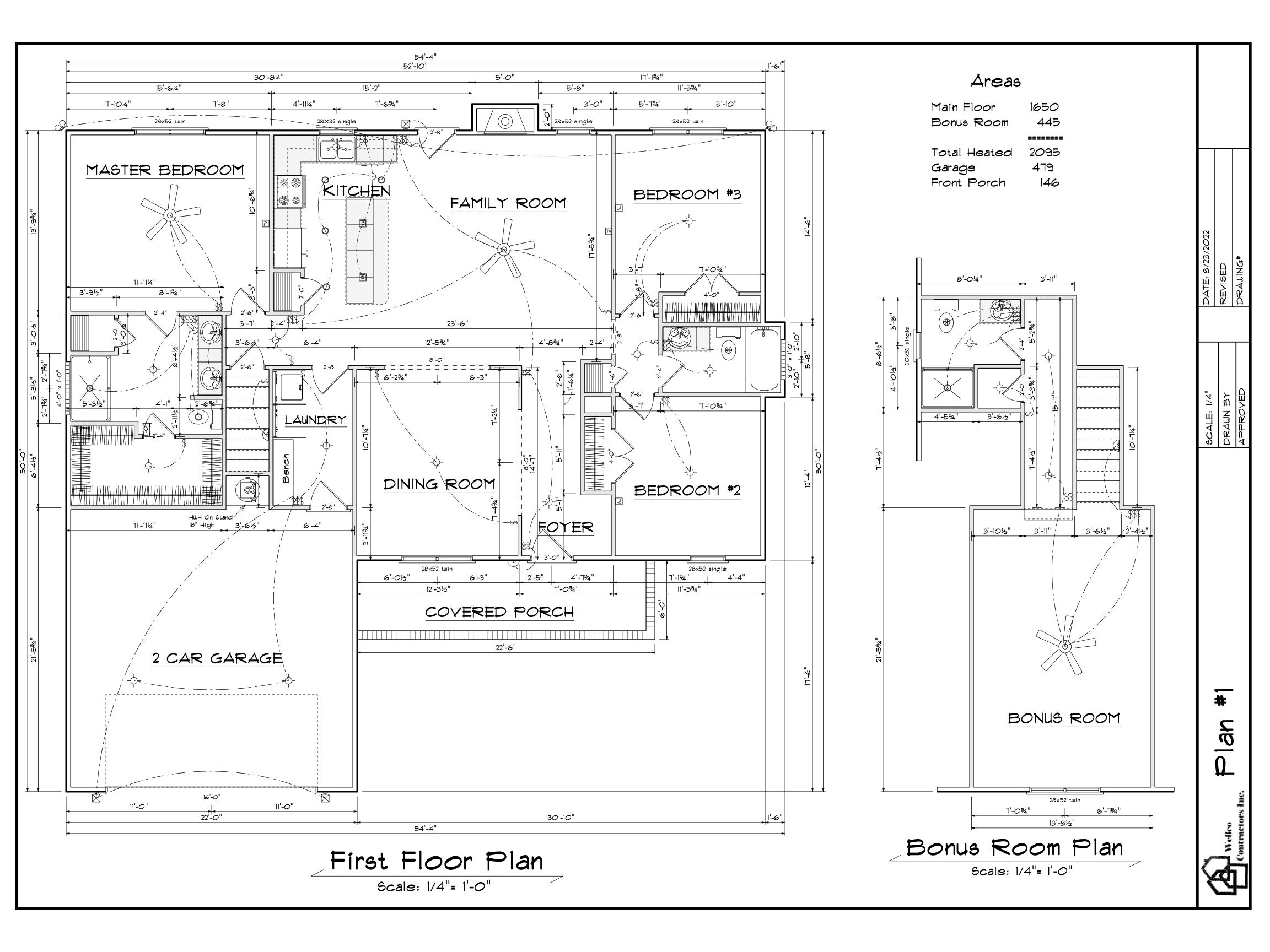
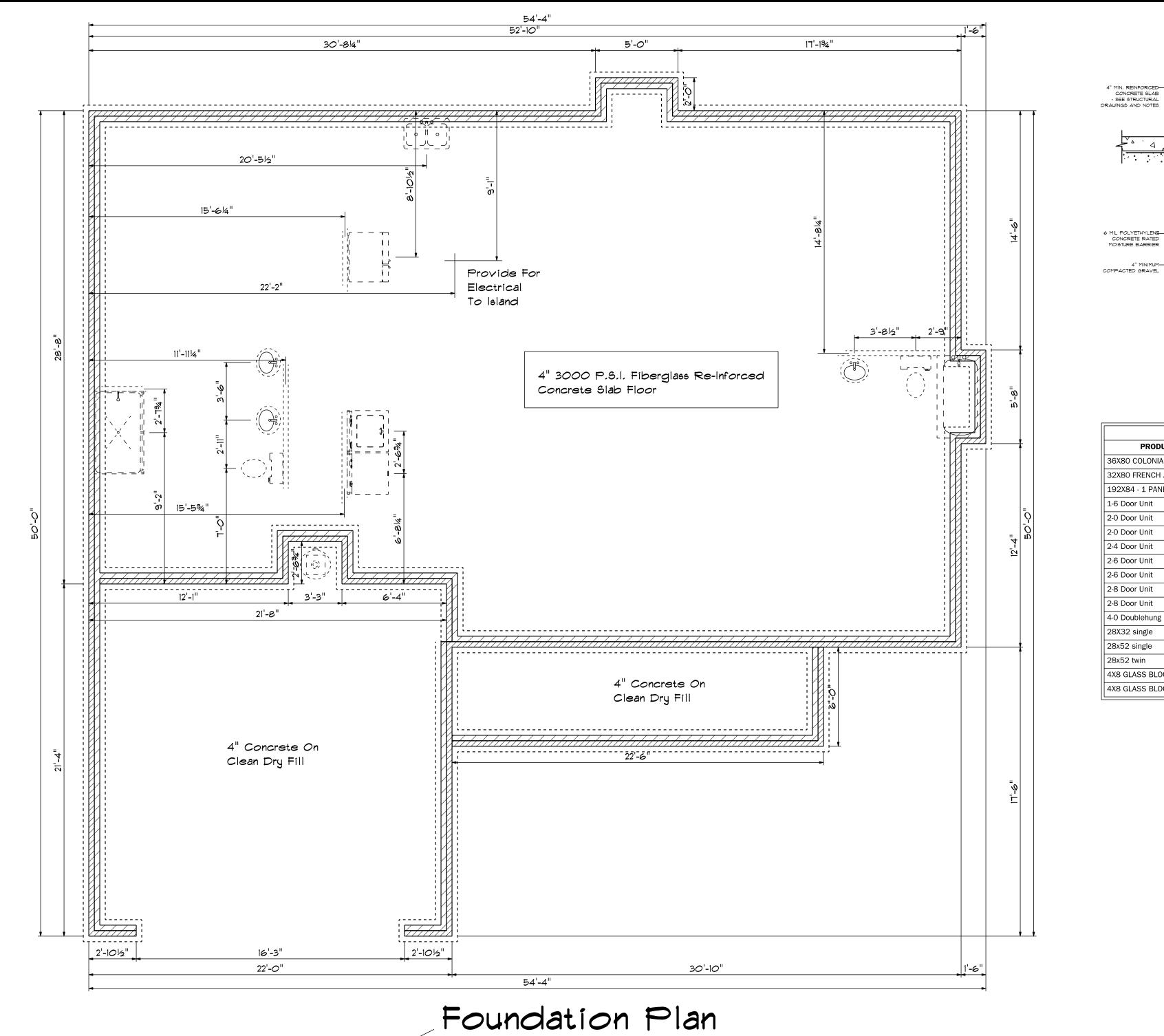
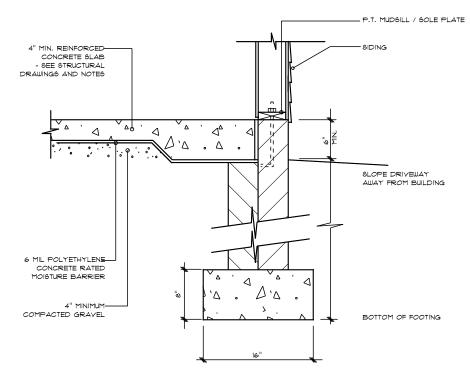


Left Elevation
Scale: 1/8"= 1'0"







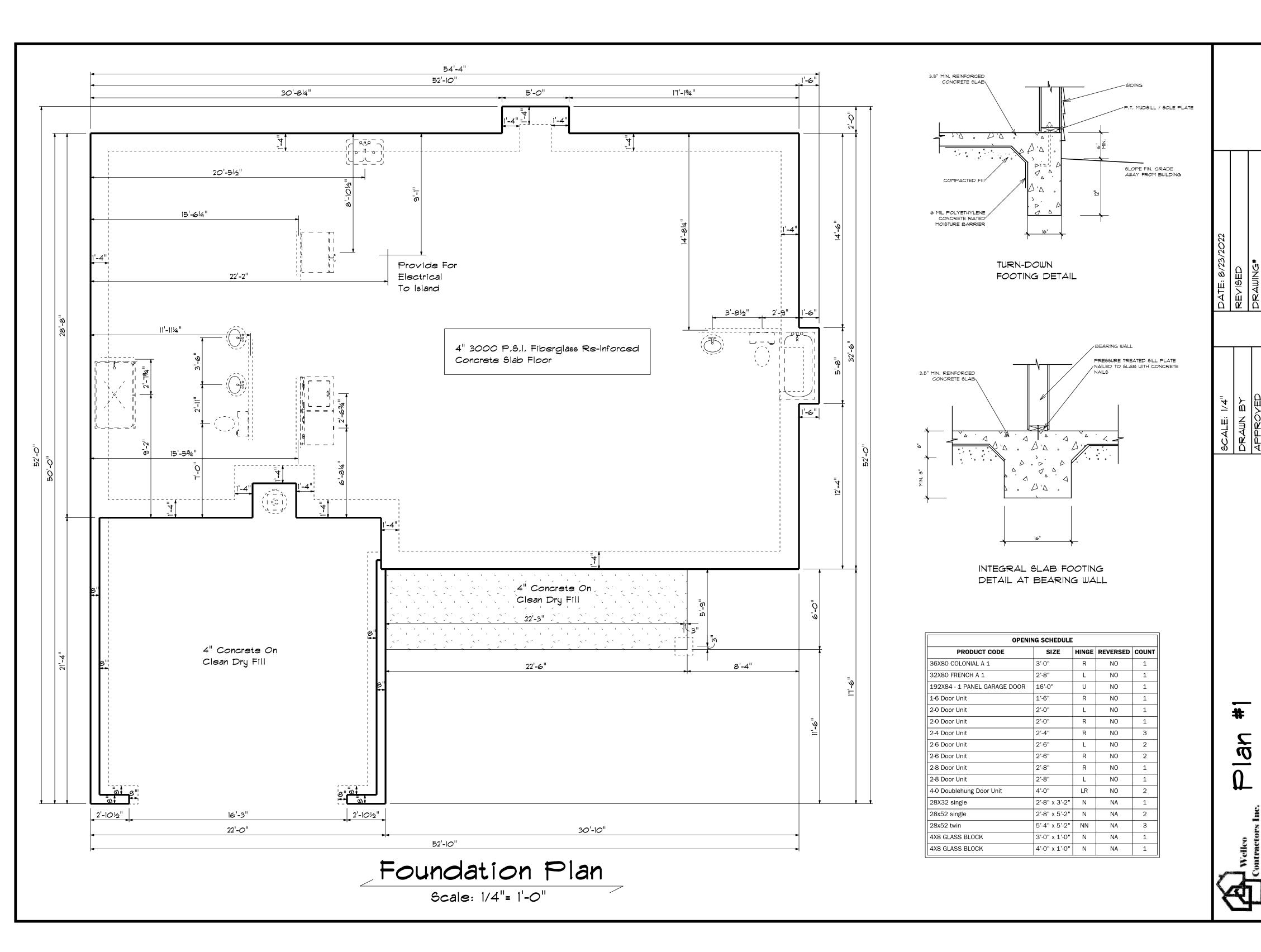


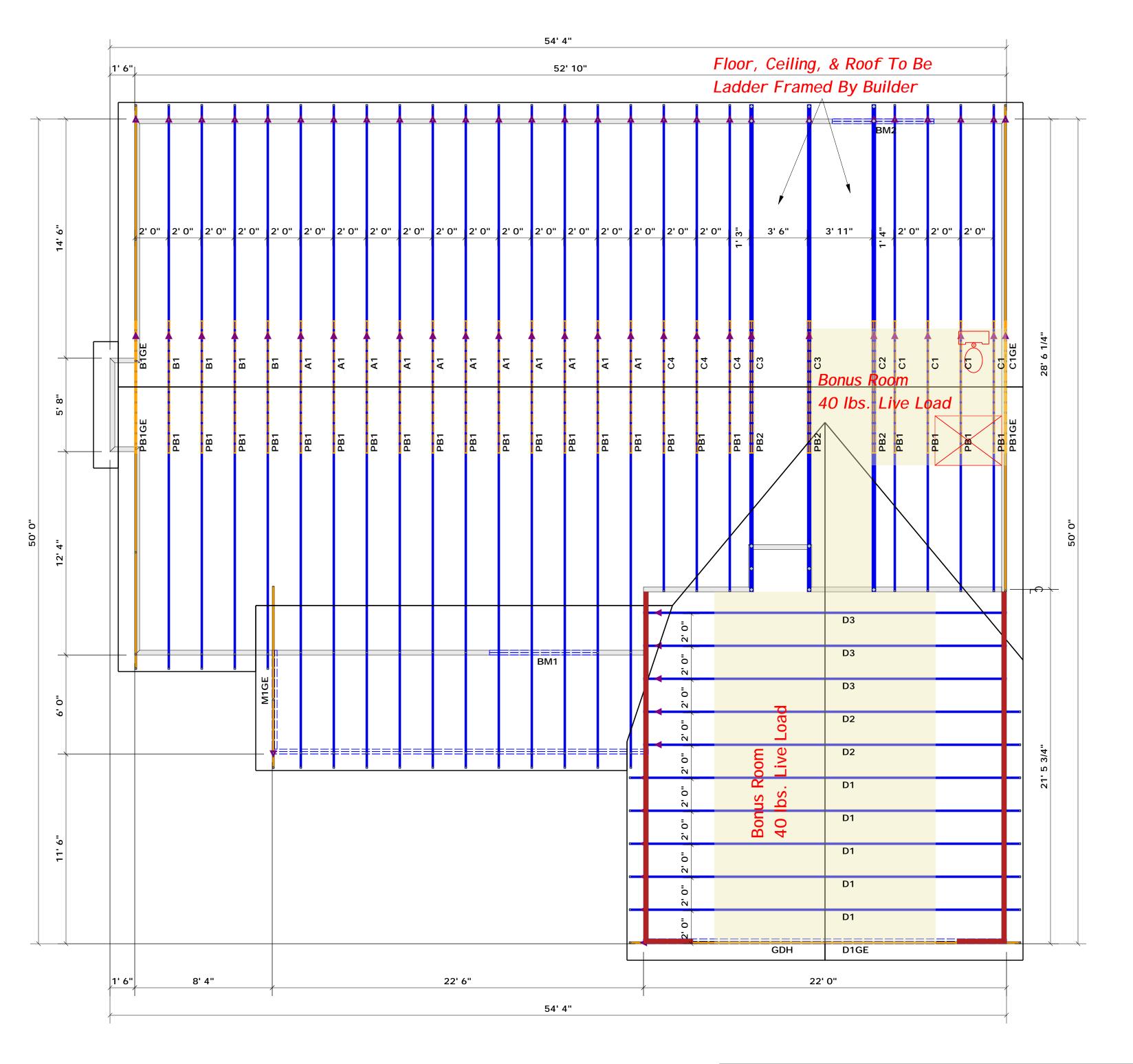
STEM WALL FOOTING DETAIL

| OPENING SCHEDULE | | | | | | | | | | |
|------------------------------|---------------|-------|----------|-------|--|--|--|--|--|--|
| PRODUCT CODE | SIZE | HINGE | REVERSED | COUNT | | | | | | |
| 36X80 COLONIAL A 1 | 3'-0" | R | NO | 1 | | | | | | |
| 32X80 FRENCH A 1 | 2'-8" | L | NO | 1 | | | | | | |
| 192X84 - 1 PANEL GARAGE DOOR | 16'-0" | U | NO | 1 | | | | | | |
| 1-6 Door Unit | 1'-6" | R | NO | 1 | | | | | | |
| 2-0 Door Unit | 2'-0" | L | NO | 1 | | | | | | |
| 2-0 Door Unit | 2'-0" | R | NO | 1 | | | | | | |
| 2-4 Door Unit | 2'-4" | R | NO | 3 | | | | | | |
| 2-6 Door Unit | 2'-6" | L | NO | 2 | | | | | | |
| 2-6 Door Unit | 2'-6" | R | NO | 2 | | | | | | |
| 2-8 Door Unit | 2'-8" | R | NO | 1 | | | | | | |
| 2-8 Door Unit | 2'-8" | L | NO | 1 | | | | | | |
| 4-0 Doublehung Door Unit | 4'-0" | LR | NO | 2 | | | | | | |
| 28X32 single | 2'-8" x 3'-2" | N | NA | 1 | | | | | | |
| 28x52 single | 2'-8" x 5'-2" | N | NA | 2 | | | | | | |
| 28x52 twin | 5'-4" x 5'-2" | NN | NA | 3 | | | | | | |
| 4X8 GLASS BLOCK | 3'-0" x 1'-0" | N | NA | 1 | | | | | | |
| 4X8 GLASS BLOCK | 4'-0" x 1'-0" | N | NA | 1 | | | | | | |

Scale: 1/4"= 1'-0"

| # | |
|-------------------|----------------|
| 0 <u>a</u> n | |
| ••- | C'EOF'S BILL'. |
| Wellen | |





▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards

Hatch Legend
Garage Walls Dropped 1'

Truss Placement Plan
SCALE: 1/4" = 1'

| Beam Legend | | | | | | | | |
|-------------|--------|-----------------------------|-------|---------|----------|--|--|--|
| PlotID | Length | Product | Plies | Net Qty | Fab Type | | | |
| BM1 | 7' 0" | 1-3/4"x 9-1/4" LVL Kerto-S | 2 | 2 | FF | | | |
| BM2 | 7' 0" | 1-3/4"x 9-1/4" LVL Kerto-S | 2 | 2 | FF | | | |
| GDH | 22' 0" | 1-3/4"x 11-7/8" LVL Kerto-S | 2 | 2 | FF | | | |

ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Curtis Quick

Curtis Quick

LOAD CHART FOR JACK STUDS

(84.5ED ON TABLÉS ROCEE(L) & (b))

NUMBER OF JACK STUDS RÉQUIRED © EA END OF
HEADED/GRODE

| | | HEADER | /GTRDE | R | | |
|------------------------|-----------------------------------|-------------------------|------------------------------------|---|-------------------------|-----------------------------------|
| END REACHON (UP 10) | REQ'D STUDS FOR (2) PLY HEADER | END REACTION (UP TO) | REQ1'S STUBS FOR (3) ALY HEADER | | END REACTION (UP TO) | REQUE STUDS FOR (4) PLY HEADER |
| 1700 | 1 | 2550 |) 1 | | 3400 | 1 |
| 3400 | 2 | 5100 |) 2 | | 6800 | 2 |
| 5100 | 3 | 7650 | 3 | | 10200 | 3 |
| 6800 | 4 | 1020 | 0 4 | | 13600 | 4 |
| 8500 | 5 | 1275 | 05 | | 17000 | 5 |
| 10200 | 6 | 1530 | 0 6 | | | |
| 11900 | 7 | | | | | |
| 13600 | 8 | | | | | |
| 15300 | 9 | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | I | | | | | |

| Wellco Contractors | CI TY / CO. | CI TY / CO. Spring Lake / Harnett | 15300 |
|----------------------|----------------------------|-------------------------------------|-------|
| Lot 149 Hidden Lakes | ADDRESS | 47 Basswodd Ct. | 9 |
| Plan 1 | MODEL | Model | |
| Seal Date | DATE REV . 09/13/22 | 09/13/22 | |
| Ouote # | DRAWN BY | DRAWN BY Curtis Quick | |
| J0822-4435 | SALES REP. | SALES REP. Lenny Norris | |

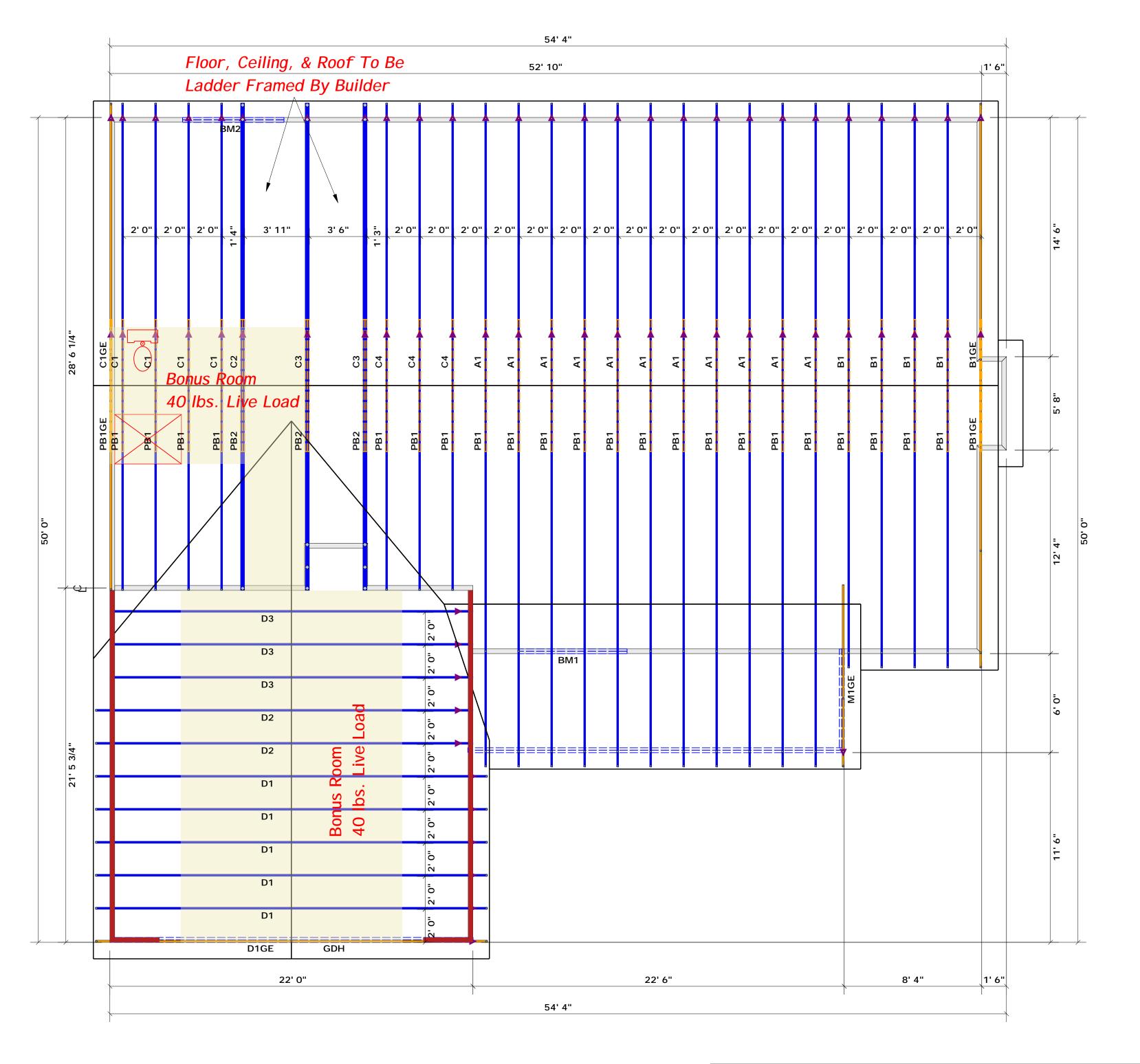
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designs are individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

SEAL DATE

QUOTE 7

JOB NAME

BUILDER



▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards

Hatch Legend
Garage Walls Dropped 1'

Truss Placement Plan
SCALE: 1/4" = 1'

Beam Legend Fab Type Length Product Plies Net Qty PlotID BM1 7' 0" 1-3/4"x 9-1/4" LVL Kerto-S FF BM2 7' 0" 1-3/4"x 9-1/4" LVL Kerto-S GDH FF 22' 0" 1-3/4"x 11-7/8" LVL Kerto-S

ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Curtis Quick

Curtis Quick

LOAD CHART FOR JACK STUDS

(BASED ON TABLES ROUBE(I) & (b))
NUMBER OF JACK STUDG REQUIRED & EA END OF

| | | • | 1 | 4EADER/ | 61 | ROER | | - | |
|-------------------------|-----------------------------------|---|---|-------------------------|----|----------------|-------------------------|----|----------------|
| END REACHION (UP 10) | REQ'O STUDS FOR (2) PLY HEADER | | | END REACTION (UP TO) | | (3) ALY HEADER | END REACTION (UP TO) | 60 | REQUESTUBS FOR |
| 1700 | 1 | | | 2550 | | 1 | 340 | Ō | 1 |
| 3400 | 2 | | | 5100 | | 2 | 680 | 0 | 2 |
| 5100 | 3 | | | 7650 | | 3 | 1020 | Ô | 3 |
| 6800 | 4 | | | 10200 | 1 | 4 | 1360 | 0 | 4 |
| 8500 | 5 | | | 12750 | ı | 5 | 1700 | 0 | 5 |
| 10200 | 6 | | | 15300 | 1 | 5 | | | |
| 11900 | 7 | | | | | | | | |
| 13600 | 8 | | | | | | | | |
| 15300 | 9 | | | | | | | | |
| | | | | | | | | | |

| actors | CITY / CO. | CI TY / CO. Spring Lake / Harnett |
|----------|---------------------------|-------------------------------------|
| en Lakes | ADDRESS | 47 Basswodd Ct. |
| | MODEL | Model |
| | DATE REV. 09/13/22 | 09/13/22 |
| | DRAWN BY | DRAWN BY Curtis Quick |
| | SALES REP. | SALES REP. Lenny Norris |

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designs are individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Seal Date

SEAL DATE

Quote

QUOTE ;

Wellco Contra

BUILDER

Lot

JOB

Client: Wellco Contractors

Project: Address:

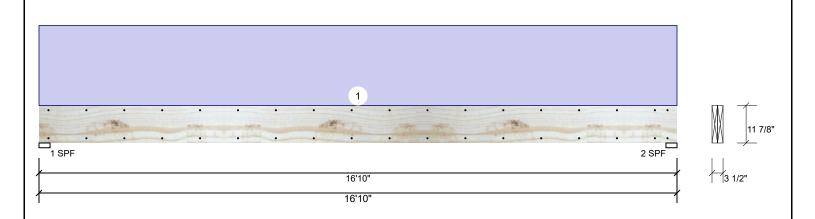
7/15/2022 Input by: Curtis Quick

Job Name: Plan 1 Beams

Project #:

Kerto-S LVL 2-Ply - PASSED 1.750" X 11.875" **GDH**

Level: Level



| Member Info | rmation | | | Rea | ctions UNP | ATTERN | NED Ib | (Uplift) | | | |
|--------------------|---------------|----------------|--------------|-----|--------------|--------|--------|--------------|-------|----------|-----------|
| Type: | Girder | Application: | Floor | Brg | Direction | Live | . [| Dead S | Snow | Wind | Const |
| Plies: | 2 | Design Method: | ASD | 1 | Vertical | 0 |) | 2182 | 0 | 0 | 0 |
| Moisture Condition | on: Dry | Building Code: | IBC/IRC 2015 | 2 | Vertical | 0 |) | 2182 | 0 | 0 | 0 |
| Deflection LL: | 480 | Load Sharing: | No | | | | | | | | |
| Deflection TL: | 360 | Deck: | Not Checked | | | | | | | | |
| Importance: | Normal - II | | | | | | | | | | |
| Temperature: | Temp <= 100°F | | | | | | | | | | |
| | | | | Bea | rings | | | | | | |
| | | | | Bea | aring Length | Dir. | Cap. R | React D/L lb | Total | Ld. Case | Ld. Comb. |
| | | | | 1 - | SPF 3.500" | Vert | 42% | 2182 / 0 | 2182 | Uniform | D |
| | | | | 2 - | SPF 3.500" | Vert | 42% | 2182 / 0 | 2182 | Uniform | D |

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|--------------|---------------|-----------|---------------|-----------------|-------|---------|
| Moment | 8689 ft-lb | 8'5" | 17919 ft-lb | 0.485 (48%) | D | Uniform |
| Unbraced | 8689 ft-lb | 8'5" | 8702 ft-lb | 0.998 (100%) | D | Uniform |
| Shear | 1859 lb | 15'6 5/8" | 7980 lb | 0.233 (23%) | D | Uniform |
| LL Defl inch | 0.000 (L/999) | 0 | 999.000 (L/0) | 0.000 (0%) | | |
| TL Defl inch | 0.453 (L/433) | 8'5 1/16" | 0.546 (L/360) | 0.831 (83%) | D | Uniform |

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 10'8 15/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

| ID | Load Type | Location | Trib Width | Side | Dead 0.9 | Live 1 | Snow 1.15 | Wind 1.6 | Const. 1.25 | Comments |
|----|-------------|----------|------------|------|----------|--------|-----------|----------|-------------|----------|
| 1 | Uniform | | | Тор | 250 PLF | 0 PLF | 0 PLF | 0 PLF | 0 PLF | |
| | Self Weight | | | | 9 PLF | | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 1 of 7

CSD DESIGN

Client: Wellco Contractors

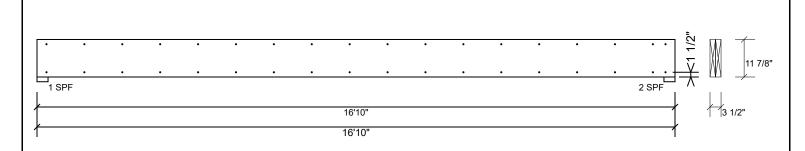
Project: Address:

7/15/2022 Input by: Curtis Quick Job Name: Plan 1 Beams Page 2 of 7

Project #:

Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED **GDH**

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c., Maximum end distance not to exceed 6".

| , , | | , | , |
|--------------------------|-----------|---|---|
| Capacity | 0.0 % | | |
| Load | 0.0 PLF | | |
| Yield Limit per Foot | 163.7 PLF | | |
| Yield Limit per Fastener | 81.9 lb. | | |
| Yield Mode | IV | | |
| Edge Distance | 1 1/2" | | |
| Min. End Distance | 3" | | |
| Load Combination | | | |
| Duration Factor | 1.00 | | |

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

(800) 622-5850

Manufacturer Info Metsä Wood

301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 www.metsawood.com/us

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Client: Wellco Contractors

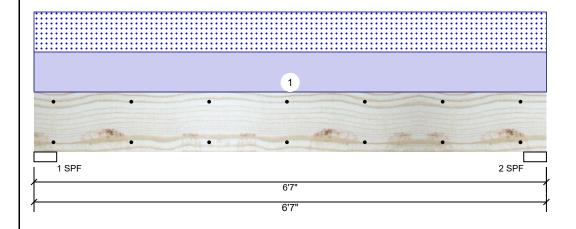
Project: Address: Date: 7/15/2022

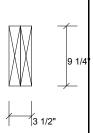
Input by: Curtis Quick Job Name: Plan 1 Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED Kerto-S LVL BM1

Level: Level





Page 3 of 7

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temp <= 100°F Temperature:

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Wind Brg Direction Live Dead Snow Const 0 1564 Vertical 1541 0 0 2 Vertical 0 1564 1541 0 0

Bearings

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+S 1 - SPF 3.500" Vert 1564 / 1541 3105 L 2 - SPF 3.500" Vert 60% 1564 / 1541 3105 L D+S

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|--------------|----------------|----------|---------------|-------------|-------|------|
| Moment | 4423 ft-lb | 3'3 1/2" | 14423 ft-lb | 0.307 (31%) | D+S | L |
| Unbraced | 4423 ft-lb | 3'3 1/2" | 10451 ft-lb | 0.423 (42%) | D+S | L |
| Shear | 2108 lb | 1' 3/4" | 7943 lb | 0.265 (27%) | D+S | L |
| LL Defl inch | 0.040 (L/1842) | 3'3 1/2" | 0.153 (L/480) | 0.261 (26%) | S | L |
| TL Defl inch | 0.080 (L/914) | 3'3 1/2" | 0.204 (L/360) | 0.394 (39%) | D+S | L |

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

| ID | Load Type | Location | Trib Width | Side | Dead 0.9 | Live 1 | Snow 1.15 | Wind 1.6 | Const. 1.25 | Comments |
|----|-------------|----------|------------|------|----------|--------|-----------|----------|-------------|----------|
| 1 | Uniform | | | Тор | 468 PLF | 0 PLF | 468 PLF | 0 PLF | 0 PLF | A1 |
| | Self Weight | | | | 7 PLF | | | | | |

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 11/3/2024 CSD DESIGN

Client: Wellco Contractors

Project: Address: Date: 7/15/2022

Input by: Curtis Quick Job Name: Plan 1 Beams

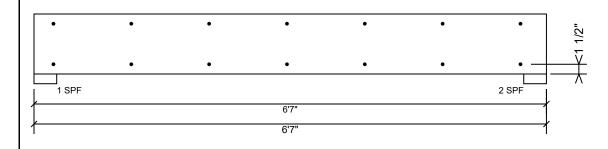
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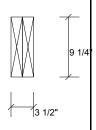
Kerto-S LVL BM1

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 4 of 7

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

| Capacity | 0.0 % |
|--------------------------|-----------|
| Load | 0.0 PLF |
| Yield Limit per Foot | 163.7 PLF |
| Yield Limit per Fastener | 81.9 lb. |
| Yield Mode | IV |
| Edge Distance | 1 1/2" |
| Min. End Distance | 3" |
| Load Combination | |
| Duration Factor | 1.00 |

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





Client: Wellco Contractors

Project: Address: Date: 7/15/2022

Input by: Curtis Quick Job Name: Plan 1 Beams

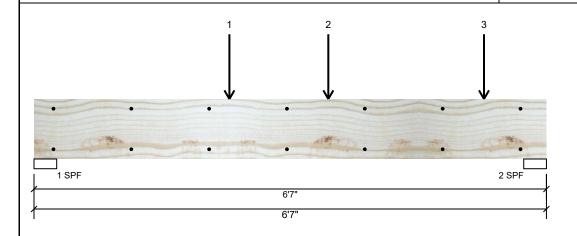
Level: Level

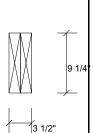
Project #:

Kerto-S LVL BM₂

1.750" X 9.250"

2-Ply - PASSED





Page 5 of 7

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temp <= 100°F Temperature:

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Wind Brg Direction Live Dead Snow Const 0 1380 1357 0 Vertical 0 2 Vertical 0 1696 1672 0 0

Bearings

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+S 1 - SPF 3.500" Vert 53% 1380 / 1357 2737 L 2 - SPF 3.500" Vert 65% 1696 / 1672 3368 L D+S

Analysis Results

| Analysis | Actual | Location | Allowed | Capacity | Comb. | Case |
|--------------|----------------|-----------|---------------|-------------|-------|------|
| Moment | 6221 ft-lb | 2'6 1/8" | 14423 ft-lb | 0.431 (43%) | D+S | L |
| Unbraced | 6221 ft-lb | 2'6 1/8" | 10451 ft-lb | 0.595 (60%) | D+S | L |
| Shear | 2870 lb | 5'6 1/4" | 7943 lb | 0.361 (36%) | D+S | L |
| LL Defl inch | 0.052 (L/1405) | 3'2 1/16" | 0.153 (L/480) | 0.342 (34%) | S | L |
| TL Defl inch | 0.105 (L/698) | 3'2 1/16" | 0.204 (L/360) | 0.515 (52%) | D+S | L |

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.

| 8 Lateral slenderness ratio based on single ply width. | | | | | | | | | | |
|--|----------------|----------|------------|------|----------|--------|-----------|----------|-------------|----------|
| ID | Load Type | Location | Trib Width | Side | Dead 0.9 | Live 1 | Snow 1.15 | Wind 1.6 | Const. 1.25 | Comments |
| 1 | Point | 2-6-2 | | Тор | 1561 lb | 0 lb | 1561 lb | 0 lb | 0 lb | C2 |
| | Bearing Length | 0-3-8 | | | | | | | | |
| 2 | Point | 3-9-6 | | Тор | 734 lb | 0 lb | 734 lb | 0 lb | 0 lb | C1 |
| | Bearing Length | 0-3-8 | | | | | | | | |
| 3 | Point | 5-9-6 | | Тор | 734 lb | 0 lb | 734 lb | 0 lb | 0 lb | C1 |

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

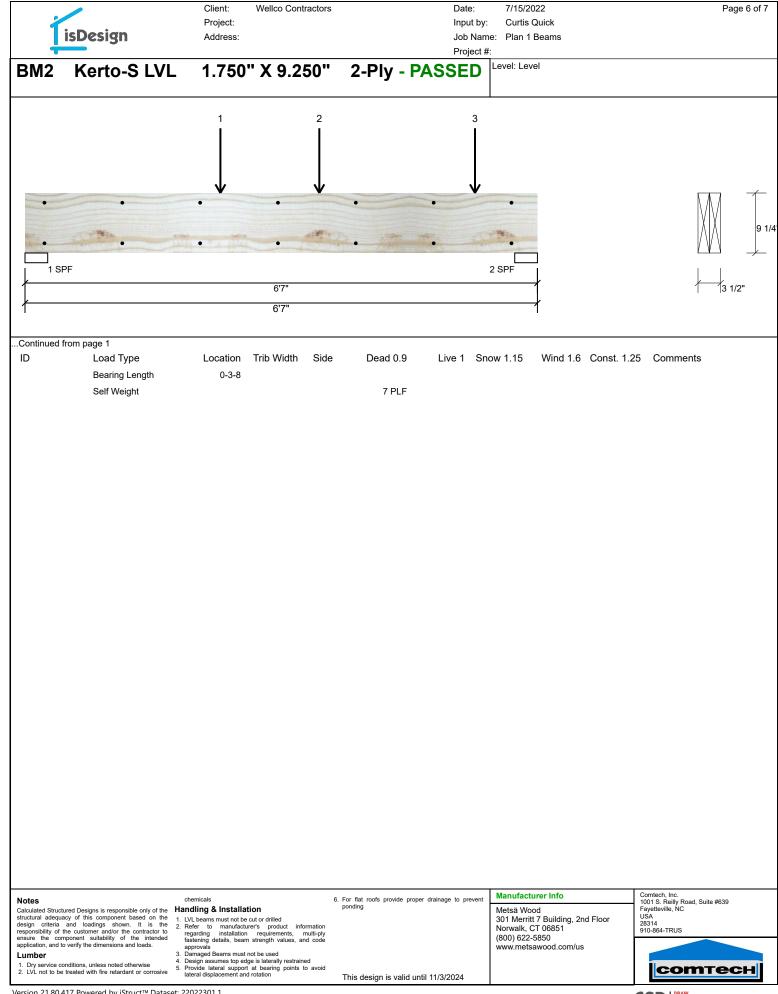
Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



CSD DESIGN



Client: Wellco Contractors

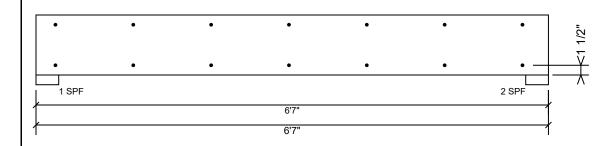
Project: Address: Date: 7/15/2022

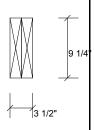
Input by: Curtis Quick Job Name: Plan 1 Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM₂

Level: Level





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Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

| Capacity | 0.0 % |
|--------------------------|-----------|
| Load | 0.0 PLF |
| Yield Limit per Foot | 163.7 PLF |
| Yield Limit per Fastener | 81.9 lb. |
| Yield Mode | IV |
| Edge Distance | 1 1/2" |
| Min. End Distance | 3" |
| Load Combination | |
| Duration Factor | 1.00 |

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

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