

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 33067-33067A 180 BIRCHWOOD GROVE - ROOF

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I53559243 thru I53559272

My license renewal date for the state of North Carolina is December 31, 2022.

North Carolina COA: C-0844



August 10,2022

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



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SEAL

036322

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1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 6-5-0, Exterior(2) 6-5-0 to 9-5-0, Interior(1) 9-5-0 to 13-7-4 zone; cantilever left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This
 connection is for uplift only and does not consider lateral forces.



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			<u>5-2-0</u> 5-2-0					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.52 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.04 2-4 -0.08 2-4 -0.00 3	l/defl >999 >762 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 17 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

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LUMBER-
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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=69(LC 12) Max Uplift 3=-60(LC 16), 2=-48(LC 12)

Max Grav 3=142(LC 2), 2=266(LC 2), 4=99(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 5-2-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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- I russ designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
 5) This trues has been designed for greater of min roof live load of 12.0 ps
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

11) n/a

12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.





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Plate Offsets (X,Y) [2:0-8-4,	0-0-5], [8:Edge,0-1-15], [9:0-5-4,0-1-8],	[10:0-5-0,0-5-4], [11:0-6-0),0-6-4], [13:0-3-8	,0-6-0]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Bop Strass Lagr	CSI. TC 0.77 BC 0.51	DEFL. Vert(LL) Vert(CT)	in (loc) -0.12 10-11 -0.23 10-11	l/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190		
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		0.05 8	n/a	11/d	Weight: 335 lb	FT = 20%		
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x8 SP DSS WEBS 2x4 SP No.3 *E 5-11: 2x6 SP No WEDGE Right: 2x4 SP No.3	xcept* 5.2	BI TC BC	RACING- DP CHORD DT CHORD	Structural wood Rigid ceiling dir	l sheathin ectly appl	g directly app ied or 10-0-0	blied or 2-11-10 oc purl oc bracing.	ins.		
REACTIONS. (size) 8=0- Max Horz 2=1 Max Uplift 8=-6 Max Grav 8=9	5-8, 2=(0-3-8 + TBE4 Simpson Strong- 55(LC 31) 07(LC 11), 2=-595(LC 10) 107(LC 2), 2=6363(LC 2)	Гіе) (req. 0-5-0)								
FORCES. (lb) - Max. Comp.//v TOP CHORD 2-3=-10072/9 7-8=-11495/7 BOT CHORD 2-14=-806/790	lax. Ten All forces 250 (lb) or less exc 54, 3-4=-9329/815, 4-5=-7505/621, 5-6= 30 30, 13-14=-806/7980, 11-13=-645/7725,	ept when shown. -7505/620, 6-7=-9568/70 10-11=-498/7925, 9-10=	4, -583/9208,							
8-9=-583/9208 WEBS 5-11=-623/804 4-11=-2713/44	Bot Clock D 2-14=-000/7500, 10-14=-000/7500, 11-13=-040/720, 10-11=-490/7920, 9-10=-305/9200, 8-9=-583/9208 WEBS 5-11=-623/8043, 6-11=-3071/278, 6-10=-216/3513, 7-10=-1675/172, 7-9=-114/2366, 4-11=-2713/448, 4-13=-417/3085, 3-13=-346/208, 3-14=-187/899									
 NOTES- 1) 2-ply truss to be connected to Top chords connected as foll Bottom chords connected as Webs connected as follows: 2 2) All loads are considered equa ply connections have been pi 3) Unbalanced roof live loads hat 4) Wind: ASCE 7-10; Vult=120n gable end zone; Lumber DOL 5) TCLL: ASCE 7-10; Pr=20.0 pi roof snow: Lumber DOL=1.11 6) This truss has been designed non-concurrent with other live 7) This truss has been designed Continued on page 2 	bigether with 10d (0.131"x3") nails as foll ows: 2x4 - 1 row at 0-7-0 oc. follows: 2x8 - 2 rows staggered at 0-4-0 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 ally applied to all plies, except if noted as rovided to distribute only loads noted as ave been considered for this design. nph Vasd=95mph; TCDL=6.0psf; BCDL= =1.60 plate grip DOL=1.60 isf (roof live load: Lumber DOL=1.15 Plats 5 Plate DOL=1.15); Category II; Exp B; f d for greater of min roof live load of 12.0 a loads.	ows: - 1 row at 0-9-0 oc. s front (F) or back (B) fac. (F) or (B), unless otherwi =6.0psf; h=30ft; Cat. II; E) te DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10 psf or 1.00 times flat roof pnconcurrent with any oth	e in the LOAD CA ise indicated. xp B; Enclosed; M osf (ground snow) i load of 11.6 psf o er live loads.	SE(S) section. IWFRS (envelog ; Pf=11.6 psf (fla on overhangs	Ply to pe) at	A DIMINING THE	SEAL 036322			
WARNING - Verify design para Design valid for use only with MiT a truss system. Before use, the b building design. Bracing indicate is always required for stability and fabrication, storage, delivery, erer Safety Information available fro	meters and READ NOTES ON THIS AND INCLUDE rek® connectors. This design is based only upon p uilding designer must verify the applicability of desi d is to prevent buckling of individual truss web and t to prevent collapse with possible personal injury a ction and bracing of trusses and truss systems, see m Truss Plate Institute, 2670 Crain Highway, Suite	D MITEK REFERENCE PAGE MI arameters shown, and is for an i gn parameters and properly inco or chord members only. Additio and property damage. For gene NS//TPI1 Quality Cr 203 Waldorf, MD 20601	II-7473 rev. 5/19/2020 I individual building com prorate this design into nal temporary and per ral guidance regarding riteria, DSB-89 and B	BEFORE USE. ponent, not to the overall manent bracing the CSI Building Com	ponent		ENGINEERING BY A MiTek 818 Soundside Road Edenton, NC 27932	LO Affiliate		

Job	Truss	Truss Type	Qty	Ply	180 BIRCHWOOD GROVE - ROOF	
						153559251
33067-33067A	CG	COMMON GIRDER	1	2		
				2	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,			8.610 s Jul	18 2022 MiTek Industries, Inc. Tue Aug 9 09:24:20 2022	Page 2
		ID:	W17blGcl	gg2WNY '	TbTxie4yHe28-Us7P5rjg7ZKr02boXripTL9MxuUuX_6s0vKto	cfypdDf

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) TDE4 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

10) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8. This connection is for uplift only and does not consider lateral forces.

11) Use Simpson Strong-Tie HHUS28-2 (22-10d Girder, 4-10d Truss) or equivalent at 5-6-0 from the left end to connect truss(es) to back face of bottom chord.

12) Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent spaced at 2-3-0 oc max. starting at 7-5-4 from the left end to 20-7-4 to connect truss(es) to back face of bottom chord.

13) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-5=-43, 5-8=-43, 2-8=-20

Concentrated Loads (lb)

Vert: 8=-1121(B) 15=-2034(B) 16=-1303(B) 17=-1382(B) 18=-1424(B) 19=-1452(B) 20=-1111(B) 21=-1111(B) 22=-1111(B)

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5-4-	4 10-2-0	18-2-0	26	-2-0	30-11-12	2 36-4-0	
Plate Offsets (X,Y) [5:0-	4 <u>4-9-12</u> 5-0.0-4-8]	8-0-0	8-	0-0	4-9-12	5-4-4	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Ren Stress Incr. YES	CSI. TC 0.50 BC 0.89 WB 0.37	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.17 13 -0.39 13-15 0.14 8	l/defl L/d >999 240 >999 180 p/a p/a	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDI 10.0	Code IRC2015/TPI2014	Matrix-S	1012(01)	0.11 0	1/4 1/4	Weight: 203 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No. 4-5,5-6: 2x6 BOT CHORD 2x4 SP No. WEBS 2x4 SP No.	2 *Except* ; SP No.2 2 3 *Except*	B Ti	RACING- OP CHORD	Structural wood except 2-0-0 oc purlins Bigid ceiling dire	sheathing direc (4-7-13 max.): 4	tly applied or 3-1-14 oc purlir 4-6.	ns,
WEDGE Left: 2x4 SP No.3 , Right: 2	x4 SP No.3	Ň	/EBS	1 Row at midpt	5-15	5, 5-11	
REACTIONS. (size) Max Horz Max Uplift Max Grav	2=0-3-8, 8=0-3-8 2=-77(LC 21) 2=-39(LC 16), 8=-39(LC 17) 2=1503(LC 2), 8=1503(LC 2)						
FORCES. (lb) - Max. Com TOP CHORD 2-3=-2648 7-8=-2648	p./Max. Ten All forces 250 (lb) or less //210, 3-4=-2326/220, 4-5=-2025/226, 5-1 //210	except when shown. 5=-2025/226, 6-7=-2326/220),				
BOT CHORD 2-16=-133	3/2254, 15-16=-133/2254, 13-15=-107/26	87, 11-13=-107/2687, 10-11	=-131/2254,				
WEBS 3-15=-384 7-11=-384	//121, 4-15=0/659, 5-15=-878/128, 5-13= //122	0/345, 5-11=-878/128, 6-11:	=0/659,				
NOTES- 1) Unbalanced roof live load 2) Wind: ASCE 7-10; Vult=' gable end zone and C-C 26-2-0, Exterior(2) 26-2-6 exposed;C-C for membe 3) TCLL: ASCE 7-10; Pr=20 roof snow: Lumber DOL- governs. Rain surcharge 4) Unbalanced snow loads 5) This truss has been desi non-concurrent with othe 6) Provide adequate draina 7) This truss has been desi 8) * This truss has been desi 8) * This truss has been desi 9) One H2.5A Simpson Stro- connection is for uplift on 10) Graphical purlin represe	As have been considered for this design. 20mph Vasd=95mph; TCDL=6.0psf; BC Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2- to 30-4-15, Interior(1) 30-4-15 to 37-2-8 rs and forces & MWFRS for reactions shu 0.0 psf (roof live load: Lumber DOL=1.15; 1.15 Plate DOL=1.15); Category II; Exp applied to all exposed surfaces with slo have been considered for this design. gred for greater of min roof live load of 1. r live loads. ge to prevent water ponding. gned for a 10.0 psf bottom chord live load signed for a live load of 20.0psf on the bc n chord and any other members. ong-Tie connectors recommended to con ly and does not depict the size or the output the size or the size	DL=6.0psf; h=30ft; Cat. II; E 1-8 to 10-2-0, Exterior(2) 10 zone; cantilever left and rig pwn; Lumber DOL=1.60 plat Plate DOL=1.15); Pg=15.0 ; B; Partially Exp.; Ct=1.10, Lu bes less than 0.500/12 in ac 2.0 psf or 1.00 times flat roo d nonconcurrent with any oth ttom chord in all areas wher nect truss to bearing walls d ientation of the purlin along	xp B; Enclosed; I-2-0 to 14-4-15, ht exposed ; end e grip DOL=1.60 osf (ground snow u=50-0-0; Min. fit cordance with IB f load of 11.6 psf her live loads. re a rectangle 3-6 ue to UPLIFT at the top and/or bo	MWFRS (envelop Interior(1) 14-4-1 vertical left and r); Pf=16.5 psf (fla at roof snow load C 1608.3.4. on overhangs 6-0 tall by 2-0-0 w it(s) 2 and 8. This ottom chord.	be) 5 to ight at	SEAL 036322	

ARKING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 rev. 5/19/2/02/ BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	6-8-4	12-10-0	18-2-0	23-6-0		29-7-12	36-4-0	
	6-8-4	6-1-12	5-4-0	5-4-0		6-1-12	6-8-4	
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	sf) 20.0 16.5/15.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Bop Strass Lagr VES	CSI. TC 0.98 BC 0.78	DEFL. Vert(LL) Vert(CT)	in (loc) -0.15 15 -0.30 15-16	l/defl L/d >999 240 >999 180	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 * 10.0	Code IRC2015/TPI2014	Matrix-S		0.13 10	11/a 11/a	Weight: 199 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE Left: 2x4 SP N	2x4 SP No.2 *E 4-5,7-8: 2x4 SP 2x4 SP No.2 2x4 SP No.2 *E 3-18,9-12: 2x4 S lo.3 , Right: 2x4 S	xcept* No.3 xcept* SP No.3 P No.3	В Т(В W	RACING- OP CHORD OT CHORD /EBS	Structural wood 2-0-0 oc purlins Rigid ceiling dir 1 Row at midpt	l sheathing directly ap ; (4-0-9 max.): 5-7. ectly applied or 10-0- 6-16, 6-1	oplied, except 0 oc bracing. 14	
REACTIONS.	(size) 2=0- Max Horz 2=-9 Max Uplift 2=-6 Max Grav 2=19	3-8, 10=0-3-8 17(LC 17) 11(LC 16), 10=-61(LC 17) 503(LC 2), 10=1503(LC 2)						
FORCES. (II TOP CHORD BOT CHORD WEBS	 b) - Max. Comp./k 2-3=-2635/19 9-10=-2635/19 2-18=-112/22 10-12=-109/22 3-18=0/277, 3 7-14=0/611, 9 	Max. Ten All forces 250 (lb) or less exc 6, 3-5=-2125/218, 5-6=-1814/230, 6-7=- 96 41, 16-18=-112/2241, 15-16=-45/2023, 1 241 -16=-608/145, 5-16=0/611, 6-16=-456/8 -14=-608/146, 9-12=0/277	ept when shown. 1814/230, 7-9=-2125/218 14-15=-45/2023, 12-14=- 3, 6-15=0/266, 6-14=-450	3, 109/2241, 6/83,				
NOTES- 1) Unbalanced 2) Wind: ASC gable end z 23-6-0, Ext exposed;C- 3) TCLL: ASC roof snow: I governs. R 4) Unbalanced 5) This truss P non-concur 6) Provide add 7) This truss P 8) * This truss S will fit betww 9) One H2.5A connection 10) Graphical	d roof live loads have E 7-10; Vult=120n cone and C-C Exte erior(2) 23-6-0 to 2 C for members an E 7-10; Pr=20.0 p Lumber DOL=1.13 tain surcharge app d snow loads have has been designed that been designed has b	ave been considered for this design. nph Vasd=95mph; TCDL=6.0psf; BCDL= erior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 27-8-15, Interior(1) 27-8-15 to 37-2-8 zo nd forces & MWFRS for reactions showr isf (roof live load: Lumber DOL=1.15 Pla 5 Plate DOL=1.15); Category II; Exp B; F blied to all exposed surfaces with slopes be been considered for this design. d for greater of min roof live load of 12.0 e loads. o prevent water ponding. d for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the bottor ord and any other members, with BCDL Tie connectors recommended to connect nd does not consider lateral forces. tion does not depict the size or the orien	=6.0psf; h=30ft; Cat. II; E to 12-10-0, Exterior(2) 1 ne; cantilever left and rig x; Lumber DOL=1.60 plat the DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10, Lu less than 0.500/12 in ac psf or 1.00 times flat rooi onconcurrent with any oth m chord in all areas wher = 10.0psf. ct truss to bearing walls d tation of the purlin along	ixp B; Enclosed; I 12-10-0 to 17-0-1 ht exposed ; end te grip DOL=1.60 psf (ground snow u=50-0-0; Min. fla cordance with IB f load of 11.6 psf ner live loads. re a rectangle 3-6 lue to UPLIFT at j the top and/or bc	/WFRS (envelop 5, Interior(1) 17- vertical left and); Pf=16.5 psf (fl. it roof snow load C 1608.3.4. on overhangs i-0 tall by 2-0-0 v it(s) 2 and 10. The ottom chord.	pe) D-15 to right at vide	SEAL 036322	The second second

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August 10,2022



	8	3-0-0	15-6-0	2	0-10-0	28-4-0		1	36-4-0	
	6	3-0-0	7-6-0		5-4-0	7-6-0			8-0-0	
Plate Offsets ()	X,Y) [4:0-3-7,0	-2-0], [6:0-3-0,0-2-0], [7:	0-3-7,0-2-0]							
LOADING (psi TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 CSI. 1.15 TC 1.15 BC YES WB Pl2014 Mate	0.93 0.55 0.27 rix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.10 14 -0.21 14-16 0.09 9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 249 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SP No.2 *Ex 5-6: 2x4 SP No.3 2x6 SP No.2 2x4 SP No.2 *Ex 3-16,8-11: 2x4 SI	cept* cept* P No.3			BRACING- TOP CHORD BOT CHORD WEBS	Structural wood 2-0-0 oc purlins Rigid ceiling dire 1 Row at midpt	sheathing (2-2-0 ma ectly applie	g directly appli ix.): 5-6. ed or 10-0-0 c 3-14, 5-13,	ied or 4-2-3 oc purlins oc bracing. 8-13	, except
REACTIONS.	(size) 2=0-3 Max Horz 2=115	9-8, 9=0-3-8 5(LC 20)								

Max Horz 2=115(LC 20) Max Uplift 2=-78(LC 16), 9=-78(LC 17) Max Grav 2=1490(LC 2), 9=1490(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2751/186, 3-5=-2023/212, 5-6=-1672/229, 6-8=-2024/212, 8-9=-2750/186

BOT CHORD 2-16=-127/2399, 14-16=-127/2399, 13-14=-0/1670, 11-13=-93/2398, 9-11=-93/2398

WEBS 3-16=0/336, 3-14=-820/180, 5-14=-12/532, 6-13=0/523, 8-13=-818/181, 8-11=0/334

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-7-14 to 2-4-2, Interior(1) 2-4-2 to 15-6-0, Exterior(2) 15-6-0 to 19-8-15, Interior(1) 19-8-15 to 20-10-0, Exterior(2) 20-10-0 to 25-0-15, Interior(1) 25-0-15 to 36-11-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



A MITEK Affilia 818 Soundside Road Edenton, NC 27932

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		8-9-8	11-6-10	17-2-	0 1	19-2-0	24	1-9-6		27-6-7	1	36-1-0	
		8-9-8	2-9-2	5-7-0	6 ¹	2-0-0	5	-7-6	1	2-9-0	1	8-6-9	I
Plate Offsets (2	X,Y) [6:0-7-8,0)-2-8], [9:0-1-4,Edge], [14	:0-2-6,0-2-8]										
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matri	0.62 0.69 0.30 x-S		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.14 -0.33 0.08	(loc) 2-15 2-15 9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 247 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SP No.2 *Ex 5-6: 2x4 SP No.2 2x6 SP No.2 2x4 SP No.2 *Ex 2 15 8 10: 2×4 SP	ccept* 3 ccept*			-	BRACIN TOP CH	I G- ORD ORD	Structura except 2-0-0 oc Rigid cei	al wood purlins iling dire	sheathir (3-5-10 i ectly app	ng directly app max.): 5-6. lied or 10-0-0	oc bracing.	ins,
	5-15,0-10. 244 0	110.5				WLD3		i now a	rmupt		5-15, 0-12	-	

REACTIONS. (size) 2=0-3-8, 9=Mechanical

Max Horz 2=130(LC 16) Max Uplift 2=-88(LC 16), 9=-74(LC 17) Max Grav 2=1504(LC 40), 9=1472(LC 40)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-2742/194, 3-5=-2489/218, 5-6=-1637/217, 6-8=-2538/233, 8-9=-2757/196
- BOT CHORD 2-15=-182/2398, 13-15=0/1635, 12-13=0/1636, 10-12=0/1636, 9-10=-94/2413
- WEBS 3-15=-572/255, 5-15=-119/820, 5-13=-122/283, 6-10=-134/888, 8-10=-597/258

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-7-14 to 2-4-2, Interior(1) 2-4-2 to 17-2-0, Exterior(2) 17-2-0 to 23-4-15, Interior(1) 23-4-15 to 36-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
 connection is for uplift only and does not consider lateral forces.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



RE USE. it, not overall nt bracing uilding Component B18 Soundside Road Edenton, NC 27932

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	8-0)-4	15-6-0		1	20-10-0		28-3-12		1	36-1-0	
	8-0)-4	7-5-12		1	5-4-0		7-5-12			7-9-4	I
Plate Offsets (X,Y) [4:0-3-2,0	-2-0], [7:0-3-2,0-2-0	0]									
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip D0 Lumber DOL Rep Stress II Code IRC20	2-0-0 DL 1.15 . 1.15 ncr YES 15/TPI2014	CSI. TC BC WB Matri	0.44 0.82 0.26 x-S	DEFL. Vert(LL) Vert(CT Horz(C	in -0.12) -0.26) 0.13	(loc) 12-13 13-15 9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 221 lb	GRIP 244/190 FT = 20%
LUMBER-						BRACING-						
TOP CHORD	2x6 SP No.2					TOP CHORD	Structur	al wood	sheathin	g directly ap	plied or 3-11-10 oc purli	ins,
BOT CHORD	2x4 SP No.2						except			• • •		
WEBS	2x4 SP No.2 *Ex	cept*					2-0-0 oc	purlins	(5-7-12 r	nax.): 5-6.		
	3-15,8-10: 2x4 S	P No.3				BOT CHORD WEBS	Rigid ce 1 Row a	iling dir t midpt	ectly appl	ied or 10-0-0 3-13, 5-1	0 oc bracing. 2, 8-12	
REACTIONS.	(size) 2=0-3	3-8, 9=Mechanical								,		

Max Horz 2=120(LC 20) Max Uplift 2=-81(LC 16), 9=-63(LC 17) Max Grav 2=1497(LC 2), 9=1444(LC 39)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2694/184, 3-5=-1996/213, 5-6=-1652/229, 6-8=-1993/215, 8-9=-2669/182
- BOT CHORD 2-15=-127/2339, 13-15=-127/2339, 12-13=-14/1654, 10-12=-92/2307, 9-10=-92/2307
- WEBS 3-15=0/335, 3-13=-776/174, 5-13=-11/518, 6-12=0/495, 8-12=-743/175, 8-10=0/332

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 15-6-0, Exterior(2) 15-6-0 to 19-8-15, Interior(1) 19-8-15 to 20-10-0, Exterior(2) 20-10-0 to 25-0-15, Interior(1) 25-0-15 to 36-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces. 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This

Continuentionagefor uplift only and does not consider lateral forces

👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-74/3 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	180 BIRCHWOOD GROVE - ROOF	
33067-33067A	Н8	HIP	1	1	15	3559257
33007-33007A			1		Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,			8.610 s Ju	18 2022 MiTek Industries, Inc. Tue Aug 9 09:24:31 2022 Pa	age 2
		ID:W	17blGclgc	2WNY_Tb	Txie4yHe28-g_HZPcrZYxjIrlxvhfPOQf6B9J9Ec0ETY7UyVXyp	bdDU

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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	5-4-4	10-2-0	18-2-0	26	-2-0	1	36-1-0	
	5-4-4	4-9-12	8-0-0	8-	0-0	1	9-11-0	
Plate Offsets (X,Y) [5:0-5-0,	0-4-8]						
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	sf) 20.0 16.5/15.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Pop Stress Ippr	CSI. TC 0.71 BC 0.67 WB 0.26	DEFL. Vert(LL) Vert(CT)	in (loc) -0.14 11 -0.28 11-13	l/defl L/c >999 240 >999 180	d PLATES 0 MT20	GRIP 244/190
BCLL	0.0 *	Code IRC2015/TPI2014	Matrix-S	1012(01)	0.09 0	11/d 11/d	Weight: 224 lb	FT = 20%
BCDL	10.0							
LUMBER- TOP CHORD BOT CHORD	2x4 SP No.2 *E 4-5,5-6: 2x6 SP 2x6 SP No.2 2x4 SP No.3 *E	xcept* No.2	E T E	BRACING- OP CHORD BOT CHORD	Structural wood 2-0-0 oc purlins Rigid ceiling dire	sheathing dire (4-7-10 max.): ectly applied or	ectly applied or 3-3-1 oc purlin: 4-6. : 10-0-0 oc bracing. 13. 5-9	s, except
WEBS	5-13 5-9 2x4 SI		V	VEBS	r Row at miupt	5-	13, 5-9	
REACTIONS.	(size) 8=M Max Horz 2=81 Max Uplift 8=-2 Max Grav 8=14	echanical, 2=0-3-8 (LC 20) 1(LC 17), 2=-39(LC 16) I34(LC 2), 2=1497(LC 2)						
FORCES. (II TOP CHORD BOT CHORD WEBS	 b) - Max. Comp./N 2-3=-2670/216 7-8=-2564/257 2-14=-146/228 3-13=-389/122 7-9=-290/171 	lax. Ten All forces 250 (lb) or le: 6, 3-4=-2353/223, 4-5=-2049/229, 1 36, 13-14=-146/2286, 11-13=-123, 2, 4-13=0/672, 5-13=-867/133, 5-1	ss except when shown. 5-6=-2043/216, 6-7=-2349/20/ /2705, 9-11=-123/2705, 8-9=-1 1=0/305, 5-9=-874/144, 6-9=0	6, 74/2196 //667,				
NOTES- 1) Unbalanced 2) Wind: ASCI gable end z 26-2-0, Exti Lumber DC 3) TCLL: ASC roof snow: I governs. R 4) Unbalanced 5) This truss h non-concur 6) Provide add 7) This truss h non-concur 6) Provide add 7) This truss h 8) * This truss will fit betwe 9) Refer to gir 10) Provide m 11) One H2.5, connection 12) Graphical	d roof live loads ha E 7-10; Vult=120n cone and C-C Exte erior(2) 26-2-0 to 3 L=1.60 plate grip E 7-10; Pr=20.0 p Lumber DOL=1.15 ain surcharge app d snow loads have as been designed has been	ave been considered for this design ph Vasd=95mph; TCDL=6.0psf; l prior(2) -0-10-8 to 2-1-8, Interior(1) 30-4-15, Interior(1) 30-4-15 to 36-1 DOL=1.60 sf (roof live load: Lumber DOL=1. 5 Plate DOL=1.15); Category II; E: blied to all exposed surfaces with s e been considered for this design. If or greater of min roof live load of e loads. b prevent water ponding. If or a 10.0 psf bottom chord live live ad for a live load of 20.0psf on the ord and any other members. truss connections. tion (by others) of truss to bearing -Tie connectors recommended to and does not consider lateral force ion does not depict the size or the	n. 3CDL=6.0psf; h=30ft; Cat. II; E 2-1-8 to 10-2-0, Exterior(2) 10 0-4 zone; C-C for members and 15 Plate DOL=1.15); Pg=15.0 kp B; Partially Exp.; Ct=1.10, L slopes less than 0.500/12 in ac f 12.0 psf or 1.00 times flat roc coad nonconcurrent with any ot bottom chord in all areas whe plate capable of withstanding connect truss to bearing walls as. e orientation of the purlin along	Exp B; Enclosed; N D-2-0 to 14-4-15, In d forces & MWFRS psf (ground snow) u=50-0-0; Min. fla cordance with IBC of load of 11.6 psf her live loads. re a rectangle 3-6 100 lb uplift at join due to UPLIFT at the top and/or bo	IWFRS (envelop nterior(1) 14-4-1 S for reactions sl roof snow load C 1608.3.4. on overhangs -0 tall by 2-0-0 w ht(s) 8. jt(s) 2. This ttom chord.	be) 5 to hown; at	SEAL 036322	

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AMITek Affilia 818 Soundside Road Edenton, NC 27932



Job	Truss	Truss Type	Qty	Ply	180 BIRCHWOOD GROVE - ROOF	
						153559259
33067-33067A	HG1	HIP GIRDER	1	2		
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,			8.610 s Ju	1 18 2022 MiTek Industries, Inc. Tue Aug 9 09:24:40 2022	Page 2
		ID:W1	7blGclgg2	2WNY Tb	Txie4yHe28-viKzIhyDQir0Q77ei23VHZ oUyHoD9aod0AwJ	VypdDL

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-43, 4-9=-53, 9-12=-43, 2-11=-20

Concentrated Loads (lb)

Vert: 4=-63(B) 9=-63(B) 20=-33(B) 14=-33(B) 23=-77(B) 24=-76(B) 25=-58(B) 27=-58(B) 28=-58(B) 29=-58(B) 30=-58(B) 31=-58(B) 32=-58(B) 33=-58(B) 36=-58(B) 37=-76(B) 38=-77(B) 40=-69(B) 41=-38(B) 42=-33(B) 43=-33(B) 44=-33(B) 45=-33(B) 46=-33(B) 47=-33(B) 48=-33(B) 49=-33(B) 50=-33(B) 51=-33(B) 52=-38(B) 53=-69(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Job	Truss	Truss Type	Qty	Ply	180 BIRCHWOOD GROVE - ROOF	
						153559260
33067-33067A	HG2	HIP GIRDER	1	2		
				_	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,			8.610 s Ju	18 2022 MiTek Industries, Inc. Tue Aug 9 09:24:48 2022	Page 2
		ID:W17bl0	Gelaa2WN	Y TbTxie4	yHe28-gFp Q2EX9stNMkBAkCNcFJAoAz85nVzSG6Mb2	2ypdDD

13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-43, 4-9=-53, 9-11=-43, 2-11=-20

Concentrated Loads (lb)

Vert: 4=-63(F) 6=-58(F) 19=-33(F) 8=-58(F) 15=-33(F) 22=-77(F) 23=-76(F) 24=-58(F) 26=-58(F) 27=-58(F) 28=-58(F) 29=-58(F) 30=-58(F) 31=-58(F) 33=-58(F) 34=-67(F) 35=-80(F) 36=-69(F) 37=-38(F) 38=-33(F) 40=-33(F) 41=-33(F) 42=-33(F) 43=-33(F) 44=-33(F) 45=-33(F) 46=-33(F) 46=-33(F) 48=-33(F) 49=-33(F) 50=-39(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.3SLIDERLeft 2x4 SP No.3 3-3-2

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=135(LC 14)

Max Uplift 4=-102(LC 14) Max Grav 4=165(LC 26), 2=269(LC 2), 5=106(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=102.



Structural wood sheathing directly applied or 5-4-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Plate Offsets (X,Y) [2	:0-3-15,0-0-6], [4:0-3-12,Edge]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCI I 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 * Rep Stress Incr YES	CSI. TC 0.77 BC 0.48 WB 0.00	DEFL. in Vert(LL) 0.04 Vert(CT) -0.08 Horz(CT) 0.08	(loc) 2-6 2-6 5	l/defl >999 >837 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 24 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP N	o.3 *Except*	BRACIN TOP CH	G- ORD Structur	al wood s	sheathing	directly ap	plied or 5-4-8 oc purlin	s, except

 TOP CHORD
 2x4 SP No.3 *Except*
 TOP CHORD
 Structural wood sheathing directly applied or 5-4-8 oc purlins, except

 4-5: 2x6 SP No.2
 2-0-0 oc purlins: 4-5.
 2-0-0 oc purlins: 4-5.

 BOT CHORD
 2x4 SP No.3
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

 SLIDER
 Left 2x4 SP No.3 2-4-5
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=Mechanical, 2=0-3-8, 6=Mechanical Max Horz 2=102(LC 14) Max Uplift 5=-47(LC 14), 2=-10(LC 14)

Max Grav 5=146(LC 2), 2=269(LC 2), 6=95(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-10-8, Exterior(2) 3-10-8 to 5-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This
 connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



TRENGINEERING BY A MITEK Affiliate 818 Soundside Road Edenton, NC 27932

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		2-4-8	3-0-0			
Plate Offsets (X,Y) [2:0-5-8,E	dge], [3:0-4-8,0-2-8]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.77 BC 0.23 WB 0.00 Matrix-P	DEFL. in (loc Vert(LL) -0.01 2-4 Vert(CT) -0.03 2-4 Horz(CT) 0.08 -4	:) I/defl L/d 5 >999 240 5 >999 180 4 n/a n/a	PLATES MT20 Weight: 24 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.3			BRACING- TOP CHORD Structural woo	od sheathing directly applie	ed or 5-4-8 oc purlir	is, except

BOT CHORD 2x6 SP No.2 WEDGE Left: 2x4 SP No.3

2-0-0 oc purlins: 3-4. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=71(LC 35) Max Uplift 4=-59(LC 7), 2=-29(LC 10) Max Grav 4=144(LC 2), 2=321(LC 2), 5=131(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs
- non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 10) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-3=-43, 3-4=-53, 2-5=-20

Continued on page 2

ᄊ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-74/3 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





1-h	Taura	Taura Tura	04.	DI		
JOD	Truss	Truss Type	Qty	PIY	180 BIRCHWOOD GROVE - ROOF	
					15355	9263
			-		10000	0200
33067-33067A	J3	JACK-OPEN GIRDER	3	1		
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,			8.610 s Ju	18 2022 MiTek Industries, Inc. Tue Aug 9 09:24:50 2022 Page	2
	ID:W17blGclgq2WNY_TbTxie4yHe28-cdxlO64U3m6bdguZl9FrhgPUP_nGZjuGwabSgwypdDB					

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 6=-44(F) 7=-11(F) 8=-16(F) 9=-13(F)

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			3-6-0					
Plate Offsets (X,Y) [2:0-3-8,	Edge]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.28 BC 0.21 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.01 2-4 -0.02 2-4 -0.00 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 14 lb	GRIP 244/190 FT = 20%
LUMBER-		BE	ACING-					

3-6-0

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=70(LC 16) Max Uplift 3=-51(LC 16), 2=-15(LC 16) Max Grav 3=89(LC 2), 2=202(LC 2), 4=66(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 3-6-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road Edenton, NC 27932



	1		2-2-0		3-6-0	
	ſ		2-2-0		1-4-0	
Plate Offsets (X,Y) [2:0-3-8,	Edge], [3:0-3-0,0-2-0]					
LOADING (psf)	SPACING-	2-0-0	CSI	DEEL	in (loc) l/defl l/d	PLATES GRIP

TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLL BCDL	20.0 16.5/15.0 10.0 0.0 * 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.18 WB 0.00 Matrix-P	Vert(LL) Vert(CT) Horz(CT)	-0.01 -0.01 -0.01 0.01	(IOC) 2-5 2-5 4	>999 >999 >999 n/a	L/d 240 180 n/a	Weight: 13 lb	244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x4 SP No.3 2x4 SP No.3		BI TC	R ACING- DP CHORD	Structura 2-0-0 oc	I wood purlins:	sheathin 3-4.	g directly ap	plied or 3-6-0 oc purlir	ns, except

2-0-0 oc purlins: 3-4. BOT CHORD Rigid ceiling directly applied or 10-0 oc bracing.

Left: 2x4 SP No.3

WEDGE

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=50(LC 16) Max Uplift 4=-25(LC 13), 2=-22(LC 16) Max Grav 4=87(LC 35), 2=235(LC 36), 5=61(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 11) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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<u> </u>	9-3-4 12-0-0 9-3-4 2-8-12	<u>18-2-0</u> 6-2-0	<u>24-4-0</u> 6-2-0	27-0-12	<u>36-4-0</u> 9-3-4	—
Plate Offsets (X,Y) [4:0-4-0,0	0-4-4], [6:0-4-0,0-4-4], [10:0-5-0,0-4-8	3], [11:0-5-0,0-4-8]				
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.50 BC 0.78 WB 0.31 Matrix-S	DEFL. ir Vert(LL) -0.32 Vert(CT) -0.47 Horz(CT) 0.07	n (loc) l/defl 2 10-11 >999 7 10-11 >928 7 8 n/a	L/d PLATES 240 MT20 180 n/a Weight: 231 lb	GRIP 244/190 FT = 20%
LUMBER-		I	BRACING-	ral wood sheathi	ng directly applied or 4-0-1 oc purlin	IS.

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-0-1 oc purlins Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-134(LC 21) Max Uplift 2=-93(LC 16), 8=-93(LC 17) Max Grav 2=1490(LC 2), 8=1490(LC 2)

2x4 SP No.2 *Except* 7-10,3-11: 2x4 SP No.3

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-2547/167, 3-5=-2280/193, 5-7=-2280/193, 7-8=-2547/168
- BOT CHORD 2-11=-183/2198, 10-11=0/1432, 8-10=-49/2198
- WEBS 5-10=-94/928, 7-10=-549/266, 5-11=-94/928, 3-11=-549/266

NOTES-

WEBS

BOT CHORD 2x6 SP No.2

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-7-14 to 2-4-2, Interior(1) 2-4-2 to 18-2-0, Exterior(2) 18-2-0 to 21-2-0, Interior(1) 21-2-0 to 36-11-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.

9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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F	9-4-4	12-0-0	18-2-0	24-4-0	26-11-12	36-1-0		
Plate Offsets (X,Y) [4	9-4-4 4:0-4-0.0-4-4]. [6:0-4-0.Edae	<u>2</u> 1. [8:0-4-0.0-4-4]. [1	6-2-0 11:0-5-0.0-4-8]. [14:0-5-0	0-2-0	2-7-12	9-1-4		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DC Lumber DOL * Code IRC20*	2-0-0 PL 1.15 1.15 cr YES 15/TPI2014	CSI. TC 0.64 BC 0.76 WB 0.75 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.20 2-14 -0.38 2-14 0.07 10	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 248 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BRACING- TOP CHORD BOT CHORD 2x6 SP No.2 TOP CHORD 2x6 SP No.3 *Except* 7-11,5-14: 2x4 SP No.2 Structural wood sheathing directly applied or 3-6-10 oc purlins. BOT CHORD								
REACTIONS. (size) 2=0-3-8, 10=Mechanical Max Horz 2=138(LC 16) Max Uplift 2=-94(LC 16), 10=-79(LC 17) Max Grav 2=1484(LC 2), 10=1434(LC 2)								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2508/157, 3-5=-2272/176, 7-9=-2257/183, 9-10=-2487/160 BOT CHORD 2-14=-176/2158, 11-14=-11/1625, 10-11=-48/2133 WEBS 7-11=-74/822, 9-11=-538/266, 5-14=-75/846, 3-14=-562/264, 5-7=-1465/155								
NOTES- 1) Unbalanced roof live I 2) Wind: ASCE 7-10; Vu gable end zone and C 36-0-4 zone;C-C for m 3) TCLL: ASCE 7-10; Pro- roof snow: Lumber DC	loads have been considered It=120mph Vasd=95mph; T()-C Exterior(2) -0-7-14 to 2-4 nembers and forces & MW2 =20.0 psf (roof live load: Lur DL=1.15 Plate DOL=1.15); C	for this design. CDL=6.0psf; BCDL: -2, Interior(1) 2-4-2 RS for reactions sh nber DOL=1.15 Pla ategory II; Exp B; I	=6.0psf; h=30ft; Cat. II; 2 to 18-2-0, Exterior(2) 1 own; Lumber DOL=1.60 tie DOL=1.15); Pg=15.0 Partially Exp.; Ct=1.10	Exp B; Enclosed; M 8-2-0 to 20-11-3, Ir) plate grip DOL=1.) psf (ground snow);	WFRS (envelop terior(1) 20-11- 50 Pf=11.6 psf (fla	pe) 3 to at		

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Refer to girder(s) for truss to truss connections.

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

11) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.





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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 8-11-14, Exterior(2) 8-11-14 to 11-11-14, Interior(1) 11-11-14 to 17-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



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- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- WEBS 4-6=-259/144, 2-8=-259/144

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 7-5-14, Exterior(2) 7-5-14 to 10-5-14, Interior(1) 10-5-14 to 14-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 5-11-14, Exterior(2) 5-11-14 to 8-11-14, Interior(1) 8-11-14

to 11-5-15 zone;C-C for members and forces & MWFRS for reactions shown; Lunber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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OTHERS 2x4 SP No.3 **REACTIONS.** (size) 1=8-11-12, 3=8-11-12, 4=8-11-12

Max Horz 1=-56(LC 10) Max Uplift 1=-26(LC 14), 3=-33(LC 15)

Max Grav 1=172(LC 2), 3=172(LC 2), 4=297(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-5-14, Exterior(2) 4-5-14 to 7-5-14, Interior(1) 7-5-14 to 8-5-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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BCDL

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD WEBS 2x4 SP No.3

10.0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-11-12 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 1=5-11-0, 3=5-11-0, 4=5-11-0 (size) Max Horz 1=35(LC 11) Max Uplift 1=-16(LC 14), 3=-21(LC 15) Max Grav 1=107(LC 2), 3=107(LC 2), 4=186(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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