJZO_7jJ_n2BsMzJTg



LOADING (psf)		SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	-0.00	2	>999	240
Snow (Pf/Pg)	11.6/15.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	-0.00	2-4	>999	180
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a
BCLL	0.0 *	Code	IRC2018/TPI2014	Matrix-P						
BCDL	10.0								Weight: 8 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical
Max Horz 2=44(LC 16)
Max Uplift 3=27(LC 16), 2=-13(LC 16)
Max Grav 3=47(LC 23), 2=148(LC 23), 4=39(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



May 6, 2021

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Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component

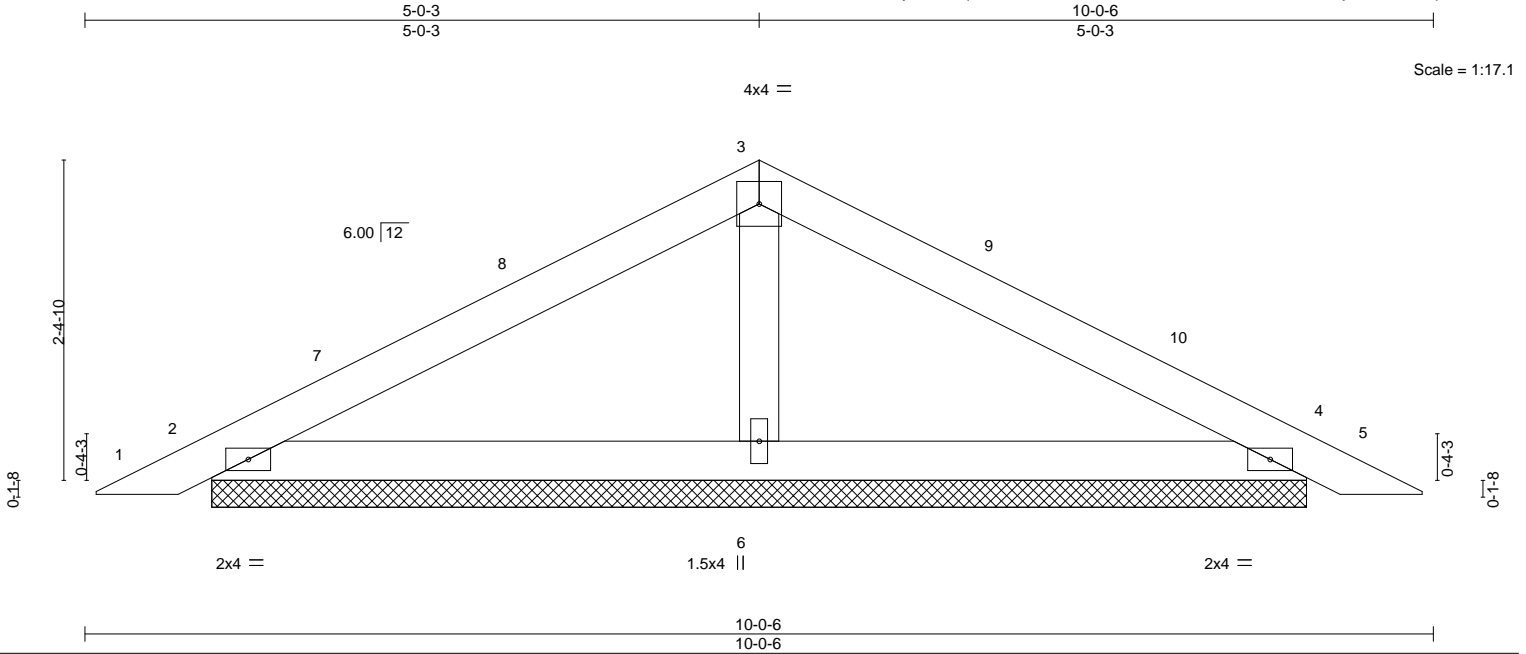


818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss PB1	Truss Type Piggyback	Qty 8	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987493
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:04 2021 Page 1
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-vAih3SP61JcM04J1POPU9XWPvitfjaGSCRokOpzJTtf



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.34	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.17	Vert(LL) 0.01 5 n/r 120		
TCDL 10.0	Lumber DOL 1.15	WB 0.05	Vert(CT) 0.02 5 n/r 120		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 4 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 31 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=8-1-12, 4=8-1-12, 6=8-1-12
Max Horz 2=-31(LC 17)
Max Uplift 2=-32(LC 16), 4=-38(LC 17)
Max Grav 2=202(LC 23), 4=202(LC 24), 6=317(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-11 to 3-4-11, Interior(1) 3-4-11 to 5-0-3, Exterior(2R) 5-0-3 to 8-0-3, Interior(1) 8-0-3 to 9-7-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) N/A
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



May 6, 2021

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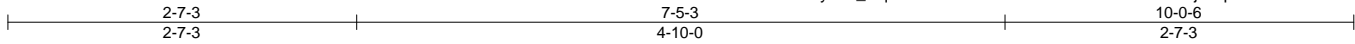
Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601
ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component



818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss PB2	Truss Type Piggyback	Qty 2	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987494
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84 Components (Dunn), Dunn, NC - 28334, 8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:05 2021 Page 1
ID:VMD62rz1yIHd_OqRtbnrIFztQ8K-NGG3GoPkodkDdEuDz5wjil2dp6DwS0UcR5XlxFzJTte



Scale = 1:17.2

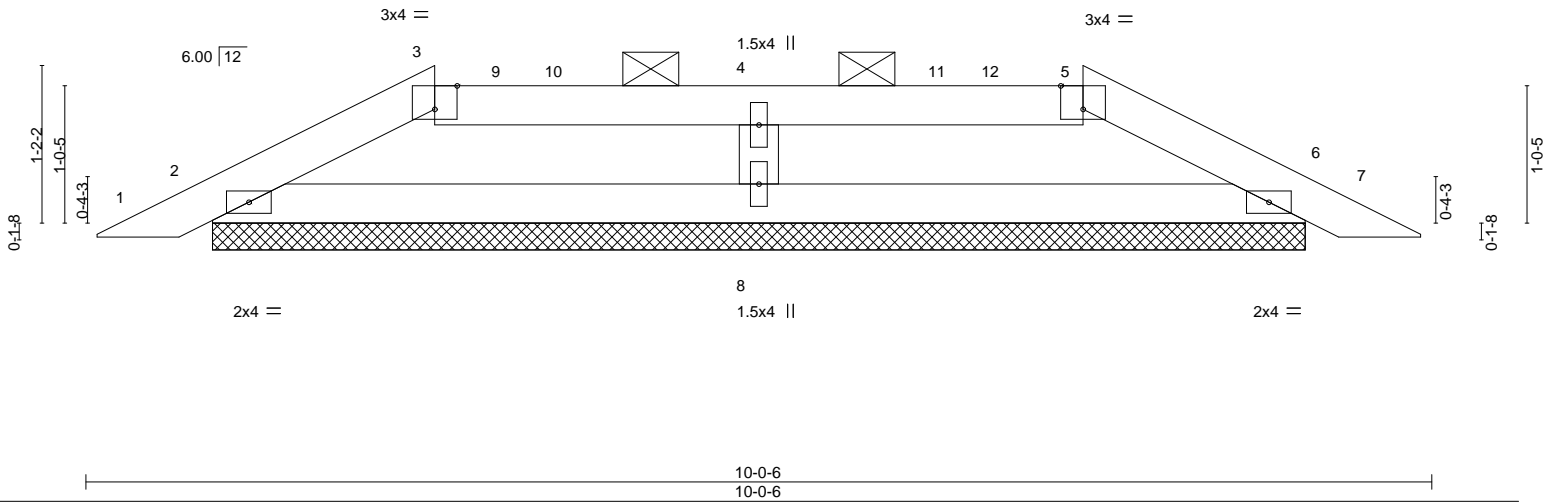


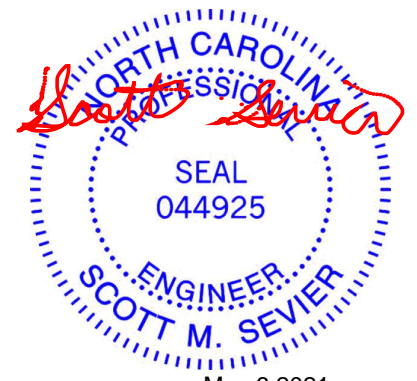
Plate Offsets (X,Y)--	[3:0-2-0,Edge], [5:0-2-0,Edge]									
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.14	Vert(LL)	0.00 7 n/r	120	MT20	197/144
Snow (Pf/Pg)	16.5/15.0	Lumber DOL	1.15	BC	0.16	Vert(CT)	0.00 7 n/r	120		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00 6 n/a	n/a		
BCLL	0.0 *	Code	IRC2018/TPI2014	Matrix-S					Weight: 28 lb	FT = 20%
BCDL	10.0									

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2	2-0-0 oc purlins (6-0-0 max.): 3-5.
WEBS 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=8-1-12, 6=8-1-12, 8=8-1-12
 Max Horz 2=-14(LC 21)
 Max Uplift 2=-20(LC 16), 6=-21(LC 17), 8=-5(LC 13)
 Max Grav 2=215(LC 41), 6=215(LC 41), 8=342(LC 40)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-11 to 2-7-3, Exterior(2R) 2-7-3 to 6-10-2, Interior(1) 6-10-2 to 7-5-3, Exterior(2E) 7-5-3 to 9-7-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - N/A
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



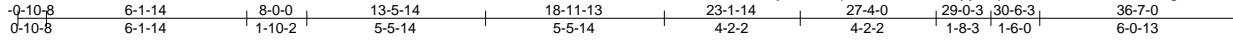
May 6, 2021

Job 150_1910_D_VO	Truss T1	Truss Type ROOF TRUSS	Qty 3	Ply 1	KB Home 150.1910.D Vo2	I45987495
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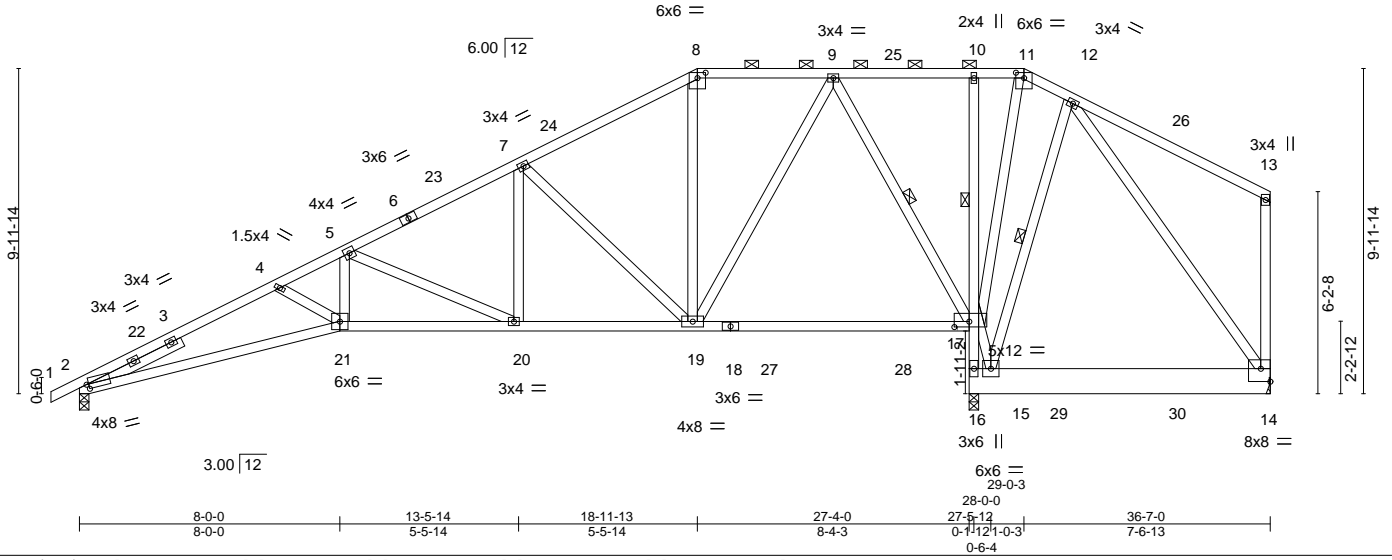
84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:08 2021 Page 1

ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-NyyCvpSd5Y6nUldoeEUQKNg2KK2lf8W273myXazJTtb



Scale = 1:70.8



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.57	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15	BC 0.95	Vert(LL) -0.26 17-19 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.99	Vert(CT) -0.42 17-19 >778 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.07 16 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 275 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 3-1-13 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 8-11.
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 2-21: 2x4 SP No.1, 10-16: 2x4 SP No.3, 14-16: 2x10 SP No.2	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except: 1 Row at midpt 10-17
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 9-17, 12-15
SLIDER Left 2x4 SP No.3 -t 3-2-13	

REACTIONS. (size) 2=0-3-8, 16=0-3-8, 14=Mechanical
 Max Horz 2=244(LC 16)
 Max Uplift 2=-46(LC 16), 16=-71(LC 16), 14=-282(LC 57)
 Max Grav 2=1071(LC 3), 16=2247(LC 3), 14=150(LC 46)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-4=-2853/277, 4-5=-2707/236, 5-7=-1487/103, 7-8=-720/91, 8-9=-579/103,
 9-10=-7/525, 10-11=-5/520, 11-12=-10/529
 BOT CHORD 2-21=-436/2551, 20-21=-321/2382, 19-20=-145/1272, 16-17=-984/0, 14-15=-359/47
 WEBS 5-21=-77/857, 5-20=-1231/192, 7-20=-24/643, 7-19=-968/183, 9-19=-77/1026,
 9-17=-1245/100, 15-17=-1094/273, 11-17=-312/0, 12-14=-85/651, 12-15=-618/137

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 18-11-13, Exterior(2R) 18-11-13 to 23-1-14, Interior(1) 23-1-14 to 29-0-3, Exterior(2R) 29-0-3 to 33-3-2, Interior(1) 33-3-2 to 36-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=282.
 - H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16. This connection is not to be used if the bearing wall does not consider lateral forces.



May 6, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss T1	Truss Type ROOF TRUSS	Qty 3	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987495
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:08 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-nyyCvpSd5Y6nUidoEQUQKNg2KK2lf8W273myXazJTb

NOTES-

- 13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 16) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

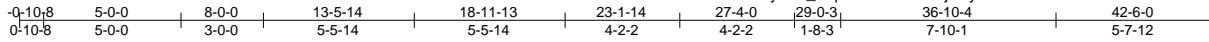
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss T2A	Truss Type ROOF TRUSS	Qty 3	Ply 1	KB Home 150.1910.D Vo2	I45987496
84 Components (Dunn), Dunn, NC - 28334,					8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:10 2021 Page 1	
					Job Reference (optional)	
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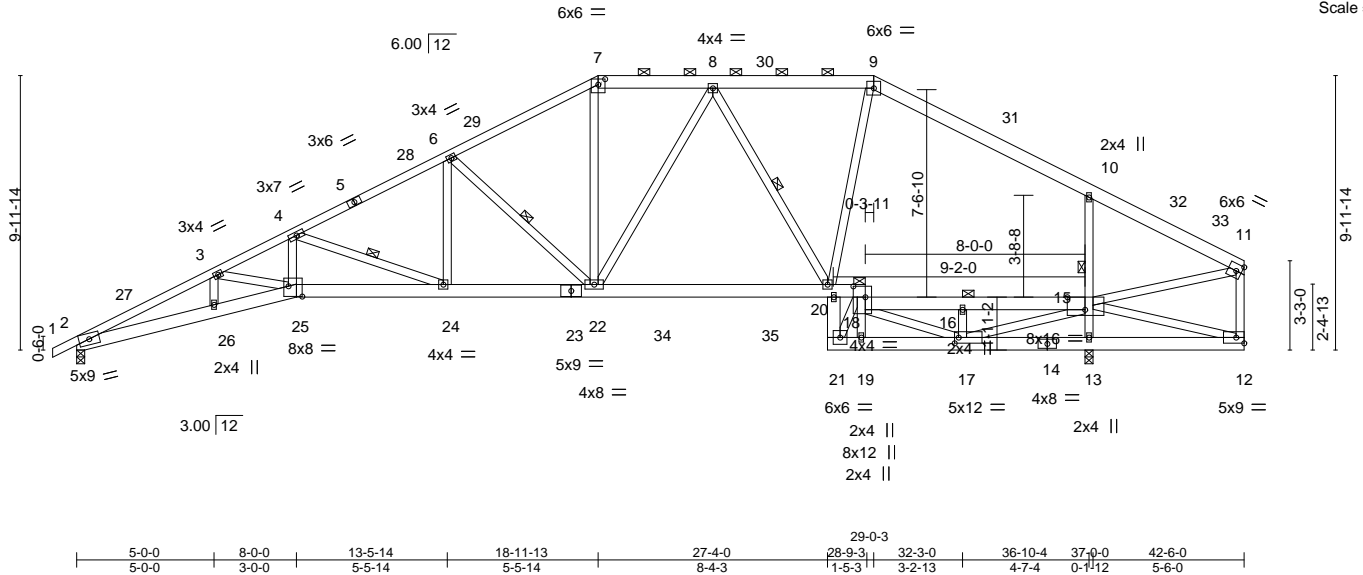


Plate Offsets (X,Y)--	[7:0-3-0,0-2-7], [15:0-7-12,Edge], [17:0-1-12,0-2-8], [18:0-4-12,0-5-4], [25:0-6-0,0-4-8]				
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.78	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15	BC 0.91	Vert(LL) -0.37 20-22 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.94	Vert(CT) -0.69 20-22 >643 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.33 13 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 343 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 7-9,9-11: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-0-14 oc purlins, except end verticals, and 2-0-0 oc purlins (5-2-1 max.): 7-9.
BOT CHORD 2x6 SP No.2 *Except* 2-25: 2x6 SP DSS	BOT CHORD Rigid ceiling directly applied or 3-1-4 oc bracing.
WEBS 2x4 SP No.3 *Except* 12-15,11-15: 2x4 SP No.1	WEBS 1 Row at midpt 4-24, 6-22, 8-20, 10-13
	JOINTS 1 Brace at Jt(s): 16, 18

REACTIONS. (size) 2=0-3-8, 13=(0-3-8 + H10A Simpson Strong-Tie) (req. 0-3-11)
 Max Horz 2=189(LC 16)
 Max Uplift 2=63(LC 16)
 Max Grav 2=1634(LC 3), 13=2360(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-5087/256, 3-4=-5044/254, 4-6=-3167/124, 6-7=-2288/104, 7-8=-1976/117, 8-9=-1452/88, 9-10=-1651/75, 10-11=-1521/0, 11-12=-1041/0
 BOT CHORD 2-26=-368/4549, 25-26=-366/4582, 24-25=-301/4587, 22-24=-110/2784, 20-22=0/1804, 18-20=0/1417, 20-21=-582/20, 19-21=-407/11, 17-19=-417/7, 13-17=-3916/0, 12-13=-4046/0, 16-18=0/3130, 15-16=0/3123
 WEBS 4-25=-54/1321, 4-24=-1925/204, 6-24=-13/827, 6-22=-1137/173, 7-22=0/792, 8-22=-64/489, 8-20=-744/87, 15-17=0/2273, 9-20=0/531, 13-15=-1997/54, 10-15=-687/201, 17-18=-1486/66, 12-15=0/4150, 18-21=-36/684, 11-15=0/1431

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 18-11-13, Exterior(2R) 18-11-13 to 23-1-14, Interior(1) 23-1-14 to 29-0-3, Exterior(2R) 29-0-3 to 33-3-2, Interior(1) 33-3-2 to 42-4-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - 6) Provide adequate drainage to prevent water ponding.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 9) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.



May 6, 2021

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ENGINEERING BY
TRENCO
 A MiTek Affiliate

818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D_VO	Truss T2A	Truss Type ROOF TRUSS	Qty 3	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987496
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:11 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnriFztQ8K-BXdLXrUVOTUML9MNKM17x0IVPX4isW?Up1_c&vzJtY

- NOTES-**
- 10) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13. This connection is for uplift only and does not consider lateral forces.
 - 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - 14) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component**

Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



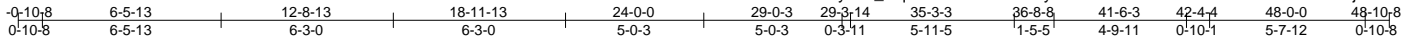
818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss T3A	Truss Type ROOF TRUSS	Qty 2	Ply 1	KB Home 150.1910.D Vo2	I45987497
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84 Components (Dunn), Dunn, NC - 28334,

8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:13 2021 Page 1

ID:VMD62rz1yIHd_OqRtbnriFztQ8K-8vlsyXVlw4I4bTVmRn4b1ROsdLncKQonHKTjDnzJTTW



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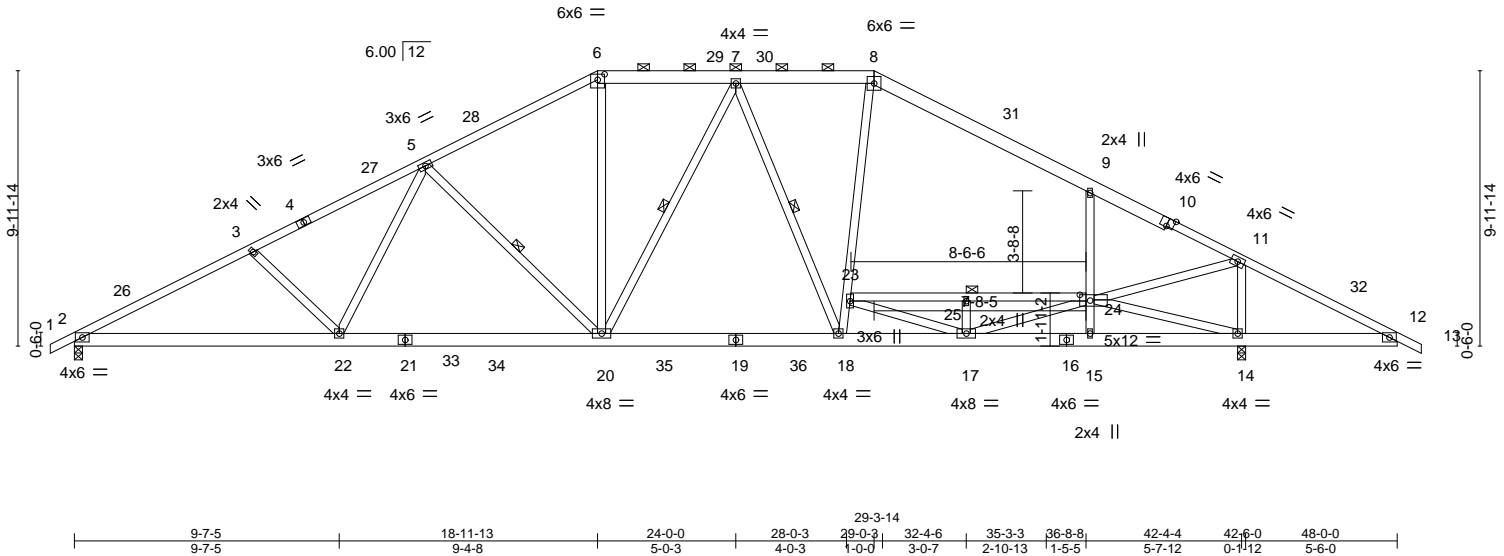


Plate Offsets (X,Y)-- [6:0-3-0,0-2-7], [10:0-3-0,Edge], [24:0-4-8,0-2-8]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.67	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15	BC 0.82	Vert(LL) -0.19 20-22 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.92	Vert(CT) -0.34 20-22 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.08 14 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 354 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 6-8,8-10: 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-5-12 oc purlins, except 2-0-0 oc purlins (5-1-5 max.): 6-8.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 12-14.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 5-20, 7-20, 7-18
	JOINTS 1 Brace at Jt(s): 25

REACTIONS. (size) 2=0-3-8, 14=(0-3-8 + H10A Simpson Strong-Tie) (req. 0-3-13)
 Max Horz 2=134(LC 16)
 Max Uplift 2=69(LC 16), 14=64(LC 17)
 Max Grav 2=1877(LC 3), 14=2413(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-3391/122, 3-5=-3177/108, 5-6=-2325/151, 6-7=-2028/155, 7-8=-1787/158,
 8-9=-2016/143, 9-11=-1956/36, 11-12=-107/571
 BOT CHORD 2-22=-166/2939, 20-22=-57/2509, 18-20=0/1948, 17-18=0/1712, 15-17=-156/771,
 14-15=-154/767, 12-14=-438/139
 WEBS 3-22=-296/159, 5-22=0/625, 5-20=-757/167, 6-20=0/739, 7-20=-96/313, 7-18=-517/90,
 11-14=-1891/118, 9-24=-567/195, 11-24=-27/2226, 23-25=-72/459, 24-25=-72/459,
 17-23=-480/75, 17-24=0/963, 14-24=-1136/267, 18-23=0/749, 8-23=0/590

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 18-11-13, Exterior(2R) 18-11-13 to 23-2-11, Interior(1) 23-2-11 to 29-0-3, Exterior(2R) 29-0-3 to 33-3-2, Interior(1) 33-3-2 to 48-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



May 6, 2021

Job 150_1910_D_VO	Truss T3A	Truss Type ROOF TRUSS	Qty 2	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987497
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84 Components (Dunn), Dunn, NC - 28334,

8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:13 2021 Page 2
ID:VMD62rz1yiHD_OqRtbnriFztQ8K-8vi5yXVlw4i4bTVmRn4b1ROsdLncKQonHKTJdznJTtW

NOTES-

- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job 150_1910_D_VO	Truss T4	Truss Type PIGGYBACK BASE	Qty 2	Ply 1	KB Home 150.1910.D Vo2	I45987498
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84 Components (Dunn), Dunn, NC - 28334, 8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:15 2021 Page 1
 ID:VMD62rz1yiHD_OqRtbnriFztQ8K-4ltnDX0Rh?oqnf8ZC636sTCt8T_oKA4keyqHgZJtTu

0-10-8	6-5-13	12-8-13	18-11-13	24-0-0	29-0-3	35-3-3	41-6-3	48-0-0	48-10-8
0-10-8	6-5-13	6-3-0	6-3-0	5-0-3	5-0-3	6-3-0	6-3-0	6-5-13	0-10-8

Scale = 1:83.6

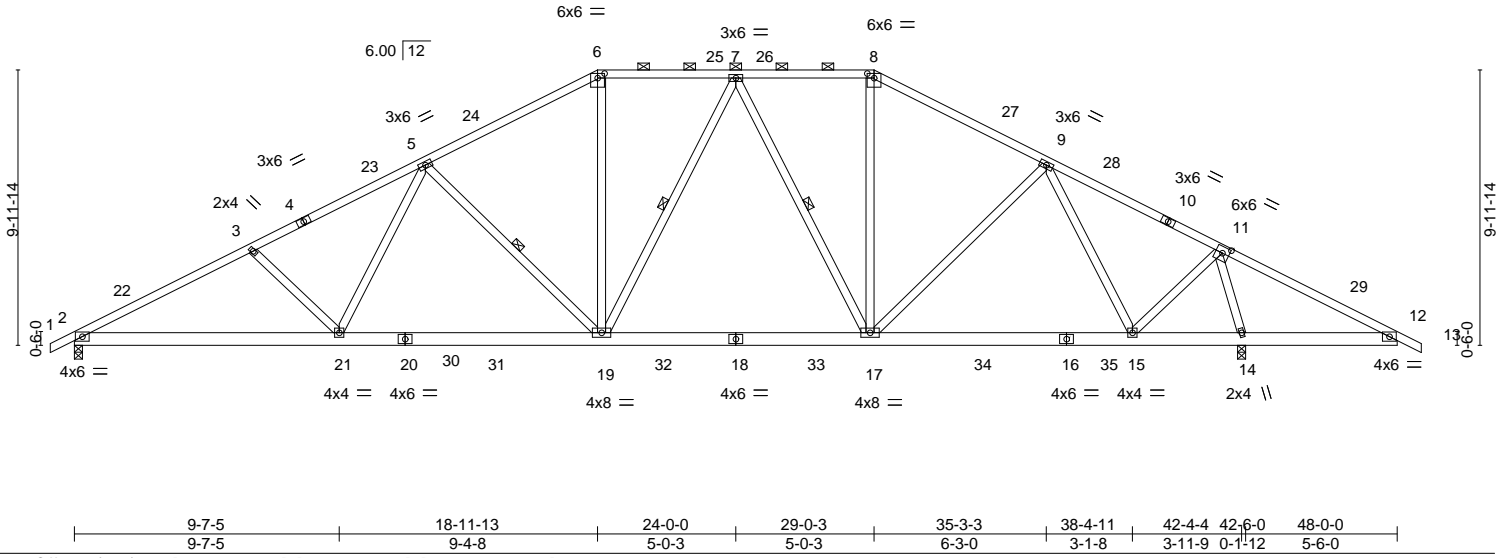


Plate Offsets (X,Y)--	[6:0-3-0,0-2-0], [8:0-3-0,0-2-0], [11:0-3-0,0-2-12]				
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.68	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15	BC 0.82	Vert(LL) -0.19 19-21 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.93	Vert(CT) -0.35 19-21 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.09 14 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 316 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD Structural wood sheathing directly applied or 2-5-4 oc purlins, except 2-0-0 oc purlins (3-10-11 max.): 6-8.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 5-19, 7-19, 7-17

REACTIONS. (size) 2=0-3-8, 14=(0-3-8 + H10A Simpson Strong-Tie) (req. 0-3-14)
 Max Horz 2=134(LC 16)
 Max Uplift 2=68(LC 16), 14=63(LC 17)
 Max Grav 2=1894(LC 3), 14=2459(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-3428/121, 3-5=-3215/108, 5-6=-2361/151, 6-7=-2054/156, 7-8=-1774/154,
 8-9=-2050/142, 9-11=-1383/81, 11-12=-105/614
 BOT CHORD 2-21=-166/2972, 19-21=-57/2544, 17-19=0/1978, 15-17=0/1568, 12-14=-469/140
 WEBS 3-21=-295/158, 5-21=0/622, 5-19=-772/171, 6-19=0/761, 7-19=-93/301, 7-17=-541/90,
 8-17=0/635, 9-17=0/374, 11-15=0/1538, 11-14=-2309/124, 9-15=-938/92

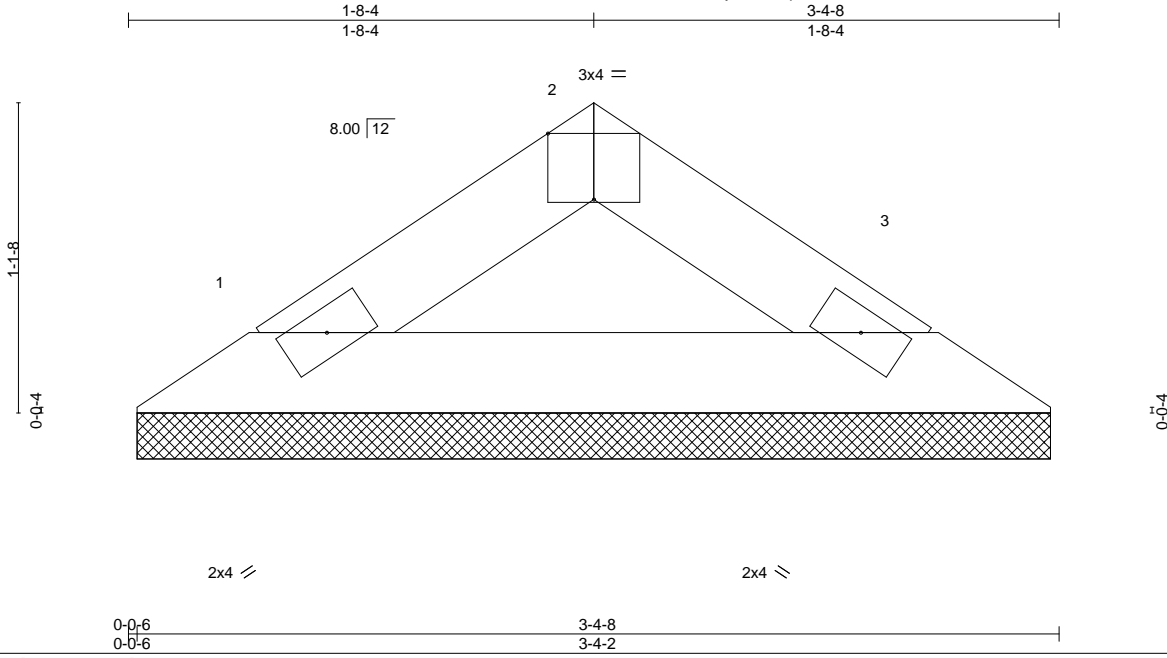
- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 18-11-13, Exterior(2R) 18-11-13 to 23-2-11, Interior(1) 23-2-11 to 29-0-3, Exterior(2R) 29-0-3 to 33-3-2, Interior(1) 33-3-2 to 48-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



May 6, 2021

Job 150_1910_D_VO	Truss V1	Truss Type Valley	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987499
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84 Components (Dunn), Dunn, NC - 28334, 8,500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:16 2021 Page 1
 ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-YUREaYyEc?7fSwEL6vdle30XeY_HX?xDziiNq6zJTt



Scale = 1:8.4

LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a - n/a	999	MT20	244/190	
Snow (Pf/Pg)	11.6/15.0	Lumber DOL	1.15	BC	0.11	Vert(CT)	n/a - n/a	999	Weight: 9 lb FT = 20%		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00 3 n/a	n/a			
BCLL	0.0 *	Code	IRC2018/TPI2014	Matrix-P							
BCDL	10.0										

LUMBER-
 TOP CHORD 2x4 SP No.3
 BOT CHORD 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 3-4-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=3-3-12, 3=3-3-12
 Max Horz 1=-16(LC 10)
 Max Uplift 1=-3(LC 14), 3=-3(LC 15)
 Max Grav 1=96(LC 2), 3=96(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

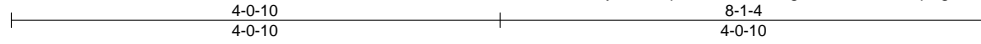
- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - N/A
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



May 6, 2021

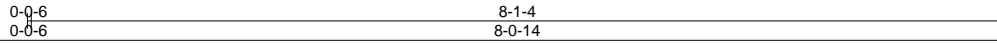
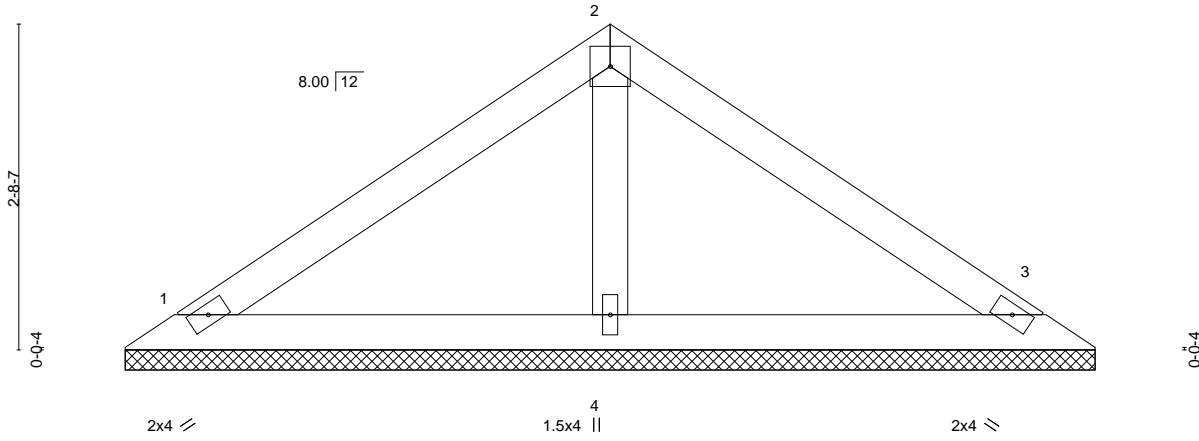
Job 150_1910_D_VO	Truss V2	Truss Type Valley	Qty 1	Ply 1	KB Home 150.1910.D Vo2 Job Reference (optional)	I45987500
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84 Components (Dunn), Dunn, NC - 28334, 8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:17 2021 Page 1
 ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-0g?couZGzJFW34pXgd8XBHYcdyluGSbNCyRwMZzJtTS



4x4 =

Scale = 1:19.1



LOADING (psf)		SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP		
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.41	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	11.6/15.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0 *	Code IRC2018/TPI2014		Matrix-P							Weight: 28 lb	FT = 20%
BCDL	10.0											

LUMBER-
 TOP CHORD 2x4 SP No.3
 BOT CHORD 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=8-0-8, 3=8-0-8, 4=8-0-8
 Max Horz 1=-48(LC 10)
 Max Uplift 1=-20(LC 14), 3=-26(LC 15)
 Max Grav 1=153(LC 2), 3=153(LC 2), 4=265(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-0-10, Exterior(2R) 4-0-10 to 7-0-10, Interior(1) 7-0-10 to 7-7-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - N/A
 - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



May 6, 2021

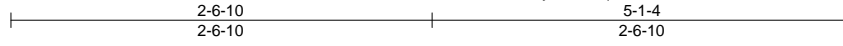
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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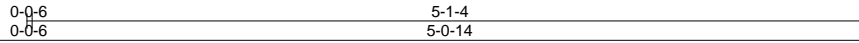
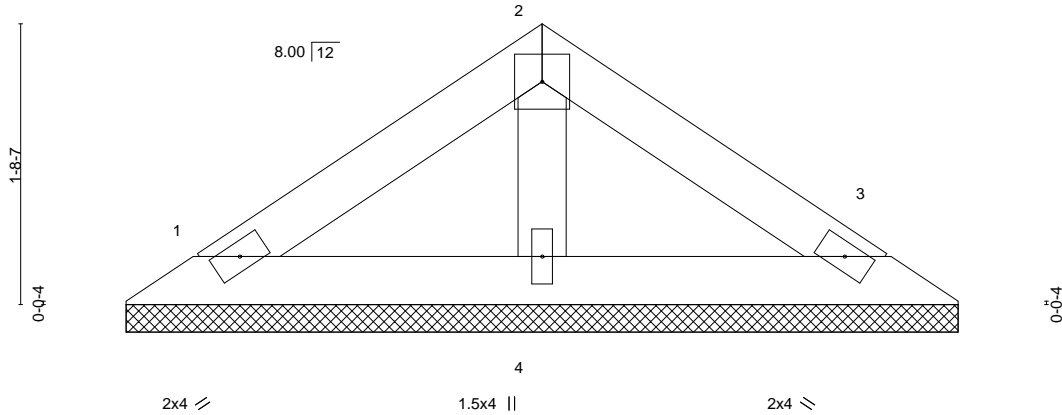
Job 150_1910_D_VO	Truss V3	Truss Type Valley	Qty 1	Ply 1	KB Home 150.1910.D Vo2	I45987501
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84 Components (Dunn), Dunn, NC - 28334, 8.500 s Feb 23 2021 MiTek Industries, Inc. Wed May 5 17:52:18 2021 Page 1
 ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-UtZ_?EZukcNNhEOjEKfmkU5soMfN?v7WQcBUu?zJTtR



4x4 =

Scale = 1:14.0



LOADING (psf)		SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.13	Vert(LL)	n/a	-	n/a	999
Snow (Pf/Pg)	11.6/15.0	Lumber DOL	1.15	BC	0.07	Vert(CT)	n/a	-	n/a	999
TCDL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	3	n/a	n/a
BCLL	0.0 *	Code IRC2018/TPI2014		Matrix-P						
BCDL	10.0								Weight: 17 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.3
 BOT CHORD 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 5-1-4 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=5-0-8, 3=5-0-8, 4=5-0-8
 Max Horz 1=-28(LC 10)
 Max Uplift 1=-11(LC 14), 3=-15(LC 15)
 Max Grav 1=89(LC 2), 3=89(LC 2), 4=154(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- N/A
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



May 6, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component

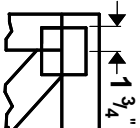
Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
 Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20 software** or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TFP 1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8
dimensions shown in ft-in-sixteenths
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TFP 1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TFP 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TFP 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Rewriting pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TFP 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

RE: 150_1910_D
KB 10x10 8pi

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:

Customer: Project Name: 150_1910_D
Lot/Block: Model:
Address: Subdivision:
City: State:

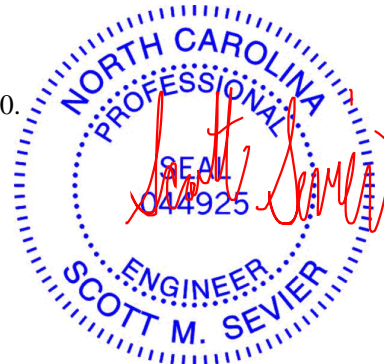
General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4
Wind Code: ASCE 7-10 Wind Speed: 115 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 5 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	I43213654	P10	11/23/2020
2	I43213655	P10E	11/23/2020
3	I43213656	VP1	11/23/2020
4	I43213657	VP2	11/23/2020
5	I43213658	VP3	11/23/2020

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383. Truss Design Engineer's Name: Sevier, Scott My license renewal date for the state of North Carolina is December 31, 2020. North Carolina COA: C-0844



IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

November 23, 2020

Job 150_1910_D	Truss P10	Truss Type COMMON	Qty 4	Ply 1	KB 10x10 8pi	143213654
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84 Components (Dunn),

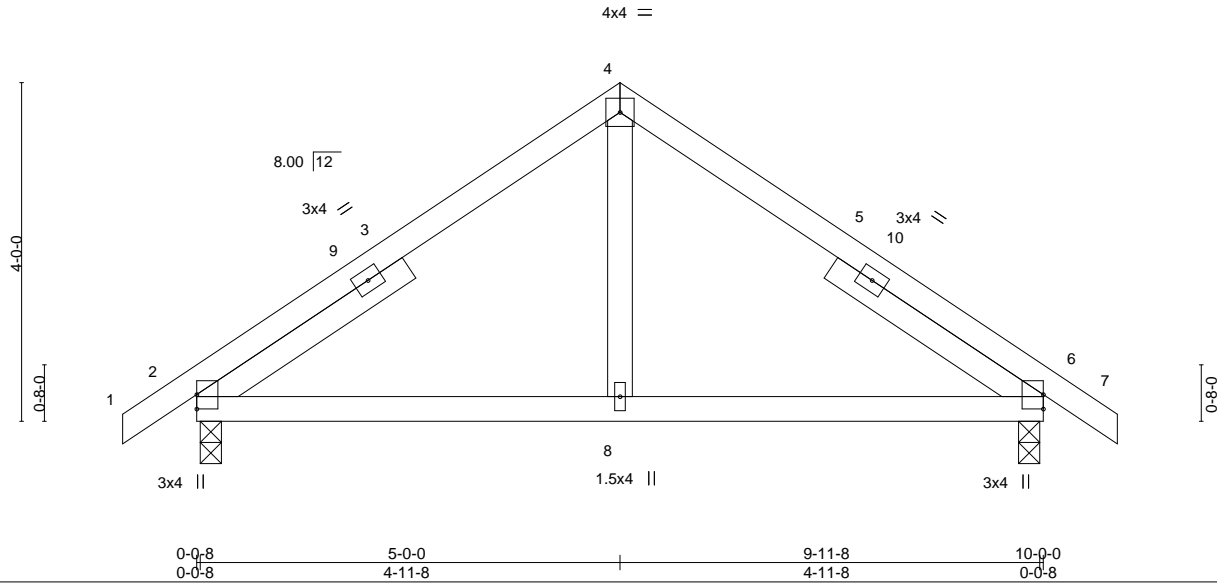
Dunn, NC - 28334,

8.420 s Aug 25 2020 MiTek Industries, Inc. Wed Oct 14 16:39:47 2020 Page 1

ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-P8cWDNYKgsUZGN2GDD67P8K8DHmnEclERLggWWayTRZQ



Scale = 1:27.2



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.30	in (loc) l/defl L/d	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.26	Vert(LL) 0.02 6-8 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.09	Vert(CT) -0.03 2-8 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 6 n/a n/a		
BCDL 10.0	Code IRC2015/TP12014			Weight: 50 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -t 2-11-3, Right 2x4 SP No.3 -t 2-11-3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-3-0
 Max Horz 2=-76(LC 12)
 Max Uplift 2=-19(LC 14), 6=-19(LC 15)
 Max Grav 2=453(LC 2), 6=453(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-4=-464/244, 4-6=-464/244
 BOT CHORD 2-8=-121/301, 6-8=-121/301

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCCL=6.0psf; BCCL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-0-0, Exterior(2) 5-0-0 to 8-0-0, Interior(1) 8-0-0 to 10-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



October 15, 2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

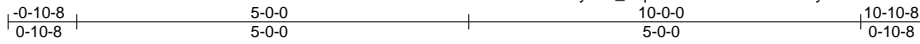
ENGINEERING BY
TRENCO
 A MiTek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	KB 10x10 8pi	143213655
150_1910_D	P10E	COMMON SUPPORTED GAB	1	1		

84 Components (Dunn), Dunn, NC - 28334,

8.420 s Aug 25 2020 MiTek Industries, Inc. Wed Oct 14 16:39:48 2020 Page 1

ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-tLAuQiZyRacQuXdsnweMxMtNxAez3vNf?QD2cyTRZP



4x4 =

Scale = 1:29.4

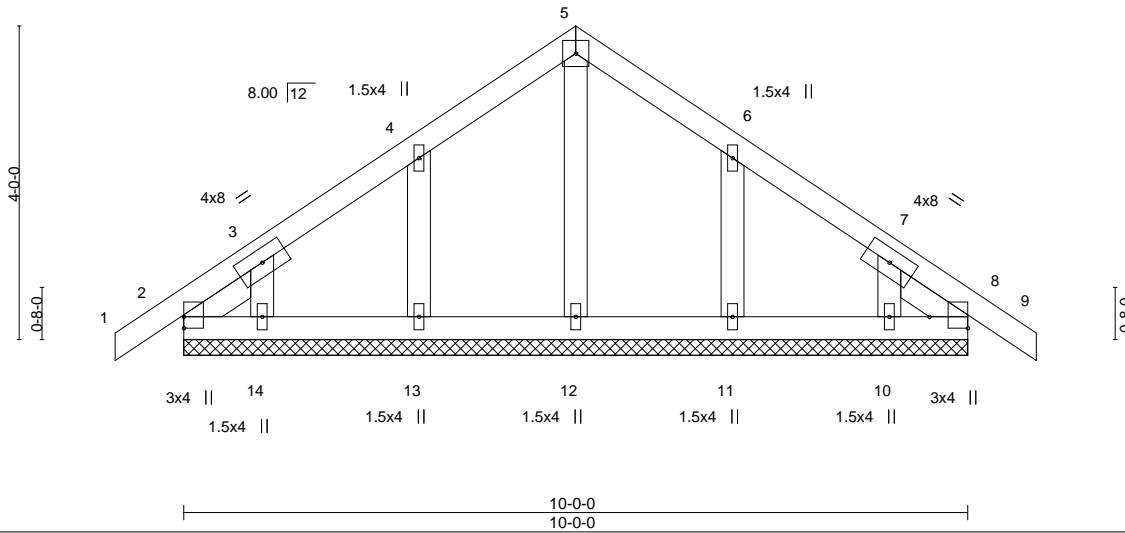


Plate Offsets (X,Y)-- [8:Edge,0-5-14]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/def	L/d	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.05	Vert(LL) -0.00	8	n/r	120	MT20	197/144
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.03	Vert(CT) -0.00	9	n/r	120		
TCDL 10.0	Lumber DOL 1.15	WB 0.03	Horz(CT) 0.00	8	n/a	n/a		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S						
BCDL 10.0	Code IRC2015/TPI2014						Weight: 53 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
OTHERS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -t 1-0-8, Right 2x4 SP No.3 -t 1-0-8

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 10-0-0.
(lb) - Max Horz 2=-76(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 2, 13, 14, 11, 10
Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 5-0-0, Corner(3) 5-0-0 to 8-0-0, Exterior(2) 8-0-0 to 10-10-8 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



October 15,2020

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



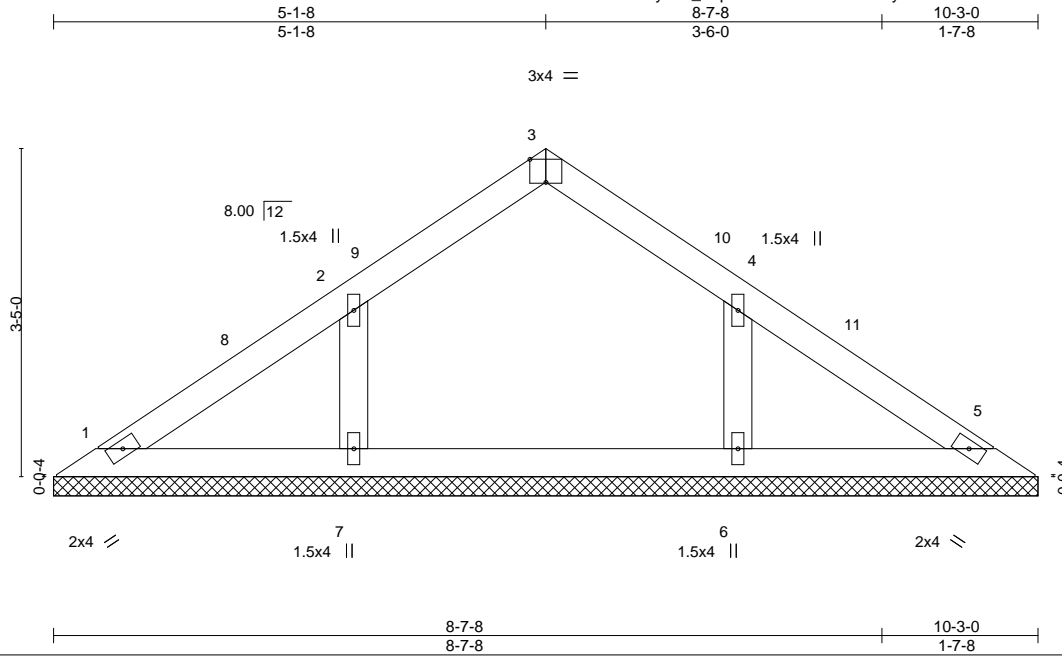
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	KB 10x10 8pi	143213656
150_1910_D	VP1	GABLE	1	1		

84 Components (Dunn), Dunn, NC - 28334,

8.420 s Aug 25 2020 MiTek Industries, Inc. Wed Oct 14 16:39:48 2020 Page 1

ID:VMD62rz1yiHD_OqRtbnrIFztQ8K-tLAuQiZyRacQuXdsnweMxMtlUh8Tz3pNf?QD2cyTRZP



Scale: 1/2"=1'

Plate Offsets (X,Y)--	[3:0-2-0,Edge]										
LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.14	Vert(LL)	n/a	-	n/a	MT20	244/190
Snow (Pf/Pg)	11.6/15.0	Lumber DOL	1.15	BC	0.17	Vert(CT)	n/a	-	n/a		
TCDL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	5	n/a		
BCLL	0.0 *	Code	IRC2015/TPI2014	Matrix-S						Weight: 36 lb	FT = 20%
BCDL	10.0										

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.3	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. All bearings 10-3-0.
 (lb) - Max Horz 1=60(LC 12)
 Max Uplift All uplift 100 lb or less at joint(s) 7, 6
 Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=255(LC 25), 6=254(LC 26)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 5-1-8, Exterior(2) 5-1-8 to 8-1-8, Interior(1) 8-1-8 to 9-9-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCCL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



October 15, 2020

Job	Truss	Truss Type	Qty	Ply	KB 10x10 8pi	I43213657
150_1910_D	VP2	Valley	1	1		

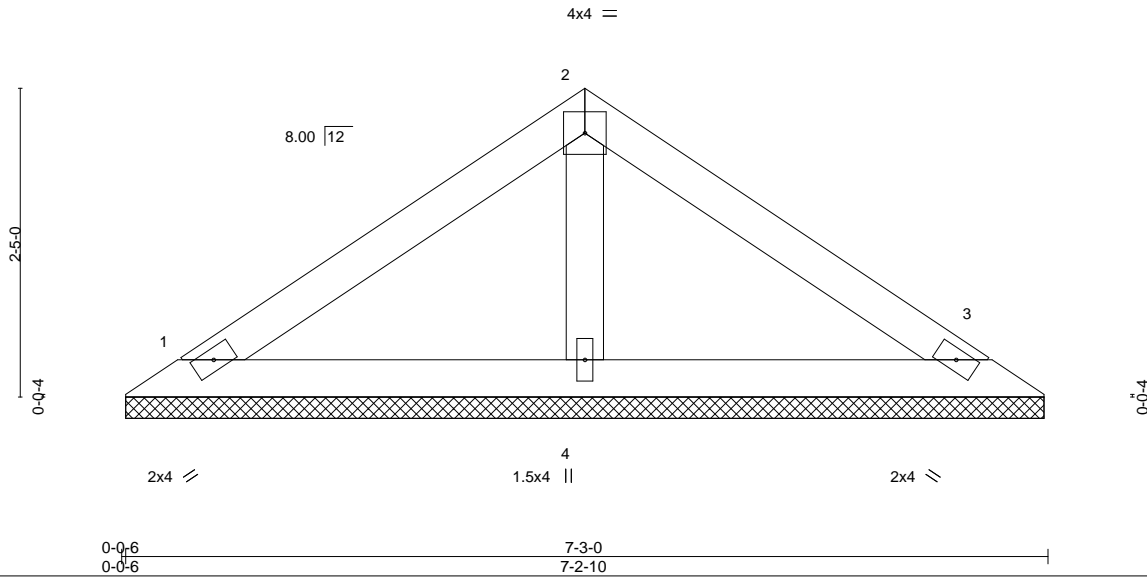
84 Components (Dunn), Dunn, NC - 28334,

8.420 s Aug 25 2020 MiTek Industries, Inc. Wed Oct 14 16:39:49 2020 Page 1

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Scale = 1:18.0



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.31	in (loc) l/defl L/d	MT20	244/190
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.17	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.03	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 3 n/a n/a		
BCDL 10.0	Code IRC2015/TP12014			Weight: 25 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.3
 BOT CHORD 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=7-2-4, 3=7-2-4, 4=7-2-4
 Max Horz 1=-41(LC 12)
 Max Uplift 1=-15(LC 14), 3=-21(LC 15)
 Max Grav 1=135(LC 2), 3=135(LC 2), 4=234(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



October 15, 2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

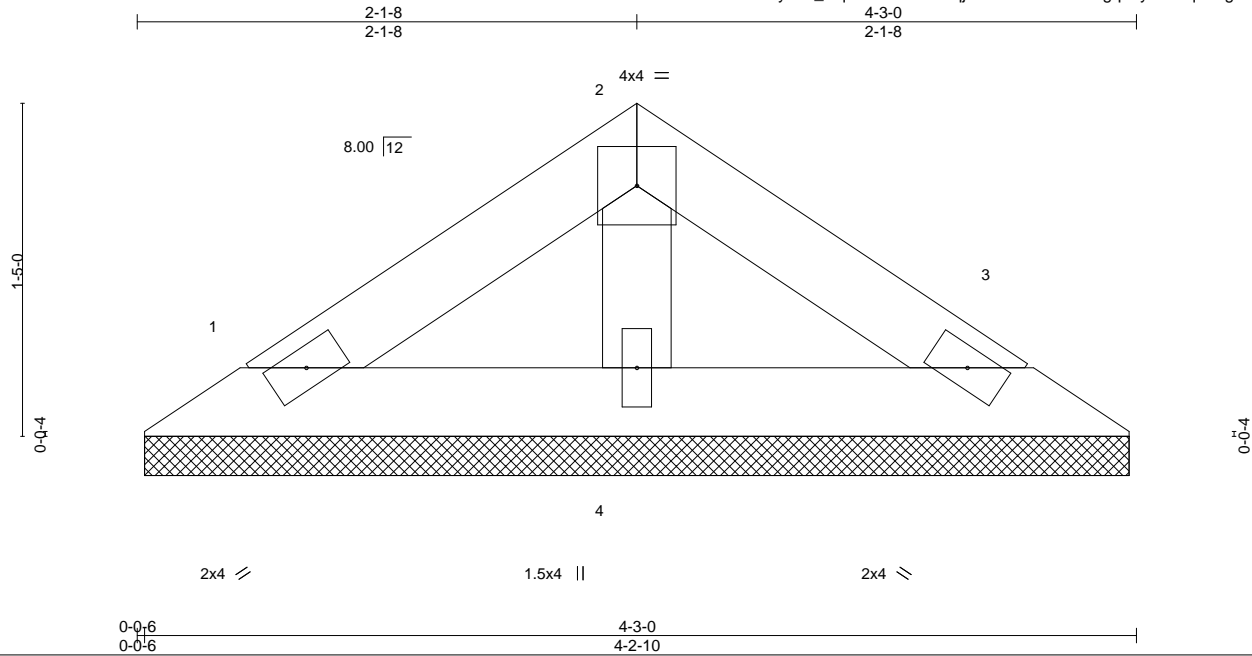


818 Soundside Road
 Edenton, NC 27932

Job 150_1910_D	Truss VP3	Truss Type Valley	Qty 1	Ply 1	KB 10x10 8pi	I43213658
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84 Components (Dunn), Dunn, NC - 28334,

8.420 s Aug 25 2020 MiTek Industries, Inc. Wed Oct 14 16:39:50 2020 Page 1
ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-qjlerOaCzns87mrvLgq0nyh1VsqrZcgg7JvK6VyTRZN



Scale = 1:9.8

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.08	in (loc) l/defl L/d	MT20	244/190
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.05	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.02	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 3 n/a n/a		
BCDL 10.0	Code IRC2015/TPJ2014			Weight: 13 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.3
BOT CHORD 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=4-2-4, 3=4-2-4, 4=4-2-4
Max Horz 1=-21(LC 10)
Max Uplift 1=-8(LC 14), 3=-11(LC 15)
Max Grav 1=70(LC 2), 3=70(LC 2), 4=122(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



October 15, 2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

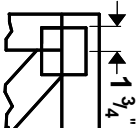
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



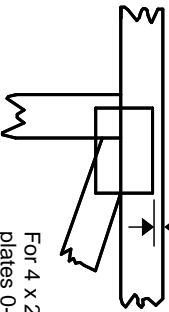
818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20 software or upon request.**

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



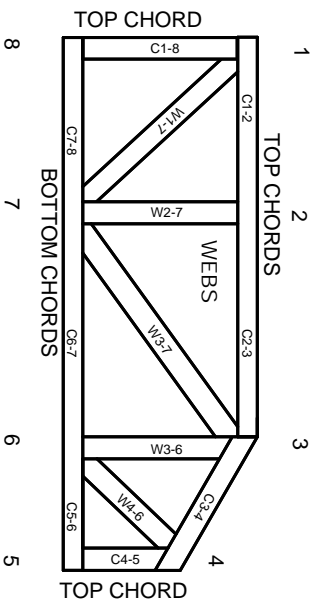
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing, Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8
dimensions shown in ft-in-sixteenths
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: Mill-7473 rev. 5/19/2020

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T or I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.