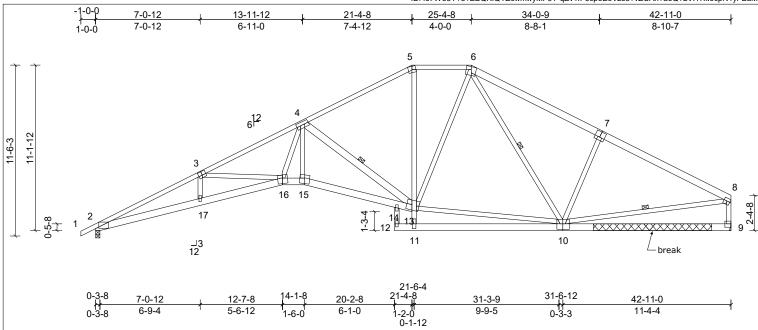


UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, clm

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Repair for a break in the bottom chord where indicated.

Attach 2x6 x 8' SP or SPF No.2 scab to each face of truss centered at the break with 2 rows of 10d (.131" x 3") nails spaced 4" oc

BRACING

WEBS

TOP CHORD

1 Row at midpt

Plate C	Seats (X, Y): [2:0-2-11,Edge], [3:0-3-0,0-3-0], [7:0-4-0,0-4-8], [8:Edge,0-2-4], [9:Edge,0-3-8], [10:0-4-12,0-4-8], [15:0-4-0,0-4-8], [16:												
Loadir	ng	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	(roof)	20.0	Plate Grip DOL	1.15	TC	0.83	Vert(LL)	-0.37	16	>999	240	MT20	244/190
TCDL		10.0	Lumber DOL	1.15	BC	0.82	Vert(CT)	-0.74	16-17	>696	180		
BCLL		0.0 *	Rep Stress Incr	YES	WB	0.82	Horz(CT)	0.38	9	n/a	n/a		
BCDL		10.0	Code	IRC2015/TPI2014	Matrix-MSH							Weight: 329 lb	FT = 20%

2x6 SP No.2 *Except* T2:2x4 SP SS, T3:2x4 SP No.2, T1:2x4 SP No.1 WERS 2x4 SP No.3 *Except* W5:2x6 SP No.2

> 2=1772/0-3-8, (min. 0-2-0), 9=1710/ Mechanical, (min. 0-1-8) Max Horiz 2=210 (LC 14)

2x6 SP No.2 *Except* B1:2x6 SP No.1, B4:2x4 SP No.3

Max Uplift 2=-256 (LC 10), 9=-198 (LC 11)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when sho

2-3=-5912/1467, 3-4=-5250/1299, 4-5=-2287/708, 5-6=-1942/690, 6-7=-2238/764, 7-8=-2426/684, 8-9=-1603/488

BOT CHORD 2-17=-1273/5368, 16-17=-1282/5391, 15-16=-829/4117, 14-15=-869/4296, 13-14=-831/4216 WEBS

3-16=-629/296. 6-10=-184/329. 7-10=-559/411. 4-13=-2809/761. 6-13=-111/586. 11-13=0/453. 5-13=-143/656. 8-10=-456/1970. 10-13=-207/1642. 4-15=-153/1102. 4-16=-338/1570

NOTES (10)

LUMBER

TOP CHORD

BOT CHORD

REACTIONS

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4)
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 256 lb uplift at joint 2 and 198 lb uplift at joint 9.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- This repair has been prepared based on information and use conditions supplied by client. Designer has made a good faith effort to outline damage and repair conditions as reported by client. When actual field conditions do not approximate those indicated on this drawing, client shall immediately inform the engineer and refrain from applying the repa



Structural wood sheathing directly applied or 2-1-10 oc purlins, except end verticals, and 2-0-0 oc purlins (4-0-9 max), 5-6. Rigid celling directly applied or 6-0-0 oc bracing.

6-10, 4-13, 8-10

This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.

