

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 150_1910_C KB Home 150.1910.C

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I42923816 thru I42923850

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



September 23,2020

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



non-concurrent with other live loads. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSIVTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932



- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 5. This connection is for uplift only and does not consider lateral forces.



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fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Qu** Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	KB Home 150.1910.C	
						I42923819
150_1910_C	AG	COMMON GIRDER	1	2		
				2	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.4	20 s Aug 2	25 2020 MiTek Industries, Inc. Wed Sep 23 08:16:02	2020 Page 2
		ID:VMD	62rz1yiHD	OgRtbnrl	FztQ8K-ff3OPwG7nMgWmEwixzlck0dhRNZCJHWFv	vTcVHpyap?h

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 3-5=-43, 1-5=-20

Concentrated Loads (lb)

Vert: 1=-1619(B) 6=-2171(B) 9=-1544(B) 10=-1541(B) 11=-1608(B) 12=-1657(B)

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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 13-9-4, Exterior(2) 13-9-4 to 16-9-4, Interior(1) 16-9-4 to 27-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.



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WEBS 4-10=0/289, 4-9=-528/151, 6-9=0/286, 7-9=-21/482

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 13-9-4, Exterior(2) 13-9-4 to 16-9-4, Interior(1) 16-9-4 to 20-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.



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basing value to geo only with these others that begins based only door parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	KB Home 150.1910.C	
150 1910 C	RE	Common Supported Gable	1	1		142923822
130_1310_0		Common Supported Cable		· ·	Job Reference (optional)	
		ID:VM	D62rz1yiH	8. D_OqRtbr	420 s Aug 25 2020 MiTek Industries, Inc. Wed Sep 23 13:42:45 2020 hrlFztQ8K-RxK7TDyjFN4ZeOAkzPhVGO0g5TAGqEvedoilp5y	Page 2 /akDO

NOTES-

13) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

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Job	Truss	Truss Type	Qty	Ply	KB Home 150.1910.C
					142923823
150_1910_C	BG	Common Girder	1	2	
				–	Job Reference (optional)

8.420 s Aug 25 2020 MiTek Industries, Inc. Wed Sep 23 13:43:32 2020 Page 2 ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-Z_guLIWCAp?uncGmEn1XgITi?PE0LUOYQcaHytyakCf

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-43, 5-6=-43, 2-7=-20 Concentrated Loads (lb)

Vert: 8=-112(B) 10=111(B) 11=84(B) 12=85(B) 13=-19(B) 14=-34(B) 15=-125(B) 16=-125(B) 17=-131(B)

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4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) N/A

11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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8-3-0	16-0-10	24-0-0	31-11-6	39-9-0 42-2-8
8-3-0	7-9-10	7-11-6	7-11-6	7-9-10 2-5-8
Plate Offsets (X,Y) [2:0-0-0,	0-0-13]			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDI 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. I TC 0.56 V BC 0.81 V	PEFL. in (loc) I/defl ert(LL) -0.34 16-18 >999 ert(CT) -0.73 16-18 >689	L/d PLATES GRIP 240 MT20 197/144 180 M18SHS 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.92 H Matrix-S	lorz(CT) 0.21 12 n/a	n/a Weight: 251 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 *E 1-4,10-11: 2x4 SP POT CHORD 2x4 SP No.1	xcept* SP No.2 or 2x4 SPF No.2	BRACING TOP CHO	- RD Structural wood sheath except end verticals, ar	ing directly applied or 3-0-0 oc purlins, id 2-0-0 oc purlins (3-3-14 max.): 4-10.
WEBS 2x4 SP No.3		WEBS	6-0-0 oc bracing: 12-13 1 Row at midpt 2 Rows at 1/3 pts	5-20, 7-18, 7-14 8-13
REACTIONS. (size) 2=0- Max Horz 2=11 Max Uplift 2=-4 Max Grav 2=17	3-8, 12=Mechanical l6(LC 16) 3(LC 13), 12=-88(LC 12) 739(LC 2), 12=1677(LC 38)			
FORCES. (lb) Max. Comp./M TOP CHORD 2-3=-3090/20' 8-10=-896/700 807 CHORD BOT CHORD 2-02=-231/26' WEBS 4-20=0/953, 5 8-13=-2713/2' 8-13=-2713/2'	lax. Ten All forces 250 (lb) or less exc. I, 3-4=-2985/162, 4-5=-2623/169, 5-7=-4 10-11=-977/57, 11-12=-1693/83 52, 18-20=-281/4150, 16-18=-297/4275, -20=-1794/186, 5-18=0/373, 7-16=0/317 10, 11-13=-71/1586	ept when shown. 150/287, 7-8=-3307/235, 14-16=-297/4275, 13-14=-229/3; ,7-14=-1169/77, 8-14=0/728,	007	
 NOTES- 1) Unbalanced roof live loads ha 2) Wind: ASCE 7-10; Vult=115n gable end zone and C-C Exte 39-9-0, Exterior(2) 39-9-0 to 4 DOL=1.60 3) TCLL: ASCE 7-10; Pr=20.0 p roof snow: Lumber DOL=1.11 governs. Rain surcharge app 4) Unbalanced snow loads have 5) This truss has been designed non-concurrent with other live 6) Provide adequate drainage to 7) All plates are MT20 plates un 8) This truss has been designed will fit between the bottom ch 10) Refer to girder(s) for truss to 11) Provide mechanical connect 12) One RT7A USP connectors only and does not consider 13) Graphical purlin representation 	ave been considered for this design. ph Vasd=91mph; TCDL=6.0psf; BCDL= prior(2) -0-10-8 to 3-4-2, Interior(1) 3-4-2 42-0-12 zone;C-C for members and force sf (roof live load: Lumber DOL=1.15 Pla 5 Plate DOL=1.15); Category II; Exp B; F blied to all exposed surfaces with slopes a been considered for this design. If or greater of min roof live load of 12.0 a loads. b prevent water ponding. less otherwise indicated. If or a 10.0 psf bottom chord live load no ad for a live load of 20.0psf on the bottor ord and any other members. b truss connections. tion (by others) of truss to bearing plate recommended to connect truss to bearing lateral forces. tion does not depict the size or the orient	6.0psf; h=25ft; Cat. II; Exp B; En to 8-3-0, Exterior(2) 8-3-0 to 14- is & MWFRS for reactions shown e DOL=1.15); Pg=15.0 psf (grou artially Exp.; Ct=1.10, Lu=50-0-0 less than 0.500/12 in accordance osf or 1.00 times flat roof load of inconcurrent with any other live lo n chord in all areas where a recta capable of withstanding 100 lb up ng walls due to UPLIFT at jt(s) 2.	Closed; MWFRS (envelope) 2-10, Interior(1) 14-2-10 to 1; Lumber DOL=1.60 plate grip and snow); Pf=16.5 psf (flat ; Min. flat roof snow load with IBC 1608.3.4. 11.6 psf on overhangs ads. ngle 3-6-0 tall by 2-0-0 wide lift at joint(s) 12. This connection is for uplift nd/or bottom chord.	SEAL 044925 September 23,2020

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	5-6-15	5-4-1	9-9	I-12	6-6-8	I	9-9-12	5-1-8	-
Plate Offsets (X,Y) [2:0-0-0,0-0	-13], [15:0-3-0,0-3-4]							
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	2-0-0 1.15 1.15 YES PI2014	CSI. TC 0.98 BC 0.95 WB 0.71 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.34 15-17 -0.74 15-17 0.15 11	l/defl L/d >999 240 >681 180 n/a n/a	PLATES MT20 Weight: 232 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.2 or 2x4 2x4 SP No.1 2x4 SP No.3	4 SPF No.2			BRACING- TOP CHORD BOT CHORD	Structural wood except end vertic Rigid ceiling dire 2-2-0 oc bracing	sheathing directly cals, and 2-0-0 oc ctly applied or 10 : 15-17.	applied or 2-8-10 oc purlir purlins (2-2-0 max.): 4-9. -0-0 oc bracing, Except:	15,
REACTIONS.	(size) 2=0-3-8	8, 11=Mechanical			WEBS	1 Row at midpt	5-17,	8-12	

27-3-4

37-1-0

Max Horz 2=134(LC 16) Max Uplift 2=-12(LC 13), 11=-45(LC 12) Max Grav 2=1739(LC 2), 11=1676(LC 2)

5-6-15

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

10-11-0

TOP CHORD 2-3=-3153/181, 3-4=-2797/179, 4-5=-2427/189, 5-6=-3260/193, 6-8=-3007/182,

8-9=-1362/125, 9-10=-1551/108, 10-11=-1654/116

 BOT CHORD
 2-18=-197/2703, 17-18=-197/2703, 15-17=-188/3190, 14-15=-182/3235, 12-14=-158/2683

 WEBS
 3-17=-454/125, 4-17=0/894, 5-17=-1061/156, 5-15=0/251, 6-14=-501/81, 8-14=0/690, 8-12=-1716/163, 9-12=0/400, 10-12=-44/1627

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-4-2, Interior(1) 3-4-2 to 10-11-0, Exterior(2) 10-11-0 to 16-10-10, Interior(1) 16-10-10 to 37-1-0, Exterior(2) 37-1-0 to 42-0-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

20-8-12

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11.

11) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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42-2-8







	6-10-15	5	13-7-0	19	10-5	_	28-1-11		34-5-0		42-2-8	_
Plate Offsets (X	6-10-15	5)-0-13] [5:0-2-	6-8-1 0 0-0-121 [8:0-4-6 F	6 6 dae] [13:0-7	-3-5 -0 0-3-41		8-3-6		6-3-5		7-9-8	
	<u>(, 1) [2.0 0 0,0</u>	, o 10], [0.0 2	0,0 0 12], [0.0 4 0,1	.ugoj, [10.0 t	0,004]		1					
TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLL) 20.0 16.5/15.0 10.0	SPACIN Plate G Lumber Rep Str	NG- 2-0-0 rip DOL 1.15 DOL 1.15 ess Incr YES		CSI. FC 0.97 BC 0.91 WB 1.00		DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.23 13-15 -0.46 13-15 0.14 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code II	RC2015/TPI2014		Matrix-S						Weight: 256 lb	FT = 20%
LUMBER-						BE	RACING.					
TOP CHORD	2x4 SP No.2 or 2 5-7,7-8: 2x6 SP I 2x4 SP No.2 or 2	2x4 SPF No.2 No.2 2x4 SPF No.2	*Except*			TC	DP CHORD	Structural wood 2-0-0 oc purlins Rigid ceiling dir	l sheathing (2-2-0 ma	g directly app ax.): 5-8. ed or 10-0-0	blied, except end vertica	als, and
WEBS	13-14,14-16: 2x4 2x4 SP No.3	SP No.1	Except			W	EBS	1 Row at midpt	oony appn	9-11	ee braomig.	
REACTIONS.	(size) 2=0-3 Max Horz 2=15 Max Uplift 2=-9(Max Grav 2=17	3-8, 11=Mecha 0(LC 16) (LC 16) 39(LC 2), 11=	anical 1676(LC 2)									
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	- Max. Comp./Ma 2-3=-3139/167 2-18=-174/268 11-12=-67/113 3-18=0/294, 3- 8-13=-55/1148	ax. Ten All fr , 3-5=-2604/19 8, 17-18=-174 3 17=-637/128, , 8-12=-494/10	orces 250 (lb) or les 95, 5-6=-2598/195, /2688, 15-17=-103/ 5-17=0/441, 5-15=- 03, 9-12=-19/797, 9	s except whe 5-8=-2344/17 2251, 13-15= 52/607, 6-15: 11=-1850/12	n shown. 5, 8-9=-175 -134/2772, 344/164, 6 2	51/156 12-13= 6-13=-	=-48/1569, 824/150,					
NOTES- 1) Unbalanced 2) Wind: ASCE gable end zo 34-5-0, Exter shown; Lumb 3) TCLL: ASCE roof snow: Lu governs. Ra 4) Unbalanced 5) This truss ha non-concurrer 6) Provide adec 7) This truss ha 8) * This truss ha 8) * This truss ha will fit betwee 9) Refer to girdf 10) One RT7A only and do 11) Graphical p	roof live loads ha 7-10; Vult=115m one and C-C Exter irior(2) 34-5-0 to 4 ber DOL=1.60 pla c 7-10; Pr=20.0 ps umber DOL=1.15 in surcharge apply snow loads have is been designed ent with other live quate drainage to is been designed thas been designed mas been designed en the bottom choc er(s) for truss to ti USP connectors bees not consider la purin representation	ve been consi ph Vasd=91m rior(2) -0-10-8 0-4-10, Interior te grip DOL=1 fi (roof live loa Plate DOL=1, lied to all expo been conside for greater of loads. prevent water for a 10.0 psf d for a live loa d for a live loa rd and any ot russ connectic recommended ateral forces. on does not d	dered for this design ph; TCDL=6.0psf; E to 3-4-2, Interior(1) r(1) 40-4-10 to 42-0 .60 d: Lumber DOL=1.1 .15); Category II; Ex sed surfaces with s red for this design. min roof live load of r ponding. bottom chord live lo d of 20.0psf on the her members, with E ons. I to connect truss to epict the size or the	n. CDL=6.0psf. 3-4-2 to 13-7 -12 zone;C-C 5 Plate DOL p B; Partially opes less th 12.0 psf or 1 ad nonconcu bottom chord 3CDL = 10.0p bearing wall: orientation o	h=25ft; Cai -0, Exterior c for membe =1.15); Pg= Exp.; Ct=1. an 0.500/12 .00 times fl: rrent with a in all areas osf. s due to UP	t. II; E> (2) 13- ers and 15.0 p 10, Lu in acc at roof ny oth where LIFT a along t	kp B; Enclosed; -7-0 to 19-6-10, d forces & MWFI usf (ground snow =50-0-0; Min. fla cordance with IB load of 11.6 psf er live loads. e a rectangle 3-6 ht jt(s) 2. This cou- the top and/or bo	MWFRS (envelo Interior(1) 19-6-1 RS for reactions); Pf=16.5 psf (fl at roof snow load C 1608.3.4. on overhangs S-0 tall by 2-0-0 v nnection is for up ottom chord.	pe) 0 to at i	South Street Street	SEAL 044925	Wer wanter

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September 23,2020



	8-2	-15	16-3-0	24	4-0-0	31-9-0		36-10-0	42-2-8	
	8-2	-15	8-0-1	7	-9-0	7-9-0	1	5-1-0	5-4-8	
Plate Offsets (X	,Y) [2:0-0-0,0)-0-13], [7:0-4-0,0-4-8]								
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLL	20.0 16.5/15.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC 0.62 BC 0.98 WB 0.68	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.21 17-19 -0.39 17-19 0.13 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code IRC2015/TP	12014	Matrix-S					Weight: 270 lb	FI = 20%
LUMBER- TOP CHORD BOT CHORD WEBS REACTIONS.	2x4 SP No.2 or / 6-7,7-8: 2x6 SP 2x4 SP No.2 or / 2x4 SP No.3 (size) 2=0- Max Horz 2=16 Max Uplift 2=-22 May Cary 2=17	2x4 SPF No.2 *Except* No.2 2x4 SPF No.2 3-8, 11=Mechanical 8(LC 16) 9(LC 16) 20(L 2), 11=1676(LC 2)			BRACING- TOP CHORD BOT CHORD WEBS	Structural wood except end verti Rigid ceiling dire 1 Row at midpt	sheathing o icals, and 2- ectly applied	directly appli 0-0 oc purlin d or 2-2-0 oc 7-17, 7-13	ed or 2-7-2 oc purlins, ns (4-9-6 max.): 6-8. : bracing.	,
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	- Max. Comp./M 2-3=-3107/157 8-9=-1849/178 2-19=-172/267 5-19=0/407, 5- 8-13=0/509, 9-	ax. Ten All forces 250 (, 3-5=-2915/165, 5-6=-23 , 9-10=-1582/120, 10-11= 9, 17-19=-138/2391, 15-1 17=-601/133, 6-17=0/730 13=-33/512, 9-12=-714/9	lb) or less except 63/203, 6-7=-206 1623/126 I7=-75/2248, 13-1 0, 7-17=-429/105, 7, 10-12=-71/1557	when shown. 6/211, 7-8=-1604 15=-75/2248, 12-1 7-15=0/430, 7-13 7	/180, 13=-61/1360 3=-983/75,					
NOTES- 1) Unbalanced r 2) Wind: ASCE gable end zou 31-9-0, Exter shown; Lumb 3) TCLL: ASCE roof snow: Lu governs. Rai 4) Unbalanced s 5) This truss have 6) Provide adeq 7) This truss have 8) * This truss have 9) Refer to girde 10) One RTTA I only and do 11) Graphical pro-	roof live loads ha 7-10; Vult=115m ne and C-C Exte ior(2) 31-9-0 to 3 ber DOL=1.60 pla 7-10; Pr=20.0 p imber DOL=1.15 in surcharge app snow loads have s been designed as been designed tas been designed tas been designed tas been designed to for truss to t USP connectors es not consider I urlin representati	ve been considered for th ph Vasd=91mph; TCDL= for(2) -0-10-8 to 3-4-2, In 47-8-10, Interior(1) 37-8-11 ate grip DOL=1.60 of (roof live load: Lumber Plate DOL=1.15); Catego lied to all exposed surface been considered for this for greater of min roof live loads. prevent water ponding. for a 10.0 psf bottom cho d for a live load of 20.0ps ord and any other membe russ connections. recommended to connect ateral forces. Ion does not depict the siz	his design. 6.0psf; BCDL=6.0 tterior(1) 3-4-2 to 7 0 to 42-0-12 zone DOL=1.15 Plate E ory II; Exp B; Parti es with slopes less design. e load of 12.0 psf ord live load nonco if on the bottom ch rs, with BCDL = 1 t truss to bearing v ze or the orientatio	Opsf; h=25ft; Cat. 16-3-0, Exterior(2 ;C-C for member DOL=1.15); Pg=1 ially Exp.; Ct=1.1 s than 0.500/12 in or 1.00 times flat oncurrent with any hord in all areas v 0.0psf. walls due to UPLI on of the purlin all	II; Exp B; Enclosed; I 2) 16-3-0 to 22-2-10, I 5 and forces & MWFI 5.0 psf (ground snow 0, Lu=50-0-0; Min. fla n accordance with IB roof load of 11.6 psf y other live loads. where a rectangle 3-6 IFT at jt(s) 2. This cor ong the top and/or bc	MWFRS (envelop nterior(1) 22-2-1 RS for reactions); Pf=16.5 psf (fla t roof snow load C 1608.3.4. on overhangs -0 tall by 2-0-0 w enection is for up ttom chord.	oe) 0 to at ride lift	Summunum Sum	SEAL 044925	

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September 23,2020



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	9-6-15	18-11-0		27-5-12	29-1-0	;	36-7-0	
Plate Offsets (X,Y) [2:0-0-0,0	9-6-15 D-0-5]	9-4-1		8-6-12	1-7-4	·	7-6-0	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.61 BC 0.93 WB 0.69 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.20 2-17 -0.44 2-17 0.03 13	l/defl >999 2 >752 7 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 250 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 2 BOT CHORD 2x4 SP No.2 or 2 WEBS 2x4 SP No.3 REACTIONS. (size) 2=0-3 Max Horz 2=23 Max Uplift 2=-33 Max Grav 2=10	2x4 SPF No.2 2x4 SPF No.2 3-8, 13=0-3-8, 11=Mechanical 4(LC 16) 5(LC 16), 13=-52(LC 16), 11=-267(LC 5 005(LC 2), 13=2078(LC 3), 11=132(LC 4	4) 3)	ACING- P CHORD T CHORD BS	Structural woor except end ver Rigid ceiling di 1 Row at midpl	d sheathing d ticals, and 2- rectly applied	directly applie 0-0 oc purlins 1 or 2-2-0 oc t 5-15, 6-15, 7	d or 3-9-5 oc purlins, s (6-0-0 max.): 6-8. bracing. 7-13, 8-13, 9-12	
FORCES. (lb) - Max. Comp./M TOP CHORD 2-3=-1550/58, BOT CHORD 2-17=-213/131 WEBS 3-17=-330/154 8-12=-24/464, 8-12=-24/464,	ax. Ten All forces 250 (lb) or less exc 3-5=-1298/49, 5-6=-551/86, 6-7=-428/1 3, 15-17=-110/895, 12-13=-305/54 , 5-17=0/539, 5-15=-758/158, 7-13=-12 9-12=-429/91, 9-11=-52/409, 7-15=-63/	≱pt when shown. 10, 7-8=0/430, 8-9=-17/352 00/124, 8-13=-833/79, 906	2					
 NOTES- 1) Unbalanced roof live loads ha 2) Wind: ASCE 7-10; Vult=115m gable end zone and C-C Exte 29-1-0, Exterior(2) 29-1-0 to 3 Lumber DOL=1.60 plate grip I 3) TCLL: ASCE 7-10; Pr=20.0 ps roof snow: Lumber DOL=1.15 governs. Rain surcharge app 4) Unbalanced snow loads have 5) This truss has been designed non-concurrent with other live 6) Provide adequate drainage to 7) This truss has been designed non-concurrent with other live 6) Provide adequate drainage to 7) This truss has been designed will fit between the bottom cho 9) Refer to girder(s) for truss to ti 10) Provide mechanical connect 11=267. 11) One RT7A USP connectors only and does not consider I 12) Graphical purlin representation 	we been considered for this design. ph Vasd=91mph; TCDL=6.0psf; BCDL= rior(2) -0-10-8 to 2-9-6, Interior(1) 2-9-6 44-3-1, Interior(1) 34-3-1 to 36-5-4 zone; DOL=1.60 sf (roof live load: Lumber DOL=1.15 Pla Plate DOL=1.15); Category II; Exp B; F lied to all exposed surfaces with slopes been considered for this design. for greater of min roof live load of 12.0 loads. prevent water ponding. for a 10.0 psf bottom chord live load no of dro a live load of 20.0psf on the bottom bottom chord live load no in (by others) of truss to bearing plate recommended to connect truss to bearing ateral forces. ion does not depict the size or the orient	6.0psf; h=25ft; Cat. II; Exp to 18-11-0, Exterior(2) 18- C-C for members and force is DOL=1.15); Pg=15.0 psf 'artially Exp.; Ct=1.10, Lu=! less than 0.500/12 in accor osf or 1.00 times flat roof lo nconcurrent with any other n chord in all areas where a = 10.0psf. capable of withstanding 10 ng walls due to UPLIFT at j tation of the purlin along the	B; Enclosed; I 11-0 to 24-1-1, es & MWFRS f f (ground snow 50-0-0; Min. fla rdance with IB4 pad of 11.6 psf live loads. a rectangle 3-6 0 lb uplift at joi it(s) 2. This cor e top and/or bo	WWFRS (envelo Interior(1) 24-1 for reactions sho); Pf=16.5 psf (f tt roof snow load C 1608.3.4. on overhangs -0 tall by 2-0-0 m nt(s) 13 except nnection is for up ttom chord.	ope) -1 to own; lat d wide (jt=lb) plift	South South South	SEAL 044925	A CONTRACTOR OF THE

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September 23,2020



	1 8	3-2-15	16-3-0	1	20-6-4	27-5	-12	1	31-9-0	36-7-0	1
	6	3-2-15	8-0-1	1	4-3-4	6-11	1-8	1	4-3-4	4-10-0	
Plate Offsets ()	X,Y) [1:0-0-0,0)-0-9], [5:0-2-8,0-0-12], [6:0-4-0	,0-4-8], [7:0-4-6,Edg	je]							
	f)										
TCLL (read)	" 20.0	SPACING- 2-0-	0 CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
	20.0	Plate Grip DOL 1.1	5 TC	0.58	Vert(LL)	-0.12 1	14-16	>999	240	MT20	197/144
Snow (Pf/Pg)	16.5/15.0	Lumber DOI 11	5 BC	0.76	Vert(CT)	-0.26	1-16	<u>_</u> 000	180		
TCDL	10.0	Bon Stross Incr. VE		0.75		0.20	11	>000 n/o	n/o		
BCLL	0.0 *			0.75	1012(01)	0.03		n/a	n/a		
BCDL	10.0	Code IRC2015/1PI2014	Matr	IX-5						weight: 246 lb	FI = 20%
					DDA CINC						
LUNIBER-					BRACING-	a					
TOP CHORD	2x4 SP No.2 or 2	2x4 SPF No.2 *Except*			TOP CHORD	Structura	l wood	sheathin	g directly applie	ed or 4-0-0 oc purlins	,
	6-7,5-6: 2x6 SP	No.2				except er	nd vert	icals, and	l 2-0-0 oc purlin	ns (6-0-0 max.): 5-7.	
BOT CHORD	2x4 SP No.2 or 2	2x4 SPF No.2			BOT CHORD	Rigid ceil	ling dire	ectly appl	ied or 6-0-0 oc	bracing.	
WEBS	2x4 SP No.3				WEBS	1 Row at	midpt		5-13, 6-11,	7-11	
									,,		
REACTIONS	(size) 1=0-3	3-9 11=0-3-8 9=Mechanical									
REAGINGING.	(312C) 1=0 (
			0/1 0 50)								
		7(LC 16), 11=-43(LC 13), 9=-23	08(LC 53)								
	Max Grav 1=93	7(LC 53), 11=2118(LC 3), 9=64	I(LC 16)								
FORCES. (lb) - Max. Comp./M	ax. Ten All forces 250 (lb) or	less except when sh	iown.							
TOP CHORD	1-2=-1589/50,	2-4=-1378/43, 4-5=-741/81, 5-	5=-334/64, 6-7=-12/	553, 7-8=-2	26/265,						
	8-9=-40/300										
BOT CHORD	1-16=-199/135	2 14-16=-108/985 13-14=-32/	603								
WERS	2-16271/136	A-16-0/465 A-14663/130 F	-1126/620 5-13-	607/87 6	13-12/837						
WLDO	2-10-2/1/100	, +-10=0/403, +-14=-003/130, 10 7 11 820/04 7 10 0/240	- 1420/023, J-13-	-031/01, 0	-13-12/037,						
	0-11=-1340/14	+9, 7-11=-029/94, 7-10=0/349,	6-10=-310/54								
NOTEO											
NOTES-											
 Unbalanced 	roof live loads ha	ve been considered for this de	sign.								
Wind: ASCE	7-10; Vult=115m	ph Vasd=91mph; TCDL=6.0ps	f; BCDL=6.0psf; h=2	25ft; Cat. II	; Exp B; Enclosed;	MWFRS (e	envelop	ce)			
aable end zo	one and C-C Exte	rior(2) 0-1-13 to 3-9-11. Interio	(1) 3-9-11 to 16-3-0	. Exterior(2	2) 16-3-0 to 21-5-1.	Interior(1)	21-5-1	to			
31-9-0 Exte	rior(2) 31-9-0 to 3	6-5-4 zone C-C for members a	nd forces & MWFRS	for reacti	ons shown. Lumber	DOI = 1.6	0 plate	arip			
						202	o plato	9			
	7 40. 0. 00 0	of (reaf live leads Lympher DOL		5). D~ 45	O not (around anou						
3) TOLL: ASUE	= 7-10; PI=20.0 ps	si (rooi live load: Lumber DOL=	1.15 Plate DOL=1.1	5); Pg=15.	o psi (ground snow); PI=10.5	psi (iii	31		'AH CARO	11,
roof snow: L	umber DOL=1.15	Plate DOL=1.15); Category II;	Exp B; Partially Exp	.; Ct=1.10	, Lu=50-0-0; Min. fla	at roof sno	w load		5	a	1111
governs. Ra	ain surcharge app	lied to all exposed surfaces wit	h slopes less than 0	.500/12 in	accordance with IB	C 1608.3.4	4.		10	FSSIG	N/2
Unbalanced	snow loads have	been considered for this desig	n.							SOLZ YN	
5) Provide ade	quate drainage to	prevent water ponding.							- <i>3</i> / \\\	ST AN	NON/
6) This truss ha	as been designed	for a 10.0 psf bottom chord live	load nonconcurren	t with any	other live loads						
7) * This trues	has been designed	d for a live load of 20 Opef on t	a bottom chord in a	ll areas wh	orre a rectangle 3-6	-0 tall by 2	2-0-0 14	ida		CEAL	1 E
i i i i i i i i i i i i i i i i i i i		and and any other members with		ii aleas wi	lere a rectarigie 5-t	-0 tall by 2	2-0-0 W	lue	- C - C - C - C - C - C - C - C - C - C	SEAL	• •
will fit betwe	en the bottom cho	ord and any other members, wi	n BCDL = 10.0pst.							044025	
 ») Refer to gird 	per(s) for truss to t	russ connections.							2 1	044920	
Provide med	chanical connection	on (by others) of truss to bearin	g plate capable of w	ithstanding	g 100 lb uplift at join	t(s) 11 exc	cept (jt=	=lb)	2 3		1 2
9=258.									2 .		1. 5
10) One RT7A	USP connectors	recommended to connect truss	to bearing walls du	e to UPLIF	T at jt(s) 1. This co	nnection is	s for up	lift	-0-	·SNOWER	A.S.
only and de	oes not consider l	ateral forces.	-						20	GINE	N 8
11) Graphical	nurlin renresentati	on does not depict the size or t	he orientation of the	nurlin alou	on the top and/or bo	ottom chore	Ь		11.	J	1.11
				r 3 0101					1	M. SE	111

September 23,2020



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⊢	6-10-15		13-7-0		20-5-12		27-5-1	2		34-5	5-0 36-7-0	
	6-10-15		6-8-1	I	6-10-12	1	7-0-0		1	6-1	1-4 ' 2-2-0	
Plate Olisets (7	X,Y) [1:0-0-0,0	0-0-9], [15:0-2-	2,0-1-8]									
LOADING (psi TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACIN Plate Gr Lumber Rep Stre Code IR	G- 2-0-0 ip DOL 1.15 DOL 1.15 siss Incr YES cC2015/TPI2014	CSI. TC BC WB Matri	0.84 0.60 0.87 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.07 -0.17 0.04	(loc) 1-17 1-17 12	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 226 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS REACTIONS.	2x4 SP No.2 or 2 2x4 SP No.2 or 2 2x4 SP No.3 (size) 1=0-3 Max Horz 1=19 Max Uplift 1=-16 Max Grav 1=96	2x4 SPF No.2 2x4 SPF No.2 3-8, 12=0-3-8, 3(LC 16) 5(LC 16), 12=-7 6(LC 53), 12=2	10=Mechanical 76(LC 13), 10=-116(L 034(LC 43), 10=60(l	.C 55) .C 54)	E T V	BRACING- TOP CHORD BOT CHORD WEBS	Structura except e Rigid cei 1 Row at	al wood nd verti iling dire t midpt	sheathin cals, and ectly appl	g directly app 2-0-0 oc pur ied or 6-0-0 c 5-12	blied or 3-2-2 oc purlins lins (4-11-11 max.): 4- oc bracing.	s, 8.
FORCES. (Ib) TOP CHORD BOT CHORD WEBS) - Max. Comp./Ma 1-2=-1640/58, 1-17=-154/139 2-17=0/291, 2- 8-11=-310/89,	ax. Ten All fc 2-4=-1066/83, 0, 16-17=-154/ 16=-723/132, 5 7-12=-951/139	rces 250 (lb) or less 4-5=-867/110, 5-7=0, 1390, 14-16=-50/573 5-16=-68/597, 5-14=0	except when sh /465 3, 12-14=-50/573)/345, 5-12=-136	own. 3, 11-12=-46 57/80, 7-11=	65/22 =-31/640,						
NOTES- 1) Unbalanced 2) Wind: ASCE gable end zz 34-5-0, Exte DOL=1.60 3) TCLL: ASCE roof snow: L governs. Ra 4) Unbalanced 5) Provide adee 6) This truss la will fit betwe 8) Refer to gird	roof live loads hat 7-10; Vult=115m one and C-C Exter- prior(2) 34-5-0 to 3 7-10; Pr=20.0 ps umber DOL=1.15 ain surcharge appl snow loads have quate drainage to as been designed has been design	ve been consid ph Vasd=91mp rior(2) 0-1-12 t i6-5-4 zone;C-(sf (roof live load Plate DOL=1. lied to all expoo been consider prevent water for a 10.0 psf I d for a live load ord and any other	lered for this design. sh; TCDL=6.0psf; BC o 3-9-10, Interior(1) 3 c for members and for d: Lumber DOL=1.15 15); Category II; Exp sed surfaces with slo ed for this design. ponding. bottom chord live load d of 20.0psf on the bx her members, with BC ns.	DL=6.0psf; h=2 -9-10 to 13-7-0, orces & MWFRS Plate DOL=1.1! B; Partially Exp. pes less than 0. d nonconcurrent ottom chord in al CDL = 10.0psf.	5ft; Cat. II; E Exterior(2) i for reactior 5); Pg=15.0 .; Ct=1.10, L 500/12 in ar t with any ot II areas whe	Exp B; Enclosed; N 13-7-0 to 18-9-1, Ins shown; Lumber psf (ground snow) Lu=50-0-0; Min. fla ccordance with IBC ther live loads. ere a rectangle 3-6	MWFRS (Interior(1) DOL=1.6); Pf=16.5 t roof snc C 1608.3. -0 tall by	envelop) 18-9-1 50 plate 5 psf (fla 5 w load 4. 2-0-0 w	ie) to grip it		CARO DR ESSION	in the second seco

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=116.
- 10) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 12. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



ENGINEERING BY REENCO A MiTek Affiliate 818 Soundside Road

Edenton, NC 27932

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	5-6-15	10-11-0	-	19-8-12			+	27-5-12				36-7-0	
Plate Offsets (X.)	<u>5-6-15</u> () [1:0-0-0.0-0-9]	5-4-1		8-9-12			<u> </u>	7-9-0				9-1-4	
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 16 TCDL BCLL BCDL	20.0 5.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES I2014	CSI. TC BC WB Matrix	0.84 0.65 0.67 -S		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.17 -0.34 0.04	(loc) 9-10 9-10 10	l/defl >620 >313 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 200 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2 BOT CHORD 2 WEBS 2 REACTIONS.	x4 SP No.2 or 2x4 S x4 SP No.2 or 2x4 S x4 SP No.3 (size) 1=0-3-8, S Max Horz 1=168(LC Max Uplift 1=-5(LC 1 Max Grav 1=937(LC	SPF No.2 SPF No.2 9=Mechanical, 10=0-3 16) (6), 9=-175(LC 35), 1 35), 9=24(LC 16), 10	3-8 0=-120(LC 13) 0=2133(LC 34)			BRA TOP BOT WEE	CING- CHORD CHORD SS	Structura except e Rigid cei 1 Row at	al wood nd verti ling dire t midpt	sheathing cals, and ectly appli	g directly ap 2-0-0 oc pu ed or 6-0-0 5-10	plied or 3-11-15 oc purli rlins (4-11-7 max.): 3-8. oc bracing.	ns,
FORCES. (lb) - TOP CHORD BOT CHORD WEBS	Max. Comp./Max. T 1-2=-1621/69, 2-3= 1-15=-172/1377, 14 2-14=-542/117, 3-1 5-10=-1364/132, 7-	en All forces 250 (l -1207/73, 3-4=-1013, I-15=-172/1377, 12-1 4=0/282, 4-14=-42/3 10=-928/132, 7-9=-1	b) or less exce /95, 4-5=-585/3 4=-73/853, 9-1 39, 4-12=-657/ [,] 1/568	pt when sho 0, 5-7=-62/7 0=-413/11 126, 5-12=-1	own. 768 19/933,								
NOTES- 1) Unbalanced ro 2) Wind: ASCE 7 gable end zone 36-5-4 zone;C 3) TCLL: ASCE 7 roof snow: Lun governs. Rain 4) Unbalanced sr 5) Provide adequ 6) This truss has 7) * This truss has will fit between 8) Refer to girder 9) Provide mecha 9=175, 10=120 10) One RTZA U	of live loads have b -10; Vult=115mph V e and C-C Exterior(2 -C for members and '-10; Pr=20.0 psf (ro nber DOL=1.15 Plat surcharge applied t how loads have been ate drainage to prev been designed for a s been designed for the bottom chord a (s) for truss to truss anical connection (b) 0. SP connectors recoi	een considered for th (asd=91mph; TCDL=) 2) 0-1-12 to 3-9-10, Ir forces & MWFRS fo of live load: Lumber I e DOL=1.15); Catego to all exposed surface n considered for this ent water ponding. a 10.0 psf bottom cho a live load of 20.0ps nd any other member connections. y others) of truss to b	is design. 6.0psf; BCDL=1 tterior(1) 3-9-10 r reactions sho DOL=1.15 Platu ory II; Exp B; Pa swith slopes I design. rd live load nor f on the bottom rs, with BCDL = earing plate ca truss to bearin	5.0psf; h=25 0 to 10-11-0 wn; Lumber e DOL=1.15 artially Exp.; ess than 0.5 concurrent chord in all = 10.0psf. pable of with g walls due	5ft; Cat. I , Exterio DOL=1. ;); Pg=15 ; Ct=1.10 500/12 in with any areas w hstandin- to UPLIF	I; Exp r(2) 10 60 pla 5.0 psf 0, Lu=5 accor other here a g 100	B; Enclosed; I I-11-0 to 16-1- te grip DOL=1 (ground snow 00-0; Min. fla dance with IB(live loads. rectangle 3-6 lb uplift at joint (s) 1. This con	MWFRS (1, Interior .60); Pf=16.5 troof snc C 1608.3. -0 tall by t(s) excep	envelop (1) 16-1 5 psf (fla w load 4. 2-0-0 w ot (jt=lb) s for up	e) -1 to t ide		SEAL 044925	wie

ıg ue to U i ji(S) only and does not consider lateral forces.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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8-3-0			17-0-12		27-5-12	2	1			
	8-3-0	I	8-9-12	1	10-5-0		1		9-1-4	
Plate Offsets (X,Y) [1:0-0-0,0-0-9	9], [5:0-4-8,0-3-0]								
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI	2-0-0 CSI. 1.15 TC 1.15 BC YES WB I2014 Matri	0.99 0.83 0.51 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.18 10-12 -0.38 10-12 0.04 10	l/defl >999 >862 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 184 lb	GRIP 197/144 FT = 20%
LUMBER-			·		BRACING-					
TOP CHORD	2x4 SP No.2 or 2x4	SPF No.2			TOP CHORD	Structural woo	d sheathin	g directly app	lied or 4-4-2 oc purlins	
BOT CHORD	2x4 SP No.2 or 2x4	SPF No.2				except end ver	ticals, and	2-0-0 oc purl	ins (2-2-0 max.): 3-8.	,
WEBS	2x4 SP No.3				BOT CHORD	Rigid ceiling di	rectly appli	ied or 10-0-0	oc bracing, Except:	
						6-0-0 oc bracir	ıg: 9-10.			
					WEBS	1 Row at midp	t	5-10		
REACTIONS.	(size) 1=0-3-8,	9=Mechanical, 10=0-3	3-8							

CTIONS. (size) 1=0-3-8, 9=Mechanical, 10=0-3-8 Max Horz 1=127(LC 16) Max Uplift 9=-129(LC 35), 10=-142(LC 13) Max Grav 1=935(LC 2), 9=10(LC 16), 10=2145(LC 34)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-1609/119, 2-3=-1408/76, 3-4=-1208/92, 4-5=-1079/43, 5-6=-94/1013

BOT CHORD 1-14=-192/1371, 12-14=-104/1348, 10-12=-68/330, 9-10=-517/15

WEBS 2-14=-288/127, 3-14=0/377, 4-12=-518/127, 5-12=0/1012, 5-10=-1701/202, 6-10=-1040/168, 6-9=-14/645

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 4-1-0, Interior(1) 4-1-0 to 8-3-0, Exterior(2) 8-3-0 to 13-5-1, Interior(1) 13-5-1 to 36-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 Unbalanced snow loads have been considered for this design.

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=129, 10=142.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

- Contraction of the 044925 S minin September 23,2020

ENGINEERING BY ENGINEERING BY A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

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Qty Ply KB Home 150.1910.C
142923835
Job Reference (optional)
8.420 s Aug 25 2020 MiTek Industries, Inc. Wed Sep 23 08:16:40 2020 Page 1
ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-YyMmNvjzq1T7K3q09xQWH0C9uhYFDVprfNpVdcyap?5
2 23-9-0 29-10-4 35-11-8 42-2-8
6-1-4 6-1-4 6-3-0

Scale = 1:73.6



23-9-0

29-10-4

35-11-8

42-2-8

11-6-8

5-7-0

17-7-12

5-7-0	5-11-8 6-1-4	6-1-4		6-1-4	6-1-	4 '	6-3-0	·
Plate Offsets (X,Y) [3:0-3-0,0)-2-7], [13:0-1-8,0-3-12]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.40 BC 0.87 WB 0.60 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.38 16-18 -0.74 16-18 0.13 12	l/defl >999 >681 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 552 lb	GRIP 197/144 FT = 20%
10.0								
LUMBER- TOP CHORD 2x6 SP No.2 *E) 1-3: 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 *E) 3-20,5-20,5-16,S REACTIONS. (size) 12=N Max Horz 2=95 Max Horz 2=95	ccept* 2 or 2x4 SPF No.2 ccept* J-16,9-13,11-13: 2x4 SP No.2 or 2x4 SF Mechanical, 2=0-3-8 (LC 53)	BR TO BO	ACING- P CHORD IT CHORD	Structural wood except end vert Rigid ceiling dir	I sheathing c icals, and 2- ectly appliec	lirectly appli 0-0 oc purlir l or 10-0-0 o	ied or 5-5-7 oc purlins. ns (5-0-12 max.): 3-11 oc bracing.	
Max Uplift 12=-3	b34(LC 9), 2=-426(LC 9) ////////////////////////////////////							
FORCES. (lb) Max. Comp./M TOP CHORD 2-3=-4667/909 9-10=-4505/909 9-10=-4505/989 10=-4505/909 BOT CHORD 2-21=-819/405 14-16=-1609/7 3-21=0/390, 3-7-16=-519/264 11-13=-1064/4 11-13=-1064/4	ax. Ten All forces 250 (lb) or less exc h, 3-4=-7088/1518, 4-5=-7087/1517, 5-7 10, 10-11=-4505/980, 11-12=-2325/576 10, 20-21=-823/4036, 18-20=-1875/8688 1391, 13-14=-1609/7391 20=-782/3418, 4-20=-573/286, 5-20=-1 h, 9-16=-327/1539, 9-14=0/350, 9-13=-3 892	ept when shown. =-8787/1905, 7-9=-8787/1 5, 16-18=-1875/8685, 775/399, 5-18=0/353, 182/693, 10-13=-551/284,	905,					
 NOTES- 1) 2-ply truss to be connected to Top chords connected as folk Bottom chords connected as webs connected as follows: 2 2) All loads are considered equa ply connections have been pr 3) Unbalanced roof live loads hat 4) Wind: ASCE 7-10; Vult=115m gable end zone; Lumber DOL=1.15 governs. Rain surcharge app 6) Unbalanced snow loads have 7) This truss has been designed non-concurrent with other live 8) Provide adequate drainage to 9) This truss has been designed 10) * This truss has been designed 	gether with 10d (0.131"x3") nails as foll ows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 row follows: 2x6 - 2 rows staggered at 0-9-0 £x4 - 1 row at 0-9-0 oc. Illy applied to all plies, except if noted as ovided to distribute only loads noted as ive been considered for this design. aph Vasd=91mph; TCDL=6.0ps; BCDL= =1.60 plate grip DOL=1.60 sf (roof live load: Lumber DOL=1.15 Plate Plate DOL=1.15); Category II; Exp B; Flied to all exposed surfaces with slopes been considered for this design. for greater of min roof live load of 12.0 loads. prevent water ponding. for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the bottom hord and any other members. truss connections.	ows: is staggered at 0-9-0 oc. oc. is front (F) or back (B) face (F) or (B), unless otherwis =6.0psf; h=25ft; Cat. II; Ex te DOL=1.15); Pg=15.0 ps Partially Exp.; Ct=1.10, Lu- less than 0.500/12 in acc psf or 1.00 times flat roof I inconcurrent with any other om chord in all areas wher	in the LOAD C/ se indicated. p B; Enclosed; N =50-0-0; Min. fla ordance with IBC load of 11.6 psf er live loads. e a rectangle 3-	ASE(S) section. //WFRS (envelo); Pf=16.5 psf (fl t roof snow load C 1608.3.4. on overhangs 6-0 tall by 2-0-0	Ply to pe) at wide	Solution Solution	SEAL 044925 MGINEER September 23,2	
WARNING - Verify design parar Design valid for use only with MIT a truss system. Before use, the b building design. Bracing indicated is always required for stability and fabrication, storage, delivery, erec Safety Information available fro	neters and READ NOTES ON THIS AND INCLUDE ek® connectors. This design is based only upon p ilding designer must verify the applicability of desis d is to prevent buckling of individual truss web and to prevent collapse with possible personal injury a tion and bracing of trusses and truss systems, see m Truss Plate Institute, 2670 Crain Highway, Suite	D MITEK REFERENCE PAGE MII arameters shown, and is for an in gn parameters and properly incor or chord members only. Additior ind property damage. For genera a ANS/TPH Quality Cri 203 Waldorf, MD 20601	-7473 rev. 5/19/2020 dividual building cor porate this design ir al temporary and pe al guidance regardin iteria, DSB-89 and i	BEFORE USE. nponent, not to the overall ermanent bracing g the BCSI Building Com	ponent		ENGINEERING BY A MITEK 818 Soundside Road Edenton, NC 27932	10 Affiliate

Job	Truss	Truss Type	Qty	Ply	KB Home 150.1910.C	
						I42923835
150_1910_C	HG1	Half Hip Girder	1	2		
				∠	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.4	20 s Aug 2	25 2020 MiTek Industries, Inc. Wed Sep 23 08:16:40 202	20 Page 2
		ID:VMI	062rz1yiHl	D OgRtbn	rlFztQ8K-YyMmNvjzq1T7K3q09xQWH0C9uhYFDVprfNp	Vdcyap?5

NOTES-

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=534.
 One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces. 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-43, 3-11=-53, 2-12=-20

Concentrated Loads (lb)

Vert: 3=-31(B) 6=-26(B) 21=-19(B) 20=-19(B) 4=-26(B) 5=-26(B) 18=-19(B) 7=-26(B) 16=-19(B) 22=-38(B) 23=-1(B) 25=-26(B) 26=-26(B) 27=-26(B) 28=-26(B) 28=-26 30=-26(B) 31=-26(B) 32=-26(B) 33=-26(B) 35=-26(B) 35=-26(B) 35=-26(B) 38=-26(B) 39=-26(B) 40=-38(B) 41=-29(B) 42=-72(B) 43=-19(B) 44=-19(B) 45=-19(B) 45=-19 46=-19(B) 47=-19(B) 48=-19(B) 49=-19(B) 50=-19(B) 51=-19(B) 52=-19(B) 53=-19(B) 54=-19(B) 55=-19(B) 55=-19(B) 57=-23(B) 54=-19(B) 55=-19(B) 55=-19

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a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1** Quality Criteria, DSB-89 and BCSI Building Component **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	KB Home 150.1910.C				
					1429	923836			
150_1910_C	HG10	HALF HIP GIRDER	1	1					
					Job Reference (optional)				
84 Components (Dunn),	Dunn, NC - 28334,		8.4	20 s Aug 2	5 2020 MiTek Industries, Inc. Wed Sep 23 08:16:44 2020 Page	e 2			
		ID:VMD62rz1viHD_OgRtbnrlFztQ8K-QkbGDHmTuGzZph8nOnVSRsMkeJ_v9HZQa?njmNyap?1							

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-43, 2-10=-53, 1-11=-20

Concentrated Loads (lb)

Vert: 2=-31(F) 5=-26(F) 19=-19(F) 7=-26(F) 8=-26(F) 20=-38(F) 21=-1(F) 23=-26(F) 24=-26(F) 25=-26(F) 26=-26(F) 27=-26(F) 28=-26(F) 29=-26(F) 30=-26(F) 31=-26(F) 32=-26(F) 32=-26(F) 33=-26(F) 33=-26(F) 35=-29(F) 36=-72(F) 37=-19(F) 38=-19(F) 40=-19(F) 41=-19(F) 42=-19(F) 43=-19(F) 43=-19(F) 44=-19(F) 45=-19(F) 45=-1

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	001]			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDI 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.26 BC 0.18 WB 0.00 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) -0.01 2-5 >999 240 Vert(CT) -0.03 2-5 >999 180 Horz(CT) 0.01 4 n/a n/a	20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 SLIDER
 Left 2x4 SP No.3 -t 2-4-9

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-11-4 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=95(LC 14) Max Uplift 4=-67(LC 14)

Max Grav 4=116(LC 26), 2=213(LC 2), 5=78(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.



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REACTIONS. (size) 5=Mechanical, 2=0-3-8, 6=Mechanical Max Horz 2=66(LC 14) Max Uplift 5=-13(LC 10), 2=-7(LC 14), 6=-7(LC 14) Max Grav 5=43(LC 2), 2=213(LC 2), 6=106(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 2-5-4, Exterior(2) 2-5-4 to 3-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Plate Offsets (X,Y) [2:0-0-0,0)-0-6], [3:0-2-0,0-2-3]								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 PCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO	CSI. TC 0.19 BC 0.08 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.01 0.01	(loc) 2-5 2-5 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 17 lb	FT = 20%
LUMBER-		BR	ACING-						

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x6 SP No.2

TOP CHORD

Structural wood sheathing directly applied or 3-11-4 oc purlins, except 2-0-0 oc purlins: 3-4.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. 4=Mechanical, 2=0-3-8, 5=Mechanical (size)

Max Horz 2=36(LC 35) Max Uplift 4=-30(LC 7), 2=-15(LC 10)

Max Grav 4=92(LC 2), 2=219(LC 2), 5=76(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 10) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 29 lb down and 19 lb up at 0-11-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 3-4=-53, 2-5=-20

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LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	sf) 20.0 11.6/15.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.05 BC 0.02 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 -0.00	(loc) 2 2 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCLL	0.0 *		Motrix D		0.00	Ũ	, ca	1.70	Woight: 7 lb	ET _ 200/
BCDL	10.0	Code IRC2015/1F12014	Matrix-P						weight. 7 ib	F1 = 20%

LUMBER-

TOP CHORD2x4 SP No.2 or 2x4 SPF No.2BOT CHORD2x4 SP No.2 or 2x4 SPF No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-7-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=36(LC 16) Max Uplift 3=-19(LC 16), 2=-12(LC 16)

Max Grav 3=30(LC 2), 2=132(LC 2), 4=31(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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BCDL

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD

10.0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-7-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. 1=0-3-8, 2=Mechanical, 3=Mechanical (size) Max Horz 1=30(LC 16)

Max Uplift 2=-23(LC 16)

Max Grav 1=62(LC 2), 2=46(LC 2), 3=31(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.



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		10-0-6						
I		10-0-6						1
Plate Offsets (X,Y) [3:0-2-0,I	Edge], [5:0-2-0,Edge]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.14 BC 0.16 WB 0.05 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (l 0.00 0.00 0.00	loc) l/defl 7 n/r 7 n/r 6 n/a	L/d 120 120 n/a	PLATES MT20	GRIP 197/144 FT = 20%
BCDL 10.0		Marix 6					troigitt. 20 lb	11 = 2070
LUMBER- BRACING-								

LUWIDER-		DRACING-	
TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2		2-0-0 oc purlins (6-0-0 max.): 3-5.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=168/8-1-12, 6=168/8-1-12, 8=274/8-1-12 Max Horz 2=-13(LC 17)

Max Horz 2=-13(LC 17) Max Uplift 2=-18(LC 16), 6=-18(LC 17), 8=-2(LC 13)

Max Grav 2=215(LC 39), 6=216(LC 39), 8=342(LC 38)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) apple and zone and C.C. Exterior(2) 0.4-11 to 6-10-2 Interior(1) 6-10-2 to 7-5-3. Exterior(2) 7-5-3 to 9-7-12 zone; C.C. for members and
- gable end zone and C-C Exterior(2) 0-4-11 to 6-10-2, Interior(1) 6-10-2 to 7-5-3, Exterior(2) 7-5-3 to 9-7-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- Gable requires continuous bottom chord bearing.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8.
- 12) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 13) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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		9-7-5		18-11-13	19-3-1 0-3-1/	0 27-5-	12	29-0-3	36-7-0		
Plate Offsets (2	X,Y) [2:0-0-0,	0-0-5], [6:0-3-0,0-2-0], [8:0)-9-8,0-2-4], [9	:Edge,0-2-4]	0-5-1-	¥ 0-2-	2	1-0-7	1-0-13		
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES I2014	CSI. TC BC WB Matrix-	0.80 0.92 0.66 S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.21 -0.46 0.03	(loc) l/de 2-16 >99 2-16 >71 12 n/	fl L/d 9 240 0 180 a n/a	PLATES MT20 MT18HS Weight: 276 lb	GRIP 197/144 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS REACTIONS.	2x4 SP No.2 or 8-9: 2x4 SP No. 2x6 SP No.2 *E 2-15: 2x4 SP No 2x4 SP No.3 (size) 2=0 Max Horz 2=2: Max Uplift 2=-2 Max Grav 2=96	2x4 SPF No.2 *Except* 1 xcept* 5.2 or 2x4 SPF No.2 3-8, 12=0-3-8, 10=Mecha 36(LC 16) (5(LC 16), 12=-88(LC 16), 38(LC 55), 12=2049(LC 2)	nical 10=-241(LC 5 , 10=159(LC 3	5) 8)	BR/ TOF BOT WE	ACING- • CHORD r CHORD BS	Structural except en Rigid ceili 1 Row at	wood shea d verticals, i ng directly a midpt	thing directly appl and 2-0-0 oc purli pplied or 2-2-0 oc 5-14, 6-14,	ed or 3-9-10 oc purlir ns (6-0-0 max.): 6-8. : bracing. 7-12, 8-12, 9-11	s,
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1510/37, 3-5=-1256/28, 5-6=-456/55, 6-7=-317/79, 7-8=-16/421, 8-9=-68/424, 9-10=-70/313 BOT CHORD 2-16=-197/1278, 14-16=-94/862, 11-12=-277/72 BOT CHORD 2-16=-335/155, 5-16=0/530, 5-14=-772/161, 7-14=-59/882, 7-12=-1168/110, 8-12=-775/73, 8-11=0/323, 9-11=-381/97											
NOTES- 1) Unbalanced 2) Wind: ASCE gable end z to 29-0-3, E Lumber DO 3) TCLL: ASCI roof snow: L governs. R 4) Unbalanced 5) This truss h non-concurr 6) Provide ade 7) All plates ar 8) This truss h 9) * This truss will fit betwee 10) Refer to gi 11) Provide mu 10=241. 12) One RT7A uplift only actions Canter transport	I roof live loads has 5 -10; Vult=116n one and C-C Exte xterior(2) 29-0-3 i L=1.60 plate grip E 7-10; Pr=20.0 p umber DOL=1.11 ain surcharge apic as been designed rent with other live equate drainage tu as been designed has been designed the bottom ch rder(s) for truss tu echanical connect USP connectors and does not con agging representat	ave been considered for the nph Vasd=91mph; TCDL= erior(2) -0-10-8 to 2-9-6, In to 34-2-5, Interior(1) 34-2-10DL=1.60 usf (roof live load: Lumber 5 Plate DOL=1.15); Categy oblied to all exposed surface a been considered for this 4 for greater of min roof live loads. Deprevent water ponding. If or a live load of 20.0ps ord and any other membe b truss connections. tion (by others) of truss to recommended to connect sider lateral forces. tion does not depict the size in the size i	his design. 6.0psf; BCDL= terior(1) 2-9-6 5 to 36-5-4 zor DOL=1.15 Pla ory II; Exp B; F es with slopes design. e load of 12.0 ord live load no f on the bottor rs, with BCDL bearing plate t truss to bearing the orient	6.0psf; h=25f to 18-11-13, l ne;C-C for me te DOL=1.15); artially Exp.; (less than 0.50 psf or 1.00 tim n concurrent w n chord in all a = 10.0psf. capable of wit ng walls due to ation of the p	t; Cat. II; Exp Exterior(2) 18 mbers and fo ; Pg=15.0 ps Ct=1.10, Lu= 00/12 in acco nes flat roof k with any other areas where hstanding 10 o UPLIFT at urlin along th	9 B; Enclosed; N 3-11-13 to 24-1 prces & MWFR f (ground snow 50-0-0; Min. fla rdance with IB(bad of 11.6 psf clive loads. a rectangle 3-6 0 Ib uplift at join jt(s) 2 and 12. ⁻⁷ e top and/or bo	AWFRS (e. -14, Interic S for reacti); Pf=16.5 t roof snov C 1608.3.4 on overha -0 tall by 2 -0 tall by 2 -0t(s) excep This conne	envelope) or(1) 24-1-12 ions shown; psf (flat w load k. ngs e-0-0 wide e-0-0 wide ot (jt=lb) ection is for d.	Survey Survey State	SEAL 044925	
WARNIN Design valiu a truss syst building des is always re fabrication, Safety Info	NG - Verify design para d for use only with Mil tem. Before use, the b sign. Bracing indicate aquired for stability and storage, delivery, ere- rmation available fro	meters and READ NOTES ON TH Fe® connectors. This design is b uilding designer must verify the a d is to prevent buckling of individi t to prevent collapse with possibl citon and bracing of trusses and t om Truss Plate Institute, 2670 Cra	IS AND INCLUDE ased only upon pa pplicability of desi ual truss web and/ e personal injury a russ systems, see ain Highway, Suite	D MITEK REFERE arameters shown, gn parameters an or chord members nd property dama ANSI/T 203 Waldorf, MD	NCE PAGE MII- and is for an inc d properly incorp s only. Additiona age. For general PI1 Quality Crit 20601	7473 rev. 5/19/2020 lividual building cor porate this design ir al temporary and pe guidance regardin eria, DSB-89 and I	BEFORE US nponent, not ito the overal rmanent brac g the BCSI Buildin	E. I cing g Component		ENGINEERING BY A MITEK 818 Soundside Road Edenton, NC 27932	LO Affiliate

Job	Truss	Truss Type	Qty	Ply	KB Home 150.1910.C				
			_			142923844			
150_1910_C	T1	ROOF TRUSS	3	1					
					Job Reference (optional)				
84 Components (Dunn),	Dunn, NC - 28334,		8.4	20 s Aug 2	25 2020 MiTek Industries, Inc. Wed Sep 23 08:16:50 2020	Page 2			
		ID:VMD62rz1yiHD_OqRtbnrlFztQ8K-FuyYUKrET6jiXcbxl2csg7cm5jxuZ0KJyxE1y1yap_x							

NOTES-

14) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

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L	9-7-5	18-11-13		24-0-0	29-0-	-3	36-10-4	37-	Q-0 4	42-6-0	
	9-7-5	9-4-8	I	5-0-3	5-0-3	3 '	7-10-1	0-1	-12	5-6-0	
Plate Offsets (X,Y) [6:0-3	-0,0-2-0], [8:0-3-0,0-2-0	0], [9:0-4-8,0-3-4]									
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DO Lumber DOL Rep Stress Ir Code IRC20	2-0-0 DL 1.15 . 1.15 nor YES 15/TPI2014	CSI. TC 0. BC 0. WB 0. Matrix-S	.80 .61 .46	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.14 18-20 -0.25 18-20 0.05 12	l/defl >999 >999 n/a	L/d 240 180 n/a		PLATES MT20 Weight: 312 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 REACTIONS. (size) 2 Max Horz 2 Max Uplift 2 Max Grav 2	or 2x4 SPF No.2 =0-3-8, 12=0-3-8 =184(LC 16) =-56(LC 16) =1500(LC 2), 12=1938((LC 2)		BR TO BO WE	i acing- P Chord It Chord EBS	Structural wood except end verti Rigid ceiling dir 1 Row at midpt	sheathing icals, and 2 ectly applie	l directly ap 2-0-0 oc pi ed or 10-0- 5-18, 7-	pplied of urlins (4 -0 oc bra 14, 9-12	r 2-11-11 oc purli -9-6 max.): 6-8. acing. :	ns,
FORCES. (lb) - Max. Comp TOP CHORD 2-3=-2638 8-9=-1105 8-9=-1105 BOT CHORD 2-20=-202 WEBS 3-20=-305)./Max. Ten All forces 124, 3-5=-2374/130, 5- 151, 9-10=-21/304 2261, 18-20=-102/1840 151, 5-20=0/536, 5-18=	250 (lb) or less exce 6=-1632/175, 6-7=-1), 14-18=-13/1221 764/157, 6-18=0/43	pt when shown 396/190, 7-8=- 88, 7-18=-35/49	n. •914/170, 96, 7-14=-7	739/93,						
 8-14=0/264 NOTES- Uhbalanced roof live load Wind: ASCE 7-10; Vult=1 gable end zone and C-C B 24-11-15 to 29-0-3, Exteri reactions shown; Lumber TCLL: ASCE 7-10; Pr=20 roof snow: Lumber DOL= governs. Rain surcharge Uhbalanced snow loads h This truss has been desig non-concurrent with other Provide adequate drainag This truss has been desig * This truss has been desig * This truss has been desig % This truss has been de	I, 9-14=0/1011, 9-12=-1 s have been considered 15mph Vasd=91mph; T Exterior(2) -0-10-8 to 3 or(2) 29-0-3 to 35-0-5, I DOL=1.60 plate grip DC 0 psf (roof live load: Lu I.15 Plate DOL=1.15); (applied to all exposed s ave been considered fo ned for greater of min ro live loads. e to prevent water pond ned for a 10.0 psf botto gned for a live load of 2 chord and any other m rs recommended to corn nsider lateral forces. ntation does not depict f IS DESIGNED AS UNII	I695/164 d for this design. CDL=6.0psf; BCDL= 4-8, Interior(1) 3-4-8 Interior(1) 35-0-5 to 4 DL=1.60 mber DOL=1.15 Plat Category II; Exp B; P surfaces with slopes I or this design. oof live load of 12.0 p ding. m chord live load nor 20.0psf on the bottom iembers, with BCDL = innect truss to bearing the size or the orient: NHABITABLE.	6.0psf; h=25ft; to 18-11-13, E: 2-4-4 zone;C-(e DOL=1.15); 1 artially Exp.; C ess than 0.500 osf or 1.00 time nconcurrent with chord in all ar = 10.0psf. y walls due to L ation of the pur	Cat. II; Exp xterior(2) 1 C for memb Pg=15.0 ps t=1.10, Lu= 0/12 in accord as flat roof I th any other eas where JPLIFT at j din along th	p B; Enclosed; N 8-11-13 to 24-1 bers and forces of (ground snow), =50-0-0; Min. fla ordance with IBC load of 11.6 psf er live loads. a rectangle 3-6 t(s) 2 and 12. The top and/or bo	//WFRS (envelog 1-15, Interior(1) & MWFRS for); Pf=16.5 psf (fli t roof snow load C 1608.3.4. on overhangs -0 tall by 2-0-0 w his connection is ttom chord.	oe) at vide for	Contraction of the second seco	LUN CONTRACTOR	H CARO SEAL 044925	Weiner Annun Annu Annun Annu

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



September 23,2020



 	9-7-5 9-7-5		18-11-13 9-4-8	24-0-0		29-0-3	35-3-3 6-3-0		42-4-4	42-6-0	48-0-0	
Plate Offsets (2	X,Y) [6:0-3-0,0	0-2-0], [8:0-3-0,0-2-0]]			000	000			0112	000	
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DO Lumber DOL Rep Stress In Code IRC201	2-0-0 DL 1.15 1.15 cr YES 15/TPI2014	CSI. TC 0 BC 0 WB 0 Matrix-S	.73 .76 .86	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.22 14-16 -0.42 14-16 0.08 14	l/defl >999 >999 n/a	L/d 240 180 n/a	F	PLATES MT20 Weight: 331 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.2 or 2 2x6 SP No.2 2x4 SP No.3	2x4 SPF No.2			BF TC BC	RACING- DP CHORD DT CHORD EBS	Structural wood 2-0-0 oc purlins Rigid ceiling dire 6-0-0 oc bracing 1 Row at midpt 2 Rows at 1/3 p	sheathin (4-1-10 r ectly appl g: 12-14. ts	g directly a nax.): 6-8. ied or 10-0 5-20, 7- 9-14	opplied or 2 -0 oc brac -20, 7-16	2-7-6 oc purlins	, except
REACTIONS.	(size) 2=0-3 Max Horz 2=13 Max Uplift 2=-50 Max Grav 2=17	3-8, 14=0-3-8 00(LC 16) 0(LC 16), 14=-40(LC 13(LC 2), 14=2230(I	: 17) LC 2)									
FORCES. (Ib TOP CHORD) - Max. Comp./M 2-3=-3091/154 8-9=-1877/188	ax. Ten All forces : , 3-5=-2852/155, 5-6 , 9-11=-83/492, 11-1	250 (lb) or less exc S=-2120/200, 6-7=- 22=-176/489	ept when show 1836/207, 7-8≕	n. -1615/202	,						
BOT CHORD WEBS	2-22=-134/266 3-22=-295/152	, 20-22=-34/2274, , 5-22=0/533, 5-20=	16-20=0/1791, 14-1 -758/157, 6-20=-4/6	16=-24/1396, 12 654, 8-16=0/559	2-14=-356/ 9, 9-16=0/3	/206 395,						

11-14=-408/172, 7-16=-531/106, 9-14=-2291/192

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 18-11-13, Exterior(2) 18-11-13 to 25-9-4, Interior(1) 25-9-4 to 29-0-3, Exterior(2) 29-0-3 to 35-9-11, Interior(1) 35-9-11 to 48-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs
- non-concurrent with other live loads.6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

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L	9-7-5	18-11-	13 2	4-0-0	29-0-3	35-3-3		42-4-4	42-6-0	48-0-0	_
	9-7-5	9-4-6	8 ' 5	5-0-3	5-0-3	6-3-0	1	7-1-1	0-1-12	5-6-0	
Plate Offsets (X	,Y) [6:0-3-0,0-	2-0], [8:0-3-0,0-2-0]									
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLL BCDL	20.0 6.5/15.0 10.0 0.0 * 10.0	SPACING-2-0Plate Grip DOL1.Lumber DOL1.Rep Stress IncrYCode IRC2015/TPI201	0-0 CSI. 15 TC 15 BC ES WB 4 Matri	0.73 0.76 0.86 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.22 14-16 -0.43 14-16 0.09 14	l/defl >999 >999 n/a	L/d 240 180 n/a	PLA MT2 Weię	TES 0 ght: 312 lb	GRIP 197/144 FT = 20%
LUMBER-				в	RACING-						
REACTIONS.	2x4 SP No.2 or 2 2x6 SP No.2 2x4 SP No.3 (size) 2=0-3 Max Horz 2=130 Max Uplift 2=-50 Max Grav 2=171	x4 SPF No.2 -8, 14=0-3-8 (LC 16) (LC 16), 14=-40(LC 17) 3(LC 2), 14=2230(LC 2)		T B V	OP CHORD SOT CHORD VEBS	Structural wood 2-0-0 oc purlins Rigid ceiling dire 6-0-0 oc bracing 1 Row at midpt 2 Rows at 1/3 pt	sheathing (4-1-7 ma ectly applie : 12-14. :s	directly app x.): 6-8. sd or 10-0-0 5-18, 7-18 9-14	lied or 2-7-6 oc bracing, 1, 7-16	6 oc purlins. Except:	except
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	- Max. Comp./Ma 2-3=-3090/154, 8-9=-1886/189, 2-20=-135/2660 3-20=-295/152, 9-16=0/397, 9-1	x. Ten All forces 250 (lb) o 3-5=-2859/155, 5-6=-2134/1 9-11=-84/491, 11-12=-177/4 0, 18-20=-34/2283, 16-18=0/ 5-20=0/529, 5-18=-757/157, 4=-2290/195, 11-14=-408/1	r less except when sh 99, 6-7=-1848/207, 7- 89 1801, 14-16=-25/1402 6-18=-4/660, 7-16=-5 72	iown. -8=-1623/203 2, 12-14=-355 534/104, 8-16	3, 5/207 6=0/563,						
NOTES- 1) Unbalanced I 2) Wind: ASCE	roof live loads hav 7-10: Vult=115mr	e been considered for this do	esign. sf: BCDI =6 0psf: h=2	5ft: Cat II: F	-xp B: Enclosed: I	WFRS (envelor	e)				

- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 18-11-13, Exterior(2) 18-11-13 to 25-9-4, Interior(1) 25-9-4 to 29-0-3, Exterior(2) 29-0-3 to 35-9-11, Interior(1) 35-9-11 to 48-10-8 zone; C-C for members and forces & MWFRS for reactions
- shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This connection is for uplift only and does not consider lateral forces.

Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
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2x4 1/

2x4 📎

0-0 CSI. .15 TC 0.04 .15 BC 0.11 ES WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
I4 Matrix-P BR	ACING-					Weight: 9 lb	F1 = 20%
1	.15 BC 0.11 ES WB 0.00 14 Matrix-P	.15 BC 0.11 Vert(CT) ES WB 0.00 Horz(CT) 14 Matrix-P BRACING- TOP CHORD S	.15 BC 0.11 Vert(CT) n/a ES WB 0.00 Horz(CT) 0.00 14 BRACING- TOP CHORD Structura	.15 BC 0.11 Vert(CT) n/a - ES WB 0.00 Horz(CT) 0.00 3 14 BRACING- TOP CHORD Structural wood	.15 BC 0.11 Vert(CT) n/a - n/a ES WB 0.00 Horz(CT) 0.00 3 n/a Matrix-P BRACING- TOP CHORD Structural wood sheathin	.15 BC 0.11 Vert(CT) n/a - n/a 999 ES WB 0.00 Horz(CT) 0.00 3 n/a n/a 14 Matrix-P BRACING- TOP CHORD Structural wood sheathing directly appr	.15 BC 0.11 Vert(CT) n/a - n/a 999 ES WB 0.00 Horz(CT) 0.00 3 n/a n/a 14 Matrix-P Weight: 9 lb Weight: 9 lb

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 1=3-3-12, 3=3-3-12 (size) Max Horz 1=-16(LC 12) Max Uplift 1=-2(LC 14), 3=-2(LC 15) Max Grav 1=96(LC 2), 3=96(LC 2)

BOT CHORD 2x4 SP No.3

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MILER KEERENCE PAGE MIL-7475 fev. or 19/2/2/0/ DEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-0-10, Exterior(2) 4-0-10 to 7-0-10, Interior(1) 7-0-10 to 7-7-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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	0-0 <u>-6</u> 0-0-6	<u>5-1-4</u> 5-0-14						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. D TC 0.13 V/ BC 0.07 V/ WB 0.02 H Matrix-P H H	DEFL. in /ert(LL) n/a /ert(CT) n/a łorz(CT) 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 17 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3OTHERS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-1-4 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=5-0-8, 3=5-0-8, 4=5-0-8 Max Horz 1=-27(LC 10) Max Uplift 1=-10(LC 14), 3=-14(LC 15) Max Grav 1=89(LC 2), 3=89(LC 2), 4=154(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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