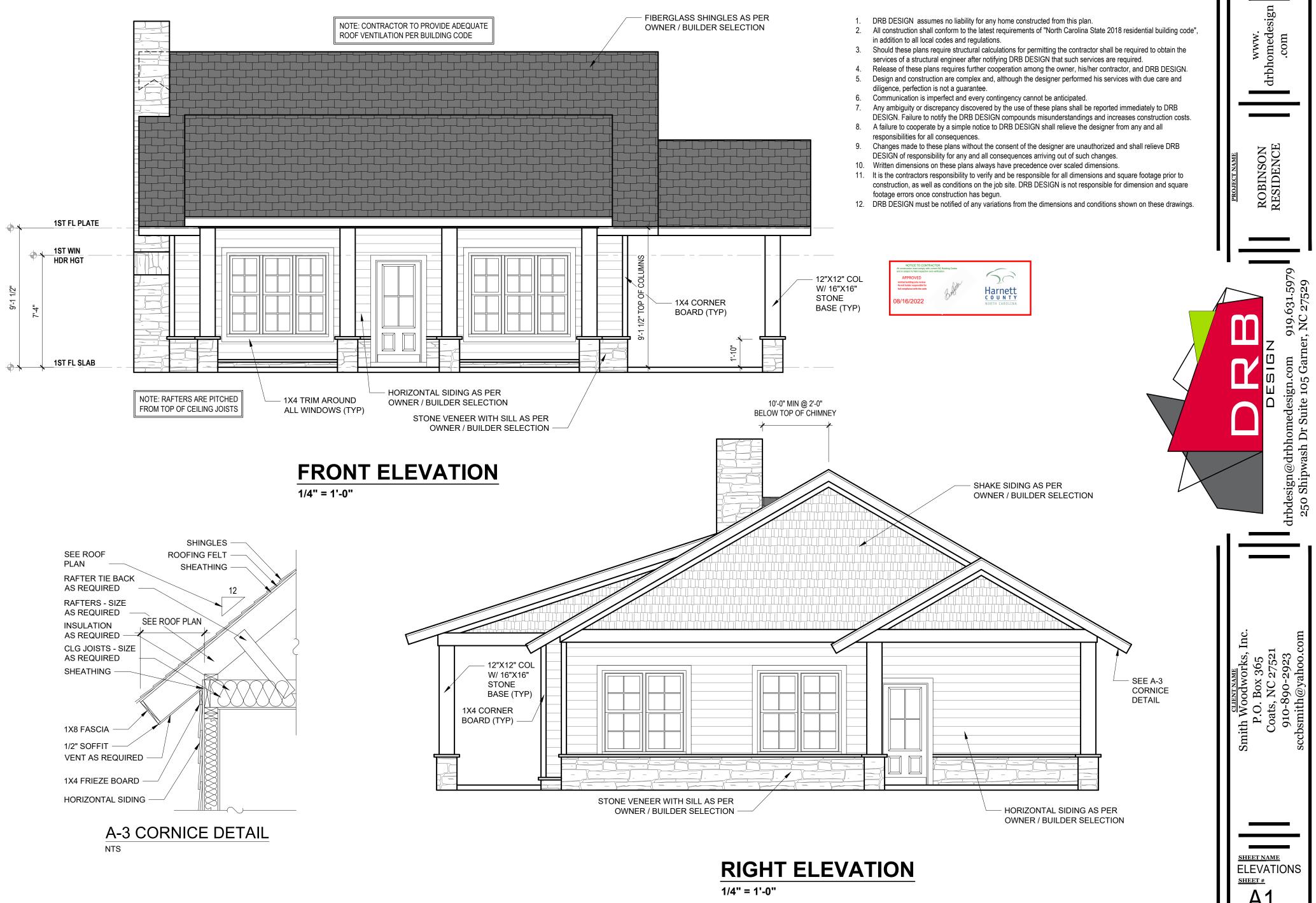
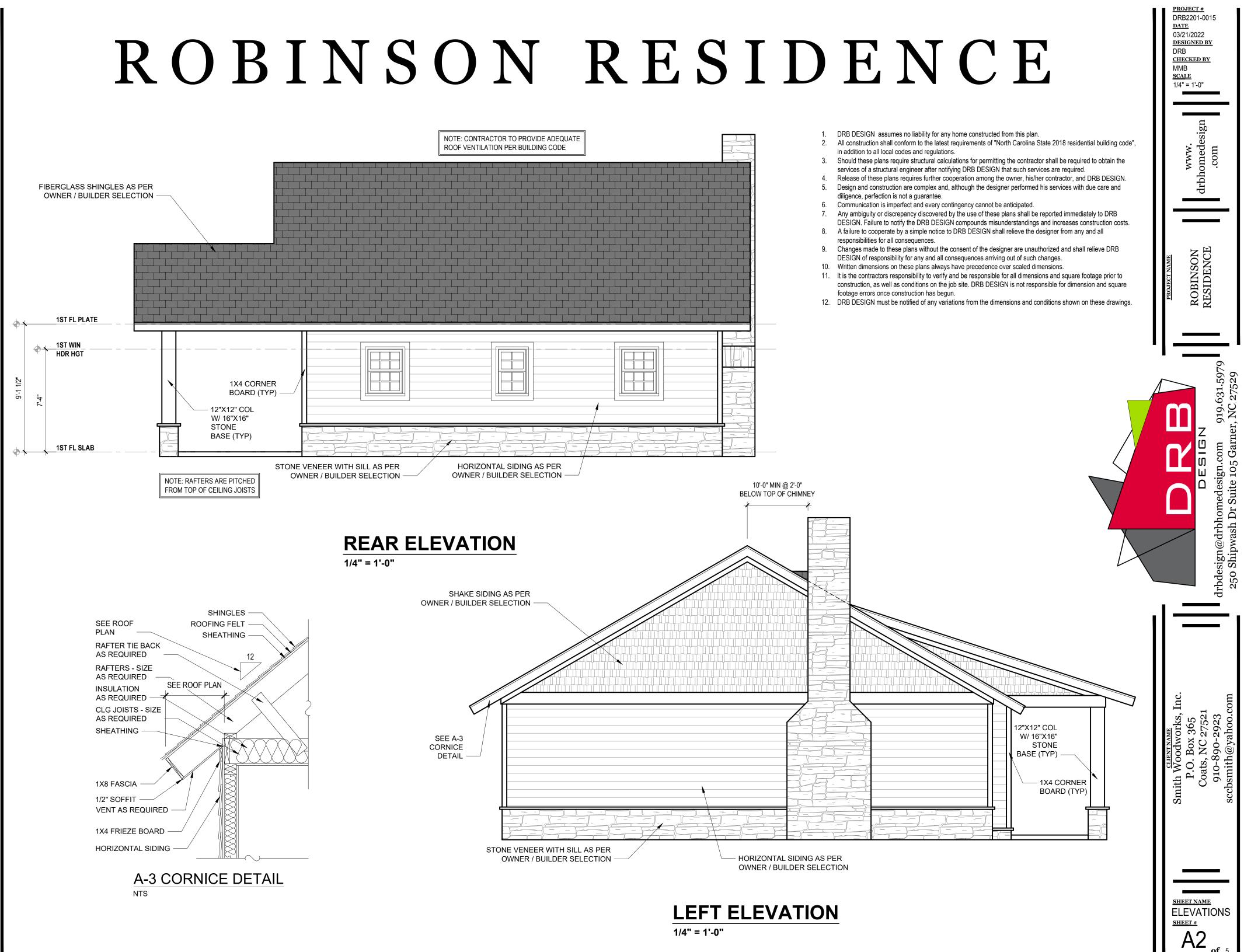
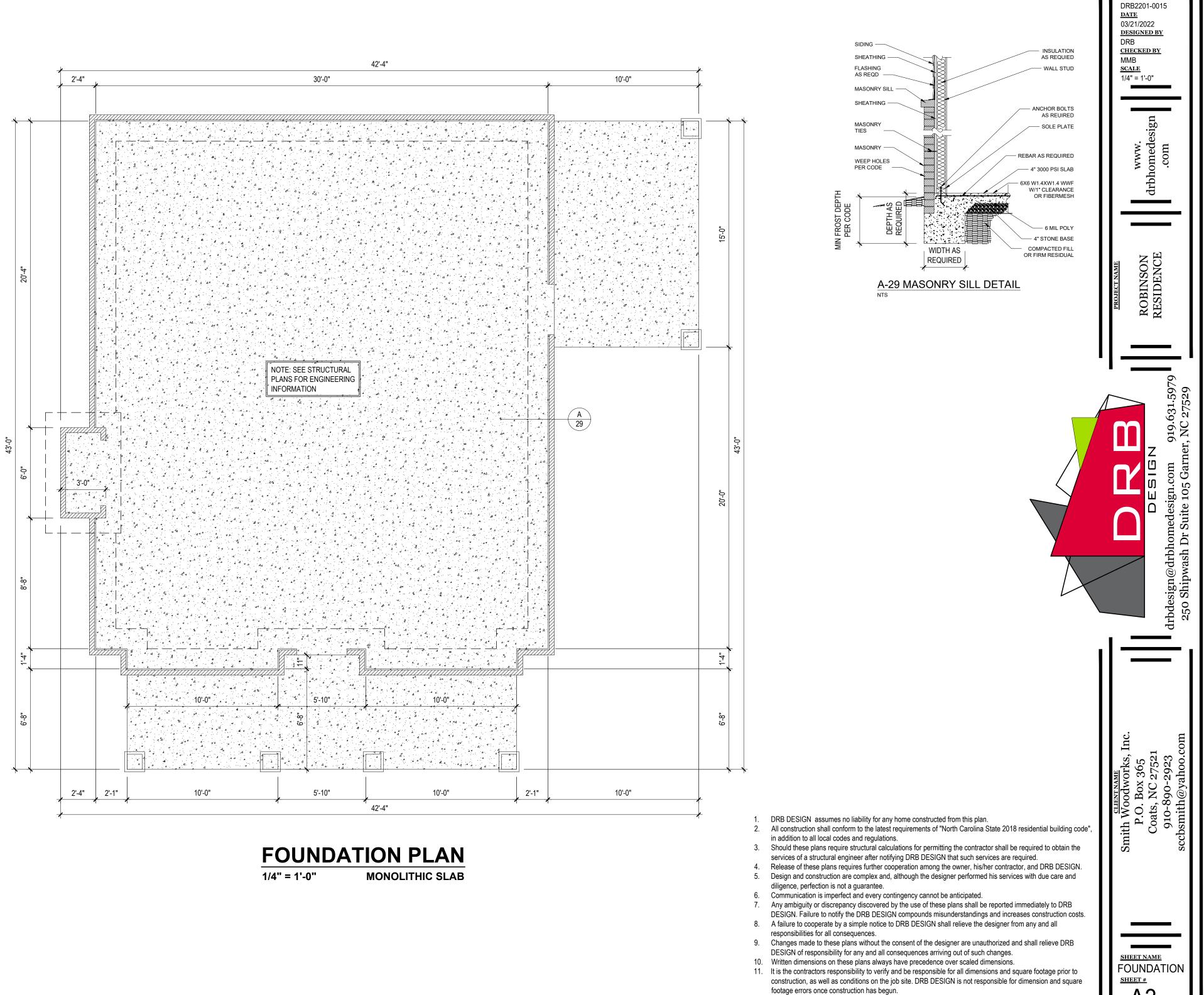
ROBINSON RESIDENCE



PROJECT # DRB2201-0015 DATE 03/21/2022

DESIGNED BY DRB CHECKED BY MMB SCALE 1/4" = 1'-0"

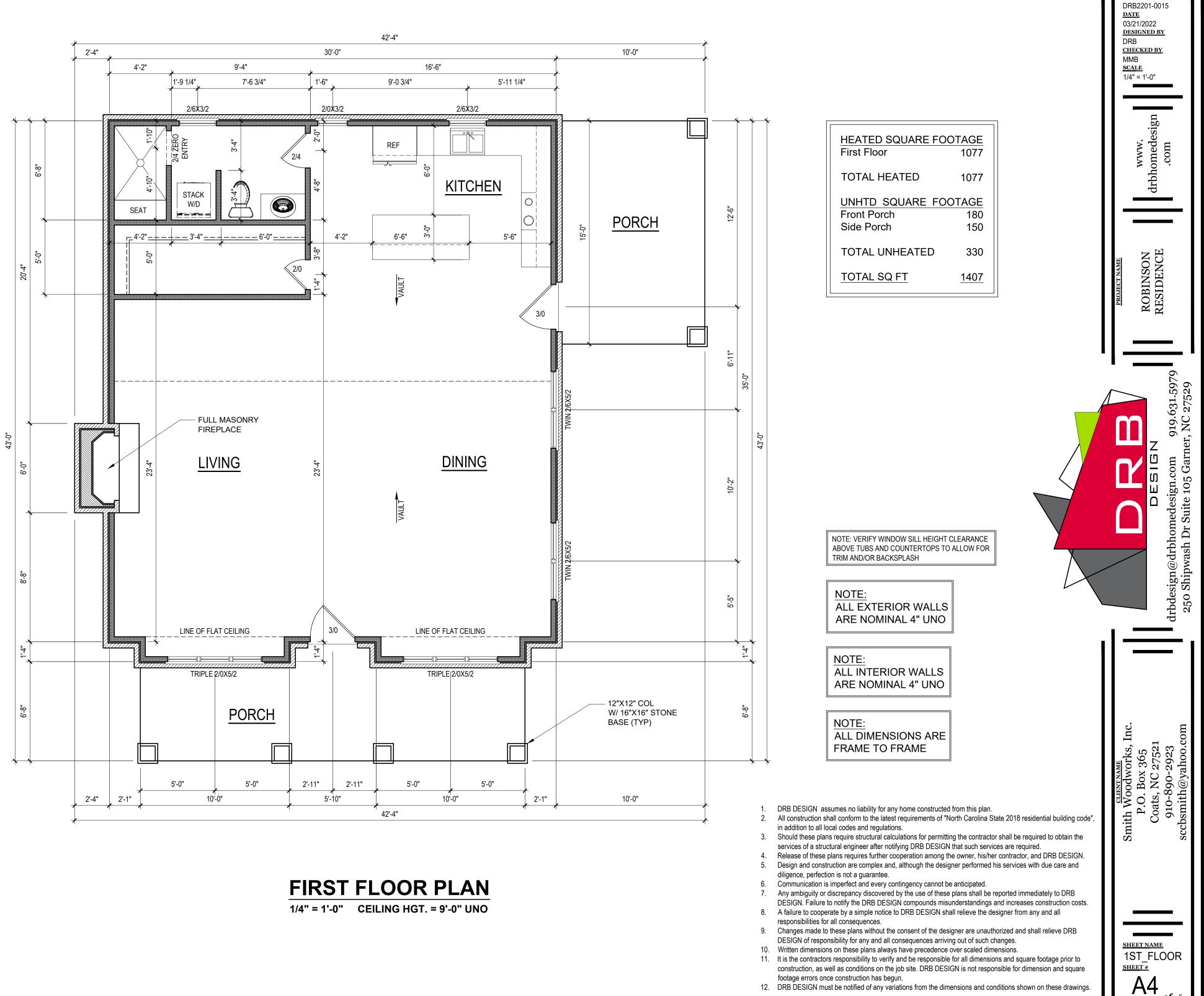




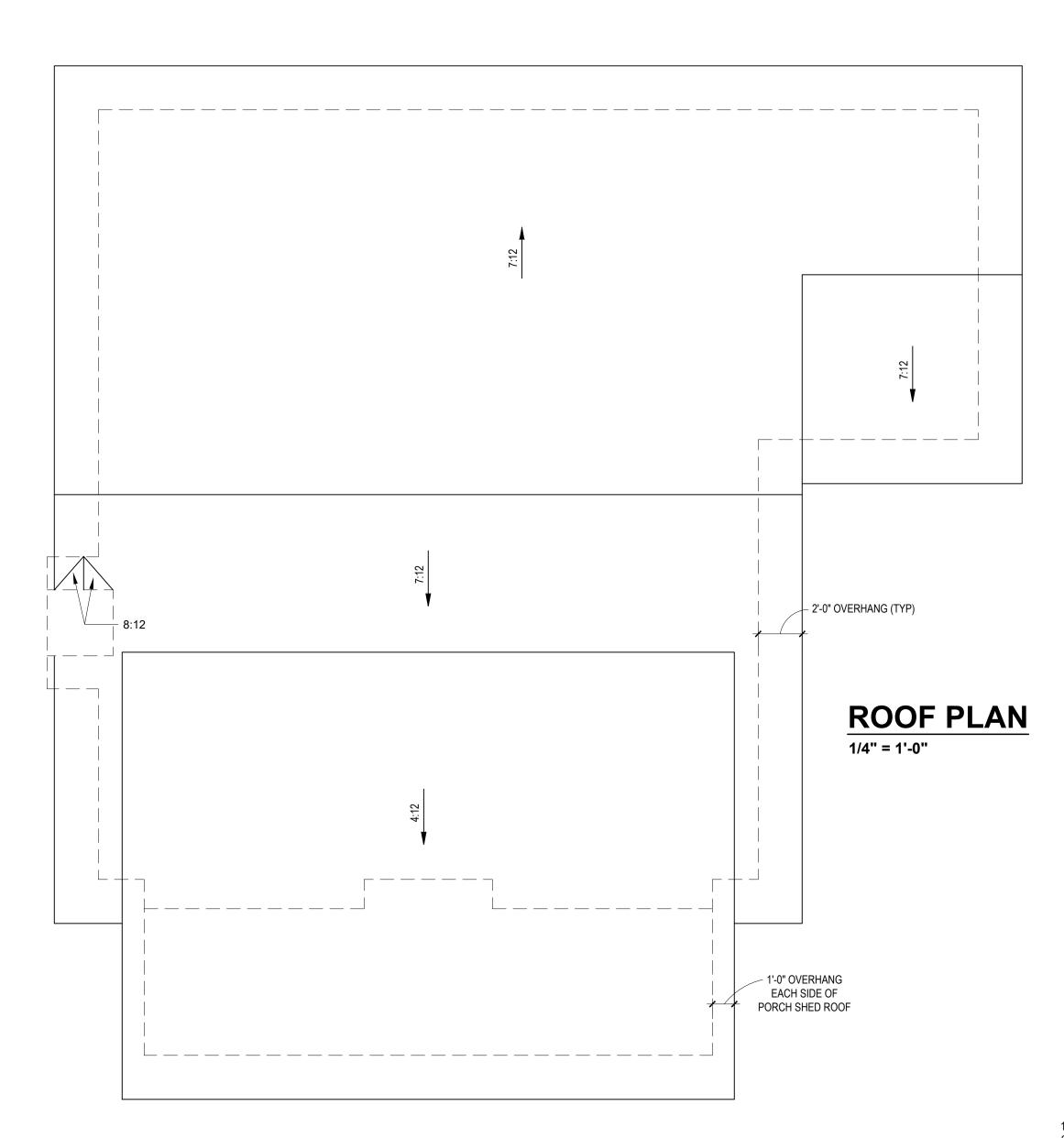
12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.

A3

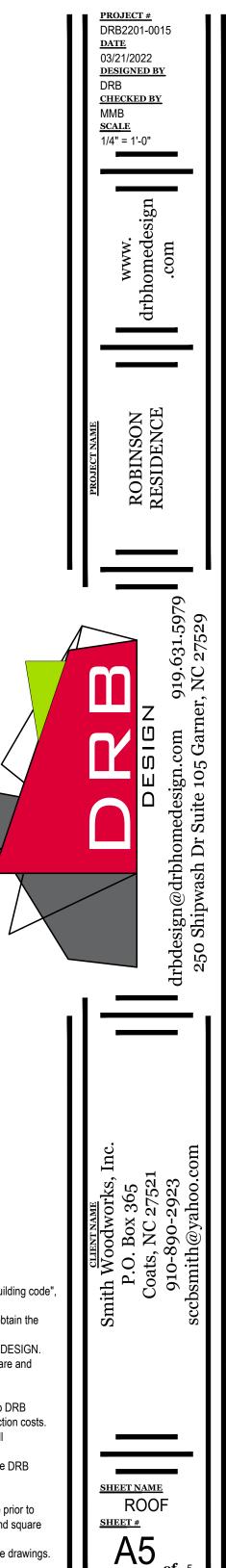
PROJECT #



PROJECT #



- 1. DRB DESIGN assumes no liability for any home constructed from this plan.
- 2. All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code", in addition to all local codes and regulations.
- 3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the services of a structural engineer after notifying DRB DESIGN that such services are required.
- Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN.
 Design and construction are complex and, although the designer performed his services with due care and
- diligence, perfection is not a guarantee.6. Communication is imperfect and every contingency cannot be anticipated.
- 7. Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB
- DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs.
 8. A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all responsibilities for all consequences.
- Changes made to these plans without the consent of the designer are unauthorized and shall relieve DRB DESIGN of responsibility for any and all consequences arriving out of such changes.
- 10. Written dimensions on these plans always have precedence over scaled dimensions.
- It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square footage errors once construction has begun.
- 12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.



DESIGN LOADS

	DESIGN L	.OADS			-													
	LIVE LOAD (PSF)	DEAD LOAD (PSF)		ECTION														
FLOOR (primary)	40	10	LL L/360	TL L/240														
FLOOR (secondary) ATTIC (w/ storage)	40 20	10 10	L/360 L/240	L/240 L/180														
ATTIC (no access) EXTERNAL BALCONY	10 40	5 10	L/240 L/360	L/180 L/240									[ONT. TURN		
ROOF ROOF TRUSS	20 20	10 20	L/240 L/240	L/180 L/180									/		CONC. W/ FTG, TYP	5" BRICK		
WIND LOAD	BAS	GED ON 120 MPH (I	EXPOSURE B)												·			
SEISMIC	BAS	ED ON SEISMIC Z	ONES A, B & C	;		-	ł									42'-4"		
					_	-	2'-4"	*						30'-0"				
TRUCTURAL NOTES: ALL CONSTRUCTION SHA CAROLINA STATE 2018 RE CODES AND REGULATION IT IS THE CONTRACTORS FOOTAGE PRIOR TO CON RESPONSIBLE FOR DIMEN CONSTRUCTION BEGINS. ALL LUMBER SHALL BE S' ALL LVL LUMBER TO BE 1 PSI, E = 1.9M PSI (I.E. ILEVEL MICROLAM) ALL LSL LUMBER IS TO BE ALL LOAD BEARING EXTE STUD (U.N.O.) AND KING S @ 8" O.C., PROVIDED THA BOTTOM OF THE WINDOW AND R602.7(2). ALL INTERIOR LOAD BEAF R602.7(1) AND R602.7(2) F INTERIOR AND EXTERIOR REFER TO 2018 NC BUILD WALLS OVER 10'-0" IN HEI ALL STRUCTURAL STEEL Fy = 50 KSI MIN. (UNO) ALL EXTERIOR LUMBER T ALL CONCRETE, fc = 3000 PRESUMPTIVE BEARING (C) 1/2"Ø ANCHOR BOLTS SP/J FROM THE CORNER. THEI ANCHOR BOLTS SHALL BI SHALL EXTEND 7" INTO C! SHALL EXTEND 7" INTO C! PSL COLUMNS DESIGNED OF PORCH COLUMNS. (U.) PROVIDE A MINIMUM OF 5	ESIDENTIAL BUILD IS. RESPONSIBILITY STRUCTION. TYNE VSIONS AND SQU/ YP #2 (UNO) .75" WIDE NOMINA E 1.55E (Fb = 2325 RIOR WINDOW HE STUDS PER TABLE T THE TOP OF TH V HEIGHT IS 1'-6". (RING HEADERS TC OR JACK STUD RE LOAD CONDITION ING CODE SECTIC GHT. SHALL BE ASTM A O BE #2 SYP PT PSI MIN. CAPACITY = 2000 F ACED AT MAXIMUN RE SHALL BE A MI E SPACED AT 3-0" ONCRETE OR MAS 00# UPLIFT & LAT N.O.) HEATHING PER SER A HEADENS SHALL THAN 500# SHALL	VING CODE", IN AD TO VERIFY ALL DI DALL ENGINEERIN ARE FOOTAGE ER AL EACH SINGLE M PSI) EADERS ARE TO B E (2002) E WINDOW HEIGH OTHERWISE REFE D BE (2) 2x10 (U.N. EQUIREMENTS FO D BE (2) 2x10 (U.N. EQUIREMENTS FO N 602 FOR CON A992 GRADE 50 PSF M OF 6'-0" O.C. AN INIMUM OF (2) BOI 'O.C. FOR BASEM SONRY. HT OF 9'-0" (UNO) ERAL CONNECTIO ECTION 602.10.4 C IOT EXCEED FOUR BE CONTINUOUS	DDITION TO AL IMENSIONS AT VG & DESIGN, RRORS ONCE MEMBER AND DE (2) 2x10 w/ (DOGETHER AND DE (2) 2x10 w/ (DOGETHER W/ (2) TI IS 6 ^{1,8} , MIN ER TO TABLES O.) REFER TO DR HEADER SF ISTRUCTION O R HEADER SF ISTRUCTION O R HEADER SF ISTRUCTION O DN AT TOP AN DF THE 2018 IF R TIMES ITS LI SLY ANCHORE	L LOCAL ND SQUARE PA IS NOT Fb = 2600 1) 2x4 JACK 2) 10d NAILS IMUM 5 R602.7(1) TABLES PANS FOR 0F ALL THAN 12" E SECTION. 0R BOLT D BOTTOM RC. EAST	20'-4"		/						4" CC WI.4 FIB 6 MIL OVER		// 6x6 OR // ER // ER // ER // ER // ER			
RUCTURAL SHEATHING N	NOTES				- +	-		i ga										
DESIGNED FOR SEISMIC ZON LESS.	NE A-C AND WIND	SPEEDS OF 120 M	/IPH OR		43'-0"						TH., CONC, F CTRON TYP @				``			
WALLS SHALL BE BRACED IN THE 2018 NCRC. BRACING REQUIREMENTS SI REFER TO SECTION R602.10. CONNECTIONS & SUPPORT (HALL BE PER TABI 4 FOR LOAD PATH	LE R602.10.3. I DETAILS INCLUE					3'-0"	 		FROJEC		U MANSORT	FIREFLAC	Ē			ς.	
$\langle 1 \rangle$ REFERENCE FIGURE R60						-		- /										
INTERIOR BRACED WALL PAI IN ACCORDANCE WITH THE O PRESCRIBED IN SECTION R6	NELS (BWP) INDIC. GB METHOD OR W	ATED SHALL BE S	SHEATHED		+		↓											
2 1/2" GYPSUM BOARD (GB (ISOLATED PANELS) OR 4 SECURE w/ 5d COOLER N SPACED @ 7" O.C. AT PA BOTTOM PLATES & 7" O.C	Í'-0" (CONTINUOUS IAILS (OR EQUAL F NEL EDGES, INCLI	S SHEATHING). PER TABLE R702.3 UDING TOP AND	3.5)		=.				,									
3/8" WOOD STRUCTURAL NAILS SPACED AT 6" O.C. INTERMEDIATE SUPPORT	. AT PANEL EDGES				8- 9													
EXTERIOR BRACED WALL PA ACCORDANCE WITH CS-WSF									1									
ACCORDANCE WITH CS-WSF R602.10.3 (UNO) ALL SHEATHABLE SURFACES									— _									
ABOVE AND BELOW OPENIN	GS AND GABLE EN	ND WALLS) SHALL	. BE		<u></u>		+	_! ///////////////////////////////////					[77777]	4				
CONTINUOUSLY SHEATHED SHEATHING WITH A MINIMUN	I THICKNESS OF 3	3/8". SHEATHING S	SHALL BE		1-4								11"	J				
SECURED WITH MINIMUM 6d PANEL EDGES AND SPACED	AT 12" O.C. AT INT	TERMEDIATE SUP	PORTS.		*					///////////////////////////////////////	///////////////////////////////////////	///////////////////////////////////////				///////////////////////////////////////	///////////////////////////////////////	///////////////////////////////////////
MINIMUM BRACED WALL PAN BE AS FOLLOWS:			OD SHALL															
- 24" ADJACENT TO OI 67% OF WALL HEIG		RE THAN							<u></u>		10'-0"			5'-10"			10'-	0"
- 30" ADJACENT TO OI 67% AND LESS THA	PENINGS GREATE				6-8								• - -9		.1			
- 48" FOR OPENINGS (0								<u>o</u>	4" CON	C. SLAB			
WALL HEIGHT $\overline{(4)}$ SHEATH INTERIOR & EXT	FRIOR								-					ON G	RADE			
FOR CS-WSP METHOD, A MIN RETURN SHALL BE PROVIDE	D AT BOTH ENDS	OF A BRACED WA	ALL LINE IN		**			<u> </u> [<u> </u>	\				
ACCORDANCE WITH FIGURE EITHER A MIN. 48" BRACED W	VALL PANEL SHAL	L BE PROVIDED A	T THE														4.01	0"
CORNER OR A HOLD-DOWN VALUE OF 800# SHALL BE FA	DEVICE WITH A MI	INIMUM UPLIFT DE	ESIGN				2'-4"	2'-1"	/		10'-0"		+	5'-10"			10'-	·U"
WALL PANEL CLOSEST TO TH																42'-4"		

(5) MINIMUM 800# HOLD-DOWN DEVICE

:4/19/2022 4:34 PM

PLOT

JAY LAST

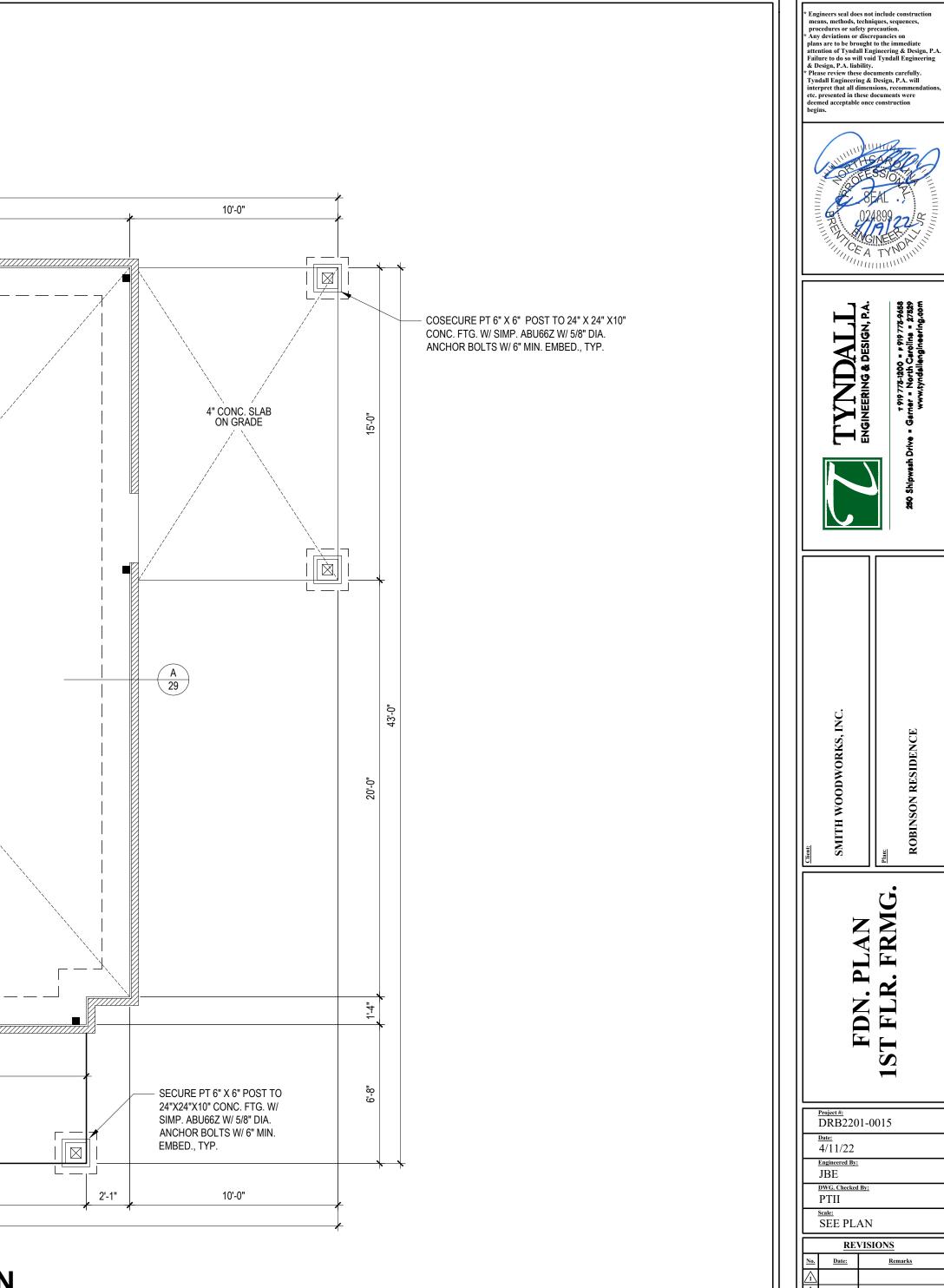
saved By:

DWG

FOUNDATION PLAN

1/4" = 1'-0"

MONOLITHIC SLAB



Sheet Number

S1

1 of 5

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION				
	(-)	(-)	LL	TL			
FLOOR (primary)	40	10	L/360	L/240			
FLOOR (secondary)	40	10	L/360	L/240			
ATTIC (w/ storage)	20	10	L/240	L/180			
ATTIC (no access)	10	5	L/240	L/180			
EXTERNAL BALCONY	40	10	L/360	L/240			
ROOF	20	10	L/240	L/180			
ROOF TRUSS	20	20	L/240	L/180			
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)						
SEISMIC	BASED ON SEISMIC ZONES A, B & C						

STRUCTURAL NOTES:

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

BWL 1

- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE 2) FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- ALL LUMBER SHALL BE SYP #2 (UNO)
- ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (I.E. iLEVEL MICROLAM) ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK 4) STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR 5) INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL
- 6) WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)

ALL EXTERIOR LUMBER TO BE #2 SYP PT ALL CONCRETE, fc = 3000 PSI MIN.

- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.) 13)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC. MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST 15)
- HORIZONTAL DIMENSION. UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE 16)
- FOUNDATION. 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
- 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.

 $\langle 1 \rangle$ REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.

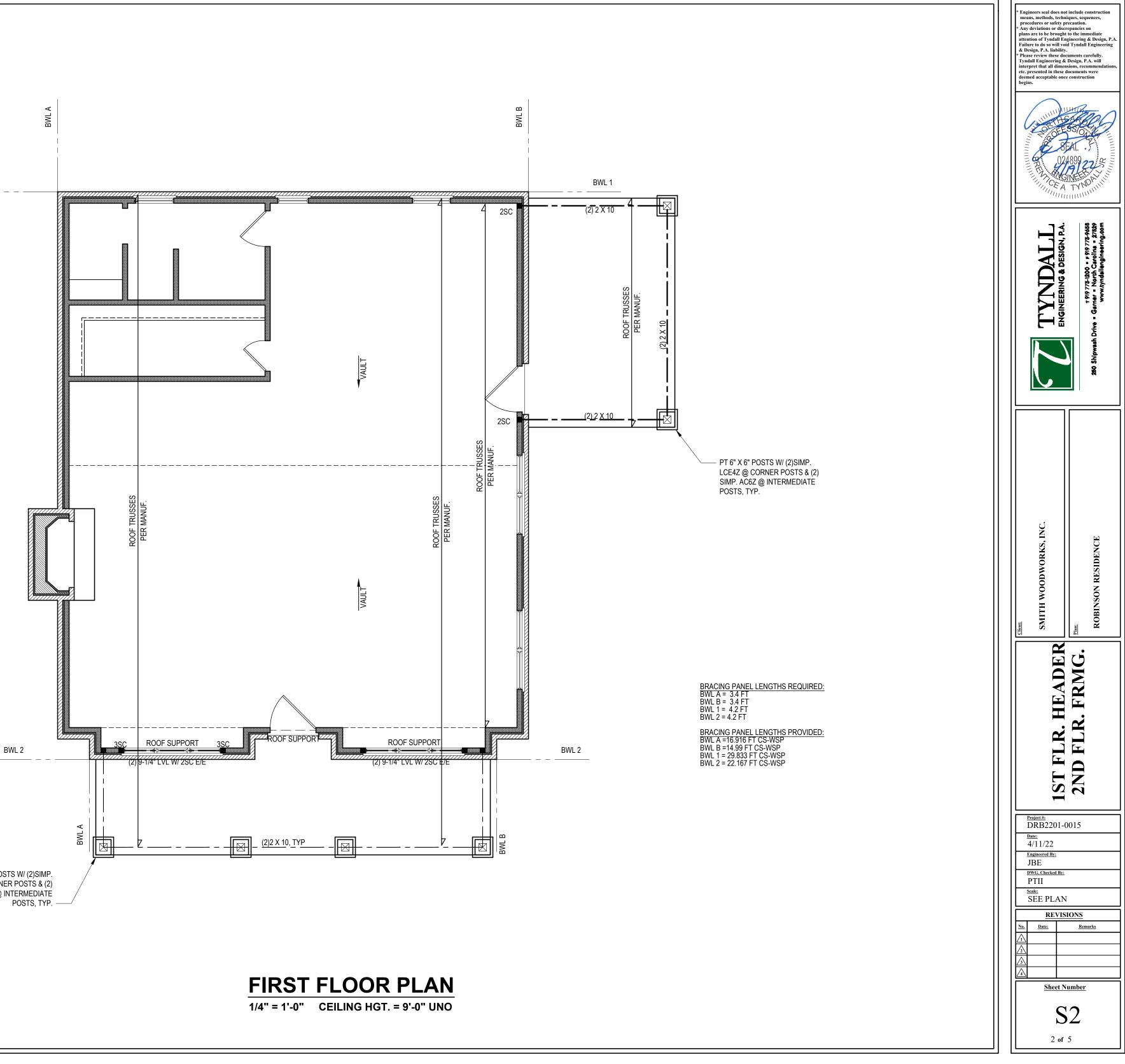
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- $\langle 2 \rangle$ 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS. 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
 - 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT
 - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT.
 - 48" FOR OPENINGS GREATER THAN 85% OF

 $\langle 4 \rangle$ SHEATH INTERIOR & EXTERIOR

WALL HEIGHT

FOR CS-WSP METHOD. A MINIMUM 24" BRACED WALL PANEL CORNER 8) RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.

5 MINIMUM 800# HOLD-DOWN DEVICE

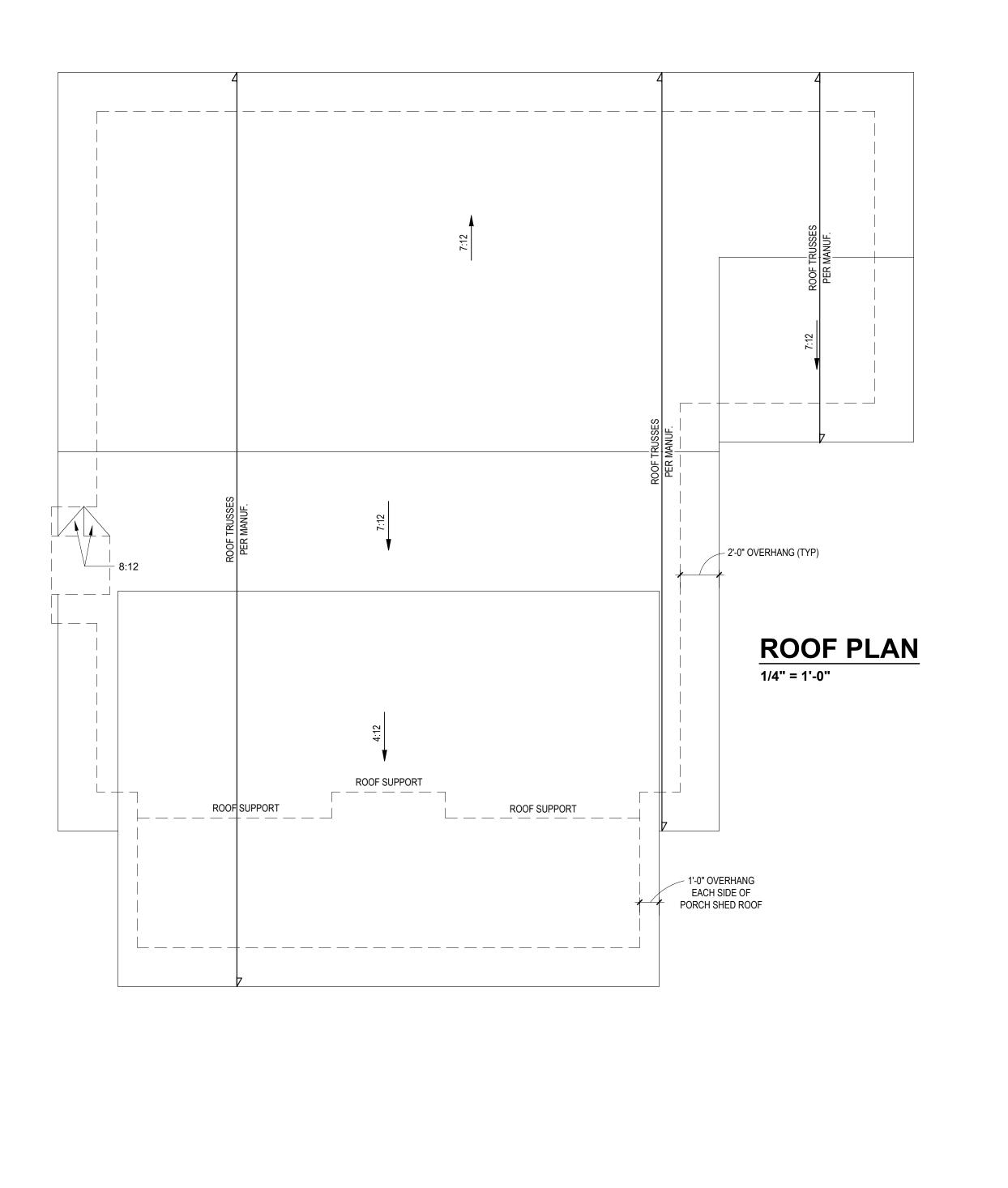


PT 6" X 6" POSTS W/ (2)SIMP. LCE4Z @ CORNER POSTS & (2) SIMP. AC6Z @ INTERMEDIATE

PM

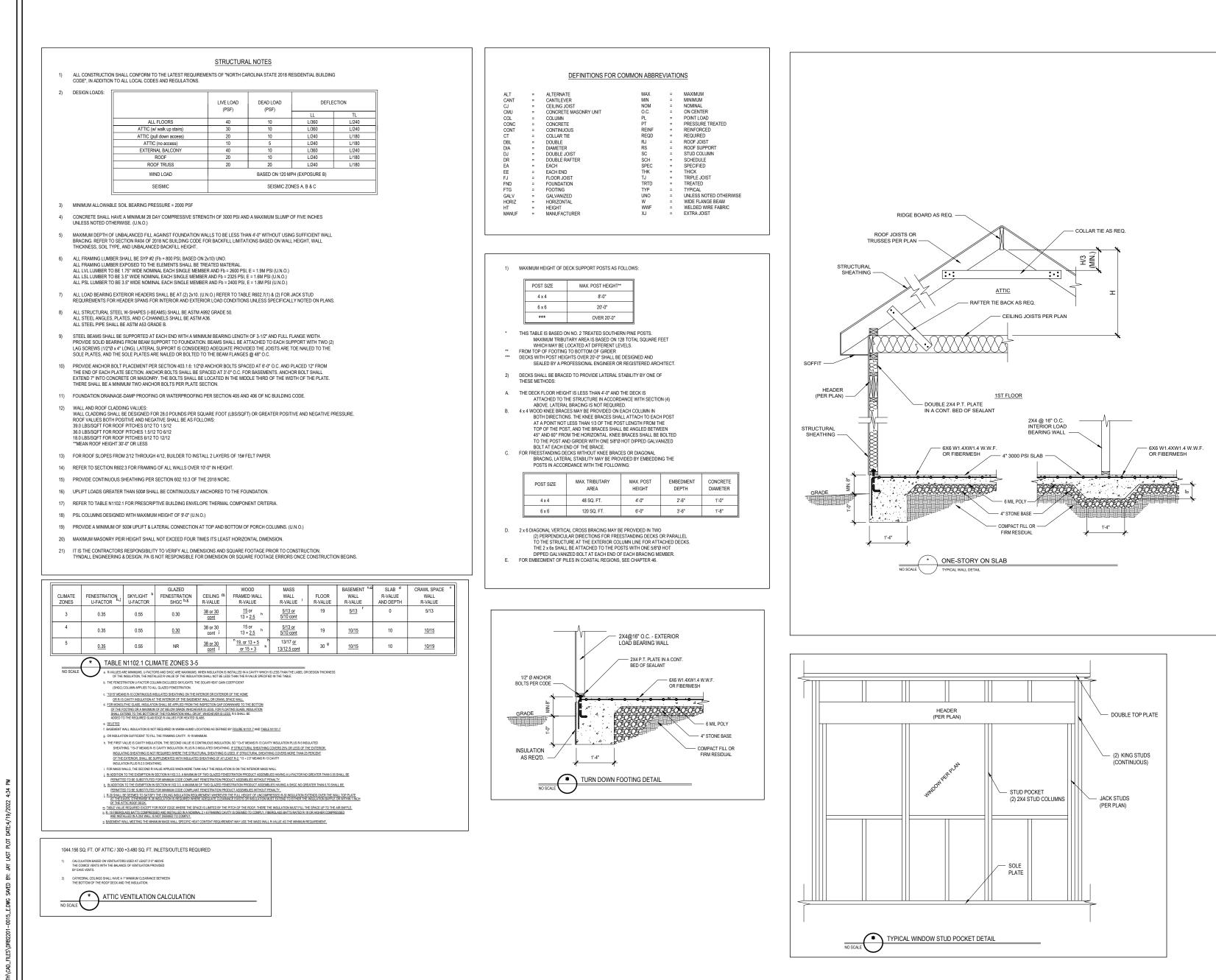
1:34

LOT



WE: Z:\DRB\DRB_2022\DRB2201-0015_BRANDON_SMITH\DRB2201-0015_BRANDON_SMITH\CAD_FILES\DRB2201-0015_E.DWG SAVED BY: JAY LAST PLOT DATE:4/19/2022 4:34 PM

means, methods, techn procedures or safety pp * Any deviations or discr plans are to be brought attention of Tyndall En Failure to do so will voi & Design, P.A. liability * Please review these do Tyndall Engineering & interpret that all dimen etc. presented in these of deemed acceptable once begins.	* Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution. * Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. liability. * Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.							
TYNDALL ENGINEERING & DESIGN, P.A.	7 919 775-1200 = # 919 775-2658 280 Shipwash Drive = Garner = North Carolina = 27529 www.tyndallengineering.com							
cliai: SMITH WOODWORKS, INC.	Pian: ROBINSON RESIDENCE							
ROOF PLAN	ROOF PLAN							
Date: 4/11/22 Engineered By: JBE DWG. Checked By: PTII Scale: SEE PLAN REVIS \u03c4 \u03c4	DRB2201-0015 Date: 4/11/22 Engineered By: JBE DWG. Checked By: PTII SEE PLAN REVISIONS No. Date: A A							
	S3 3 of 5							



M

