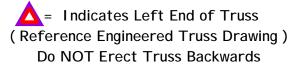


		Product	ts					Truss Placeme
PlotID	Length	Product		Plies	Net Qty	Fab Type	(1	Scale: 1/4"=1'
BM1	4' 0"	1-3/4"x 16" LVL Kerto-	S	2	2	FF		~
BM2	7' 0"	1-3/4"x 9-1/4" LVL Ker	to-S	2	4	FF		All Walls Sh
GDH	20' 0"	1-3/4"x 18" LVL Kerto-	S	2	2	FF	C	onsidered Lo
		Produc	cts					
PlotID	Lengt	h Product	Plie	es N	let Qty	Fab Type		Dimensior
BM3	8' 0"	2x12 SP No.2	2	2	2	FF]	1. All exterior wall to wa
							-	face of sheathing unles 2. All interior wall dimer

All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

TI	ROC RUS eilly R Fayet Phon	OF & SES load Ir teville	& FL & B ndustr , N.C. 0) 864	OOF EAN ial Par 28309 -8787	₹ ∕IS k
deemed requirem attached requirem size and reactions 15000#. retained reaction Tables.	reactions to comply hents. The Tables (hents) to number of s greater A register that exce A register	e less that y with the e contract derived f determin of wood s than 3000 red desig the sup eeds thos ed design the sup	n or equa e prescrip tor shall rom the p e the mir studs req D# but no n profess port syste e specific n profess port syste	I to 3000# trive Code refer to th orescription uired to s t greater to sional sha em for any ed in the a ional shal em for all	e ve Code Indation upport than II be y attachec
Signatur			<mark>d La</mark> d La	a <mark>ndr</mark> ndry	<u>y</u>
LOA		ART FO	DR JA	CK STU	IDS
			REQUISE: /GIRDER BG SCP SA AN (S) 0 1 1 2 3 3 0 4 0 5	S B EA ENE	00 1 00 2 00 3 00 4
CITY / CO. Harnett Co. / Harnett	204 Solomon Drive	Roof	DATE REV. 08/10/22	DRAWN BY David Landry	SALES REP. Marshall Naylor
СІ ТҮ / СО.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
Benjamin Stout Real Estate	JOB NAME Lot 10 Liberty Meadows	Cypress / 2GRF, CP	N/A		J0822-4070
BUILDER	JOB NAME	PLAN	SEAL DATE N/A	QUOTE #	JOB #
These t compor design See ind identifie designe perman for the	russes ar nents to b at the spe ividual de ed on the er is respe ent bracin overall st	e designe e incorpo ecification esign she placemen onsible fo ng of the ructure.	ed as ind prated int n of the b ets for ea nt drawin or tempor roof and The desig	GRAM ON ividual bu to the buil puilding de ach truss g. The bu ary and floor syst n of the ti rs, beams,	ilding ding esigner design ilding tem and russ



for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the <u>truss delivery package or online @ sbcindustry.com</u>



RE: J0822-4070 Lot 10 Liberty Meadows Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Custome Lot/Block

Customer: Benjamin Stout Real Estate Lot/Block: 10	Project Name: J0822-4070 Model: Cypress
Address: 204 Solomon Drive	Subdivision: Liberty Meadows
City:	State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.3 Wind Speed: 130 mph Floor Load: N/A psf

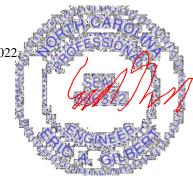
This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E16497598	A1	12/23/2021
2	E16497599	A1GE	12/23/2021
3	E16497600	A2	12/23/2021
4	E16497601	A3	12/23/2021
5	E16497602	B1	12/23/2021
6	E16497603	B1GE	12/23/2021
7	E16497604	M1	12/23/2021
8	E16497605	M2	12/23/2021
9	E16497606	V1GE	12/23/2021
10	E16497607	V2GE	12/23/2021
11	E16497608	V3	12/23/2021
12	E16497609	V4	12/23/2021
13	E16497610	V5	12/23/2021
14	E16497611	V6	12/23/2021
15	E16497612	V7	12/23/2021

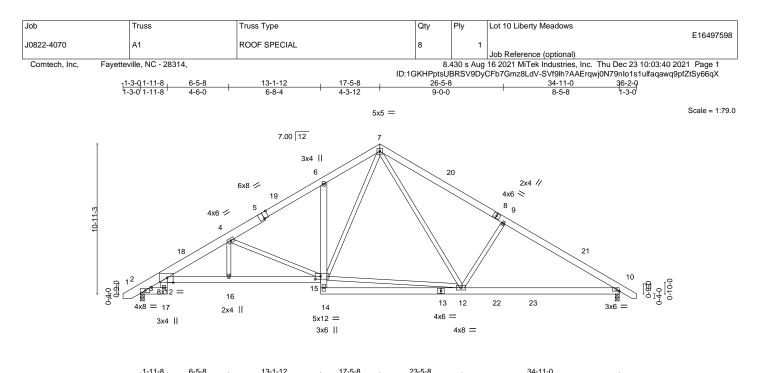
The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

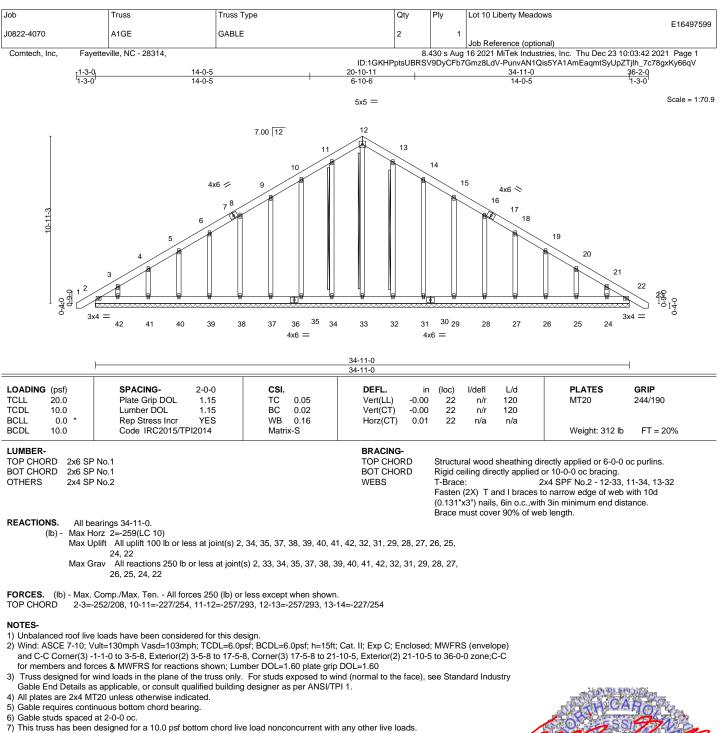


	1-11-8		13-1-12		17-5-8	23-5-8		_		34-11-0			
Plate Offsets (X,Y)	1-11-8	4-6-0 5:0-4-0,Edge], [15:0-4-	6-8-4	•	4-3-12	6-0-0				11-5-8			
	[3.0-3-4,∟uge], [3	.0-4-0,∟ugej, [13.0-4-	12,0-2-0]										
LOADING (psf)	SPACING	- 2-0-0	CSI.			DEFL.	in	(loc)	l/defl	L/d	PLA	TES	GRIP
TCLL 20.0	Plate Grip	DOL 1.15	тс	0.46		Vert(LL) -	0.14	1Ò-1Ź	>999	360	MT2	20	244/190
TCDL 10.0	Lumber D		BC	0.57		Vert(CT) -	0.29	10-12	>999	240			
BCLL 0.0 *	Rep Stress	s Incr YES	WB	0.97		Horz(CT)	0.16	10	n/a	n/a			
BCDL 10.0	Code IRC	2015/TPI2014	Matrix	k-S		Wind(LL)	0.09	16	>999	240	Wei	ght: 281 lb	FT = 20%
LUMBER-						BRACING-							
	No.1 *Except*					TOP CHORD		Structu	ral wood	sheathing o	directly applie	ed or 5-0-6 o	oc purlins
	3 SP 2400F 2.0E					BOT CHORD					d or 10-0-0 or		
BOT CHORD 2x6 SP						201 0110112		i ligita e	oning and			o braoing.	
WEBS 2x4 SP	No.2												
	e) 2=0-3-8, 10=												
	orz 2=-259(LC 10												
	olift 2=-93(LC 12)												
Max Gi	rav 2=1450(LC 1), 10=1459(LC 1)											
FORCES. (Ib) - Max. (Comp /Max Ten	- All forces 250 (lb) o	r lass avcant	when s	hown								
		89/491, 4-6=-1959/43				33/492							
	-2149/442		2,07-1021	,000,1	0- 100	<i>bor</i> 102,							
		6=-329/2548, 6-15=-2	54/197. 12-14	=-2/353	3. 10-1	2=-250/1758							
		5=-10/881, 7-15=-228/											
	,	,		,									
NOTES-													
1) Unbalanced roof live													
2) Wind: ASCE 7-10; V													
		Interior(1) 3-5-9 to 17-					1-10-	5 to 36-	0-0 zone	;C-C			
		or reactions shown; Lu											
This truss has been of												24431 10.80	LTR Show
4) * This truss has been					all area	as where a rectangle	e 3-6-	-0 tall b	/ 2-0-0 w	ide	35	100	1. 40
		any other members, w						(.)	_		- SAFA	18-04	DO VEA
5) Provide mechanical (STON 1	2 dest	12 Martin
 This truss is designed referenced standard 		with the 2015 Internat	unal Residen		ie sect	10115 K502.11.1 and	1 K80	2.10.28	DIN			OL STATE	
relefenceu standard	ANGI/TELT.									4		10	
											2	SEA	









8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 34, 35, 37, 38, 39, 40, 41, 42, 32, 31, 29, 28, 27, 26, 25, 24, 22.

10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

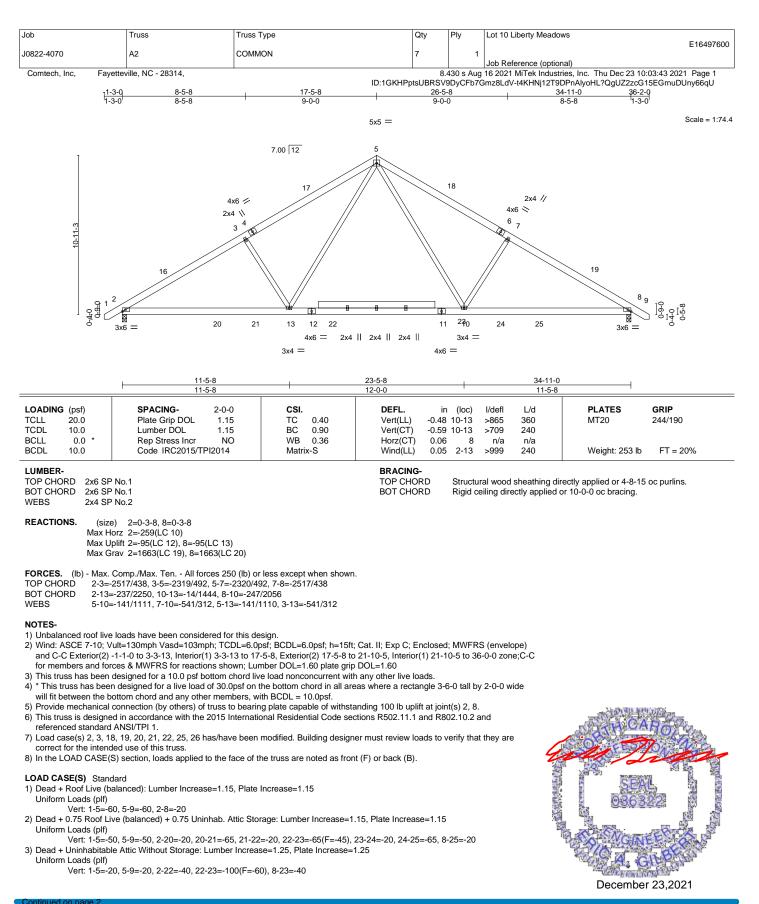
11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



December 23,2021



🗥 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange, delivery, erection and bracing of trusses and truss even and/or chord members only. Additional building design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is to here only upon parameters and properly incorporate this design is the overall building designer must verify the applicability of design parameters and properly incorporate this design is the overall building designer must verify the applicability of design parameters and properly incorporate this design is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings systems, see **ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Job	Truss	Truss Type	Qty	Ply	Lot 10 Liberty Meadows
					E16497600
J0822-4070	A2	COMMON	7	1	
					Job Reference (optional)
Comtech, Inc, Fa	etteville, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Thu Dec 23 10:03:43 2021 Page 2

ID:1GKHPptsUBRSV9DyCFb7Gmz8LdV-t4KHNj12T9DPnAlyoHL?QgUZ2zcG15EGrnuDUny66qU

LOAD CASE(S) Standard

 Dead + Uninhabitable Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-20, 5-9=-20, 2-20=-20, 20-21=-80, 21-22=-20, 22-23=-80(F=-60), 23-24=-20, 24-25=-80, 8-25=-20

19) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf) Vert: 1-2=-56, 2-5=-61, 5-8=-43, 8-9=-38, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20

Horz: 1-2=6, 2-5=11, 5-8=7, 8-9=12

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-38, 2-5=-43, 5-8=-61, 8-9=-56, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20 Horz: 1-2=-12, 2-5=-7, 5-8=-11, 8-9=-6

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-31, 2-5=-36, 5-8=-45, 8-9=-40, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20

Horz: 1-2=-19, 2-5=-14, 5-8=5, 8-9=10

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-40, 2-5=-45, 5-8=-36, 8-9=-31, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20

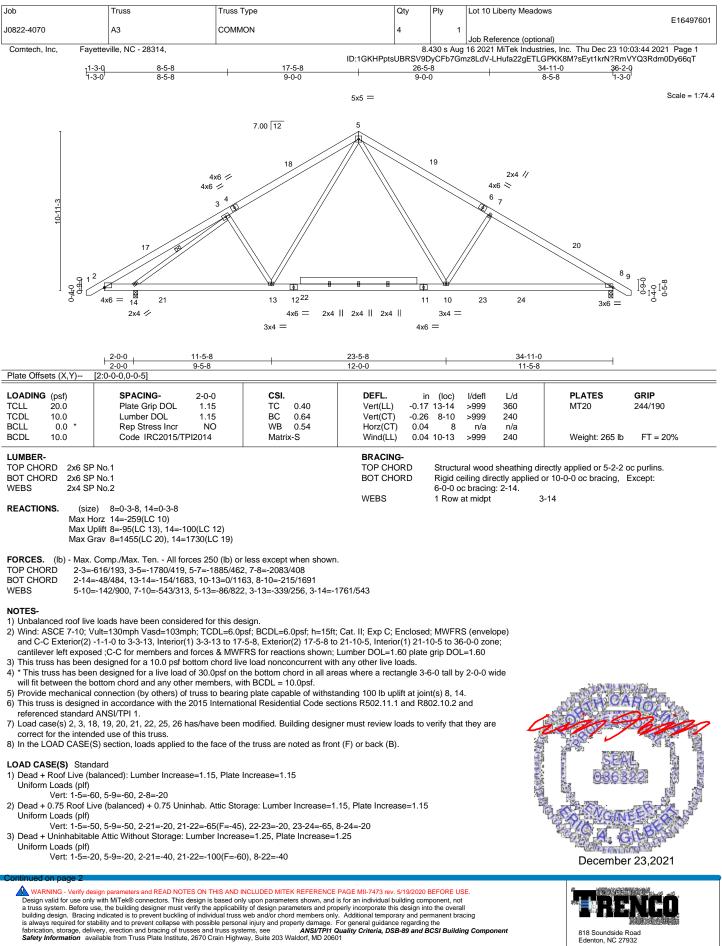
Horz: 1-2=-10, 2-5=-5, 5-8=14, 8-9=19 25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-50, 5-9=-20, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-20, 5-9=-50, 2-20=-20, 20-21=-65, 21-22=-20, 22-23=-65(F=-45), 23-24=-20, 24-25=-65, 8-25=-20





Job	Truss	Truss Type	Qty	Ply	Lot 10 Liberty Meadows
					E16497601
J0822-4070	A3	COMMON	4	1	
					Job Reference (optional)
Comtech, Inc,	Fayetteville, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Thu Dec 23 10:03:45 2021 Page 2

ID:1GKHPptsUBRSV9DyCFb7Gmz8LdV-pTS2oO3J?nT71UvLwiNTV5ZvamKfVyoZI5NKYfy66qS

LOAD CASE(S) Standard

- 18) Dead + Uninhabitable Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-5=-20, 5-9=-20, 2-21=-20, 21-22=-80(F=-60), 22-23=-20, 23-24=-80, 8-24=-20

19) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-56, 2-5=-61, 5-8=-43, 8-9=-38, 2-14=-3, 14-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

Horz: 1-2=6, 2-5=11, 5-8=7, 8-9=12 20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-38, 2-5=-43, 5-8=-61, 8-9=-56, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

- Horz: 1-2=-12, 2-5=-7, 5-8=-11, 8-9=-6
- 21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-31, 2-5=-36, 5-8=-45, 8-9=-40, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

- Horz: 1-2=-19, 2-5=-14, 5-8=5, 8-9=10
- 22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-40, 2-5=-45, 5-8=-36, 8-9=-31, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20 Horz: 1-2=-10, 2-5=-5, 5-8=14, 8-9=19

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

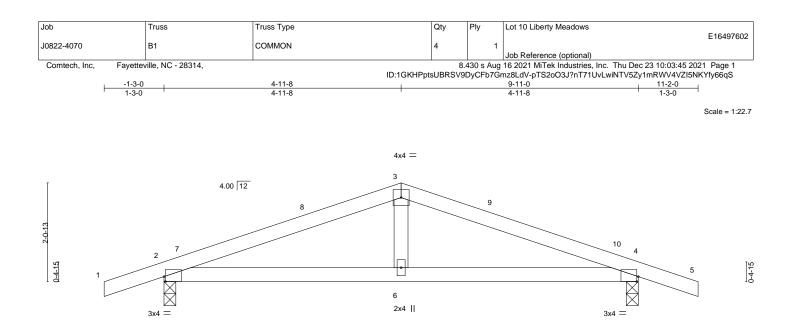
Vert: 1-5=-50, 5-9=-20, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-20, 5-9=-50, 2-21=-20, 21-22=-65(F=-45), 22-23=-20, 23-24=-65, 8-24=-20

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSETPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





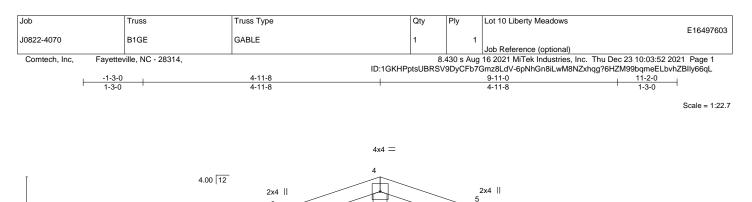
		11-8 11-8		<u>9-11-0</u> 4-11-8	
Plate Offsets (X,Y)	[2:0-0-6,Edge], [4:0-0-6,Edge]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.24 BC 0.20 WB 0.05 Matrix-S	DEFL. i Vert(LL) 0.04 Vert(CT) -0.05 Horz(CT) 0.07	3 2-6 >999 240	PLATES GRIP MT20 244/190 Weight: 37 lb FT = 20%
Max U	No.1 No.2	11	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di Rigid ceiling directly applied	rectly applied or 6-0-0 oc purlins. or 7-8-15 oc bracing.
FORCES. (lb) - Max. TOP CHORD 2-3=- BOT CHORD 2-6=-	Comp./Max. Ten All forces 250 (lb) o 654/750, 3-4=-654/750 624/567, 4-6=-624/567 293/227	r less except when shown.			
 Wind: ASCE 7-10; V and C-C Exterior(2) left and right expose This truss has been * This truss has been will fit between the b 	loads have been considered for this d ult=130mph Vasd=103mph; TCDL=6.0 -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 4- d;C-C for members and forces & MWF designed for a 10.0 psf bottom chord li n designed for a live load of 30.0psf on ottom chord and any other members. connection (by others) of truss to beari	psf; BCDL=6.0psf; h=15ft; 11-8, Exterior(2) 4-11-8 to RS for reactions shown; Lu /e load nonconcurrent with the bottom chord in all are	9-4-5, Interior(1) 9-4-5 umber DOL=1.60 plate of any other live loads. as where a rectangle 3-	to 11-2-0 zone; porch grip DOL=1.60 -6-0 tall by 2-0-0 wide	

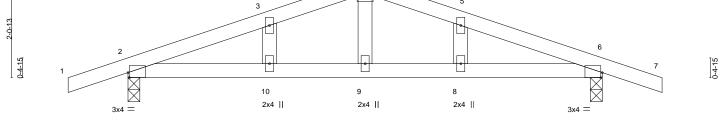
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb 2=191, 4=191.
6) This true is designed in accordance with the 2015 Interpretional Posidential Code sections RE02 11 1 and R802 10 2 and

6) This trues is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.









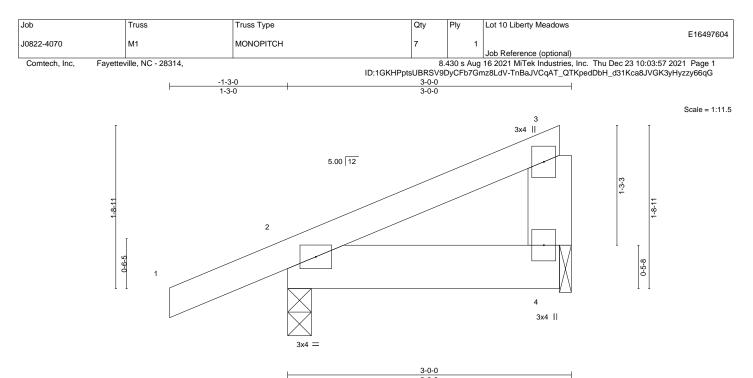
		-11-8 -11-8		<u>9-11-0</u> 4-11-8	
Plate Offsets (X,Y)	[2:0-0-6,Edge], [6:0-0-6,Edge]	-11-0		4-11-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.18 BC 0.23 WB 0.04 Matrix-S	DEFL. in (loc) Vert(LL) 0.04 8 Vert(CT) -0.04 10 Horz(CT) -0.01 6	8 >999 240 >999 240	PLATES GRIP MT20 244/190 Weight: 39 lb FT = 20%
LUMBER- TOP CHORD 2x4 SI 30T CHORD 2x4 SI WEBS 2x4 SI	P No.1			tural wood sheathing di ceiling directly applied	rectly applied or 6-0-0 oc purlins.
Max H Max U Max C FORCES. (Ib) - Max. TOP CHORD 2-3= 30T CHORD 2-10	ze) 2=0-3-0, 6=0-3-0 Horz 2=-42(LC 13) Jplift 2=-271(LC 8), 6=-271(LC 9) Grav 2=469(LC 1), 6=469(LC 1) . Comp./Max. Ten All forces 250 (lb) o =655/778, 3-4=-607/789, 4-5=-607/790, =-660/573, 9-10=-660/573, 8-9=-660/57 =-291/185	5-6=-655/778			
NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-10; 1 gable end zone and Lumber DOL=1.60 3) Truss designed for Gable End Details a 4) Gable studs spaced 5) This truss has been 6) * This truss has been 6) * This truss has been 6) * This truss has been	re loads have been considered for this d Vult=130mph Vasd=103mph; TCDL=6.0 d C-C Exterior(2) zone; porch left and rig plate grip DOL=1.60 wind loads in the plane of the truss only as applicable, or consult qualified buildin	psf; BCDL=6.0psf; h=15ft; ht exposed;C-C for memb For studs exposed to wi g designer as per ANSI/TF ve load nonconcurrent with the bottom chord in all are	ers and forces & MWFRS for re nd (normal to the face), see Sta Pl 1. any other live loads. as where a rectangle 3-6-0 tall	arctions shown; andard Industry by 2-0-0 wide	HCARO TO TO

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=271, 6=271.

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







							3-0-0					
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	ТС	0.09	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.02	Vert(CT)	-0.00	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-P	Wind(LL)	0.00	2-4	>999	240	Weight: 16 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=76(LC 12) Max Uplift 2=-98(LC 8), 4=-38(LC 8) Max Grav 2=210(LC 1), 4=84(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify
- capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.

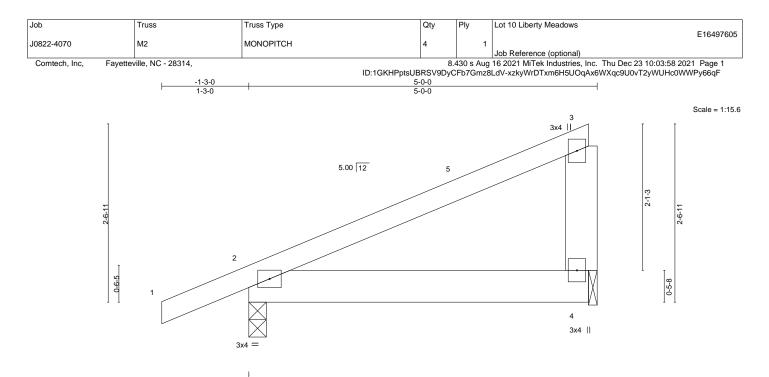


Structural wood sheathing directly applied or 3-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.26 BC 0.08 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.01 0.00 0.01	(loc) 2-4 2-4 2-4	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 26 lb	GRIP 244/190 FT = 20%
LUMBER-			BRACING-					_	

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=79(LC 12)

Max Uplift 2=-83(LC 8), 4=-57(LC 8) Max Grav 2=281(LC 1), 4=174(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

- capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.

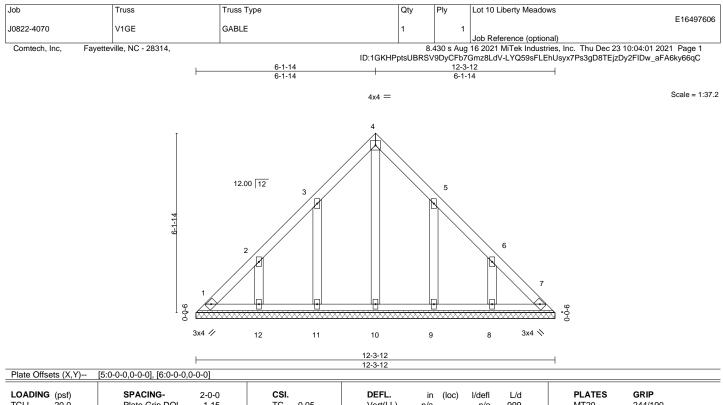


Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.07 Matrix-S	Horz(CT) 0.00 7 n/a n/a	Weight: 66 lb FT = 20%
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP MT20 244/190
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) n/a - n/a 999	
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) n/a - n/a 999	

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-3-12.

(lb) - Max Horz 1=-174(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 11=-143(LC 12), 12=-148(LC 12), 9=-142(LC 13), 8=-149(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 11, 12, 9, 8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

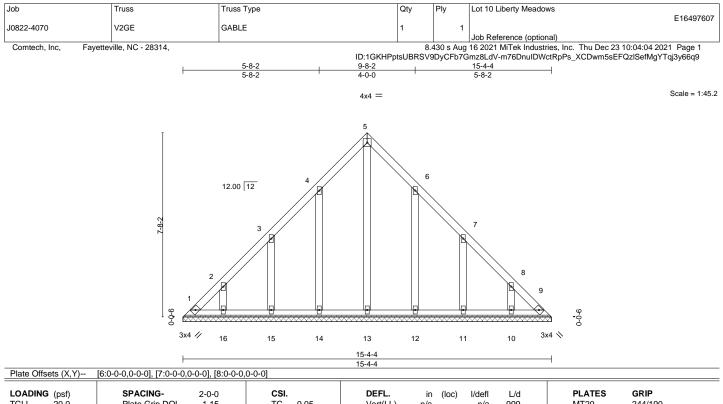
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 11=143, 12=148, 9=142, 8=149.

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.05 BC 0.03 WB 0.15 Matrix-S	DEFL. ir Vert(LL) n/z Vert(CT) n/z Horz(CT) 0.00	a - n/a 999 a - n/a 999	PLATES GRIP MT20 244/190 Weight: 92 lb FT = 20%
	SP No.1 SP No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di Rigid ceiling directly applied	rectly applied or 6-0-0 oc purlins. or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No 2

REACTIONS. All bearings 15-4-4

Max Horz 1=-220(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 9 except 14=-142(LC 12), 15=-143(LC 12), 16=-128(LC 12), 12=-140(LC 13), 11=-144(LC 13), 10=-128(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 16, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 1-2=-290/181, 8-9=-255/169 TOP CHORD

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

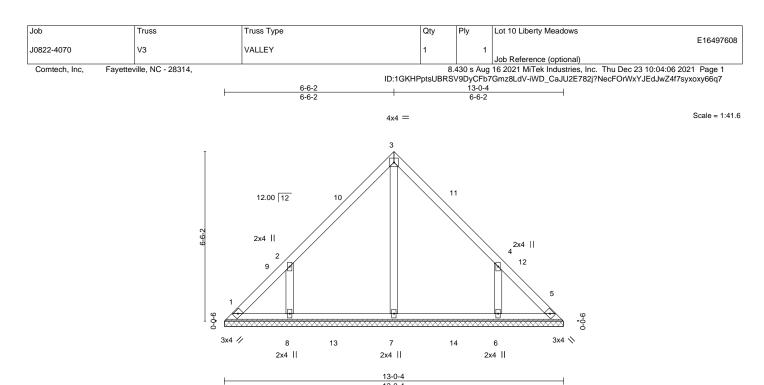
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9 except (jt=lb) 14=142, 15=143, 16=128, 12=140, 11=144, 10=128.

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🛕 WARNING - Verify design pa rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange, delivery, erection and bracing of trusses and truss even and/or chord members only. Additional building design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is to here only upon parameters and properly incorporate this design is the overall building designer must verify the applicability of design parameters and properly incorporate this design is the overall building designer must verify the applicability of design parameters and properly incorporate this design is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings systems, see **ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (lo	c) l/defl	L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.14	Vert(LL)	n/a	- n/a	999	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.15	Vert(CT)	n/a	- n/a	999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.09	Horz(CT)	0.00	5 n/a	n/a	
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 60 lb FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. All bearings 13-0-4.

(lb) - Max Horz 1=-148(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-163(LC 12), 6=-162(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=384(LC 19), 8=374(LC 19), 6=374(LC 20)

WEBS 2-8=-358/290, 4-6=-358/290

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-6-2, Exterior(2) 6-6-2 to 10-10-15, Interior(1) 10-10-15 to 12-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=163, 6=162.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



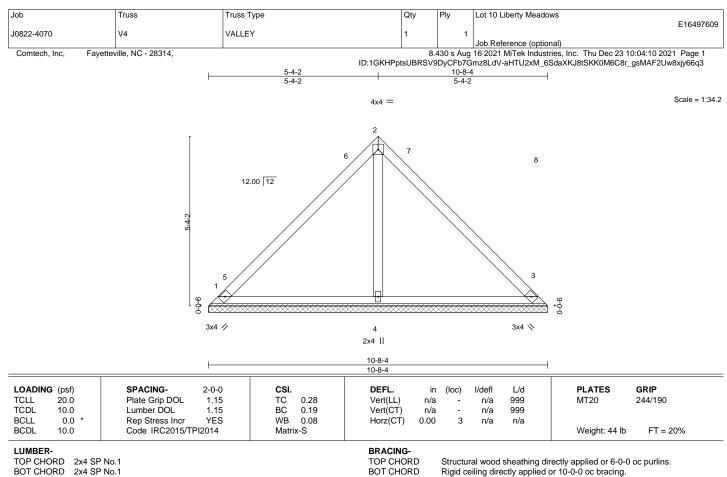
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oullapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=10-8-4, 3=10-8-4, 4=10-8-4

Max Horz 1=-120(LC 8)

Max Uplift 1=-30(LC 13), 3=-30(LC 13)

Max Grav 1=226(LC 1), 3=226(LC 1), 4=346(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

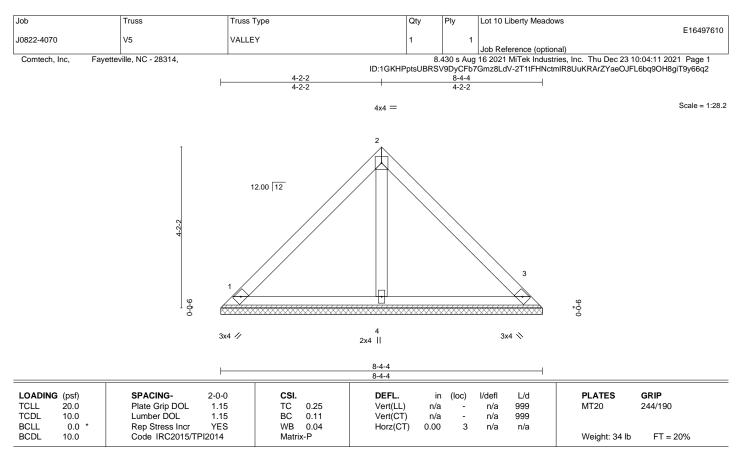
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-4-2, Exterior(2) 5-4-2 to 9-8-15, Interior(1) 9-8-15 to 10-4-0 zone;C-C for
- members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🛕 WARNING - Verify design pa rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSETPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=8-4-4, 3=8-4-4, 4=8-4-4

Max Horz 1=92(LC 9) Max Uplift 1=-33(LC 13), 3=-33(LC 13)

Max Grav 1=186(LC 1), 3=186(LC 1), 4=239(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0ps; BCDL=6.0ps; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

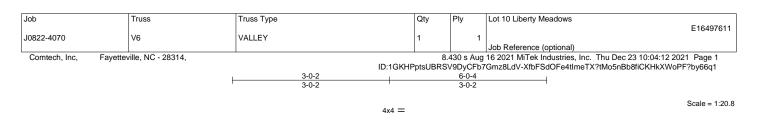


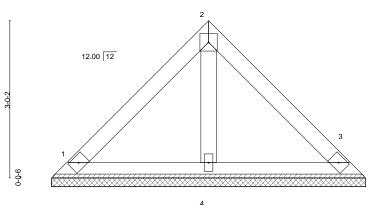
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛕 WARNING - Verify design pa rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSETPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601









2x4 || 6-0-4

	1		1			6-0-4					1	
LOADING (psf	f) S	PACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Ó P	late Grip DOL	1.15	тс	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	0 L	umber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0	0* R	ep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	0 C	ode IRC2015/TP	12014	Matri	<-P						Weight: 24 lb	FT = 20%
											-	
LUMBER-						BRACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=6-0-4, 3=6-0-4, 4=6-0-4

Max Horz 1=64(LC 9)

Max Uplift 1=-23(LC 13), 3=-23(LC 13)

Max Grav 1=129(LC 1), 3=130(LC 1), 4=166(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0ps; BCDL=6.0ps; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



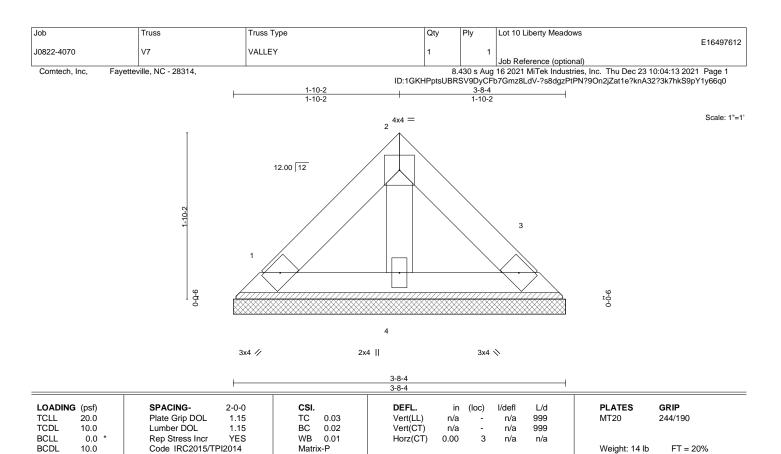
0-<u>0</u>-6

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

3x4 🔨





BRACING-

TOP CHORD

BOT CHORD

LUMBER-	

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=3-8-4, 3=3-8-4, 4=3-8-4

Max Horz 1=-36(LC 8) Max Uplift 1=-13(LC 13), 3=-13(LC 13)

Max Grav 1=73(LC 1), 3=73(LC 1), 4=93(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0ps; BCDL=6.0ps; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

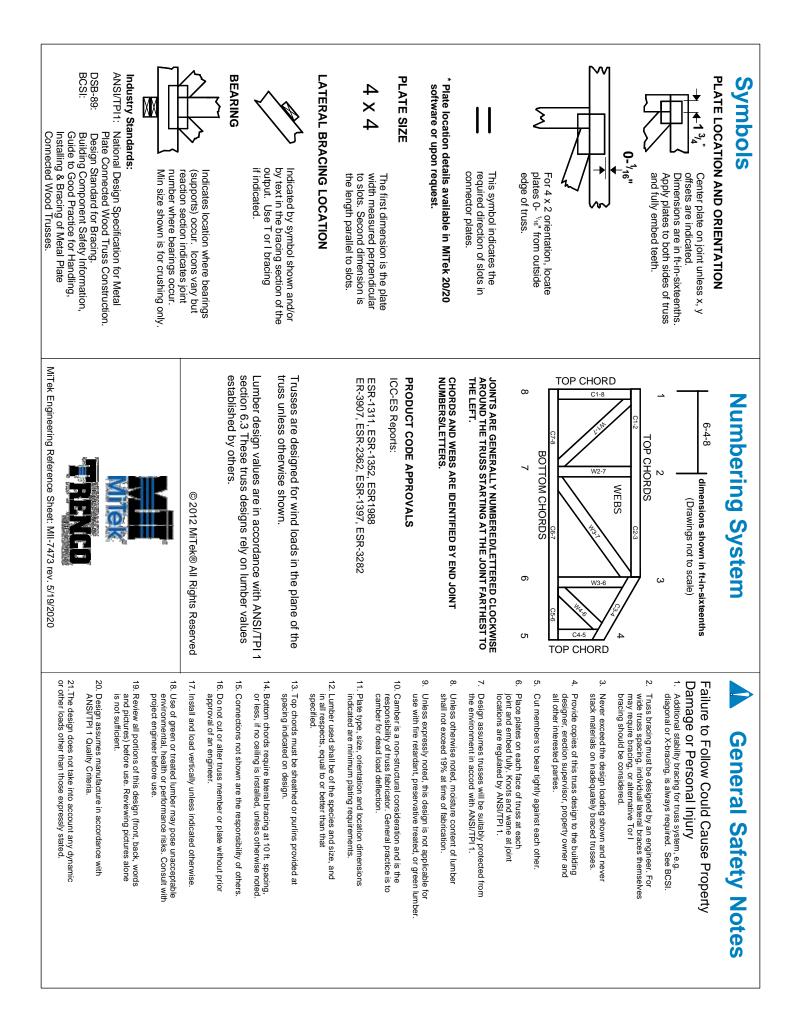
7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

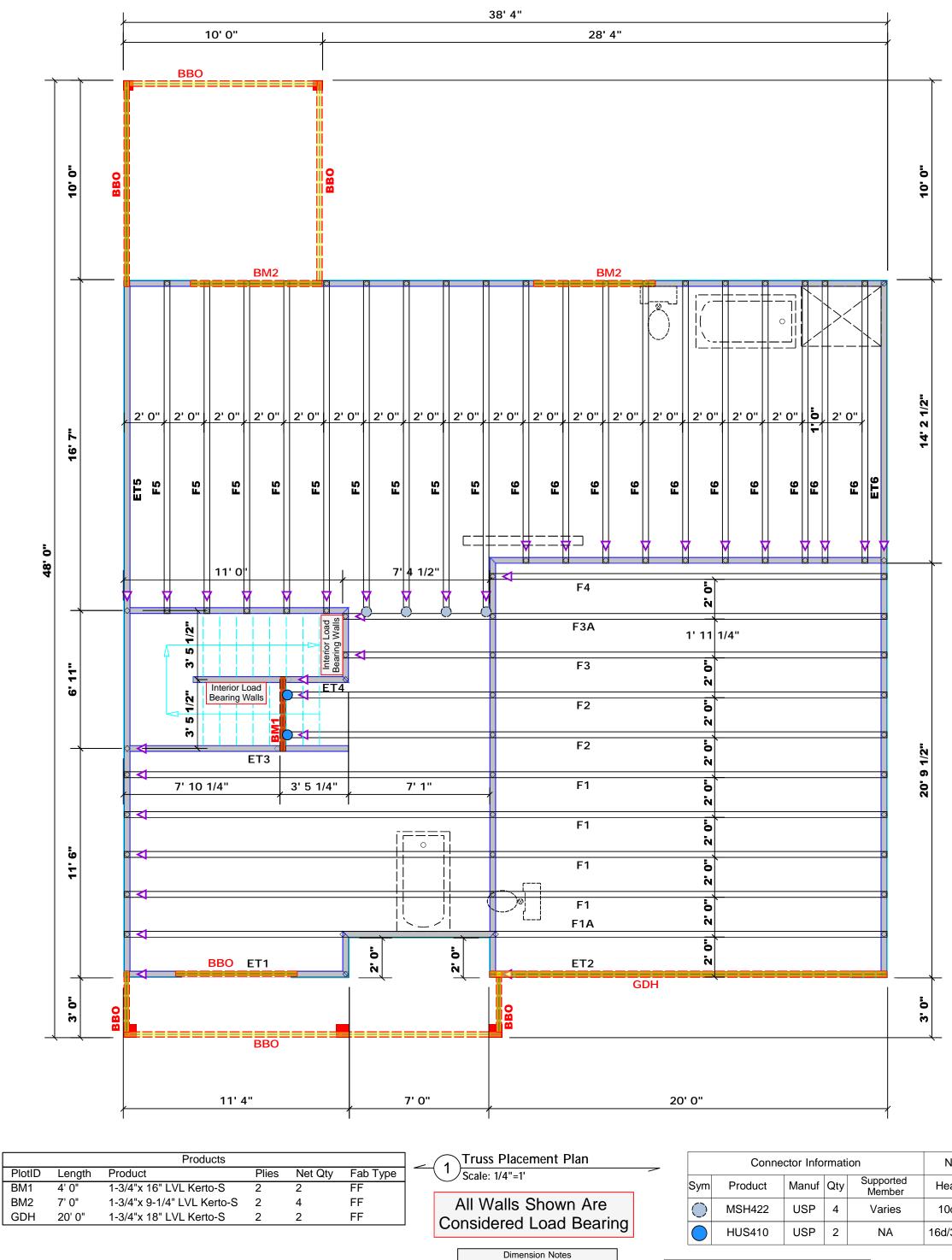


Structural wood sheathing directly applied or 3-8-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.







All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame well unless noted otherwise

face of frame wall unless noted otherwise

		Conne	Nail Information				
-	Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	\bigcirc	MSH422	USP	4	Varies	10d/3"	10d/3"
	\bigcirc	HUS410	USP	2	NA	16d/3-1/2"	16d/3-1/2"

Plumbing Drop Notes Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	TI	RUS eilly R Fayet Phon	OF 8 SES oad Ir teville	& B dustr , N.C.)) 864	OOF EAN ial Par 28309 -8787	1S k
	deemed requirem attached requirem size and reaction 15000#. retained reaction Tables.	to compl nents. The Tables (number s greater A register to design that exce A register to design s that exc	y with the e contract derived f determin of wood s than 3000 red design of the supp edd the supp edd the supp edd the supp edd to supp the supp edd to supp the supp	e prescrip for shall r rom the p e the min studs requ D# but no b# but no bort syste e specifie n profess port syste 0#.	andr	e re Code ndation upport han II be / ttached I be
	LOA	D CH			K STU	DS
	NUW		ON TABLE CK STUDS HEADER	REQUIRED) A (b)) @ EA END	OF
	NOLLO 10140) 1700 3400 5100 6800 8500 10200 11900 13600 15300	BOYSHAN(2) acysonus (25) 1 2 3 4 5 6 7 8 9	NOLLW W W (04-40) 25500 51000 76500 102000 127500	BONGHAN(S) 1 2 3 4 5	940 944 3400 6800 1020 1360	g g 0 1 0 2 00 3 00 4
	CITY / CO. Harnett Co. / Harnett	204 Solomon Drive	Floor	08/10/22	DRAWN BY David Landry	SALES REP. Marshall Naylor
	СІ ТҮ / СО.	ADDRESS	MODEL	DATE REV. 08/10/22	DRAWN BY	SALES REP.
	Benjamin Stout Real Estate	JOB NAME Lot 10 Liberty Meadows	Cypress / 2GRF, CP	N/A		J0822-4071
	BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #
ng)	These to comport design See ind identified designed for the support and col designed consult	russes ar nents to b at the spe ividual de ed on the er is respe ent bracin overall st t structur umns is t er. For ge BCSI-B1	e designe be incorpo ecification esign she placemen onsible fo ng of the ructure. T e includin the respon neral guid and BCS	ed as indi prated into n of the b ets for ea the drawing roof and 'ne desig ig header nsibility of dance reg I-B3 prov	GRAM ON vidual bu o the buil uilding de ch truss s g. The bui ary and floor syst n of the tui s, beams, of the buil arding br ided with sbcindus	ilding ding esigner. design ilding rem and russ walls, ding acing, the

▲ = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

1	-	Clie Pro		enjamin Sto Sypress	ut Real Esta	ate		Date: Input by:	8/10/202 David La	ndry				Page 1 of
	sDesign	Ado	dress: 2	04 Solom	on Drive					berty Meadow	s			
				10.00		-		Project #:	J0822-40 evel: Level_)71				
BM1	Kerto-S LV	L 1.7	50'' X	16.00	0" 2-	-Ply - P	ASSE	D ľ	evel. Level					
1 SPF	1	2 SPF												1'4"
	nformation						Reactio	ons UNF	ΔTTFRN	IED lb (Up	lift)			
Туре:	Girder	I	Applicatio	n: F	loor			irection	Live	Dead	-	Snow	Wind	Cor
Plies:	2		Design Me		SD		U U	ertical	345	140		0	0	00
Moisture Co	ndition: Dry		Building C		3C/IRC 201	5		ertical	345	140		0	0	
Deflection LI	L: 480		Load Sha	ring: N	lo									
Deflection T	L: 240		Deck:	N	lot Checked	l								
Importance:	Normal - II													
Temperature	e: Temp <= 100°I	F												
							Bearin	gs						
							Bearin	g Length	Dir.	Cap. React	D/L lb	Total Lo	d. Case	Ld. Com
							1 - SPI	= 3.500"	Vert	9% 14	0 / 345	485 L		D+L
							2 - SPI	= 3.500"	Vert	9% 14	0 / 345	485 L		D+L
Analysis R				a			٦							
Analysis		Location Allo		Capacity	Comb.	Case								
Moment		1'10 1/2" 345		0.010 (1%)		L								
Unbraced		1'10 1/2" 298		0.012 (1%)		L								
Shear	409 lb	2'1 1/2" 119		0.034 (3%)		L								
LL Defl incl	ר 0.001 (L/54618)	1'10 1/2" 0.0	83 (L/480)	0.009 (1%)	L	L								
TL Defl incl		1'10 1/2" 0.1	66 (L/240)	0.006 (1%)	D+L	L								
TE Dell'Illo	(L/38886)			0.000 (170)		-								
Design No	otes													
1 Provide s may also 2 Fasten al	upport to prevent latera be required at the inter plies using 3 rows of 1	ior bearings by	y the buildin	ng code.	-									
4 Girders a	i 6". ast page of calculations re designed to be supp be laterally braced at e	orted on the bo			oads.									
6 Bottom m	ust be laterally braced	at end bearing												
7 Lateral si	enderness ratio based Load Type	• • •		ib Width	Side	Dead 0.9	Live	- 1 Sno	w 1.15	Wind 1.6 C	onet 1	.25 Comm	nents	
1	Uniform	LUC			Near Face		184 F		0 PLF	0 PLF		PLF F2	101110	
T					NCAI FACE		10 4 F	L1	UILF	VILF	01			
	Self Weight					12 PLF								
	ed Designs is responsible only of y of this component based on nd loadings shown. It is	the 1. LVL beams i the 2. Refer to	must not be cut c manufacturer's		pond nation ulti-ply	flat roofs provide p ling	roper drainage	to prevent	Manufacture Metsä Wood 301 Merritt 7 Norwalk, CT (800) 622-58	Building, 2nd F 06851	loor	Comtech, Inc. 1001 S. Reilly Fayetteville, N USA 28314 910-864-TRUS	с	639
design criteria a responsibility of the	e customer and/or the contractor		installation re						18001 622-58	50				
design criteria a responsibility of the ensure the comp	 customer and/or the contractor onent suitability of the intend verify the dimensions and loads. 		installation re etails, beam stre	ength values, and	code				www.metsaw					

	1		Client:	Benjamin Stout Re	eal Estate	Dat	te:	8/10/2022	Page 2 of
1			Project:	Cypress			ut by:	David Landry	
	isDesign		Address:	204 Solomon [Drive		o Name		
							oject #:	J0822-4071	
BM1	Kerto-S L	.VL	1.750"	X 16.000"	2-Ply	- PASSED) '	Level: Level	
			-						
•	• •	•	-						MI 1
.	•	•	<1 1/2"						WW Lata
			$\overline{\Sigma}$						1'4"
	• •	• —	Т Т						Ш 🖵
1 SPF	-	2 SPF	1						
<i> </i>	3'9"	,	ł						3 1/2"
		,	-						
	3'9"		I						
Multi-Ply	y Analysis								
Fasten all	plies using 3 ro	ws of 10c	d Box nails	(.128x3") at 12"	o.c Maxim	um end distan	nce no	ot to exceed 6".	
Capacity	1 5	50.1 %							
Load		123.0 P							
Yield Limit pe Yield Limit pe		245.6 Pl 81.9 lb.	LF						
Yield Mode		IV							
Edge Distand		1 1/2"							
Min. End Dis Load Combir		3" D+L							
Duration Fac		1.00							
Notes		che	emicals			ovide proper drainage to p	prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Struct	tured Designs is responsible of lacy of this component based		lling & Installa		ponding		Γ	Metsä Wood 301 Merritt 7 Building, 2nd Eloor	Fayetteville, NC USA
design criteria responsibility of	and loadings shown. It the customer and/or the cont	is the 2 Ref ractor to reg	L beams must not be fer to manufactu arding installation	e cut or drilled urer's product information n requirements, multi-ply				301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
ensure the cor	mponent suitability of the to verify the dimensions and loa	intended fas ids. app	tening details, bear provals	n strength values, and code				(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service ce	conditions, unless noted otherwi	3. Dai 4. De:	maged Beams must sign assumes top eo	ge is laterally restrained					
	e treated with fire retardant or	0, FIG	ovide lateral suppor eral displacement an	t at bearing points to avoid d rotation	This design is	valid until 11/3/2024			соттесн
Version 21.90	417 Powered by iStruct	W Detects 220	61001.1						CONTRACTOR AND A DESCRIPTION

Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

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	-	Client:	Benjamin Stout Re	al Estate		Date:	8/10/202 David La				Page 3 of
Tis	Design	Project: Address:	Cypress 204 Solomon D	rive		Input by: Job Name:	David La Lot 10 Li	ndry berty Meadows			
-	-		201 00101101 2			Project #:	J0822-40	-			
3 M 2	Kerto-S LV	L 1.750'	" X 9.250"	2-Ply	- PAS	SED	evel: Level				
					3						
	2										
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1 SPF	End Grain		6'7"			2 SPF End G	Frain			<u> </u>	3 1/2"
ļ			6'7"							I	3 1/2
I			07				I				
	formation							IED lb (Uplif	-		
Type: Plies:	Girder 2	Applica	ation: Floor Method: ASD		3	Direction Vertical	Live 1109	Dead 2160	Snow 1369	Wind 0	Con
Moisture Con				C 2015		Vertical	1109	2160 2160	1369	0	
Deflection LL	.: 480		haring: No		-	· or it out		2.00	1000		
Deflection TL	.: 240	Deck:	Not Ch	ecked							
Importance:	Normal - II										
Temperature	Temp <= 100°F										
					Beari	ngs					
					Bear	ing Length	Dir.	Cap. React D/	L lb Tota	Ld. Case	Ld. Com
						PF 3.500"	Vert	39% 2160 / 1	859 4019	9 L	D+0.75(L
nalysis Re					End Graii	า					
Analysis K		ocation Allowed	Capacity Co	mb. Case		PF 3.500"	Vert	39% 2160 / 1	859 4019	θL	D+0.75(L
Moment		3'3 1/2" 14423 ft-lb	0.397 (40%) D+0		End	-					
Unbraced		3'3 1/2" 10451 ft-lb	0.548 (55%) D+(Grair	1					
Shear	2727 lb	1' 3/4" 7943 lb	0.343 (34%) D+(
LL Defl inch	0.048 (L/1526)	3'3 1/2" 0.153 (L/48	. ,	, ,							
		3'3 1/2" 0.306 (L/24									
esign No		, , , , , , , , , , , , , , , , , , ,	, , ,	· · /							
	Ipport to prevent lateral	movement and rotati	on at the end bearing	is Lateral sunno	rt l						
may also t 2 Fasten all	be required at the interio plies using 2 rows of 10	r bearings by the bui	lding code.	, II							
to exceed 3 Refer to la	6". Ist page of calculations f	or fasteners required	for specified loads								
	e designed to be suppor										
•	must be supported equa										
	be laterally braced at en ust be laterally braced at	0									
	nderness ratio based or	-									
ID	Load Type	Location	Trib Width Side	e Dead 0	.9 L	ive 1 Snov	v 1.15	Wind 1.6 Con	st. 1.25 Co	omments	
1	Uniform		Тор	113 PI	LF 337	' PLF	0 PLF	0 PLF	0 PLF F5	5	
2	Uniform		Тор	416 PI	LF () PLF 41	6 PLF	0 PLF	0 PLF A2	2	
3	Uniform		Тор	120 PI			0 PLF	0 PLF	0 PLF W		
-	Self Weight		100	7 PI					2.2		
Notes		chemicals	ion	 For flat roofs provid ponding 	de proper draina	ge to prevent	Manufacture		Comtec 1001 S.	Reilly Road, Suite	#639
structural adequacy	d Designs is responsible only of the of this component based on the	1. LVL beams must not be	cut or drilled			:		Building, 2nd Floor		ville, NC	
responsibility of the	nd loadings shown. It is the customer and/or the contractor to ment suitability of the intended	 Refer to manufacture regarding installation 	rer's product information requirements, multi-ply			1	Norwalk, CT (800) 622-58	06851	28314 910-864	1-TRUS	
application, and to ve	erify the dimensions and loads.	fastening details, beam approvals 3. Damaged Beams must r	strength values, and code					/ood.com/us			
Lumber 1. Dry service condi	itions, unless noted otherwise ated with fire retardant or corrosive	 Design assumes top edg Provide lateral support 	ge is laterally restrained at bearing points to avoid							COMT	OCT I

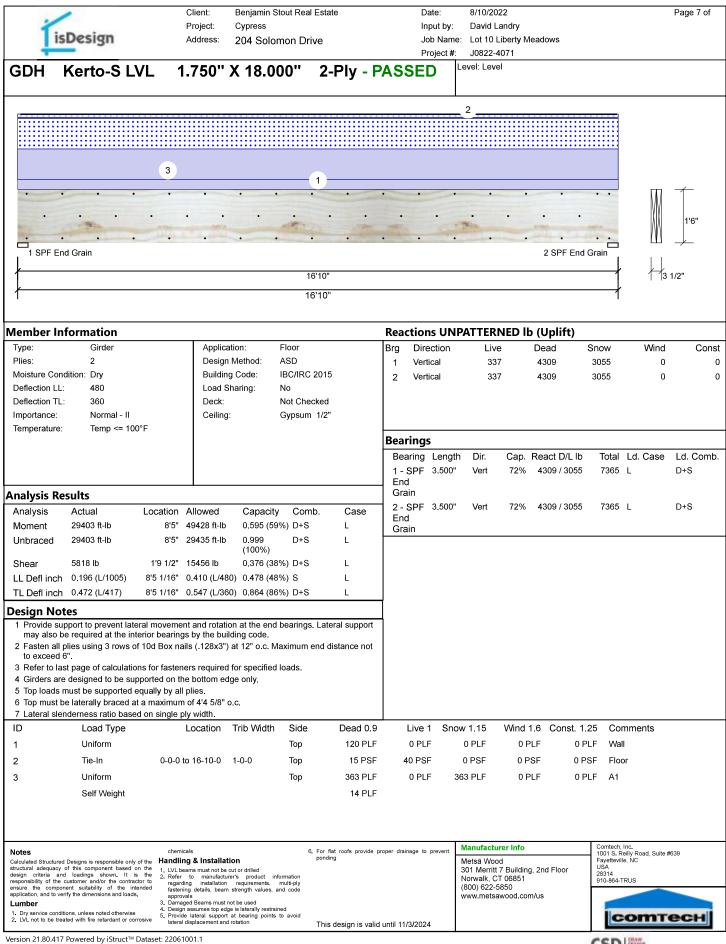
1	1		Client:	Benjamin Stout Re	eal Estate	Date:	8/10/2022	Page 4 of
Ť	inDesign		Project:	Cypress		Input by:		
1	isDesign		Address:	204 Solomon [Drive	Job Nam Project #	he: Lot 10 Liberty Meadows : J0822-4071	
	Karta C I V		4 750		2 01.7		Level: Level	
BM2	Kerto-S L	VL	1./50	X 9.250	2-Piy	- PASSED		
•	•	•		•	•	•	•	$\Lambda \Lambda \Lambda = 1$
							•	9 1/4
•	•	•		•	•	•	• <u> </u>	
	PF End Grain					2 SPF End	Grain	
				6'7"				1 13 1/2"
1				6'7''			1	
Multi-Ply	/ Analysis							
Fasten all	plies using 2 rows	s of 10d E	Box nails	(.128x3") at 12"	o.c Maximu	im end distance n	ot to exceed 6".	
Capacity Load		0.0 % 0.0 PLF						
Yield Limit pe	er Foot	163.7 PLF						
Yield Limit pe	er Fastener	81.9 lb.						
Yield Mode Edge Distand	ce	IV 1 1/2"						
Min. End Dist		3"						
Load Combin Duration Fac		1.00						
Notes		chemic	als		6. For flat roofs provid	de proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Struct	tured Designs is responsible only o acy of this component based on	f the Handlin	g & Installat		ponding	- preparation of prevent	Metsä Wood 301 Merritt 7 Building, 2nd Eleer	Fayetteville, NC USA
design criteria responsibility of	and loadings shown. It is the customer and/or the contractor	the 2 Refer or to regardi	ng installation	er's product information requirements, multi-ply			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (200) 622 5850	28314 910-864-TRUS
application, and to	mponent suitability of the inter to verify the dimensions and loads.	nded fasteni approv	ng details, beam	strength values, and code			(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service co 2. IVI. not to be	onditions, unless noted otherwise a treated with fire retardant or corro	 Design Provide 	assumes top edg alateral support	e is laterally restrained at bearing points to avoid				соттесн
	417 Powered by iStruct™ D	latora	displacement and	rotation	This design is v	alid until 11/3/2024		
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is	Design		Client: Project: Address:	Cypress	tout Real Estate non Drive			Date: Input by: Job Name: Project #:	8/10/20 David L Lot 10 L J0822-4	andry .iberty M	eadows			Page 5 of
BM3	SP #2 2	2.000'	' X 12.	000"	2-Ply -	PASS	SED		evel: Leve					
			1										Π	4
I SPF E	• end Grain				2 SPF Er	e nd Grain]							11 1
 			5'6" 6'1"			1	7							;"
/lember Inf	formation						Reacti	ons UNP	ATTERI	NED IL	o (Uplift)			
Type: Plies: Moisture Conc Deflection LL: Deflection TL: Importance:	480 240 Normal - II		Applicat Design I Building Load Sh Deck:	/lethod: Code: aring:	Floor ASD IBC/IRC 2015 No Not Checked		1 \	Direction /ertical /ertical	Live ()	Dead 1265 1265	Snow 1265 1265	Wind 0 0	Cor
Temperature:	Temp <= 100°	F						ng Length PF 3.500"	Dir. Vert	Cap. 43%	React D/L lb 1265 / 1265	Total 2531	Ld. Case L	Ld. Com D+S
Analysis Real Analysis Moment Unbraced			Allowed 4548 ft-lb 4171 ft-lb	Capacity 0.723 (72 0.789 (79	%) D+S	Case L L		PF 3.500"	Vert	43%	1265 / 1265	2531	L	D+S
Shear LL Defl inch TL Defl inch	1508 lb 0.019 (L/3590) 0.038 (L/1795)		4528 lb 0.141 (L/480 0.281 (L/240		%) S	L L L								
 may also be Fasten all p to exceed 6 Refer to lass Girders are Top loads n Top must be Bottom must 	port to prevent later- e required at the inte lies using 2 rows of ". t page of calculation designed to be supp- nust be supported ec e laterally braced at o st be laterally braced	rior bearing 10d Box na s for fastend ported on th jually by all end bearing at end bea	s by the build ils (.128x3") a ers required f e bottom edg plies. Is. rings.	ing code. at 12" o.c. M or specified	aximum end dis									
<u>8 Lateral sien</u> ID 1	iderness ratio based Load Type Uniform			Trib Width	Side Top	Dead 0.9 416 PLF			v 1.15 6 PLF	Wind 1 0 P	1.6 Const. 1 'LF 0 F	.25 Co PLF A2	mments	
									Manufactu	rer Info		Comtech, 1001 S. F Fayettevil USA 28314 910-864-		639
														есн

-		Benjamin Stout Real Estate	Date:	8/10/2022	Page 6 of
Lipposign		Cypress	Input by:	David Landry	
isDesign	Address:	204 Solomon Drive		Lot 10 Liberty Meadows	
			Project #:	J0822-4071 evel: Level	
BM3 SP #2	2.000" X 12.0	000" 2-Ply - PAS	SED [
• •	• •	• • •] .		\overline{M} 1
			<1 1/2"		
		• • •	\overline{V}		11 1/4"
	• •]		
1 SPF End Grain		2 SPF End Grain	1		
l l	5'6"	1			// _{3"}
/ /	6'1"		1		
Multi-Ply Analysis					
	ows of 10d Box pails (128x3") at 12" o.c Maximum	and distance no	t to exceed 6"	
Capacity	0.0 %		end distance no	t to exceed o.	
Load	0.0 PLF				
Yield Limit per Foot Yield Limit per Fastener	202.6 PLF 101.3 lb.				
Yield Mode	IV				
Edge Distance	1 1/2"				
Min. End Distance Load Combination	3"				
Duration Factor	1.00				
				Manufacturer Info	Comtech, Inc. 1001 S. Bailly Road, Suite #639
			F		1001 S. Reilly Road, Suite #639 Fayetteville, NC USA
					28314 910-864-TRUS
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		This design is valid	until 11/3/2024		Connech



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	-	Client:	Benjamin Stout Re	al Estate	Date:	:	8/10/2022	Page 8 of
~		Project:	Cypress		Input	by:	David Landry	-
	isDesign	Address:	204 Solomon E	Drive	Job N	lame:	Lot 10 Liberty Meadows	
-					Proje	ect #:	J0822-4071	
GDH	Kerto-S LVI	_ 1.750''	X 18.000"	2-Plv -	PASSED	L	evel: Level	
				,				
•	• • •	• •	• •	• •	• •	•	• • •	$\overline{}$
								1'6"
	• •		• •	•				1'6"
-;	• • •	• •	• •	• •	• •	•	• • •	
	End Grain						2 SPF End	Grain //
/				16'10"				
				16'10"				I
Multi-Ply	Analysis							
Fasten all	plies using 3 rows o	of 10d Box nails	(.128x3") at 12"	o.c Maximu	um end distance	e no	t to exceed 6".	
Capacity		0.0 %						
Load Yield Limit pe		0.0 PLF 245.6 PLF						
Yield Limit pe		1.9 lb.						
Yield Mode		V						
Edge Distanc		1/2"						
Min. End Dist Load Combin		;" 						
Duration Fact		.00						
								1
Notes		chemicals	1	For flat roofs prov ponding	ide proper drainage to prev		Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
structural adequa	ured Designs is responsible only of the cy of this component based on the and loadings shown. It is the	e 1. LVL beams must not be	cut or drilled	r J			Metsä Wood 301 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA 28314
responsibility of t	and loadings shown. It is the he customer and/or the contractor to apponent suitability of the intende	o regarding installation	er's product information requirements, multi-ply strength values, and code			1	Norwalk, CT 06851 800) 622-5850	910-864-TRUS
	overify the dimensions and loads.	 fastening details, beam approvals Damaged Beams must r 					www.metsawood.com/us	
1. Dry service co	nditions, unless noted otherwise treated with fire retardant or corrosiv	 Design assumes top edg Provide lateral support 	e is laterally restrained at bearing points to avoid					соттесн
		lateral displacement and	rotation	This design is	valid until 11/3/2024			
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Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

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RE: J0822-4071 Lot 10 Liberty Meadow Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Customer: Lot/Block:

Customer: Benjamin Stout Real Estate Lot/Block: 10	Project Name: J0822-4071 Model: Cypress
Address: 204 Solomon Drive City:	Subdivision: Liberty Meadow State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.3 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 14 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E16497613	ET1	12/23/2021
2	E16497614	ET2	12/23/2021
3	E16497615	ET3	12/23/2021
4	E16497616	ET4	12/23/2021
5	E16497617	ET5	12/23/2021
6	E16497618	ET6	12/23/2021
7	E16497619	F1	12/23/2021
8	E16497620	F1A	12/23/2021
9	E16497621	F2	12/23/2021
10	E16497622	F3	12/23/2021
11	E16497623	F3A	12/23/2021
12	E16497624	F4	12/23/2021
13	E16497625	F5	12/23/2021
14	E16497626	F6	12/23/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

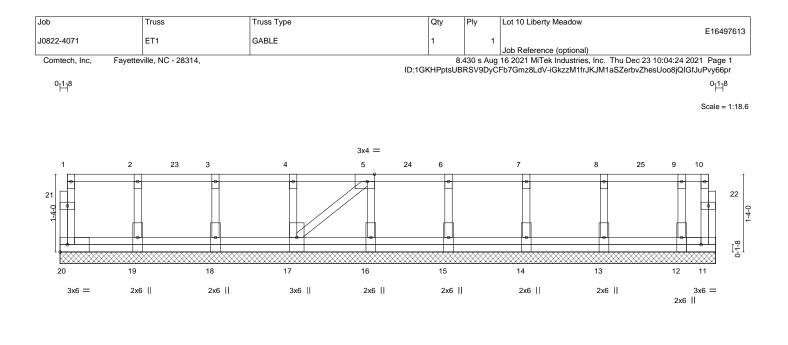
My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

December 23, 2021



L	1-4-0	2-8-0	4-0-0	5-4-0	6-8-0	8-0-0	9-4-0	10-8-0	11-3-0
	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	0-7-0
Plate	Offsets (X,Y) [5:0-1-8,Edge]			1				
LOAI TCLL TCDL BCLL BCDL	- 10.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/	2-0-0 1.00 1.00 YES TPI2014	CSI. TC 0.07 BC 0.00 WB 0.03 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl n/a - n/a n/a - n/a 0.00 17 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 69 lb	GRIP 244/190 FT = 20%F, 11%E
TOP BOT WEB	LUMBER- Control 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat)					except end ver	d sheathing directly a ticals. ectly applied or 6-0-i		oc purlins,

REACTIONS. All bearings 11-3-0. (lb) - Max Uplift All uplift 100 lb o

Max Uplift All uplift 100 lb or less at joint(s) 11
 Max Grav All reactions 250 lb or less at joint(s) 20, 19, 18, 17, 16, 15, 14, 13, 12

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11.

7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

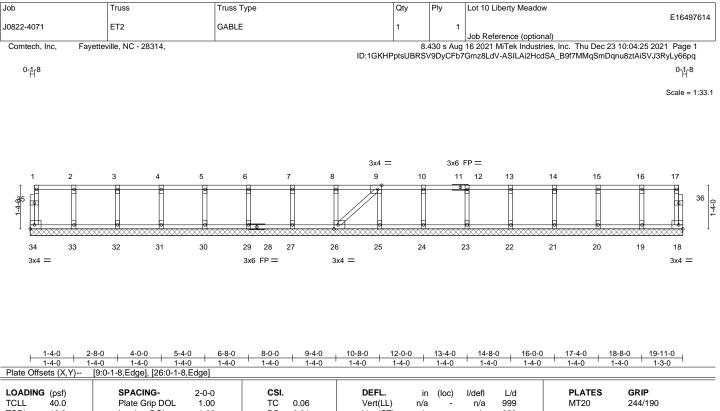
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 11-20=-10, 1-10=-100

Concentrated Loads (lb) Vert: 4=-26 7=-26 23=-26 24=-26 25=-26 December 23,2021





TCLL TCDL BCLL	40.0 10.0 0.0	Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	TC 0.06 BC 0.01 WB 0.03	Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	a - r	/a 999 /a 999 /a n/a	MT20	244/190
BCDL	5.0	Code IRC2015/TPI2014	Matrix-S	1.0.2(0.1)			Weight: 90 lb	FT = 20%F, 11%E
LUMBER	!-			BRACING-				
TOP CHO	ORD 2x4 SF	P No.1(flat)		TOP CHORD	Structural w	ood sheathing di	irectly applied or 6-0-0	oc purlins,
BOT CHO	ORD 2x4 SF	P No.1(flat)			except end	verticals.		
WEBS	2x4 SF	P No.3(flat)		BOT CHORD	Rigid ceiling	directly applied	or 10-0-0 oc bracing.	

2x4 SP No.3(flat) OTHERS

REACTIONS.

 All bearings 19-11-0.
 (b) - Max Grav All reactions 250 lb or less at joint(s) 34, 18, 33, 32, 31, 30, 29, 27, 26, 25, 24, 23, 22, 21, 20, 19

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

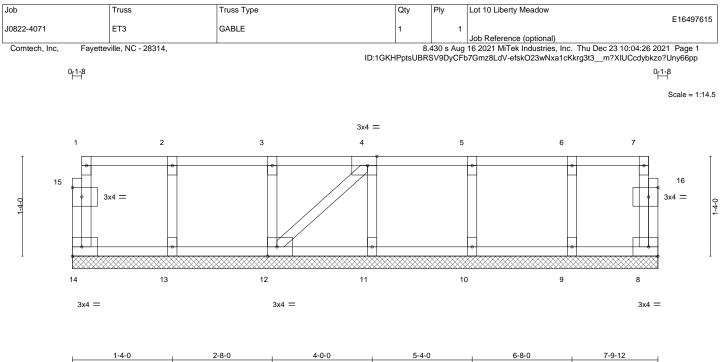
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.





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I	1-4-0	-4-0 1-4-0	1-4-	0 1-4-0	¹ 1-1-12	1
Plate Offsets (X,Y)	[4:0-1-8,Edge], [12:0-1-8,Edge],	[15:0-1-8,0-1-8], [16:0-1-8,0-1-4	8]			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	TC 0.06 BC 0.01	DEFL. ii Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	a - n/a 999		GRIP 444/190 FT = 20%F, 11%E
BOT CHORD 2x4 S WEBS 2x4 S	SP No.1(flat) SP No.1(flat) SP No.3(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing d except end verticals. Rigid ceiling directly applied	,	: purlins,

REACTIONS. All bearings 7-9-12.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 14, 8, 13, 12, 11, 10, 9

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

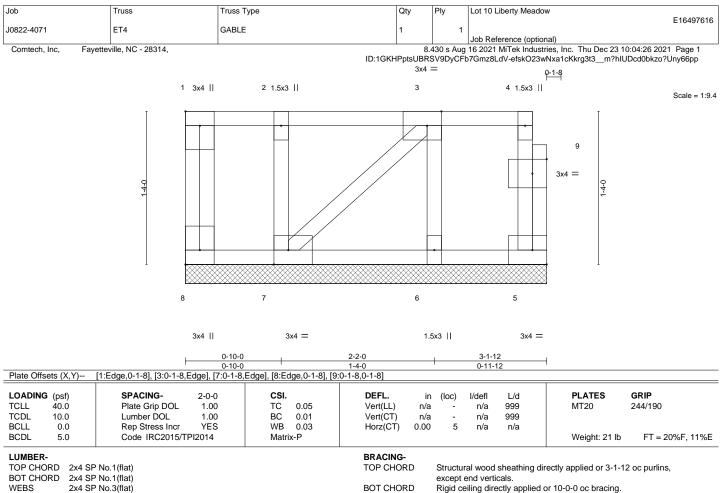
4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.







2x4 SP No.3(flat) WFBS 2x4 SP No.3(flat) OTHERS

REACTIONS. All bearings 3-1-12.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.





🛕 WARNING - Verify design pa rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSETPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



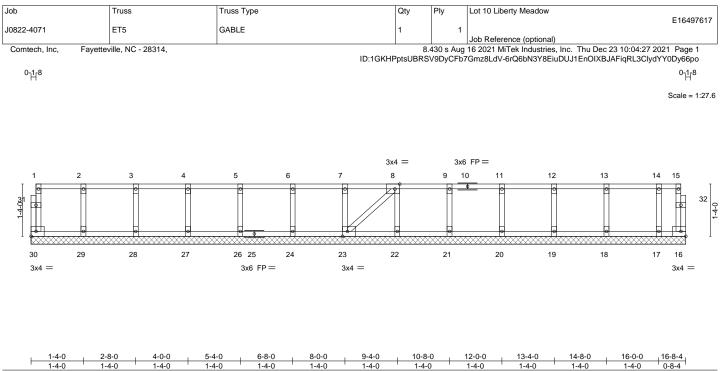


Plate Offsets (X,Y)	[8:0-1-8,Edge], [23:0-1-8,Edge]		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) n/a - n/a 999 MT20 244/190 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 16 n/a n/a Weight: 77 lb FT = 20%F, 1 FT = 20%F, 1 FT = 20%F, 1 FT = 20%F, 1
	P No.1(flat) P No.1(flat)	1	BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD WEBS 2x4 SP No.1(flat) 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat)

REACTIONS.

DNS. All bearings 16-8-4. (lb) - Max Grav All reactions 250 lb or less at joint(s) 30, 16, 29, 28, 27, 26, 24, 23, 22, 21, 20, 19, 18, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

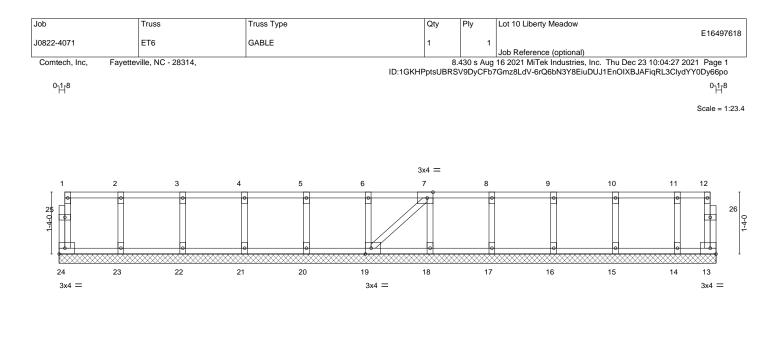
5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



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1	1-4-0	2-8-0	4-0-0	5-4-0	6-8-0	8-0-0	9-4-0	10-8-0	12-0-0	13-4-0) 14-2-0
F	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	0-10-0
Plate C	Offsets (X,Y)	[7:0-1-8,Edge],	[19:0-1-8,Edge]								
LOADI TCLL TCDL BCLL BCDL	ING (psf) 40.0 10.0 0.0 5.0	SPACIN Plate Gri Lumber Rep Stre Code IR	ip DOL 1.00 DOL 1.00) T) E	CSI. FC 0.06 3C 0.01 WB 0.03 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 13	l/defl L/d n/a 999 n/a 999 n/a n/a	MT	ATES 20	GRIP 244/190 FT = 20%F, 11%E
LUMB TOP C BOT C WEBS	HORD 2x4 S HORD 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat)		I		BRACING- TOP CHOP BOT CHOP	RD Structo exception	ural wood sheathin t end verticals. ceiling directly appl	5 <i>j</i> 11		oc purlins,

2x4 SP No.3(flat) OTHERS REACTIONS.

DNS. All bearings 14-2-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 24, 13, 23, 22, 21, 20, 19, 18, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

All plates are 1.5x3 MT20 unless otherwise indicated.
 Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

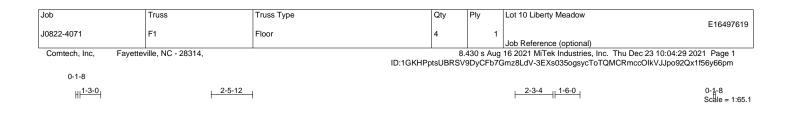
5) Gable studs spaced at 1-4-0 oc.

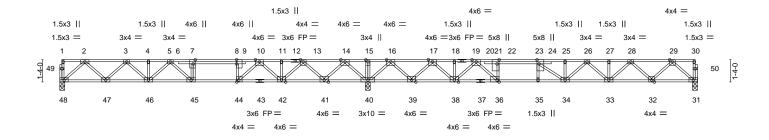
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



December 23,2021







	18-5-12 18-5-12				38-3-0 19-9-4		
Plate Offsets (X,Y)	[7:0-3-0,Edge], [8:0-3-0,0-0-0], [22:0-3-	0,Edge], [23:0-3-0,Edge],	[36:0-1-8,Edge], [44:0-	1-8,Edge]			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.96 BC 0.81 WB 0.71	Vert(LL) -0.2	in (loc) l/defl 27 34-35 >864 36 34-35 >655 36 31 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S				Weight: 211 lb	FT = 20%F, 11%
BOT CHORD 2x4 S	SP No.1(flat) SP No.1(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	except end ver	icals.	rectly applied or 2-2-0 or 6-0-0 oc bracing.	oc purlins,
8-1 15- 22- 28-	=-1560/0, 3-4=-2546/0, 4-5=-2546/0, 5-7= D=-2888/105, 10-11=-1673/754, 11-13=- 16=0/3309, 16-17=-64/1187, 17-18=-176 23=-3236/0, 23-25=-3338/0, 25-26=-3338 29=-1704/0 48=0/394 46-47=0/2157, 45-46=0/2787	673/754, 13-14=-128/140 7/545, 18-20=-1767/545, 2 8/0, 26-27=-2818/0, 27-28=	7, 14-15=0/3309, 20-22=-3258/0, =-2818/0,				
BOT CHORD 47- 41-	48=0/939, 46-47=0/2157, 45-46=0/2787, 42=-1069/990, 40-41=-1989/0, 39-40=-19	925/0, 38-39=-852/1004, 3	6-38=-294/2360,				
WEBS 2-4 13- 5-4 28 17-	36=0/3236, 34-35=0/3236, 33-34=0/3146 8=-1248/0, 2-47=0/864, 3-47=-830/0, 3-4 41=-1322/0, 13-42=0/1058, 10-42=-801/(5=-530/142, 7-45=-88/295, 8-44=-798/0, -33=0/619, 26-33=-445/18, 26-34=-20/26 39=-1403/0, 17-38=0/1136, 20-38=-907/0 34=-351/0, 23-34=-99/656	6=-14/529, 14-40=-1757/0), 10-44=0/1338, 5-46=-32 29-31=-1348/0, 29-32=0/9 2, 16-40=-1842/0, 16-39=0	, 14-41=0/1364, 8/67, 59, 28-32=-916/0, 0/1446,			1. Spotter con	antorn:
NOTES- 1) Unbalanced floor I	ive loads have been considered for this d MT20 unless otherwise indicated.	esign.				C CAR	Rolling States

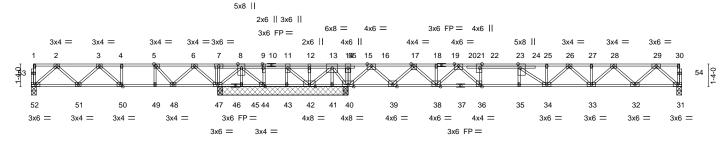
 Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.





Job		Truss	Truss Type	Qty		Ply	Lot 10 Liberty Meadow	
								E16497620
J0822-4071		F1A	Floor	1		1		
							Job Reference (optional)	
Comtech, Inc,	Fayette	/ille, NC - 28314,			8.4	30 s Aug	16 2021 MiTek Industries, Inc. Thu Dec 23	3 10:04:31 2021 Page 1
				ID:1GKHPptsUE	BRS	/9DyCFb	7Gmz8LdV-?cfdRI72CTCJi6doTcTEh1Th7	J0hHjGKtFWm9?y66pk
0-1-8								
1-3-0		1-10-0	<mark>1-2-8 1-2-8 1-2-8 1-2-8 1-2-8 -9-0</mark>				2-1-8	0-1-8 Scale: 3/16"=1'



	10-11-8	14-9-8 18-7-8			38-3-0				
	10-11-8	3-10-0 3-10-0			19-7-8				
Plate Offsets (X,Y)-	[5:0-1-8,Edge], [9:0-3-0,Edge], [22:0-3-	0,Edge], [23:0-3-0,Edge],	[36:0-1-8,Edge], [44:0-1	-8,Edge], [50:0-1-8	3,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.71 BC 0.75 WB 0.67 Matrix-S	Vert(LL) -0.23	n (loc) l/defl 3 34-35 >999 34-35 >746 4 31 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 221 lb	GRIP 244/190 FT = 20%F, 11%E		
BOT CHORD 2x4	l SP No.1(flat) I SP No.1(flat) I SP No.3(flat)	1	BRACING- TOP CHORD BOT CHORD	except end vertic	cals.	ectly applied or 6-0-0 c or 6-0-0 oc bracing.	oc purlins,		
REACTIONS. All bearings 7-8-0 except (jt=length) 52=0-3-8, 31=0-3-8. (lb) Max Uplift All uplift 100 lb or less at joint(s) except 41=-793(LC 4), 42=-419(LC 4), 43=-275(LC 4) Max Grav All reactions 250 lb or less at joint(s) 42, 43, 45 except 52=560(LC 3), 47=830(LC 3), 47=764(LC 1), 40=3094(LC 7), 40=3081(LC 1), 44=399(LC 7), 31=878(LC 4)									
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-901/0, 3-4=-1142/0, 4-5=-1768/0, 6-7=-96/503, 7-8=-106/487, 8-9=0/380, 9-11=0/380, 11-12=0/792, 12-13=0/792, 13-14=0/3016, 14-16=0/3018, 16-17=0/638, 17-18=-1159/0, 18-20=-1159/0, 20-22=-2752/0, 22-23=-2728/0, 23-25=-2972/0, 25-26=-2972/0, 26-27=-2570/0, 27-28=-2570/0, 28-29=-1579/0 BOT CHORD 51-52=0/591, 50-51=0/1157, 49-50=0/1142, 48-49=0/1142, 47-48=0/420, 45-47=-318/0,									
4- 3:	4-45=-318/0, 43-44=-380/0, 42-43=-380/0, 9-40=-1647/0, 38-39=0/350, 36-38=0/1799	41-42=-1769/0, 40-41=-17	769/0,						
WEBS 14 5- 2 1	32-33=0/2177, 31-32=0/949 WEBS 14-40=-299/0, 2-52=-784/0, 2-51=0/432, 3-51=-356/0, 6-47=-837/0, 6-48=0/551, 5-48=-571/0, 13-40=-1985/0, 13-41=0/771, 11-43=-60/286, 29-31=-1261/0, 29-32=0/876, 28-32=-833/0, 28-33=0/534, 26-33=-358/0, 16-40=-1830/0, 16-39=0/1403, 17-39=-1366/0, 17-38=0/1101, 20-38=-871/0, 20-36=0/1328, 22-36=-794/0,								
 NOTES- 1) Unbalanced floor live loads have been considered for this design. 2) All plates are 1.5x3 MT20 unless otherwise indicated. 3) Plates checked for a plus or minus 1 degree rotation about its center. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 793 lb uplift at joint 41, 419 lb uplift at joint 43. 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. 6) CAUTION, Do not erect truss backwards. 									
LOAD CASE(S) Standard 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 31-52=-10, 1-30=-100									

December 23,2021



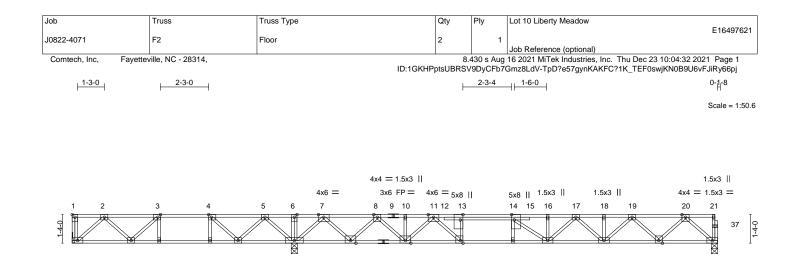
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Lot 10 Liberty Meadow			
J0822-4071	F1A	Floor	1	1	E16497620			
			•		Job Reference (optional)			
Comtech, Inc, Fayetteville, NC - 28314,				8.430 s Aug 16 2021 MiTek Industries, Inc. Thu Dec 23 10:04:31 2021 Page 2				
		ID:1GKHPptsUBRSV9DyCFb7Gmz8LdV-?cfdRI72CTCJi6doTcTEh1Th7J0hHjGKtFWm9?y66pk						

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 10=-69 12=-69 8=-69 55=-69





30	2	9	28	
4x6 =	3x6	FP	=	
			4x6	=

27

4x4 =

26

1.5x3 ||

25

3x6 =

24

3x6 =

23

4x4 =

22

3x6 =

29

30

Ě

31

3x10 =

33

1.5x3 ||

32

 	10-4-8			<u> </u>		
Plate Offsets (X,Y)		Edge], [13:0-3-0,Edge], [14:0-3-0,Edge], [27:0-1-			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.77 BC 0.85 WB 0.63 Matrix-S	Vert(LL) -0.29	n (loc) l/defl L/d 9 25-26 >821 480 9 25-26 >604 360 6 22 n/a n/a	PLATES MT20 Weight: 163 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 WEBS 2x4	SP No.1(flat) SP No.1(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin except end verticals. Rigid ceiling directly appl	g directly applied or 5-10-1 ied or 6-0-0 oc bracing.	oc purlins,
Max	size) 36=Mechanical, 31=0-3-8, 22=0-3- ; Uplift 36=-26(LC 4) ; Grav 36=490(LC 3), 31=1947(LC 1), 22=					
()	ux. Comp./Max. Ten All forces 250 (lb) o 3=-721/150, 3-4=-860/441, 4-5=-419/882,					

- 8-10=-2401/0, 10-11=-2401/0, 11-13=-3738/0, 13-14=-3719/0, 14-16=-3669/0, 16-17=-3669/0, 17-18=-3045/0, 18-19=-3045/0, 19-20=-1819/0
- 35-36=-18/514, 34-35=-441/860, 33-34=-441/860, 32-33=-441/860, 31-32=-1193/33. BOT CHORD 30-31=-576/0, 28-30=0/1695, 27-28=0/2937, 26-27=0/3719, 25-26=0/3719, 24-25=0/3432, 23-24=0/2533. 22-23=0/1075 WEBS 2-36=-685/24, 2-35=-183/287, 3-35=-189/395, 5-31=-990/0, 5-32=0/752, 4-32=-948/0, 4-33=0/301, 3-34=-264/0, 7-31=-1699/0, 7-30=0/1314, 8-30=-1280/0, 8-28=0/1005, 11-28=-774/0, 11-27=0/1261, 13-27=-758/0, 20-22=-1428/0, 20-23=0/1035,
 - 19-23=-993/0, 19-24=0/697, 17-24=-526/0, 17-25=0/322, 16-25=-250/79, 14-25=-439/328

NOTES-

36

3x6 =

35

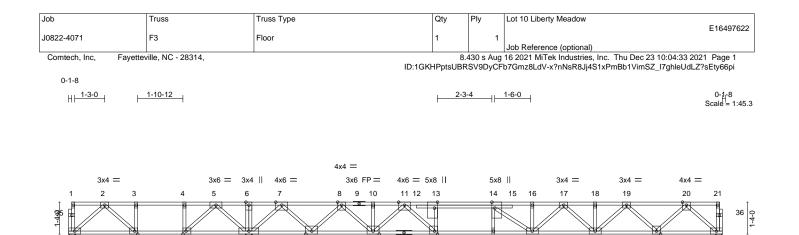
34

1.5x3 ||

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 36.
 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.
- Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.







29

4x6 =

28

3x8 M18AHS FP =

27

4x4 =

26

27-3-8

19-9-4

25

3x6 =

24

3x6 =

23

4x4 =

				19-9-4			
Plate Offsets (X,Y)	[13:0-3-0,Edge], [14:0-3-0,Edge], [27:0-	1-8,Edge], [32:0-1-8,Edge	ej, [33:0-1-8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.85 BC 0.85 WB 0.62 Matrix-S	Vert(LL) -0.2	in (loc) l/defl 29 25-26 >817 39 25-26 >598 36 22 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 149 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1 (flat) P No.1 (flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	except end ver	ticals.	ectly applied or 5-9-9 o r 6-0-0 oc bracing.	oc purlins,
	e) 34=0-3-8, 31=0-3-8, 22=0-3-8 lplift 34=-112(LC 4) krav 34=328(LC 3), 31=1806(LC 1), 22=	996(LC 7)					
TOP CHORD 2-3=- 8-10=	Comp./Max. Ten All forces 250 (lb) o -381/510, 3-4=-381/510, 4-5=-381/510, -2479/0, 10-11=-2479/0, 11-13=-3798/ /=-3711/0, 17-18=-3074/0, 18-19=-3074	5-6=0/1504, 6-7=0/1504, 0, 13-14=-3780/0, 14-16=	7-8=-907/0,				

 BOT CHORD
 33-34=-159/298, 32-33=-510/381, 31-32=-1003/24, 29-30=0/1785, 27-29=0/3008, 26-27=0/3780, 25-26=0/3780, 24-25=0/3468, 23-24=0/2554, 22-23=0/1082

 WEBS
 2-34=-393/211, 2-33=-476/113, 5-31=-838/0, 5-32=0/903, 4-32=-461/0, 7-31=-1673/0, 7-30=0/1302, 8-30=-1244/0, 8-29=0/966, 20-22=-1438/0, 20-23=0/1045, 19-23=-1002/0, 19-24=0/706, 17-24=-536/0, 17-25=0/330, 11-29=-743/0, 11-27=0/1229, 13-27=-738/0, 14-25=-493/278

 \boxtimes

31

3x10 =

30

4x6 =

NOTES-

Ň

34

3x6 =

33

3x4 =

7-6-4

76/

32

3x4 =

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 1.5x3 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 34.
 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.

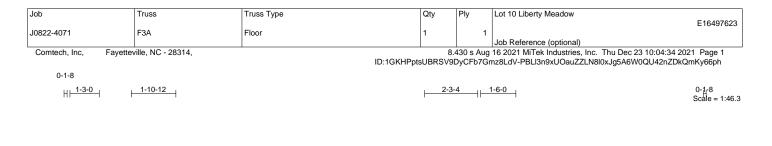


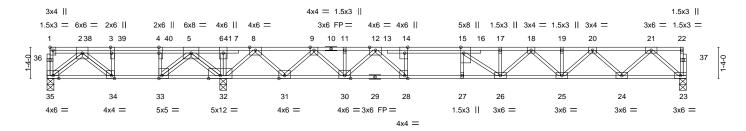
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22

3x6 =







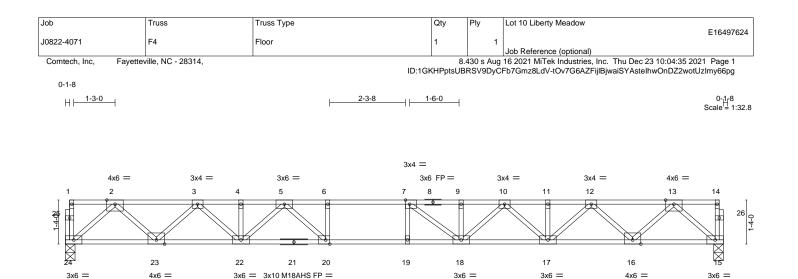
	7-6-4			27-3-8		
	7-6-4			19-9-4		
Plate Offsets (X,Y)	[1:Edge,0-1-8], [3:0-3-0,Edge], [4:0-3-0	,Edge], [14:0-3-0,Edge], [*	15:0-3-0,Edge], [28:0-1-8		:0-1-8,Edge]	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO	CSI. TC 0.79 BC 0.88 WB 0.66	Vert(LL) -0.25	n (loc) l/defl L/d 5 26-27 >928 480 5 26-27 >677 360 5 23 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 169 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing except end verticals. Rigid ceiling directly applie 6-0-0 oc bracing: 32-33,31	ed or 10-0-0 oc bracing,	• •
REACTIONS. (size	e) 35=0-3-8, 32=0-3-8, 23=0-3-8			3 • • • •,•	- ,	
Max G	Grav 35=1662(LC 3), 32=3831(LC 1), 23	B=915(LC 7)				
9-11= 17-18 BOT CHORD 34-36 28-30 WEBS 6-32= 3-34= 21-24	-2360/0, 3-4=-2360/0, 4-5=-2360/0, 5-6 =-1518/0, 11-12=-1518/0, 12-14=-3071/ 8=-3209/0, 18-19=-2730/0, 19-20=-2730 5=0/1750, 33-34=0/2360, 32-33=-583/8 0=0/2135, 27-28=0/3047, 26-27=0/3047 =-874/0, 2-35=-2265/0, 2-34=-2/810, 5-3 =-536/0, 8-32=-1779/0, 8-31=0/1395, 9- 4=0/929, 20-24=-886/0, 20-25=0/589, 1 8=0/1364, 14-28=-816/0, 15-26=-269/47	0, 14-15=-3047/0, 15-17= 1/0, 20-21=-1659/0 33, 31-32=-1290/0, 30-31: , 25-26=0/3034, 24-25=0/ 232=-2781/0, 5-33=0/2696, 31=-1342/0, 9-30=0/1070 3-25=-413/0, 17-26=-297/	-3209/0, =-107/736, '2296, 23-24=0/991 '4-33=-1680/0, , 21-23=-1317/0,			
 Plates checked for a Recommend 2x6 str Strongbacks to be a CAUTION, Do not e Hanger(s) or other c down at 3-2-4, and device(s) is the resp 	connection device(s) shall be provided s 878 lb down at 5-2-4, and 857 lb down	its center. oc and fastened to each tr strained by other means. ufficient to support concer at 7-2-4 on top chord. Ti	ntrated load(s) 878 lb don he design/selection of su	wn at 1-2-4, 878 lb uch connection	Contraction of the second seco	Martin Contraction
Uniform Loads (plf)	dard palanced): Lumber Increase=1.00, Plate =-10, 1-22=-100	Increase=1.00			SE 0861	

Concentrated Loads (lb) Vert: 38=-798(B) 39=-798(B) 40=-798(B) 41=-798(B)

A. GIL CONFERENCE PARTY December 23,2021







3x4 =

			19-11-0 19-11-0					
Plate Offsets (X,	Y) [7:0-1-8,Edge], [20:0-1-8,Edge]						T	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.99 BC 0.74 WB 0.55 Matrix-S		in (loc) -0.35 18-19 -0.47 18-19 0.07 15	l/defl >683 >498 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 105 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2 WEBS 2 REACTIONS.	2x4 SP No.1(flat) 2x4 SP No.1(flat) *Except* 15-21: 2x4 SP 2400F 2.0E(flat) 2x4 SP No.3(flat) (size) 24=0-3-8, 15=0-3-8 Max Grav 24=1075(LC 1), 15=1075(LC 1)		BRACING- TOP CHORI BOT CHORI				rectly applied, except of or 10-0-0 oc bracing.	end verticals.
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	Max. Comp./Max. Ten All forces 250 (lb) o 2-3=-2007/0, 3-4=-3409/0, 4-5=-3409/0, 5-6 9-10=-4232/0, 10-11=-3412/0, 11-12=-3412/ 23-24=0/1174, 22-23=0/2804, 20-22=0/3891 16-17=0/2808, 15-16=0/1172 2-24=-1560/0, 2-23=0/1159, 3-23=-1108/0, 3 6-20=-395/0, 13-15=-1558/0, 13-16=0/1159, 10-18=0/460. 9-18=-251/64. 7-18=-606/291	=-4323/0, 6-7=-4323/0, 7-5 0, 12-13=-2005/0 , 19-20=0/4323, 18-19=0/ -22=0/823, 5-22=-655/0, 1	9=-4232/0, 4323, 17-18=0/389 5-20=0/865,	,				
NOTES- 1) Unbalanced fl	oor live loads have been considered for this d	esign.						

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 1.5x3 MT20 unless otherwise indicated.

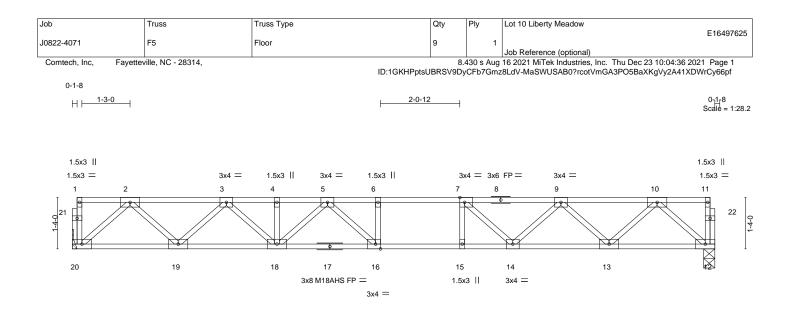
4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



December 23,2021





 			<u>16-8-4</u> 16-8-4					
Plate Offsets (X,Y)	[7:0-1-8,Edge], [16:0-1-8,Edge]		10-0-4					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.54 BC 0.91 WB 0.43 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.22 16-18 -0.30 16-18 0.05 12	l/defl >892 >666 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 87 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
LUMBER- TOP CHORD BRACING- 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.								
(ize) 20=Mechanical, 12=0-3-8 Grav 20=898(LC 1), 12=898(LC 1)							
TOP CHORD 2-3 9-1 BOT CHORD 19 12 WEBS 2-2	x. Comp./Max. Ten All forces 250 (lb) oi =-1619/0, 3-4=-2661/0, 4-5=-2661/0, 5-6= 0=-1624/0 :20=0/970, 18-19=0/2244, 16-18=0/2936, :13=0/970 10=-1289/0, 2-19=0/903, 3-19=-869/0, 3-11 3=-861/0, 9-14=0/557, 5-18=-374/0, 5-16=	3022/0, 6 ⁻ 7=-3022/0, 7-9 15-16=0/3022, 14-15=0/3 3=0/567, 10-12=-1289/0, <i>1</i>)=-2612/0, 022, 13-14=0/2243	3,				

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x6 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

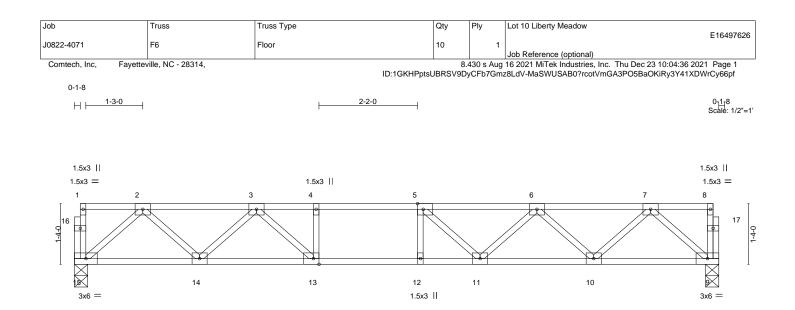
5) Refer to girder(s) for truss to truss connections.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.







L			14-2-0			
1			14-2-0			1
Plate Offsets (X,Y)	[5:0-1-8,Edge], [13:0-1-8,Edge]					
LOADING (psf) TCLL 40.0	SPACING- 2-0-0 Plate Grip DOL 1.00	CSI. TC 0.54		n (loc) l/defl L/d 5 11-12 >999 480	PLATES MT20	GRIP 244/190
TCDL 10.0 BCLL 0.0	Lumber DOL 1.00 Rep Stress Incr YES	BC 0.78 WB 0.34	- ()) 11-12 >854 360		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 73 lb	FT = 20%F, 11%E
LUMBER-			BRACING-			
	PNo.1(flat) PNo.1(flat)		TOP CHORD	Structural wood sheathing dire except end verticals.	ectly applied or 6-0-0) oc purlins,
WEBS 2x4 SF	P No.3(flat)		BOT CHORD	Rigid ceiling directly applied o	r 10-0-0 oc bracing.	
REACTIONS. (size	e) 15=0-3-8, 9=0-3-8					

Max Grav 15=759(LC 1), 9=759(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1309/0, 3-4=-2153/0, 4-5=-2153/0, 5-6=-2019/0, 6-7=-1326/0

BOT CHORD 14-15=0/813, 13-14=0/1799, 12-13=0/2153, 11-12=0/2153, 10-11=0/1821, 9-10=0/806 WEBS 2-15=-1080/0, 2-14=-0/690, 3-14=-683/0, 3-13=0/649, 7-9=-1070/0, 7-10=0/723, 6 10-690/0, 6 14=0/625, 6 14-20/625, 6 14-20/625, 6 14-0/625,

6-10=-689/0, 6-11=0/352, 5-11=-383/24, 4-13=-299/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.





