



	Products							
PlotID	Length	Product	Plies	Net Qty				
BM1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2				
GDH	22' 0"	1-3/4"x 14" LVL Kerto-S	2	2				



All Walls Shown Are Considered Load Bearing

Roof Area	= 3931.6 sq.ft.
Ridge Line	= 143.67 ft.
Hip Line	= 11.94 ft.
Horiz. OH	= 171.72 ft.
Raked OH	= 208.75 ft.
Decking	= 135 sheets

Dimension Notes
 All exterior wall to wall dimensions are to face of sheathing unless noted otherwise All interior wall dimensions are to face of frame wall unless noted otherwise
3. All exterior wall to truss dimensions are to
face of frame wall unless noted otherwise

Hate	ch Legend
	Box Storage
	Drop Beam
	Flush Beam

Conne	ctor Info	rmati	Nail Information			
duct	Manuf	Qty	Supported Member	Header	Truss	
S26	USP	25	NA	16d/3-1/2"	16d/3-1/2"	
26-2	USP	1	NA	16d/3-1/2"	10d/3"	

Truss Placement Plan

Scale: 1/4"=1'

	TI	ROC RUS eilly R Fayett Phone Fax:	DF & SES oad In teville, e: (910)	FE 4 FL(& B dustri N.C. 2) 864- 864-4	CH DOR EAN al Parl 28309 8787 444	is «						
	THIS IS These tr compon the spec design s placeme for temp system a truss su and colu For gene and BCS <u>online</u>	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com										
	Bearing deemed requirem attached requirem size and reaction: 15000# retained reaction: Tables Signatur	reactions to comply to comply Tables (number of s greater A register to design that exce A register to design s that exce	less thar y with the e contract derived fi determine of wood s than 3000 red design the supp eds those ed design the supp eed 1500	a or equal prescrip or shall r rom the p e the min ttuds requ # but not n profess port syste e specifie n profess port syste 0#. d La	to 3000# tive Code efer to the rescriptivi imum fou irred to si greater t is greater t innal shal m for any d in the a onal shal m for all inndr	are e code ndation upport han II be / ttached I be						
			David									
	Clayton / Johnston	Sugarberry Place	Roof	11/30/22	Jonathan Landry	Lenny Norris						
	сі ТҮ / со.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.						
	Wellco Contractors	Lot 148 Hidden Lakes	Plan 11 / 2GLF, CP	N/A		J1122-5849						
	BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #						
)	LOA NUM NUM NUM NUM NUM NUM NUM NUM NUM NUM	(BASED) BER OF JA 803 SQUIS 4,58 1 2 3 4 5 6 7 8 9	ART FC ON TABLE (CL STUDS) HEADER/ 2550 5100 7650 10200 12750	DR JAC S R902 5(1) GIRDER GIRDER 1 2 3 4 5 5 6	CK STU (* 60)) (* 64 END (* 64	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0						



/



Trenco RE: J1122-5849 818 Soundside Rd Lot 148 Hidden Lakes Edenton, NC 27932 Site Information: Customer: Wellco Contractors Project Name: J1122-5849 Lot/Block: 148 Model: Plan 11 Address: Sugarberry Place Subdivision: Hidden Lakes State: NC City: Clayton General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4 Wind Code: ASCE 7-10 Wind Speed: 150 mph Roof Load: 40.0 psf Floor Load: N/A psf This package includes 32 individual, dated Truss Design Drawings and 0 Additional Drawings. No. Seal# Truss Name Date No. Seal# Truss Name Date 154232521 9/15/2022 154232541 M1-GR 9/15/2022 A1 21 1 2 154232522 A1GE PB 9/15/2022 22 154232542 9/15/2022 3 154232523 A2 9/15/2022 23 154232543 PBGE 9/15/2022 4 154232524 9/15/2022 154232544 VC1 9/15/2022 A3 24 5 154232525 A3GE 9/15/2022 25 154232545 VC2 9/15/2022 9/15/2022

VC3 6 154232526 A4 9/15/2022 26 154232546 7 154232527 Β1 9/15/2022 27 154232547 VC4 8 154232528 B1GE 9/15/2022 28 154232548 VC5 9 154232529 B2 9/15/2022 29 154232549 VD1 VD2 10 154232530 B2-GR 9/15/2022 30 154232550 154232531 C1 9/15/2022 31 154232551 VG1 11 154232532 C1SG 9/15/2022 32 154232552 VG2 12 13 154232533 C2 9/15/2022 C2-GR 14 154232534 9/15/2022 D1-GR 15 154232535 9/15/2022 16 154232536 D1GE 9/15/2022

9/15/2022

9/15/2022 9/15/2022

9/15/2022

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

G1

H1 H1GE

G1GE

Truss Design Engineer's Name: Gilbert, Eric

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154232537

154232538

154232539

154232540

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



1 of 1

Gilbert, Eric

September 15, 2022

9/15/2022

9/15/2022

9/15/2022

9/15/2022

9/15/2022

9/15/2022



Exterior(2) 20-4-0 to 26-6-1, Interior(1) 26-6-1 to 32-8-0 zone;C-C for members and forces & MWFRS for reactions show DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=186, 10=186.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Job	Truss	Truss Type	Qty	Ply	Lot 148 Hidden Lakes		
					154232522		
J1122-5849	A1GE	PIGGYBACK BASE SUPPO	1	1			
					Job Reference (optional)		
Comtech, Inc,	Fayetteville, NC - 28314,			8.430 s Jar	1 6 2022 MiTek Industries, Inc. Thu Sep 15 14:15:13 2022 Page 2		
ID:pRI1C9Effk0ZVsLDXhFWTRyeOMT-iVnlovEC1KywSmLR41PTSqlUsoXTw7zmMkxWpBydMUy							

NOTES-

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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¹⁰⁾ Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 32, 33, 34, 35, 23 except (jt=lb) 1=245, 37=138, 38=175 , 39=165, 40=145, 41=329, 29=128, 28=177, 27=168, 26=132, 25=316.



	5-2-12	17-5-0	25-5-0	37-9	-0
Plate Offsets (X,Y)	[3:0-3-4,0-3-12], [4:0-3-12,0-3-8]	12-2-4	0-0-0	12-4	~0
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.72 BC 0.74 WB 0.73 Matrix-S	DEFL. in Vert(LL) -0.47 Vert(CT) -0.66 Horz(CT) 0.03 Wind(LL) 0.43	(loc) l/defl L/d 9-11 >845 360 9-11 >598 240 9 n/a n/a 9-11 >920 240	PLATES GRIP MT20 244/190 Weight: 267 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 SUDE Picht 2x4 SP No.2			BRACING- TOP CHORD S BOT CHORD 6	Structural wood sheathing dir 2-0-0 oc purlins (6-0-0 max.): Rigid ceiling directly applied c 5-0-0 oc bracing: 2-14.	ectly applied or 4-0-9 oc purlins, except 4-5. rr 10-0-0 oc bracing, Except:
REACTIONS. (siz Max H	e) 9=Mechanical, 14=0-3-8 lorz 14=356(LC 9)		WEBS 1	Row at midpt 6	-11

Max Uplift 9=-185(LC 13), 14=-308(LC 12) Max Grav 9=1450(LC 20), 14=1786(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-1021/712, 3-4=-1523/524, 4-5=-1158/603, 5-6=-1485/657, 6-9=-1717/658
- BOT CHORD 2-14=-596/1024, 12-14=-374/1262, 11-12=-111/1128, 9-11=-294/1209
- WEBS 3-14=-2106/1400, 4-12=0/445, 5-11=-133/632, 6-11=-492/388, 3-12=-376/325

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 17-5-0, Exterior(2) 17-5-0 to 21-9-13, Interior(1) 21-9-13 to 25-5-0, Exterior(2) 25-5-0 to 29-9-13, Interior(1) 29-9-13 to 37-9-0 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=185, 14=308.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





REACTIONS. (size) 11=0-3-8, 7=Mechanical Max Horz 11=-347(LC 8) Max Uplift 11=-157(LC 12), 7=-176(LC 13) Max Grav 11=1301(LC 2), 7=1394(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1428/572, 2-3=-1066/623, 3-4=-1394/675, 4-7=-1629/675, 1-11=-1259/572

BOT CHORD 10-11=-421/549. 8-10=-113/1029. 7-8=-326/1149

1-10=-263/1036, 2-10=0/374, 3-8=-135/596, 4-8=-521/397 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-1, Interior(1) 4-9-1 to 10-7-8, Exterior(2) 10-7-8 to 16-10-3, Interior(1) 16-10-3 to 18-7-8, Exterior(2) 18-7-8 to 24-9-9, Interior(1) 24-9-9 to 30-11-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) All plates are MT20 plates unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=157.7=176.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



September 15,2022

🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This show include up and the reference Packet Mitra's dev. of value before Use. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 148 Hidden Lakes	
	1005				154232525	
J1122-5849	A3GE	PIGGYBACK BASE SUPPO	1	1		
					Job Reference (optional)	
Comtech, Inc, Fayetteville, NC - 28314, 8430 s Jan 6 2022 MiTek Industries, Inc. Thu Sep 15 14:15:17 2022 Page						
	ID:pRI1C9Effk0ZVsLDXhFWTRyeOMT-aH1odHHj5YTMwNeCJtUPdgSArPuQsxyMHMvjyyydMUu					

NOTES-

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27, 29, 30, 31 except (jt=lb) 36=334, 32=136, 33=176, 34=160, 35=453, 25=131, 24=177, 23=168, 22=130, 21=331, 20=135.

Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14

7) Attic room checked for L/360 deflection.



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September 15,2022



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.93 BC 0.61 WB 0.12 Matrix-S	DEFL. ir Vert(LL) -0.19 Vert(CT) -0.33 Horz(CT) 0.01 Wind(LL) 0.11	n (loc) I/defi L/d 9 11-13 >999 360 8 11-13 >765 240 9 n/a n/a 11-13 >999 240	PLATES GRIP MT20 244/190 Weight: 198 lb FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x10 S WEBS 2x6 SF 5-14: 2 WEDGE Left: 2x4 SP No.2 , Rig	P No.1 P No.1 P No.1 *Except* x4 SP No.2 ht: 2x4 SP No.2		BRACING- TOP CHORD BOT CHORD JOINTS	Structural wood sheathing di Rigid ceiling directly applied 1 Brace at Jt(s): 14	rectly applied or 2-2-0 oc purlins. or 10-0-0 oc bracing.
REACTIONS. (size Max H Max G	e) 1=0-3-8, 9=0-3-8 orz 1=-361(LC 8) rav 1=1395(LC 21), 9=1425(LC 21)				
FORCES. (lb) - Max. TOP CHORD 1-3=- BOT CHORD 1-13= WEBS 3-13=	Comp./Max. Ten All forces 250 (lb) or 1776/90, 3-4=-895/282, 4-5=-59/319, 6- =0/1055, 11-13=0/1055, 9-11=0/1055 =11/850, 7-11=-41/848, 4-14=-1397/45-	less except when shown. 7=-1028/264, 7-9=-1866/68 4. 6-14=-1397/454	3		

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 10-7-0, Exterior(2) 10-7-0 to 14-11-13, Interior(1) 14-11-13 to 21-11-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Ceiling dead load (10.0 psf) on member(s). 3-4, 6-7, 4-14, 6-14; Wall dead load (5.0psf) on member(s).3-13, 7-11

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

7) Attic room checked for L/360 deflection.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidal truss evel and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rerection and bracing of trusses shaft muss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 September 15,2022





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Edenton, NC 27932





				10-0	-0	10-1	J-0		20-1-0			
				10-0	-8	5-2	-8	1	4-10-0	1		
Plate Off	sets (X,Y)	[2:0-7-9,0-0-2], [8:0-7-9,	0-0-2]									
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.15	Vert(LL)	-0.10	2-12	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.35	Vert(CT)	-0.16	2-12	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.59	Horz(CT)	0.01	8	n/a	n/a		

Matrix-S

н	IIM	IRF	R-	

10.0

Plate LOAD

BCDL

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No 2 Left 2x6 SP No.1 3-8-1, Right 2x6 SP No.1 3-6-7 SLIDER

BRACING-TOP CHORD BOT CHORD

Wind(LL)

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 182 lb

FT = 20%

240

>999

0.01 8-10

REACTIONS. (size) 2=0-3-8, 10=0-3-8, 8=0-3-0 Max Horz 2=-346(LC 8)

Max Uplift 2=-105(LC 13), 10=-103(LC 12), 8=-61(LC 13) Max Grav 2=738(LC 20), 10=604(LC 1), 8=393(LC 1)

Code IRC2015/TPI2014

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-4=-705/314, 4-5=-640/375, 5-6=-644/386, 6-8=-326/182
- BOT CHORD 2-12=-202/592

NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 10-0-8, Exterior(2) 10-0-8 to 14-5-5, Interior(1) 14-5-5 to 20-10-6 zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 2=105, 10=103.



Scale = 1:69.4

rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. 🛕 WARNING - Verify design pai Design valid for use only with MiTek® connectors. This show include up and the reference Packet Mitra's dev. of value before Use. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



^{4-12=-446/361, 5-12=-296/526, 6-12=-59/305, 6-10=-624/244} WFBS



15-0-8

20-1-0

10-0-8

5-0-8



	SPACING- 2	-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
Y)	[2:0-7-9,0-0-2], [12:0-7-9,0-0	-2]								
		ſ	10-0-8	5-2-8		1	4-10-0	1		
		1	10-0-8	3x10 = 15-3-0		1	20-1-0			
				4x6 —						

Vert(LL)

Vert(CT)

Horz(CT)

Wind(LL)

BRACING-TOP CHORD

BOT CHORD

JOINTS

-0.09 2-18

0.01 12-14

12

-0.15 2-18

0.01

>999

>999

>999

1 Brace at Jt(s): 19, 20

n/a

360

240

n/a

240

Rigid ceiling directly applied or 10-0-0 oc bracing.

MT20

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 219 lb

-	 	
		•

Plate Offsets (X,

LOADING (psf)

20.0

10.0

10.0

0.0

TCLL

TCDL

BCLL

BCDL

LUMBER-	
TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2
OTHERS	2x4 SP No.2
SLIDER	Left 2x6 SP No.1 3-8-1, Right 2x6 SP No.1 3-6-7

REACTIONS. (size) 2=0-3-8, 15=0-3-8, 12=0-3-0 Max Horz 2=-433(LC 8) Max Uplift 2=-198(LC 12), 15=-313(LC 13), 12=-67(LC 9)

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

Max Grav 2=701(LC 19), 15=701(LC 1), 12=320(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-4=-669/324, 4-5=-580/361, 5-6=-555/399, 6-7=-568/413, 7-8=-583/377, 8-9=-435/254

BOT CHORD 2-18=-276/620 WEBS

4-19=-434/438, 18-19=-455/461, 6-18=-344/492, 18-20=-77/402, 20-21=-65/392, 9-21=-74/391, 9-15=-371/101, 16-21=-257/178, 10-14=-300/233

1.15

1.15

YES

TC

BC

WB 0.68

Matrix-S

0.15

0.34

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 2=198. 15=313.



244/190

FT = 20%

Scale = 1:69.4

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15-0-8

20-1-0

4-10-0

10-0-8

5-0-8



Plate Offsets (X,Y)-- [2:0-7-9,0-0-2], [8:0-7-9,0-0-2]

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d PLATES GRIF	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL) -0.10 2-11 >999 360 MT20 244/1	90
TCDL	10.0	Lumber DOL 1.15	BC 0.35	Vert(CT) -0.16 2-11 >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.61	Horz(CT) 0.01 8 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.01 8-9 >999 240 Weight: 179 lb FT	= 20%

10-0-8

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No 2 SLIDER Left 2x6 SP No.1 3-8-1, Right 2x6 SP No.1 3-6-7

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 8=Mechanical, 2=0-3-8, 9=0-3-8 Max Horz 2=-347(LC 8)

Max Uplift 8=-46(LC 13), 2=-106(LC 13), 9=-101(LC 12) Max Grav 8=346(LC 1), 2=740(LC 20), 9=603(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-707/314, 4-5=-642/375, 5-6=-646/393, 6-8=-319/190

BOT CHORD 2-11=-201/593

4-11=-446/361, 5-11=-306/529, 6-11=-62/303, 6-9=-620/243 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 10-0-8, Exterior(2) 10-0-8 to 14-5-5, Interior(1) 14-5-5 to 20-1-0 zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (it=lb) 2=106.9=101.



Scale = 1:69.4

September 15,2022

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⁹⁾ Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=158, 2=776, 9=846.

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1485 lb down and 206 lb up at 1-8-12, 1485 lb down and 206 lb up at 3-8-12, 1485 lb down and 206 lb up at 5-8-12, 1485 lb down and 206 lb up at 7-8-12, 1485 Ib down and 206 lb up at 9-8-12, and 1485 lb down and 206 lb up at 11-8-12, and 1485 lb down and 206 lb up at 13-8-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Continued on page 2

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818 Soundside Road Edenton, NC 27932

September 15,2022

Job		Truss	Truss Type	Qty	Ply	Lot 148 Hidden Lakes	
							154232534
J1122-5849		C2-GR	COMMON	1	2		
					_	Job Reference (optional)	
Comtech, Inc,	Fayettev	rille, NC - 28314,			3.430 s Ja	n 6 2022 MiTek Industries, Inc. Thi	u Sep 15 14:15:27 2022 Page 2
			ID:	RI1C9Effk0Z	/sLDXhF\	VTRyeOMT-HCdakiP_kdjx7wP7vzfl	11ntqNRAUCNYqavKFJNydMUk

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-60, 5-8=-60, 2-8=-20

Concentrated Loads (lb)

Vert: 13=-1287(B) 14=-1287(B) 15=-1287(B) 16=-1287(B) 17=-1287(B) 18=-1287(B) 19=-1287(B)

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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 148 Hidden Lakes	
						154232535
J1122-5849	D1-GR	Common Girder	1	2		
				_	Job Reference (optional)	
Comtech, Inc, Fa	ayetteville, NC - 28314,			3.430 s Jar	6 2022 MiTek Industries	, Inc. Thu Sep 15 14:15:29 2022 Page 2
		ID:pRI1C	9Effk0ZVsL	DXhFWTR	yeOMT-EalL9NRFGEzfND	ZW0OiD6Cy?YFpggFB72DpMNFydMUi

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 1-5=-20 Concentrated Loads (Ib)

Vert: 10=-1209(F) 11=-1204(F) 12=-1057(F) 13=-1057(F) 14=-1057(F) 15=-1057(F) 16=-1204(F) 17=-1204(F) 18=-1204(F)

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			18-8-0					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.03 BC 0.01 WB 0.04 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.00 12 -0.00 12 0.00 12	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 130 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 18-8-0. (lb) -Max Horz 2=-135(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 17, 12 except 20=-101(LC 12), 21=-112(LC 12), 22=-111(LC 12), 23=-131(LC 12), 16=-114(LC 13), 15=-110(LC 13), 14=-120(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 19, 20, 21, 22, 23, 17, 16, 15, 14, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing. 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 17, 12 except (jt=lb) 20=101, 21=112, 22=111, 23=131, 16=114, 15=110, 14=120.

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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7-11-8 15-11-0 7-11-8 7-11-8 LOADING (psf) SPACING-CSL DEFL. PLATES GRIP 2-0-0 in (loc) l/defl I/d 0.29 244/190 20.0 Plate Grip DOL тс -0.02 >999 TCLL 1.15 Vert(LL) 360 MT20 2-6 TCDL BC 0.33 Vert(CT) 10.0 Lumber DOL 1.15 -0.05 2-6 >999 240 WB BCLL 0.0 Rep Stress Incr YES 0.09 Horz(CT) 0.01 n/a 4 n/a Code IRC2015/TPI2014 BCDL 10.0 Matrix-S Wind(LL) 0.02 2-6 >999 240 Weight: 90 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 4=0-3-0 Max Horz 2=75(LC 11) Max Uplift 2=-130(LC 12), 4=-130(LC 13) Max Grav 2=677(LC 1), 4=677(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-856/382, 3-4=-856/378

BOT CHORD 2-6=-174/655, 4-6=-174/655 WEBS 3-6=0/380

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 7-11-8, Exterior(2) 7-11-8 to 12-4-5, Interior(1) 12-4-5 to 16-7-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=130, 4=130.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

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and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 6-6-8, Exterior(2) 6-6-8 to 10-11-5, Interior(1) 10-11-5 to 13-10-6 zone; C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



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September 15,2022



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5-4-0 5-4-0 LOADING (psf) SPACING-DEFL. PLATES 2-0-0 CSI. in (loc) l/defl I/d GRIP 20.0 Plate Grip DOL 0.08 -0.02 >999 244/190 TCLL TC Vert(LL) 360 1.15 1-5 MT20 TCDL BC Vert(CT) 10.0 Lumber DOL 1.15 0.26 -0.04 >999 240 1-5WB BCLL 0.0 Rep Stress Incr 0.00 Horz(CT) 0.00 NO n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-P Wind(LL) 1-5 >999 240 Weight: 56 lb FT = 20% 0.02

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 WFBS

REACTIONS. (size) 1=0-3-0, 5=0-3-0 Max Horz 1=85(LC 4)

Max Uplift 1=-136(LC 4), 5=-316(LC 4)

Max Grav 1=561(LC 1), 5=598(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) 2-ply truss to be connected together as follows:
- Top chords connected with 10d (0.131"x3") nails as follows: 2x6 2 rows staggered at 0-9-0 oc, 2x4 1 row at 0-9-0 oc. Bottom chords connected with 10d (0.131"x3") nails as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to
- y index to be provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=136. 5=316.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 326 lb down and 66 lb up at 1-3-12, and 326 lb down and 66 lb up at 3-3-12, and 228 lb down and 172 lb up at 4-11-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-20, 1-4=-20

Concentrated Loads (lb)

Vert: 5=-100(B) 6=-326(B) 7=-326(B)



Scale = 1:16.2

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BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.



BRACING-

TOP CHORD

BOT CHORD

н	111	IRF	R.	

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. (size) 2=6-8-9, 4=6-8-9, 6=6-8-9

Max Horz 2=-100(LC 10)

Max Uplift 2=-56(LC 12), 4=-65(LC 13)

Max Grav 2=182(LC 1), 4=182(LC 1), 6=223(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCF 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; b=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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	[2.0 2 1,0 1 0], [0.0 2 1,0 1 0]		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.00 6 n/r 120 MT20 244/190 Vert(CT) 0.00 6 n/r 120 MT20 244/190
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	
TCDL 10.0	Lumber DOL 1.15	BC 0.02	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.04	Horz(CT) 0.00 6 n/a n/a
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Weight: 32 lb FT = 20%
LUMBER-			BRACING-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 6-8-9.

(lb) - Max Horz 2=-125(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-177(LC 12), 8=-176(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; bell=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=177, 8=176.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.26 BC 0.18 WB 0.18 Matrix-S	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defl L/d - n/a 999 - n/a 999 5 n/a n/a	PLATES GRIP MT20 244/190 Weight: 85 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF OTHERS 2x4 SF	2 No.1 2 No.1 2 No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing di Rigid ceiling directly applied o 1 Row at midpt 3	rectly applied or 6-0-0 oc purlins. or 10-0-0 oc bracing. 8-8

REACTIONS. All bearings 17-4-4.

(lb) - Max Horz 1=-267(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-320(LC 12), 6=-320(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=427(LC 22), 9=582(LC 19), 6=582(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-250/238, 3-4=-250/238

WEBS 2-9=-569/466, 4-6=-569/466

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-8-8, Interior(1) 4-8-8 to 8-8-8, Exterior(2) 8-8-8 to 13-1-5, Interior(1) 13-1-5 to 17-0-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=320, 6=320.



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1 1000 0110000 (74,17)			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP Vert(LL) n/a - n/a 999 MT20 244/190 Vert(CT) n/a - n/a 999 MT20 244/190
TCLL 20.0	Plate Grip DOL 1.15	TC 0.18	
TCDL 10.0	Lumber DOL 1.15	BC 0.17	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.11	Horz(CT) 0.00 5 n/a n/a
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 66 lb FT = 20%
LUMBER-			BRACING-

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. All bearings 14-0-4.

(lb) - Max Horz 1=214(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-257(LC 12), 6=-257(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=408(LC 19), 8=439(LC 19), 6=438(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-8=-468/408, 4-6=-468/408

NOTES-

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 7-0-8, Exterior(2) 7-0-8 to 11-5-5, Interior(1) 11-5-5 to 13-8-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=257.6=257.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES GRIP
TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	TC 0.19 BC 0.09 WB 0.08	Vert(LL) Vert(CT) Horz(CT) (n/a - n/a -).00 5	n/a n/a n/a	999 999 n/a	MT20 244/190
LUMBER- TOP CHORD 2x4 SI	P No.1	Matrix-S	BRACING- TOP CHORD	Struct	ural wood	sheathing di	rectly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.1 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. All bearings 10-8-4.

(lb) - Max Horz 1=-160(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) except 1=-148(LC 10), 5=-121(LC 11), 8=-259(LC 12), 6=-259(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=391(LC 19), 6=391(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2-8=-493/461, 4-6=-493/461

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-4-8, Exterior(2) 5-4-8 to 9-9-5, Interior(1) 9-9-5 to 10-4-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

joint 5, 259 lb uplift at joint 8 and 259 lb uplift at joint 6.



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6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 148 lb uplift at joint 1, 121 lb uplift at



				7-4-10		0-0-6		
	G (psf)	SPACING- 2-0-0 Plate Grip DOI 1.15	CSI . TC 0.24	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP
TCDL	10.0	Lumber DOL 1.15	BC 0.08	Vert(CT)	n/a -	n/a 999	W120	244/130
BCLL	0.0 * 10.0	Code IRC2015/TPI2014	Matrix-P	Horz(CT) 0.).00 3	n/a n/a	Weight: 30 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2 BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=7-4-4, 3=7-4-4, 4=7-4-4 Max Horz 1=-107(LC 8) Max Uplift 1=-53(LC 13), 3=-53(LC 13)

Max Grav 1=164(LC 1), 3=163(LC 1), 4=210(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 1 and 53 lb uplift at joint 3.



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				4-0-10		0-0-6	5	
	i (psf) 20.0	SPACING- 2-0-0 Plate Grip DOI 1.15	CSI. TC 0.06	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.02	Vert(CT)	n/a -	n/a 999	INT20	244/100
BCLL BCDL	0.0 * 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.01 Matrix-P	Horz(CT) (0.00 3	n/a n/a	Weight: 15 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-1-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=4-0-4, 3=4-0-4, 4=4-0-4 Max Horz 1=-54(LC 10)

Max Uplift 1=-27(LC 13), 3=-27(LC 13)

Max Grav 1=82(LC 1), 3=82(LC 1), 4=106(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 1 and 27 lb uplift at joint 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidal truss evel and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rerection and bracing of trusses shaft muss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





ACTIONS. (SIZE) 1=11-1-9, 3=11-1-9, 4=11-1-9 Max Horz 1=-43(LC 8) Max Uplift 1=-50(LC 12), 3=-57(LC 13), 4=-44(LC 12) Max Grav 1=184(LC 23), 3=184(LC 24), 4=431(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-4=-285/264

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-7-13 to 5-0-10, Interior(1) 5-0-10 to 5-7-8, Exterior(2) 5-7-8 to 10-0-5, Interior(1) 10-0-5 to 10-7-4 zone; C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 57 lb uplift at joint 3 and 44 lb uplift at joint 4.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oullapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3x4 💋

2x4 ||

	2-3-8			1				4-7-1	
	2-2-12			1				2-3-9	
SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC	0.04	DEFL. Vert(LL	in .) n/a	(loc) -	l/defl n/a	L/d 999	PLATES MT20
Rep Stress Incr Code IRC2015/TF	YES 912014	WB Matri	0.02 0.02 x-P	Horz(C	T) 0.00	3	n/a n/a	999 n/a	Weight: 13 lb

LUMBER-

TCLL TCDL

BCLL

BCDL

LOADING (psf)

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.2

20.0

10.0

0.0

10.0

0-0-12

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-7-1 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

3x4 📚

REACTIONS. (size) 1=4-5-9, 3=4-5-9, 4=4-5-9 Max Horz 1=-15(LC 10) Max Uplift 1=-20(LC 12), 3=-23(LC 13), 4=-6(LC 12) Max Grav 1=67(LC 1), 3=67(LC 1), 4=128(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psr bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 30.0psr on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 1, 23 lb uplift at joint 3 and 6 lb uplift at joint 4.



GRIP

244/190

FT = 20%

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This down the seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





REACTIONS. All bearings 14-2-9.

(lb) - Max Horz 1=57(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-125(LC 12), 6=-125(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=282(LC 1), 8=313(LC 23), 6=313(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-8=-240/282, 4-6=-240/282

NOTES-

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-7-13 to 5-0-10, Interior(1) 5-0-10 to 7-2-0, Exterior(2) 7-2-0 to 11-6-13, Interior(1) 11-6-13 to 13-8-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=125. 6=125.



🛕 WARNING - Verify design pa rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This show include up and the reference Packet Mitra's dev. of value before Use. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidal truss evel and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rerection and bracing of trusses shaft muss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







CSD

isD	esign	Client: Project: Address:	Wellco Construction Plan 11 Sugarberry Plac Clayton, NC 275	n e 27	Date: Input by: Job Nam Broiget #	11/30/2022 Jonathan Landry e: Lot 148 Hidden Lakes	Page 2 of
BM1 K	erto-S LVL	_ 1.750'	' X 11.875	" 2-Ply	- PASSED	Level: Level	
	• •	•••	••••	•	•••	• •	
	•••	•••	•	•••	•••	•••	
1 SPF				40101			
 				13'3"			3 1/2"
Fasten all plies	using 4 rows of	10d Box nails (.128x3") at 12"	o.c Maximum	end distance n	ot to exceed 6".	
Capacity Load	96 36	.4 % 3.0 PLF					
Yield Limit per Foot Yield Limit per Faste	ener 94	6.5 PLF .1 lb.					
Yield Mode Edge Distance	IV 1 1	/2"					
Min. End Distance Load Combination	3" D+	S					
Duration Factor	1.1	5					
Notes		chemicals		6. For flat roofs provide p	proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Desi structural adequacy of thi design criteria and less	igns is responsible only of the is component based on the adings shown it in the	Handling & Installati	on ut or drilled	ponding		Metsä Wood 301 Merritt 7 Building, 2nd Flo	Fayetteville, NC USA 28314
responsibility of the custor ensure the component application, and to verify the	mer and/or the contractor to suitability of the intended e dimensions and loads	 Refer to manufacture regarding installation fastening details, beam approved. 	ers product information requirements, multi-ply strength values, and code			Norwalk, CT 06851 (800) 622-5850	910-864-TRUS
Lumber 1. Dry service conditions,	unless noted otherwise	approvais 3. Damaged Beams must n 4. Design assumes top edg 5. Provide lateral support	ot be used a is laterally restrained at bearing points to avoid			www.metsawood.com/us	Comtoout
2. LVL not to be treated w	with fire retardant or corrosive	lateral displacement and	rotation	This design is valid	l until 11/3/2024		CONTECH

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

CSD



isDesign	Client: Wellco Constructi Project: Plan 11 Address: Sugarberry Pla Clayton, NC 27	on Date: Input by Ce Job Nan 527 Project #	11/30/2022 : Jonathan Landry ne: Lot 148 Hidden Lakes # J1122-5849	Page 4 of
GDH Kerto-S LVL	1.750" X 14.000"	2-Ply - PASSED	Level: Level	
1 SPF End Grain	· · · · ·	· · · · ·	2 SPF End (
<u>/</u>		16'10"		3 1/2"
1		16'10"		
Multi-Ply Analysis Fasten all plies using 3 rows of Capacity 0.0 Load 0.0 Yield Limit per Foot 245 Yield Limit per Fastener 81.3 Yield Mode IV Edge Distance 1 1/ Min. End Distance 3" Load Combination Duration Factor Duration Factor 1.00	10d Box nails (.128x3") at 12 % PLF 66 PLF 9 lb. /2" 0	' o.c Maximum end distance r	not to exceed 6".	
Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	chemicals Handling & Installation 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	6, For flat roofs provide proper drainage to prevent ponding	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

CSD



Fayetteville, N.C. 28309 (910) 864-TRUS

Fay	etteville, N.C. 28309 (910) 864-TRUS		SUPERINTENDANT	Jason Wellons	SALE	ES REP	Lenny Norr	ris
			JOBSITE PHONE #	(910) 263-0276	SALE	ES AREA	David Lanc	dry
Π	Wellco Contractors, Inc.	JOB NAME:L	₋ot 148 Hidden Lakes		LOT # 148	SUBDIV: Hidden	Lakes	
S O L	PO Box 766	MODEL:Roof	TAG: Pla	an 11 / 2GLF, CP	JOB CATEG	ORY: B & S - Build	and Ship	
D T O	Spring Lake, NC 28390 (910) 436-3131	DELIVERY INS	TRUCTIONS:					
ынны но	Wellco Contractors Sugarberry Place Clayton, NC 27527	SPECIAL INST Copied from Lot	RUCTIONS: 135 Hidden Lakes (J1122-56	525)		PLAN SE	AL DATE:	N/A
	1	1					BY D	ATE

11

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11/23/22

Johnston

Jason Wellons

REQ. QUOTE DATE

ORDER DATE

ORDERED BY

COUNTY

DELIVERY DATE

DATE OF INVOICE

BUILDING DEPARTMENT OVERHANG Roof Order END CUT RE				HANG INFO	HEEL HEIGI	нт	00-0	04-05	REQ. L	AY	OUTS		REQ.	EN	GINE	ERING		QUO	TE	JL	1	1/30/22
Roof Order			END CL	JT RETURN														LAYO	TUC	JL	1	1/30/22
			PLUME	5	GABLE STU	DS	16	N. OC		JO	BSITE	1			JOE	SITE	1	CUTT	ING	JL	1	1/30/22
ROOF T	RUS	SES		DADING FORMATION	TCLL-TCDL-B0	сіі-всі 0.0,10.	L STR	LESS INCR.	R		OF TRUS	ss s	SPAC	ING	:24.0	IN. C).C. (TYP.)				
PROFILE	QTY	PIT	ГСН	TYPE	BASE	LUN	IBER	OVER	HANG		DEAC	ידור	SINC									
	PLY	TOP	BOT	ID	0/A	TOP	BOT	LEFT	RIGH	Т	REAC		5113									
	10	10.00	0.00	PIGGYBACK A1	32-08-00 32-08-00	2 X 6	2 X 6				Joint 1 1509.2 -186.3 I	lbs. bs.	Joi 1؛ 1-	nt 10 509.2 86.3) 2 lbs. Ibs.							
	1	10.00	0.00	PIGGYBACK A1GE	32-08-00 32-08-00	2 X 6	2 X 6		00-11-0	00	Joint 1 351.1 II -245.2 II	bs. bs.	Joi 2 -	nt 23 47.9 78.3	lbs. Ibs.	Joir 2 [.] -3	nt 25 70.6 16.1	bs. bs.	Joint 2 180 -132	26 .9 lbs. .0 lbs.	Joir 19 -11	nt 27 99.5 lbs. 67.8 lbs.
	6	4.00	0.00	PIGGYBACK A2	37-09-00 37-09-00	2 X 6	2 X 6	00-11-00			Joint 9 1383.2 -184.5	lbs. bs.	Joi 17 -3	nt 15 785.7 608.4	7 lbs.							
	6	10.00	0.00	PIGGYBACK A3	30-11-08 30-11-08	2 X 6	2 X 6				Joint 7 1328.7 -175.8 II	lbs. bs.	Joi 12 -1	nt 12 280.9 57.3) bs.							
				PIGGYBACK	30-11-08						Joint 20		Joi	nt 21		Joir	nt 22		Joint 2	23	Joir	nt 24

-													
									-175.8 lbs.	-157.3 lbs.			
1	10.00	0.00	PIGGYBACK A3GE	30-11-08 30-11-08	2 X 6	2 X 6			Joint 20 287.7 lbs. -136.7 lbs.	Joint 21 281.5 lbs. -310.3 lbs.	Joint 22 184.8 lbs. -146.2 lbs.	Joint 23 198.7 lbs. -164.9 lbs.	Joint 24 200.7 lbs. -176.6 lbs.
4	10.00	0.00	PIGGYBACK A4	27-03-08 27-03-08	2 X 6	2 X 6			Joint 7 1185.3 lbs. -147.3 lbs.	Joint 12 1180.1 lbs. -137.7 lbs.			
7	12.00	0.00	ATTIC B1	21-02-00 21-02-00	2 X 6	2 X 10	00-11-00	00-11-00	Joint 2 1430.0 lbs. 42.2 lbs.	Joint 10 1424.7 lbs. 57.1 lbs.			
1	12.00	0.00	GABLE B1GE	21-02-00 21-02-00	2 X 6	2 X 10	00-11-00	00-11-00	Joint 2 1333.1 lbs. -111.5 lbs.	Joint 12 1390.9 lbs. -96.7 lbs.			
1	12.00	0.00	ATTIC B2	21-02-00 21-02-00	2 X 6	2 X 10		00-11-00	Joint 1 1394.6 lbs. 50.6 lbs.	Joint 9 1425.4 lbs. 56.9 lbs.			
1 2 Ply	12.00	0.00	ATTIC B2-GR	21-02-00 21-02-00	2 X 6	2 X 10		00-11-00	Joint 1 1683.9 lbs. -32 0 lbs	Joint 3 179.4 lbs. -32.0 lbs	Joint 4 230.4 lbs. 30 4 lbs	Joint 9 2138.1 lbs. 85.3 lbs	

DATE11/30/22 PAGE 1

J1122-5849

000006558

Net 10 Days

ORDER #

QUOTE #

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TERMS

CUSTOMER ACCT #

CUSTOMER PO #



KIGH	ly kodd industrial Park P.O. box 40408		000111		Conniston			10	Het To Days		
Fay	etteville, N.C. 28309 (910) 864-TRUS		SUPERINTEND	ANT	Jason Wellons		SALE	S REP	Lenny Norris		
			JOBSITE PHON	IE #	(910) 263-0276		SALE	S AREA	David Landry		
Π	Wellco Contractors, Inc.	JOB NAME:	Lot 148 Hidden La	ikes		LOT #	148	SUBDIV: Hidden	Lakes		
S O L	PO Box 766	MODEL:Roof TAG: P			n 11 / 2GLF, CP	JOB C	ATEG	l and Ship			
D HO	Spring Lake, NC 28390 (910) 436-3131	DELIVERY INS	TRUCTIONS:								
s	Wellco Contractors										
H I P T	Sugarberry Place	SPECIAL INST Copied from Lot	RUCTIONS: t 135 Hidden Lakes (J1122-56	25)						
٥	Clayton, NC 27527						PLAN SEAL DATE:				

REQ. QUOTE DATE

ORDER DATE

ORDERED BY

COUNTY

DELIVERY DATE

DATE OF INVOICE

11

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11/23/22

Johnston

Jason Wellons

															DAIL
BUILDING DEPARTMENT	OVERH	ANG INFO	HEEL HEIGHT	00-04-05	RE	Q.	LAYOUTS		REQ.	EN	GINEERING		QUOTE	JL	11/30/22
Roof Order	END CUT	RETURN											LAYOUT	JL	11/30/22
	PLUMB		GABLE STUDS	16 IN. OC			JOBSITE	1			JOBSITE	1	CUTTING	JL	11/30/22

PROFILE QTY PI PLY TOP				FORMATION	TCLL-TCDL-B	0.0,10	.0	LESS INCR.	RO	OF TRUSS S	PACING:24.0	IN. O.C. (TYP	.)	
PROFILE		PIT	CH	TYPE	BASE O/A	LUN	IBER	OVER	HANG	REACTIO	NS			
	1	12.00	0.00	COMMON C1	20-01-00 20-01-00	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 738.4 lbs. -105.3 lbs.	Joint 8 393.1 lbs. -61.0 lbs.	Joint 10 604.4 lbs. -103.4 lbs.		
	1	12.00	0.00	GABLE C1SG	20-01-00 20-01-00	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 701.4 lbs. -198.1 lbs.	Joint 12 319.9 lbs. -67.3 lbs.	Joint 15 701.2 lbs. -313.0 lbs.		
	2	12.00	0.00	COMMON C2	20-01-00 20-01-00	2 X 6	2 X 6	00-11-00		Joint 2 739.9 lbs. -106.5 lbs.	Joint 8 346.4 lbs. -45.9 lbs.	Joint 9 603.0 lbs. -101.2 lbs.		
	1 2 Ply	12.00	0.00	COMMON C2-GR	20-01-00 20-01-00	2 X 6	2 X 8	00-11-00		Joint 2 5633.7 lbs. -776.2 lbs.	Joint 8 297.7 lbs. -158.0 lbs.	Joint 9 6175.5 lbs. -846.2 lbs.		
	1 2 Ply	6.00	0.00	COMMON D1-GR	18-08-00 18-08-00	2 X 6	2 X 6			Joint 1 6559.5 lbs. -968.4 lbs.	Joint 5 5794.8 lbs. -865.7 lbs.			
	1	6.00	0.00	COMMON D1GE	18-08-00 18-08-00	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 176.5 lbs. -51.3 lbs.	Joint 3 65.2 lbs. -27.8 lbs.	Joint 4 51.6 lbs. 15.5 lbs.	Joint 12 108.5 lbs. -16.4 lbs.	Joint 14 128.5 lbs. -119.5 lbs.
\bigtriangleup	5	6.00	0.00	COMMON G1	15-11-00 15-11-00	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 677.2 lbs. -130.3 lbs.	Joint 4 677.2 lbs. -130.3 lbs.			
	1	6.00	0.00	COMMON G1GE	15-11-00 15-11-00	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 128.6 lbs. -28.5 lbs.	Joint 10 128.6 lbs. -11.1 lbs.	Joint 12 163.5 lbs. -75.8 lbs.	Joint 13 158.9 lbs. -62.7 lbs.	Joint 14 166.2 lbs. -55.5 lbs.
	3	12.00	0.00	COMMON H1	13-01-00 13-01-00	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 570.1 lbs. -86.1 lbs.	Joint 6 570.1 lbs. -86.1 lbs.			
	1	12.00	0.00	COMMON H1GE	13-01-00 13-01-00	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 265.9 lbs. -106.0 lbs.	Joint 10 228.3 lbs. -55.7 lbs.	Joint 12 284.7 lbs. -358.6 lbs.	Joint 13 190.7 lbs. -148.2 lbs.	Joint 14 205.2 lbs. 15.3 lbs.

DATE11/30/22 PAGE 2 J1122-5849

0000006558

Net 10 Days

ORDER #

QUOTE #

INVOICE #

TERMS

CUSTOMER ACCT #

CUSTOMER PO #

: N/A DATE BY



Fayetteville, N.C. 28309 (910) 864-TRUS

			JOBSITE PHONE #	(910) 263-0276		SALES AREA	David La	indry
	Wellco Contractors, Inc.	JOB NAME:L	ot 148 Hidden Lakes		LOT #	148 SUBDIV: Hidd	en Lakes	
S O L	PO Box 766	MODEL:Roof	TAG: Pla	n 11 / 2GLF, CP	JOB C	ATEGORY: B & S - Bu	ild and Ship	
д но	Spring Lake, NC 28390 (910) 436-3131	DELIVERY INS	TRUCTIONS:					
ынна но	Wellco Contractors Sugarberry Place Clayton, NC 27527	SPECIAL INST Copied from Lot	RUCTIONS: 135 Hidden Lakes (J1122-562	25)		PLAN S	EAL DATE:	N/A
							BY	DATE

11

11

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11/23/22

Johnston

Jason Wellons

Jason Wellons

REQ. QUOTE DATE

ORDER DATE

ORDERED BY

COUNTY

DELIVERY DATE

DATE OF INVOICE

SUPERINTENDANT

BUILDING DEPARTMENT OVERHANG INFO HEEL HEIGHT 00-04-05 REQ. LAYOUTS REQ. ENGINEERING QUOTE JL 11/30/22 END CUT RETURN LAYOUT JL 11/30/22 Roof Order 11/30/22 1 CUTTING JL PLUMB GABLE STUDS 16 IN. OC JOBSITE JOBSITE 1 LOADING TCLL-TCDL-BCLL-BCDL STRESS INCR. **ROOF TRUSSES** ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.) INFORMATION 20.0,10.0,0.0,10.0 1.15

PROFILE			Сп	ITPE	BASE	LUN	IBER	OVER	HANG	REACTIONS				
	PLY	TOP	BOT	טו	0/A	TOP	BOT	LEFT	RIGHT					
	1			MONOPITCH	05-04-00	2.0	2 2 2			Joint 1	Joint 5			
	2 Ply	4.00	0.00	MI-GR	05-04-00	2 7 0	2.7.0			560.8 lbs.	597.7 lbs.			
										-135.7 lbs.	-315.7 lbs.			
^				PIGGYBACK	06-08-09	0 V 4	0 1 4			Joint 2	Joint 4	Joint 6		
	26	10.00	0.00	PB	06-08-09	2 X 4	2 X 4			182.0 lbs.	182.0 lbs.	222.9 lbs.		
										-55.7 lbs.	-64.8 lbs.	8.2 lbs.		
^		40.00	0.00	GABLE	06-08-09	2 1	2 × 4			Joint 2	Joint 6	Joint 8	Joint 9	Joint 10
	2	10.00	0.00	PBGE	06-08-09	2 × 4	2 X 4			91.4 lbs.	78.2 lbs.	200.7 lbs.	119.0 lbs.	202.0 lbs.
										-34.4 lbs.	-11.8 lbs.	-175.6 lbs.	23.5 lbs.	-176.8 lbs.
					47.04.04					laint 4	laint C	laint C	laint 0	laint O
	1	12.00	0.00	VALLE Y	17-04-04	2 8 1	2 8 1			Joint 1	JOINT 5	JOINT 6	JOINT 8	Joint 9
416	· ·	12.00	0.00	VOI		274	274			221.1 IDS.	100.1 IDS.	361.6 IDS.	427.1 IDS.	302.2 IDS.
										-47.0 IDS.	-3.7 IDS.	-319.0 lbs.	50.3 IDS.	-319.0 lbs.
				VALLEY	14 00 04					loint 1	loint 5	loint 6	loint 7	loint 9
	1	12 00	0.00	VALLET VC2	14-00-04	2 X 4	2 X 4			161.8 lbc	135.3 lbc	138 / lbc	407.6 lbs	138.8 lbc
415		12.00	0.00							-57 0 lbs	-21.8 lbs	-257 1 lbs	407.0 lb3.	-257.2 lbs
										-57.0 lb3.	-21.0103.	-207.1 103.	52.7 103.	-237.2 103.
				VALLEY	10-08-04					Joint 1	Joint 5	Joint 6	Joint 7	Joint 8
	1	12.00	0.00	VC3	10-08-04	2 X 4	2 X 4			164.7 lbs	151 7 lbs	390.9 lbs	220.3 lbs	391.0 lbs
										-147.6 lbs.	-121.3 lbs.	-258.9 lbs.	43.5 lbs.	-258.8 lbs.
				VALLEY	07-04-04					Joint 1	Joint 3	Joint 4		
	1	12.00	0.00	VC4	07-04-04	2 X 4	2 X 4			163.5 lbs.	163.5 lbs.	210.0 lbs.		
										-53.0 lbs.	-53.0 lbs.	11.8 lbs.		
				VALLEY	04-00-04					Joint 1	Joint 3	Joint 4		
	1	12.00	0.00	VC5	04-00-04	2 X 4	2 X 4			82.3 lbs.	82.3 lbs.	105.7 lbs.		
-										-26.7 lbs.	-26.7 lbs.	6.0 lbs.		
				VALLEY	11-01-09					Joint 1	Joint 3	Joint 4		
	1	6.00	0.00	VD1	11-01-09	2 X 4	2 X 4			184.1 lbs.	184.1 lbs.	431.0 lbs.		
										-49.5 lbs.	-57.0 lbs.	-44.1 lbs.		
										1				
				VALLEY	04-05-09					Joint 1	Joint 3	Joint 4		
and the	1	6.00	0.00	VD2	04-05-09	2 X 4	2 X 4			67.0 lbs.	67.1 lbs.	128.5 lbs.		
										-20.3 lbs.	-22.9 lbs.	-6.4 lbs.		

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Net 10 Days

Lenny Norris

ORDER #

QUOTE #

INVOICE #

SALES REP

TERMS

CUSTOMER ACCT #

CUSTOMER PO #



_								
Reil	ly Road Industrial Park P.O. Box 40408		COUNTY	Johnston	TERI	MS	Net 10 Days	
Fay	etteville, N.C. 28309 (910) 864-TRUS		SUPERINTENDANT	Jason Wellons	SALE	ES REP	Lenny No	rris
			JOBSITE PHONE #	(910) 263-0276	SALE	ES AREA	David Lan	dry
Π	Wellco Contractors, Inc.	JOB NAME:L	₋ot 148 Hidden Lakes		LOT # 148	Lakes		
S O L	PO Box 766	MODEL:Roof	TAG: Pla	n 11 / 2GLF, CP	JOB CATEG	GORY: B & S - Build and Shi		
D HO	Spring Lake, NC 28390 (910) 436-3131	DELIVERY INS	TRUCTIONS:					
ынны но	Wellco Contractors Sugarberry Place Clayton, NC 27527	SPECIAL INST Copied from Lot	RUCTIONS: 135 Hidden Lakes (J1122-56	25)		PLAN SE	AL DATE:	N/A
							BY	DATE

11

11

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11/23/22

Jason Wellons

REQ. QUOTE DATE

ORDER DATE

ORDERED BY

DELIVERY DATE

DATE OF INVOICE

BUILDING DEPARTMENT	OVERH	ANG INFO	HEEL HEIGHT	00-04-05	REC	Q. LAYOUTS		REQ. E	NGINEERING		QUOTE	JL	11/30/22
Roof Order	END CUT	RETURN									LAYOUT	JL	11/30/22
	PLUMB		GABLE STUDS	16 IN. OC		JOBSITE	1		JOBSITE	1	CUTTING	JL	11/30/22

ROOF T	RUS	SES		DADING FORMATION	TCLL-TCDL-B0 20.0,10.0,0	сільвор 0.0,10.	l str 0	ESS INCR.	RO	OF TRUSS S	PACING:24.0	IN. O.C. (TYP	.)		
PROFILE	QTY PLY	PIT TOP	СН вот	TYPE ID	BASE O/A	LUN TOP	IBER BOT	OVER	HANG RIGHT	REACTIO	NS				
	1	6.00	0.00	VALLEY VG1	14-02-09 14-02-09	2 X 4	2 X 4			Joint 1 74.1 lbs. -11.4 lbs.	Joint 5 74.1 lbs. -1.6 lbs.	Joint 6 312.8 lbs. -125.2 lbs.	Joint 7 282.3 lbs. 6.8 lbs.	Joint 8 312.8 lbs. -125.4 lbs.	
	1	6.00	0.00	VALLEY VG2	07-06-09 07-06-09	2 X 4	2 X 4			Joint 1 129.8 lbs. -39.4 lbs.	Joint 3 129.8 lbs. -44.2 lbs.	Joint 4 249.7 lbs. -12.8 lbs.			

ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
25	Hangers, USP	HUS 26			SIMPSON (HUS26)
2	LVL Beams (Sized)	LVL, 1-3/4" x 11-7/8" (S)	14-00-00		BM1
2	LVL Beams (Sized)	LVL, 1-3/4" x 14" (S)	22-00-00		GDH
1	Hangers, USP	THD26-2			SIMPSON (HHUS26-2)

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