

THE LEE AT ATHERSTONE COMMUNITY

SQUARE FOOTAGES

FIRST FLOOR (HTD.) = 815 sf SECOND FLOOR (HTD.) = 1255 sf 2070 sf

GARAGE = 583 sf FRONT PORCH = 76 sf

TOTAL

= 2729 sf

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D1-D7 STANDARD DETAILS

ENGINEER

SUMMIT ENGINEERING

2520 WHITEHALL PARK DRIVE SUITE 250 CHARLOTTE, NC 28273 704-504-1717

ARCHITECT

COX ARCHITECTURE & DESIGN, PLLC

R. CRAIG COX 1310 SOUTH TRYON STREET SUITE 111 CHARLOTTE, NC 28203 980-237-3827

WWW.COXARCHITECTURE.COM CRAIG@COXARCHITECTURE.COM

GENERAL CONTRACTOR

LGI HOMES

SCOTT STERLING
V.P. OF CONSTRUCTION FOR MID-ATLANTIC 704-953-3824



EE COMMUNITY ANGIER, NORTH CAROLINA THE LE

PERMIT SET FOR CONSTRUCTION

● 14 OCTOBER 2021

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COVER SHEET

FLOOR PLAN NOTES

-CONTRACTORS TO FIELD VERIFY ALL DIMENSIONS & NOTIFY ARCHITECT OF ANY DISCREPANCIES, ERRORS OR OMISSIONS PRIOR TO EXECUTION OF WORK.

-CLEANUP TO OCCUR DAILY.

TYPICAL ATTIC INSULATION.

-G.C. TO VERIFY FINISH GRADE @ HOUSE TO DETERMINE NUMBER OF STEPS.

-MECHANICAL CONTRACTOR TO COORDINATE W/ ARCHITECT LOCATION OF MAIN TRUNK & DISTRIBUTION LINES, REGISTERS (CENTER ALL REGISTERS ON WINDOWS), THERMOSTATS, AIR HANDLER & CONDENSERS. -CEILING HEIGHTS LISTED ARE DIMENSIONED TO FRAMING (TOP OF SUBFLOOR TO UNDERSIDE OF FRAMING ABOVE)

-CONCRETE SLABS & SETTING BEDS TO ACCOMMODATE FOR ADEQUATE WATER DRAINAGE AT GARAGES AND PORCHES ADEQUATE WATER DRAINAGE AT GARAGES AND PORCHES
ATTIC ACCESS DROP-DOWN STAIRS TO CONFORM WITH
LOCAL AUTHORITIES BASED ON IRC (R807.1) MINIMUM NET
CLEAR OPENING 20° x 30°. ALL ATTIC ACCESS STAIRS TO
BE WEATHER STRIPPED & SEALED WITH R-VALUES THAT
CONFORM WITH LOCAL AUTHORITIES BASED ON IRC
(M1102.24) GG TO PROVIDE & INSTALL INSULATION DAMS TO
RESTRICT TYPICAL ATTIC INSULATION FROM FALLING
THROUGH ATTIC ACCESS OPENING. RIGIO FOAM BOX COVER
TO BE INSTALLED & SEALED AROUND FRAMING OF OPENING.
NOT TO MIRPOR OF OR PROVIDED TO THE POLICENT IOT TO IMPEDE OR OBSTRUCT PERFORMANCE OF ADJACENT

-HOSE BIBB(S) TO BE LOCATED 24" ABOVE FIRST FLOOR

WINDOW NOTES

-ALL WINDOW DIMENSIONS ARE BASED ON M.I. WINDOW ROUGH OPENING CALL OUTS, UNO. FINAL SELECTION OF WINDOW SIZES ARE TO BE VERIFIED IN FIELD. -WINDOWS TO BE INSTALLED BY CERTIFIED WINDOW INSTALLER PER MANUFACTURER'S INSTALLATION INSTRUCTIONS.

-WINDOW SUPPLIER TO SPECIFY & ORDER TEMPERED GLASS IN WINDOWS AS REQ'D BY LOCAL CODE.

IN WINDOWS AS RED'D BY LOCAL CODE.

-G. AND WINDOW SUPPLIER TO VERIFY THAT EACH BEDROOM
TO HAVE A MINIMUM OF ONE WINDOW WHICH MEETS
EMERGENCY EGRESS AS REQUIRED BY LOCAL AUTHORITIES
BASED ON IRC 2018, WINDOW SUPPLIER TO ADD EGRESS
HARDWARE TO CASEMENT WINDOWS IF NECESSARY.

-TOP OF INTERIOR CASING @ ADJACENT DOORS & WINDOWS TO ALIGN WHEN HEADER CALL OUTS ARE EQUAL

DOOR NOTES

-ATTIC ACCESS DOORS TO INCLUDE WEATHER STRIPPING & INSULATION

-TOP OF INTERIOR CASING @ ADJACENT DOORS & WINDOWS TO ALIGN WHEN HEADER CALL OUTS ARE EQUAL -DOOR SUPPLIER TO SPECIFY & ORDER TEMPERED GLASS IN DOORS AS REQ'D BY LOCAL CODE.

DOOR & WINDOW LEGEND

— HEIGHT: 6'-8" WIDTH: 3'-0" DOORS: P = POCKET WINDOWS: SH = SINGLE HUNG F = FIXED

INSULATION NOTES

INSULATION VALUES PER 2018 NCRC CH. 11 ENERGY CONSERVATION CODE

ANGIER, NC HARNETT COUNTY - CLIMATE ZONE 4A

TABLE N1102.1.2

CEILING: R-38 FLOOR: R-19 WALL: R-15 SLAB: R-10, 2FT

DESIGN SPECIFICATIONS

USE GROUP: (IBC 310)

"R-3" ONE & TWO FAMILY DWELLING

CONSTRUCTION CLASS: (IBC 601)
"TYPE V-B" UNPROTECTED

HEIGHT & AREA LIMIT: (LOCAL ZONING) 35' MAXIMUM 2 STORY HEIGHT

EMERGENCY ESCAPE: (IRC 310-311)

EGRESS OR RESCUE WINDOWS FROM SLEEPING ROOMS SHALL HAVE MINIMUM OF 4.0 SQ. FT. NET CLEAR OPENING.

MINIMUM 20" WIDTH.

MINIMUM 22" HEIGHT. MAXIMUM 44" SILL HEIGHT

GARAGE / HOUSE CEILING / ASSEMBLY: (IRC 702)

½" GYPSUM WALL BOARD

5/4" TYPE "X" GYPSUM BOARD CEILING WHERE LIVING IS ABOVE 20 MINUTE RATED GARAGE / HOUSE DOOR

ATTIC VENTILATION: (IRC 806)

[TOTAL ATTIC SQ. FT.] / [300] = SQ. FT. AREA REQUIRED

RIDGE VENT:

[LINEAR FEET OF VENT] X [18 SQUARE INCHES IN FREE AREA] / 12 = SQ. FT. PROVIDED [LINEAR FEET OF VENT] X [7 SQUARE INCHES IN FREE AREA] / 12 = SQ. FT. PROVIDED

EDGE SHINGLE OVER VENT:

[LINEAR FEET OF VENT] X [9 SQUARE INCHES IN FREE AREA] / 12 = SQ. FT. PROVIDED

ROOF LOUVER VENTS: [NUMBER OF VENTS] X [70 SQUARE INCHES IN FREE AREA] / 12 = SQ. FT. PROVIDED

CRAWL SPACE VENTILATION: (IRC 408)

[TOTAL CRAWL SPACE SQ. FT.] / [300] = SQ. FT. AREA REQUIRED FOUNDATION VENT:

FREE SPACE PROVIDED BY VENT = F [FREE AREA REQUIRED] / F = NUMBER OF VENTS REQUIRED SQUARE FOOTAGES

FIRST FLOOR (HTD.) SECOND FLOOR (HTD.) = 1255 sf 2070 sf = 583 sf = 76 sf = 2729 sf

FLOOR PLAN LEGEND

1 ROD, 2 SHELVES 2 ROD, 2 SHELVES 2R 2S HR HANGING ROD CO W D CASED OPENING WASHER, DRYER D/W FRIG LS M DISH WASHER REFRIGERATOR LAZY SUSAN MIRROR SHOWER HEAD **⊕**⊕ RAIN HEAD TEMPERED GLASS

WALL SCHEDULE

FRAMED WALLS

---- OVERHEAD/BELOW

ALL WALLS ARE 2x4 WOOD STUD WALLS, UNO 5 1/2" DIMENSION INDICATES 2x6 WOOD STUD WALL

STAIR NOTES

-STAIR FABRICATOR / INSTALLER TO VERIFY THAT STAIRS MEET ALL REQ'D CODES

-ADJUSTMENTS TO STAIR TO BE CONFIRMED W/ ARCHITECT & CONTRACTOR PRIOR TO STAIR CONSTRUCTION

CEILING HEIGHT NOTES

8' - 1 $\frac{1}{2}$ " CEILING HEIGHTS ON FIRST FLOOR 8' - 1 $\frac{1}{2}$ " CEILING HEIGHTS ON SECOND FLOOR

MEASURED FROM TOP OF SUBFLOOR / CONCRETE SLAB TO BOTTOM OF FLOOR JOISTS / ROOF TRUSSES

COLUMN NOTES

COLUMNS TO BE: AFCO OR COLUMN OF EQUAL BEARING CAPACITY. (6000 # MINIMUM)
TOP CONNECTION: (2) #8 - ½* x 3" STAINLESS STEEL SCREWS FRE SIDE INSERTED INTO BEAM.
BOTTOM CONNECTION: (3) UBS - #18043
BRACKETS FASTENED WITH (2) ½* x 1½*
SCREWS INTO COLUMN & (2) ½* x 3 ½*
CONCRETE SCREWS THROUGH FASTENER
INTO CONCRETE

ELECTRICAL PANEL/METER

MAXIMUM DISTANCE BETWEEN ELECTRICAL PANEL & ELECTRICAL METER (NEC 230.70) TO BE DETERMINED BY LOCAL AUTHORITY.

GENERAL NOTES

-DO NOT SCALE DRAWINGS; DESIGNATED DIMENSIONS SHALL BE USED IN PREFERENCE TO MEASUREMENTS BY SCALE.

-GENERAL CONTRACTOR SHALL VERIFY AND COMPLY TO ALL LOCAL & NATIONAL BUILDING CODES. CONTACT ARCHITECT IF INSPECTORS REQUIRE REVISIONS OR ALTERATIONS TO DRAWNOS.

-ALL SUB-CONTRACTORS SHALL BE RESPONSIBLE FOR DAMAGE TO OTHER TRADES.

B'FAST H.S. 94 sf COAT H.S. 5 sf CONTRACTOR TO PROVIDE & INSTALL CONTINUOUS FIRE SEPARATION PROTECTION @ GARAGE WALLS & CEILING. **KITCHEN** FOR SEPARATION SHALL HAVE & GWB OR EQUIVALENT W/FASTENERS @ 6" O.C. PER LOCAL AUTHORITIES BASED ON IRC TABLE 702.1(3) -MINIMUM ¾" TYPE X GWB @ GARAGE CEILING -MINIMUM X* GWB @ GARAGE WALLS -MINIMUM NO. 26 GAGE SHEET STEEL HVAC DUCT WHERE PENETRATIONS OCCUR PER LOCAL AUTHORITIES BASED ON IRC SECTION 302.5-302.7 STUDY / HOME OFFICE 108 sf SLOPING 42" TALL | | 81 | 2x4 HALF-WALL 10'-6¹/₄" 10'-6<u>1</u>" 21'-02" (HOUSE FRAMING)







7'-4"

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CONC. SLAB FLOOR PLAN & GENERAL NOTES



SQUARE FOOTAGES

FIRST FLOOR (HTD.) = 815 sf SECOND FLOOR (HTD.) = 1255 sf 2070 sf

GARAGE = 583 s FRONT PORCH = 76 s

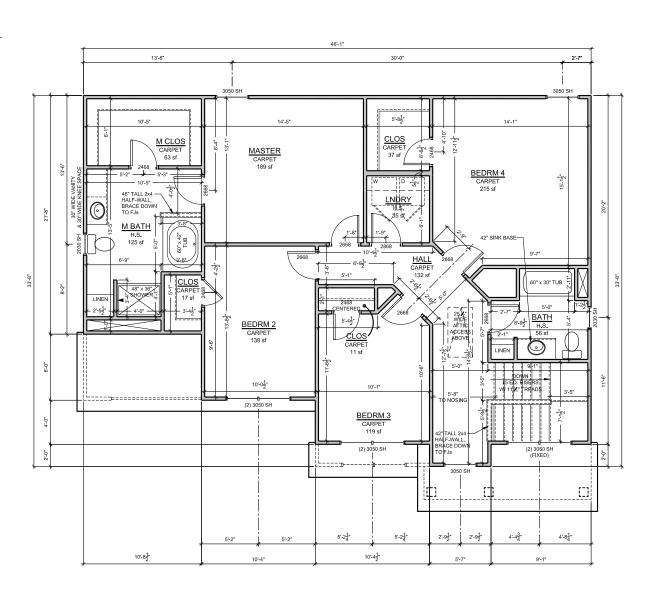
= 2729 sf

TOTAL

CEILING HEIGHT NOTES

 8° - 1 ½" CEILING HEIGHTS ON FIRST FLOOR 8° - 1 ½" CEILING HEIGHTS ON SECOND FLOOR

MEASURED FROM TOP OF SUBFLOOR / CONCRETE SLAB TO BOTTOM OF FLOOR JOISTS / ROOF TRUSSES



1 SECOND FLOOR PLAN
A2.1 1/4" = 1'-0"

THE LEE

ANGIER, NORTH CAROLINA

PERMIT SET
FOR CONSTRUCTION

A2.1

INSULATION NOTES

INSULATION VALUES PER 2018 NCRC CH. 11 ENERGY CONSERVATION CODE

ANGIER, NC HARNETT COUNTY - CLIMATE ZONE 4A

TABLE N1102.1.2

CEILING: R-38 FLOOR: R-19 WALL: R-15 SLAB: R-10, 2FT

STAIR WAYS NOTE: COMPLY WITH SECTION R-311, R-312

HANDRAILS & GUARDRAILS NOTE: COMPLY WITH SECTION R-311, R-312

-CONTRACTORS TO FIELD VERIFY ALL DIMENSIONS & NOTIFY ARCHITECT OF ANY DISCREPANCIES, ERRORS OR OMISSIONS PRIOR TO EXECUTION OF WORK.

-ALL ROOF PENETRATIONS TO BE PLACED ON REAR SIDE OF MAIN RIDGE OR AS SPECIFIED BY ARCHITECT. PAINT TO MATCH SHINGLE COLOR.

-ATTIC INSULATION TO BE BATT. INSUL. PER CODE, PROVIDE BAFFLES @ PERIMETER TO ALLOW 2" FOR AIRFLOW FROM EAVE VENTS TO RIDGE VENTS.

-ROOF SHEATHING TO BE $\frac{1}{2}$ " T&G PLYWOOD W/ METAL CLIPS @ ENDS.

-ALL BATHROOM & DRYER VENT PENETRATIONS TO RUN TOWARD REAR OF HOUSE & VENT IN REAR OUTSIDE WALL OR ROOF BEHIND MAIN RIDGE.

-GUTTER & DOWNSPOUT INSTALLER TO PROVIDE ADEQUATE UNITS PER MANUFACTURER SPECIFICATIONS BASED ON ROOF COVERAGE. SUB-CONTACTOR TO VERIFY NUMBER & LOCATION OF DOWNSPOUTS

-ALL SHINGLED ROOFS WITH A PITCH OF 4:12 OR LESS REQUIRE (2) LAYERS OF 30# FELT PAPER PER LOCAL AUTHORITIES BASED ON IRC 2018

STAIR DETAIL / NOTES

ROOF NOTES CEILING HEIGHT NOTES

 8^{\prime} - 1 ½" CEILING HEIGHTS ON FIRST FLOOR 8^{\prime} - 1 ½" CEILING HEIGHTS ON SECOND FLOOR

MEASURED FROM TOP OF SUBFLOOR / CONCRETE SLAB TO BOTTOM OF FLOOR JOISTS / ROOF TRUSSES



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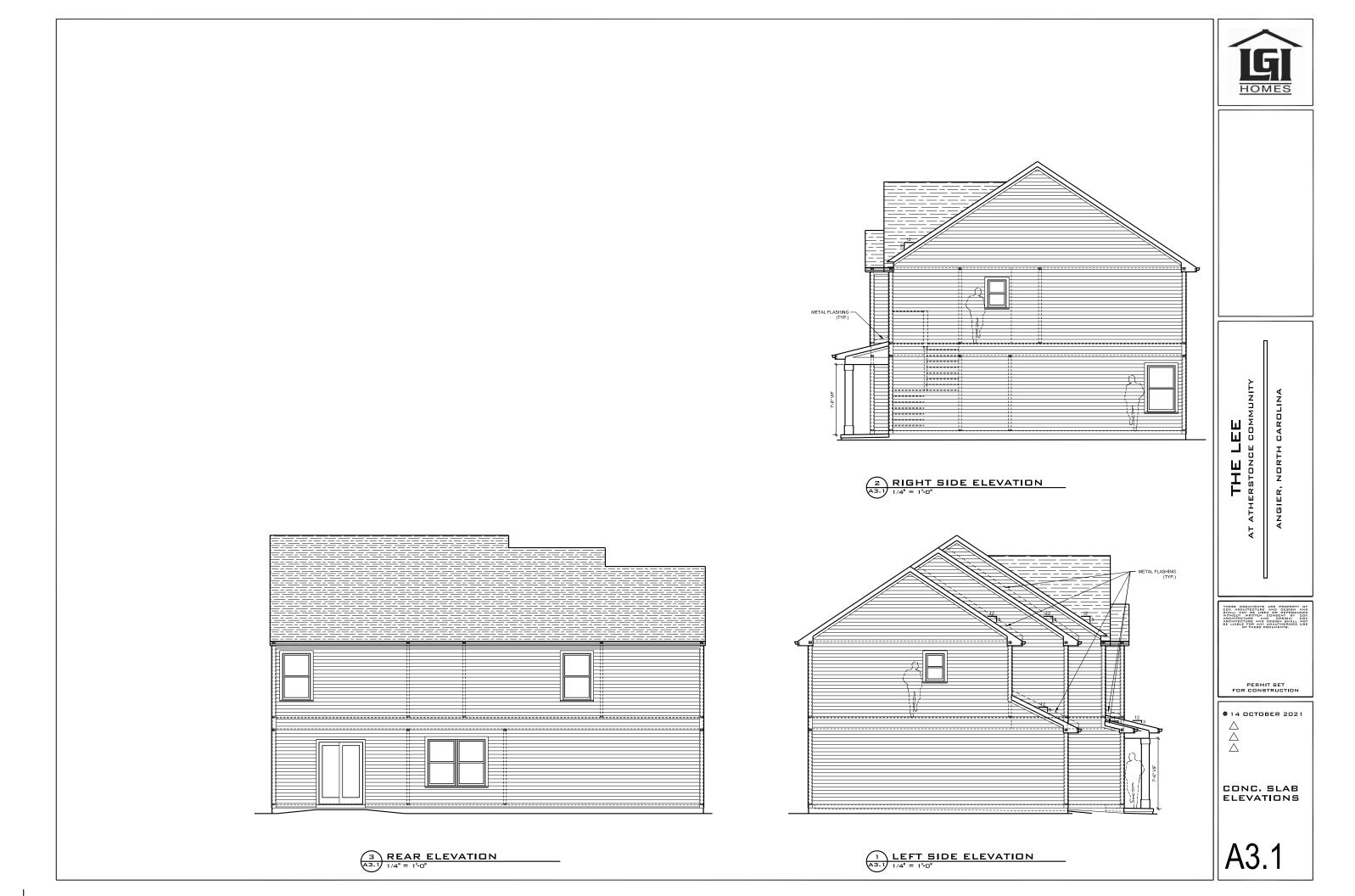
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CONC. SLAB ELEVATIONS & STAIR DETAILS

A3.0





ELECTRICAL LEGEND 120 OUTLET, 18" A.F.F. TO CENTER OR 12" ABOVE COUNTERTOPS SMOKE/CARBON MONOXIDE DETECTOR DOOR BELL DOOR BELL CHIME DOOR BELL TRANSFORMER WEATHER PROOF GFI 120 OUTLET \$ WALL SWITCH, 48" A.F.F. TO CENTER \$D DIMMER SWITCH, 48" A.F.F. TO CENTER 120 QUADRAPLEX OUTLET, 18* A.F.F. TO CENTER PHONE, 18" A.F.F. TO CENTER, W.
INDICATES WALL MOUNTED @ 48" A.F.F.

DATA, 18" A.F.F. TO CENTER, W.
INDICATES WALL MOUNTED @ 48" A.F.F.

CABLE JUNCTION BOX FLUORESCENT LIGHT FIXTURE

ZENON UNDER CABINET LIGHT TO BE MTD. TO
BOTTOM OF WALL CAB, NEAR FRONT EDGE CEILING LIGHT FIXTURE (LED)

WALL LIGHT FIXTURE PLUG MOLD TO BE MTD. TO BOTTOM OF WALL CAB. NEAR WALL → LED TAPE LIGHT 4" RECESSED EYEBALL FIXTURE FAN/LIGHT RECESSED FIXTURE EP ELECTRICAL PANEL
EM ELECTRICAL METER
TWH TANKLESS WATER HEATER FANILIGHT RECESSED DAMP LOCATION FIXTURE +HB HOSE BIBB +G GAS CONNECTION FLOOD LIGHT +GSO GAS SHUT-OFF THERMOSTAT

EXHAUST FAN

ELECTRICAL NOTES

-LIGHT FIXTURES IN CLOSETS TO COMPLY WITH SECTION
410.8 OF THE LATEST VERSION OF THE NEC HANDBOOK

ELECTRICAL PANEL / METER

[53 _____53 ____53]

5'-2¹/₂*

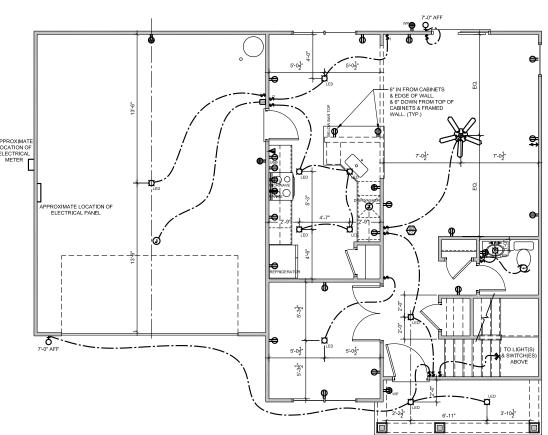
CELING

(SD)

2 SECOND FLOOR ELECTRICAL PLAN
E1.0 1/4" = 1'.0"

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4'-22"



1 FIRST FLOOR ELECTRICAL PLAN
E1.D 1/4" = 1'.0"



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ATHERSTONCE COMMUNITY ANGIER, NORTH CAROLINA

PERMIT SET FOR CONSTRUCTION

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ELECTRICAL PLAN & LEGEND

E1.0

DESIGN SPECIFICATIONS:

Construction Type: Commerical ☐ Residential ☒

Applicable Building Codes:

- 2018 North Carolina Residential Building Code
- ASCE 7—10: Minimum Design Loads for Buildings and Other Structures

Design	Load	ds:	
4	_	•	

1. Roof	
1.1 Live	20 PSF
1.2 Dead	10 PSF
1.3 Snow	15 PSF
1.3.1 Importance Factor	
2. Floor Live Loads	
2.1 Typ. Dwelling	40 PSF
2.2 Sleeping Areas	30 PSF
2.3 Balconies (exterior) and Decks	
2.4 Garage Parking	
3. Floor Dead Loads	
3.1 Conventional 2x	10 PSF
3.2 I-Joist	15 PSF
3.3 Floor Truss	
4. Ultimate Wind Speed (3 sec. gust)	130 MPH
4.1 Exposure	
4.2 Importance Factor	
4.3 Wind Base Shear	
4.3.1 Vx =	
4.3.2 Vy =	
··	

MEAN ROOF HT.	UP TO 30'	30'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

5. Component and Cladding (in PSF)

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6.1 Site ClassD
6.2 Design CategoryC
6.3 Importance Factor1.0
6.4 Seismic Use Group1
6.5 Spectral Response Acceleration
6.5.1 Sms = %g
6.5.2 Sm1 = %g
6.6 Seismic Base Shear
$6.6.1 \ Vx =$
6.6.2 Vy =
6.7 Basic Structural System (check one)
■ Bearing Wall
□ Building Frame
☐ Moment Frame
□ Dual w/ Special Moment Frame
☐ Dual w/ Intermediate R/C or Special Steel
☐ Inverted Pendulum
6.8 Arch/Mech Components Anchored?No



STRUCTURAL PLANS PREPARED FOR:

THE LEE

PROJECT ADDRESS: TBD

OWNER:

LGI Homes 7201 Creedmoor Road, Suite 147 Raleigh, NC 27613

ARCHITECT/DESIGNER:

Dwell In Design Architecture, LLC Guilford, CT

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory & Testing, INC. before construction begins.

PLAN ABBREVIATIONS:

AB	Anchor Bolt	OC	On Center	
ACI	American Concrete Institute	PCF	Pounds per Cubic Foot	
ASCE	American Society of Civil Engineers	PCI	Pounds per Cubic Inch	
AFA	American Fiberboard Association	PSF	Pounds per Square Foot	
AFF	Above Finished Floor	PSI	Pounds per Square Inch	
AISC	American Institute for Steel Construction		Pressure Treated	
	American Plywood Association		Stud Column	
AWS	American Welding Society	SER	Structural Engineer of Record	
CJ	Ceiling Joist	SJ	Single Joist	
CLR	Clear	SPF	Spruce Pine Fir	
DBL	DBL Double		Simpson Strong Tie	
DJ	DJ Double Joist		Single Truss	
DSP	Double Stud Pocket	STD	D Standard	
EA	Each	TJ	Triple Joist	
EE	Each End	TOF	Top of Footing	
EOS	Edge of Slab	TSP	Triple Stud Pocket	
EW	Each Way	TYP	Typical	
HDG	Hot Dipped Galvanized	UNO	Unless Noted Otherwise	
NDS	Nation Design Spec. for Wood	WWF	Welded Wire Fabric	
NTS	Not to Scale			

SHEET LIST:

Sheet No.	Description
CS1	Cover Sheet, Specifications, Revisions
CS2	Specifications Continued
S1.0m	Monolithic Slab Foundation
S1.0s	Stem Wall Foundation
S1.0c	Crawl Space Foundation
S1.0b	Basement Foundation
S2.0	Basement Framing Plan
S3.0	First Floor Framing Plan
S4.0	Second Floor Framing Plan
S5.0	Roof Framing Plan
S6.0	Basement Bracing Plan
S7.0	First Floor Bracing Plan
S8.0	Second Floor Bracing Plan

REVISION LIST:

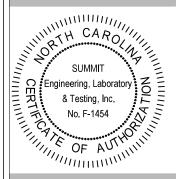
	Revision No.	Date	Project No.	Description
İ	0	10.4.19	24616	Original Engineering
	1	12.18.19	24616R	Added rear sliding door and covered porch
	2	10.14.21		Updated Seals
Ī				



STRUCTURAL MEMBERS ONLY



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LGI Homes 7201 Creedmoor Road, Suite 14 Raleigh, NC 27613

CURRENT DRAWING

DATE: 10/14/2021

Coversheet

THE LEE

SCALE: 1/8"=1'-0"

PROJECT #: 1203-08R: 24616R

DRAWN BY: BAF

CHECKED BY: CTB

ORIGINAL DRAWING

DATE 10/04/19 PROJECT # 24616

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

CHE

CS1

GENERAL STRUCTURAL NOTES:

- 1. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, INC. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
- 2. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- 7. This structure and all construction shall conform to all applicable sections of the international residential code.
- 8. This structure and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions

FOUNDATIONS:

- Foundations shall be constructed in accordance with chapter 4 of the 2018 NC Residential Building Code (Special consideration shall be given to Chapter 45 in wind zones 130mph and above)
- 2. Footing sizes based on a presumptive soil bearing capacity of 2000 PSF. Contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction
- 3. Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC
- 4. The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 8. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bearing in the middle third of the piers. Pilasters to be bonded to perimeter foundation wall
- 10. Crawl spaced to be graded level and clear of all debris
- 11. Provide foundation waterproofing and drain with positive slope to outlet as required by site conditions
- 12. Energy efficiency compliance and insulation of the structure to be in accordance with chapter 11 of the 2018 NCRC

CONCRETE

- 1. Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows: 3.1. Footings: 5%
- 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER
- Concrete slabs—on—grade shall be constructed in accordance with ACI 302.1R—96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab—on—grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- 7. Control or saw cut joints shall be spaced in interior slabs—on—grade at a maximum of 15'-0" O.C. and in exterior slabs—on—grade at a maximum of 10'-0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.
- 10. All welded wire fabric (W.W.F.) for concrete slabs—on—grade shall be placed at mid—depth of slab. The W.W.F. shall be securely supported during the concrete pour. Fibermesh may be used in lieu of W.W.F.

CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs—on—grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strenath.
- 2. Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- Fibermesh shall comply with ASTM C1116, any local building code requirements, and shall meet or exceed the current industry standard.
- 5. Steel Reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- 7. Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice
- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- Where reinforcing dowels are required , they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

WOOD FRAMING:

- Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern Yellow Pine (SYP) #2.
- LVL or PSL engineered wood shall have the following minimum design values:
 - 2.1. E = 1,900,000 psi
 - 2.2. Fb = 2600 psi2.3. Fv = 285 psi
 - 2.3. FV = 285 psi2.4. Fc = 700 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15.
 All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted.
- Lag screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS specifications.
- 6. All beams shall have full bearing on supporting framing members unless otherwise noted.
- 7. Exterior and load bearing stud walls are to be 2x4 SYP#2 @16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- Individual studs forming a column shall be attached with one 10d nail @6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be fully blocked at all floor levels to ensure proper load transfer.
- Multi-ply beams shall have each ply attached wth (3)10d nails @ 24" O.C.
- 10. Flitch beams and four and five ply beams shall be bolted together with (2) rows of 1/2" dia. through bolts staggered @24" O.C. w/ 2" edge distance and (2) bolts located at 6" from each end, unless noted otherwise.

WOOD TRUSSES:

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7—10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- 3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB—91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- 5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

WOOD STRUCTURAL PANELS:

- 1. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- 2. All structurally required wood sheathing shall bear the mark of the APA.
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- 4. Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)—8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 5. Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)—8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

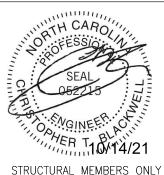
- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- 3. Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

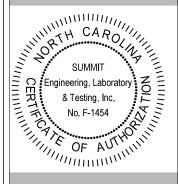
STRUCTURĂL STEEL:

- Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and of the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- 2. All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
- 3. Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes for shopt and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.



SUMMIT

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LGI Homes 7201 Creedmoor Road, Suite 14 Raleigh, NC 27613

CURRENT DRAWING

DATE: 10/14/2021

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Cove

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SCALE: 1/8"=1'-0"

PROJECT #: 1203-08R: 24616R

DRAWN BY: BAF

CHECKED BY: CTB

ORIGINAL DRAWING

DATE 10/04/19 PROJECT # 24616

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

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FOUNDATION NOTES:

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- STRUCTURAL CONCRETE TO BE Fc = 3000 PSI, PREPARED AND PLACED IN
- ACCORDANCE WITH ACI STANDARD 318.
 FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE ENFORCEMENT OFFICIAL.
 FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000
- PSF. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
 FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE
- ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF
- 6. MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE. PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.
- PROVIDE FOUNDATION WATERPROOFING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS. PROVIDED PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH
- CAROLINA RESIDENTIAL BUILDING CODE.
- 10. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK CRAWL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.
- FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.16. MINIMUM 1/2" DIA. BOLTS SPACED AT 6"-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MASONR" OR CONCRETE, ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.

DJ = DOUBLE JOIST SJ = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END TR = TRIPLE RAFTER

OC = ON CENTER PL = POINT LOAD CL = CENTER LINE

- 14. ALL PIERS TO BE 16 "X16" MASONRY AND ALL PILASTERS TO BE 8"X16" MASONRY, TYPICAL, (UNO)
- WALL FOOTINGS TO BE CONTINUOUS CONCRETE SIZES PER STRUCTURAL PLAN. A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER, OR HIS QUALIFIED REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUMMIT ENGINEERING LABORATORY & TESTING, INC. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
- ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED IIIALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED. THE IN OUR PROJECT WALL FLAN FOR FANEL LOCATIONS AND ANT RECUIRED HOLD-DOUBL, ADDITIONAL INFORMATION PER SECTION R602.10.8 AND FIGURE R602.10.1 OF THE 2015 IRC.

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP I PER TABLE R405.1

ANCHOR SPACING SHALL BE REDUCED TO 4'-0" ON CENTER IN LOCATIONS WHERE THE DESIGN WIND SPEED IS 130 MPH

ROOF TRUSS AND FLOOR JOIST LAYOUTS AND THEIR CORRESPONDING LOADING DETAILS, WERE NOT PROVIDED TO SUMMIT ENGINEERING, LABORATORY & TESTING, INC. (SUMMIT) PRIOR TO THE INITIAL DESIGN. THEREFORE, TRUSS AND JOIST DIRECTIONS WERE ASSUMED BASED ON THE INFORMATION PROVIDED BY LGI HOMES SUBSEQUENT PLAN REVISIONS BASED ON ROOF TRUSS AND FLOOR JOIST LAYOUTS SHALL BE NOTED IN THE REVISION LIST, INDICATING THE DATE THE LAYOUTS WERE PROVIDED. SHOULD ANY DISCREPANCIES BECOME APPARENT THE CONTRACTOR SHALL NOTIFY SUMMIT IMMEDIATELY.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY LGI HOMES COMPLETED/REVISED ON 12/19/2019. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

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ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

MONOLITHIC SLAB FOUNDATION SCALE: 1/8"=1"

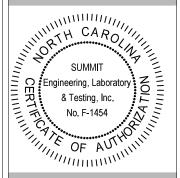
FOUNDATION SCHEDULE DESCRIPTION 16"5Q x10"D NONE 24"5Q x 10"D 30"5Q x 10"D NONE 36"5Q x 12"D NONE 42"5Q x 12"D (4) *4 E Ш 48"5Q x 12"D (6) *4 E.W. 4" THICK POURED CONCRETE SLAB W/ FIBER MESH ON 6 MIL POLY ON \otimes COMPACTED SOIL

4" THICK POURED CONCRETE SLAB ON ₿ COMPACTED SOIL

ABBREVIATIONS: W = WIDTH, D = DEPTH, SQ = SQUARE,

B.D. = BOTH DIRECTIONS, CONT. = CONTINUOUS, MONO = MONOLITHIC SLAB FOOTING

3070 HAMMOND BUSINESS PLACE, SUITE 171 RALFIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM



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CURRENT DRAWING DATE: 10/14/2021

SCALE: 1/8"=1'-0"

PROJECT # 1203-08R 24616R

DRAWN BY: BAF

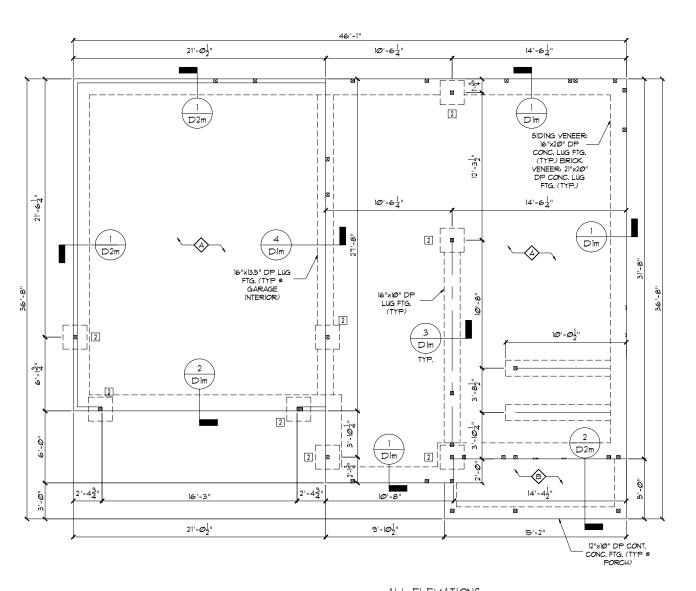
CHECKED BY: CTB

ORIGINAL DRAWING

DATE 10/04/19 PROJECT # 24616

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

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ALL ELEVATIONS



STRUCTURAL MEMBERS ONLY

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS.
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- RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.
- TO RESIST ALL PORCES ENCOUNTERED DURING ERECTION

 REOPERTIES (USED IN THE DESIGN ARE AS FOLLOWS.

 MICROLLAM (LVL), F_b = 2600 PSI, F_v = 285 PSI, E = 19x10⁶ PSI

 PARALLAM (PSI,), F_b = 3900 PSI, F_v = 290 PSI, E = 125x10⁶ PSI

 ALL WOOD MEMBERS SHALL BE 7.5 YP UNLESS NOTED ON PLAN ALL STUD COLUMNS

 AND JOISTS SHALL BE 7.5 SYP (WAO).
- ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 12 SYP STUD COLUMN AT EACH
- END UNLESS NOTED OTHERWISE.

 ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615
- AND SHALL HAVE A MINIMUM COVER OF 3".
 FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.16. MINIMUM 1/2" DIA, BOLTS SPACED AT 6'-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE, ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION, MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE, ANCHOR SPACING SHALL BE REDUCED TO 4"-0" ON CENTER IN LOCATIONS WHERE THE DESIGN WIND SPEED IS 130 MPH.
- CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN
- PERPENDICULAR TO RAFTERS.

 10. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA, THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/DTF, MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP 12, DROPPED, FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP *2, DROPPED. (UNLESS
- ABBREVIATIONS:

DJ = DOUBLE JOIST SJ = SINGLE JOIST GT = GIRDER TRUSS FT = FLOOR TRUSS SC = STUD COLUMN EE = EACH END DR = DOUBLE RAFTER TR = TRIPLE RAFTER TJ = TRIPLE JOIST OC = ON CENTER CL = CENTER LINE PL = POINT LOAD

NOTE:

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

NOTE: SHADED WALLS INDICATE LOAD BEARING WALLS

JOIST & BEAM SIZES SHOWN ARE MINIMUMS. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

ROOF TRUSS AND FLOOR JOIST LAYOUTS AND THEIR CORRESPONDING LOADING DETAILS, WERE NOT PROVIDED TO SUMMIT ENGINEERING, LABORATORY & TESTING, INC. (SUMMIT) PRIOR TO THE INITIAL DESIGN. THEREFORE, TRUSS AND JOIST DIRECTIONS WERE ASSUMED BASED ON THE INFORMATION PROVIDED BY LGI HOMES SUBSEQUENT PLAN REVISIONS BASED ON ROOF TRUSS AND FLOOR JOIST LAYOUTS SHALL BE NOTED IN THE REVISION LIST, INDICATING THE DATE THE LAYOUTS WERE PROVIDED. SHOULD ANY DISCREPANCIES BECOME APPARENT THE CONTRACTOR SHALL NOTIFY SUMMIT IMMEDIATELY.

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN

NOTE: 1ST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF IIIALL (TYP INO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

ROOF TRUSS UPLIFT CONNECTOR SCHEDULE				
MAX. UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FNE	
600 LBS	H2.5A	PER WALL SHEATHIN	G & FASTENERS	
1200 LBS	(2) H2.5A	CS16 (END = 11")	DTT2Z	
145Ø LBS	HT52Ø	C616 (END = 11")	DTT2Z	
2000 LBS	(2) MT92Ø	(2) CSI6 (END = II")	DTT2Z	
2900 LBS	(2) HTS2Ø	(2) C516 (END = 11")	HTT4	
3685 LBS	LGT3-9D92.5	MSTC52	HTT4	
	MAX. UPLIFT 600 LBS 1200 LBS 1450 LBS 2000 LBS 2900 LBS	MAX. UPLIFT ROOF TO WALL 600 LBS H2.5A 1200 LBS (2) H2.5A 1450 LBS HT520 2000 LBS (2) MT520 2900 LBS (2) HT520	MAX. UPLIFT ROOF TO WALL FLOOR TO FLOOR 600 LBS	

- ALL PRODUCTS LISTED ARE SIMPSON STRONG-TIE FOUIVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS.

 2. UPLIFT VALUES LISTED ARE FOR SYP 2 GRADE MEMBERS. 3. REFER TO TRUSS LAYOUT PER MANUF, FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS, CONNECTORS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE
- 4. CONTACT SUMMIT FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

≣	HEADER SCHEDULE			
ND	TAG	SIZE	JACKS (EACH END)	
25	A	(2) 2x6	(1)	
25	В	(2) 2x8	(2)	
	С	(2) 2xlØ	(2)	
	D	(2) 2xl2	(2)	
	E	(2) 9-1/4" LSL/LVL	(3)	
	F	(2) 11-7/8" LSL/LVL	(3)	
	G	(3) 2x8	(2)	
	Н	(3) 2x1Ø	(2)	
	1	(3) 2xl2	(2)	

NOTES:

. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.

2. ALL HEADERS TO BE DROPPED (UN.O.).

3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (UNO.).

BEAM SCHEDULE			
TAG	SIZE		
ві	(1) 11-7/8" FLOOR JOIST OR TRUSS		
B2	(2) 11-7/8" FLOOR JOISTS OR TRUSS		
B3	(1) 14 FLOOR JOIST OR TRUSS		
B4	(2) 14" FLOOR JOISTS OR TRUSS		
B5	(1) 9-1/4" LVL		
В6	(2) 9-1/4" LVL		
B 1	(1) 11-7/8" LVL		
B8	(2) -7/8" L∨L		
В9	(1) 14" LVL		
BIØ	(2) 14" LVL		
BII	(2) 2×1Ø		

BEAM SIZES SHOWN ARE MINIMUM SIZES, LARGER SIZES MAY BE SUBSTITUTED FOR EASE OF CONSTRUCTION. BEAMS ARE TO BE FLUSH WITH TOP OF FLOOR UNLESS

U	WALL STUD SCHEDULE				
		FT HEIC	‡HT/		
STUD SIZE		STUD SPACING (O.C.)			
	ROOF ROOF & ROOF & NON-LOAD				
2×4	24"	16"	12"	24"	
2x6	24"	24"	16"	24"	

<u>NOTES:</u> I. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX. OF 16" O.C.

3. TWO STORY WALLS SHALL BE FRAMED w/ 2x4 STUDS

@ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED w/ CROSS BRACING @ 6'-O" O.C. VERTICALLY.

KING STUD REQUIREMENTS					
OPENING WIDTH	KINGS (EACH END)				
(FT)	16" O.C.	24" O.C.			
LESS THAN 3'-0"	(1)	(1)			
3'-Ø TO 4'-Ø"	(2)	(1)			
4'-0" TO 8'-0"	(3)	(2)			
8'-0" TO 12'-0"	(5)	(3)			
12'-0" TO 16'-0"	(6)	(4)			
KING STUD DEGUIDEMENTS ABOVE DO NOT ADDLY TO					

PORTAL FRAMED OPENINGS

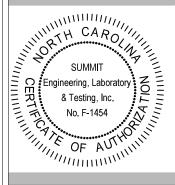
LINTEL SCHEDULE					
TAG	SIZE	OPENING SIZE			
1	L3x3x1/4"	LESS THAN 6'-0"			
2	L5x3x1/4"	6'-0" TO 10'-0"			
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"			
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS			
SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER					

LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR 3) ALL HEADERS WITH BRICK ABOVE: (1)(UNO)



STRUCTURAL MEMBERS ONLY

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Suite Road, 3 Framing Imoor 2761 Creed 2 0 omes 음 leigh, I First \vdash LGI | 7201 Ralei

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CURRENT DRAWING

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DATE: 10/14/2021

SCALE: 1/8"=1'-0"

PROJECT # · 1203-08R · 24616R

DRAWN BY: BAF

CHECKED BY: CTB

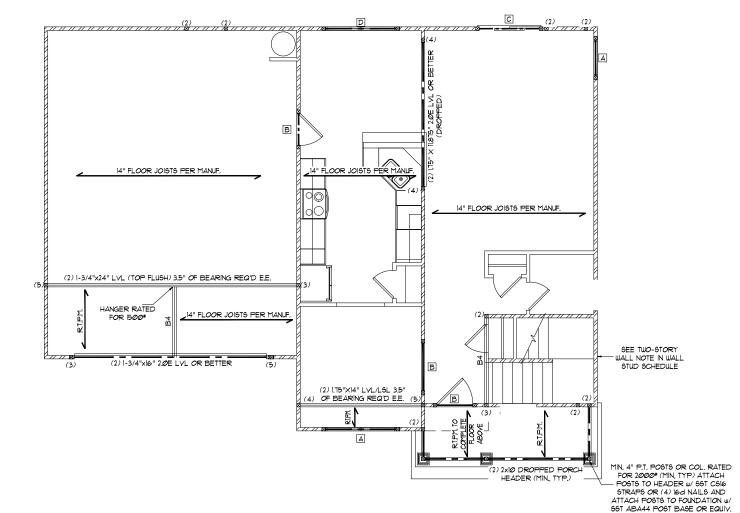
ORIGINAL DRAWING

PROJECT # DATE 10/04/19 24616

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ALL ELEVATIONS

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- CODE UITH ALL LOCAL AMENOMENTS.

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- TO RESIST ALL PORCES ENCOUNTERED DURING ERECTION

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 PARALLAM (PSI,), F_b = 3900 PSI, F_v = 290 PSI, E = 125x10⁶ PSI

 ALL WOOD MEMBERS SHALL BE 7.5 YP UNLESS NOTED ON PLAN ALL STUD COLUMNS

 AND JOISTS SHALL BE 7.5 SYP (WAO).
- ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 2 SYP STUD COLUMN AT EACH
- END UNLESS NOTED OTHERWISE.

 ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615
- ALL REINFORCING SIEEL SHALL DE GRADE 80 DARS CONFORTING 10 ASHT ABIS AND SHALL HAVE A MINIMM COVER OF 3".
 FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.16. MINIMM 12" DIA BOLTS SPACED AT 6"-0" ON CENTER WITH A T" HINIMM PMEEDIFFIN TNO MASONRY OR CONCRETE. ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION, MINIMM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE, ANCHOR SPACING SHALL BE REDUCED TO 4"-0" ON CENTER IN LOCATIONS WHERE THE DESIGN WIND SPEED IS 130 MPH.
- CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN
- PERPENDICULAR TO RAFTERS.

 10. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 12" DIA THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL I/DTI: MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP 12, DROPPED. FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP *2, DROPPED. (UNLESS
- 12. ABBREVIATIONS:

DJ = DOUBLE JOIST SJ = SINGLE JOIST GT = GIRDER TRUSS FT = FLOOR TRUS SC = STUD COLUMN DR = DOUBLE R. EE = EACH END TR = TRIPLE RAF	
SC = STUD COLUMN DR = DOUBLE RA	Ť
	5
EE - EACH END TO - TOIDI E DAE	\FTE
LL - LACH LIND IN - INITLE RAF	TER
TJ = TRIPLE JOIST OC = ON CENTER	
CL = CENTER LINE PL = POINT LOAD)

NOTE

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

NOTE: SHADED WALLS INDICATE LOAD BEARING WALLS

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

ROOF TRUSS AND FLOOR JOIST LAYOUTS AND THEIR CORRESPONDING LOADING DETAILS, WERE NOT PROVIDED TO SUMMIT ENGINEERING, LABORATORY & TESTING, INC. (SUMMIT) PRIOR TO THE INITIAL DESIGN. THEREFORE, TRUSS AND JOIST DIRECTIONS LIERE ASSUMED BASED ON THE INFORMATION PROVIDED BY LGI HOMES. SUBSEQUENT PLAN REVISIONS BASED ON ROOF TRUSS AND FLOOR JOIST LAYOUTS SHALL BE NOTED IN THE REVISION LIST, INDICATING THE DATE THE LAYOUTS WERE PROVIDED. SHOULD ANY DISCREPANCIES BECOME APPARENT THE CONTRACTOR SHALL NOTIFY SUMMIT IMMEDIATELY.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY LGI HOMES COMPLETED/REVISED ON 12/19/2019. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

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ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR FRAMING PLAN SCALE: 1/8"=1"

NOTE: 1ST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF IIIALL (TYP INO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

ROOF TRADS OF EIT CONNECTOR SCHEDULE						
MAX, UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND			
600 LBS	H2.5A	PER WALL SHEATHIN	G & FASTENERS			
1200 LBS	(2) H2.5A	CS16 (END = 11")	DTT2Z			
145Ø LBS	HT52Ø	C616 (END = 11")	DTT2Z			
2000 LBS	(2) MT62Ø	(2) CSI6 (END = II")	DTT2Z			
2900 LBS	(2) HTS2Ø	(2) CSI6 (END = II")	HTT4			
3685 LBS	LGT3-SDS2.5	MSTC52	HTT4			

- ALL PRODUCTS LISTED ARE SIMPSON STRONG-TIE FOULVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS.

 2. UPLIFT VALUES LISTED ARE FOR SYP 2 GRADE MEMBERS. 3. REFER TO TRUSS LAYOUT PER MANUF, FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS, CONNECTORS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE
- 4. CONTACT SUMMIT FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

ROOF TRUSS UPLIFT CONNECTOR SCHEDULE					HEA	DER SCHED	ULE
MAX. UPLIFT ROOF TO WALL FLOOR TO FLOOR FLOOR TO FND			1	TAG	SIZE	JACKS (EACH END)	
600 LBS	110 = 4			1	A	(2) 2x6	(1)
600 LD3	H2.5A	PER WALL SHEATHING & FASTENERS		1	В	(2) 2x8	(2)
1200 LBS	(2) H2.5A	CSI6 (END = II") DTT2Z			С	(2) 2xlØ	(2)
145Ø LB6	HT52Ø	CSI6 (END = 11")	DTT2Z		D	(2) 2xl2	(2)
2000150	(0) 147000	(2) COK (FND 111)	D##07	1	E	(2) 9-1/4" LSL/LVL	(3)
2000 LBS	(2) MTS2Ø	(2) CSI6 (END = II")	DTT2Z	1	F	(2) 11-7/8" LSL/LVL	(3)

NOTES:

. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.

2. ALL HEADERS TO BE DROPPED (UN.O.).

(3) 2x8

(3) 2x1Ø

(3) 2x12

3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (UNO.).

BEAM SCHEDULE					
TAG SIZE					
ы	(1) 11-7/8" FLOOR JOIST OR TRUSS				
B2	(2) 11-7/8" FLOOR JOISTS OR TRUSS				
B3	(1) 14 FLOOR JOIST OR TRUSS				
B4	(2) 14" FLOOR JOISTS OR TRUSS				
B5	(1) 9-1/4" LVL				
В6	(2) 9-1/4" LVL				
B1	(1) 11-7/8" LVL				
B8	(2) II-7/8" LVL				
В9	(1) 14" LVL				
BIØ	(2) 14" LVL				
BII	(2) 2×1Ø				

BEAM SIZES SHOWN ARE MINIMUM SIZES, LARGER SIZES MAY BE SUBSTITUTED FOR EASE OF CONSTRUCTION. BEAMS ARE TO BE FLUSH WITH TOP OF FLOOR UNLESS

WALL STUD SCHEDULE						
(10 FT HEIGHT)						
STUD SIZE	STUD SPACING (O.C.)					
	ROOF ONLY	ROOF & 1 FLOOR	ROOF & 2 FLOORS	NON-LOAD BEARING		
2×4	24"	16"	12"	24"		
2x6	24"	24"	16"	24"		

<u>NOTES:</u> 1. BRACED WALLS STUDS SHALL BE A MAX, *O*F 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX. OF 16" O.C.

3. TWO STORY WALLS SHALL BE FRAMED w/ 2x4 STUDS

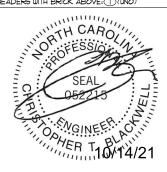
@ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED w/ CROSS BRACING @ 6'-O" O.C. VERTICALLY.

KING STUD REQUIREMENTS					
OPENING WIDTH	KINGS (EACH END)				
(FT)	16" O.C.	24" O.C.			
LESS THAN 3'-0"	(1)	(1)			
3'-Ø TO 4'-Ø"	(2)	(1)			
4'-0" TO 8'-0"	(3)	(2)			
8'-0" TO 12'-0"	(5)	(3)			
12'-0" TO 16'-0"	(6)	(4)			
KING STUD DEGUIDEMENTS ABOVE DO NOT ADDLY TO					

PORTAL FRAMED OPENINGS

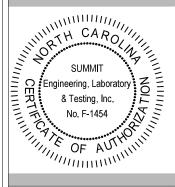
LINTEL SCHEDULE						
TAG	SIZE	OPENING SIZE				
1	L3x3x1/4"	LESS THAN 6'-0"				
2	L5x3x1/4"	6'-0" TO 10'-0"				
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"				
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS				
SECURE LINTEL TO HEADER III/ (2) 1/2" DIAMETER						

LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR 3). ALL HEADERS WITH BRICK ABOVE: (1)(UNO)



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CURRENT DRAWING

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DATE: 10/14/2021

SCALE: 1/8"=1'-0"

PROJECT # · 1203-08R · 24616R

DRAWN BY: BAF

CHECKED BY: CTB

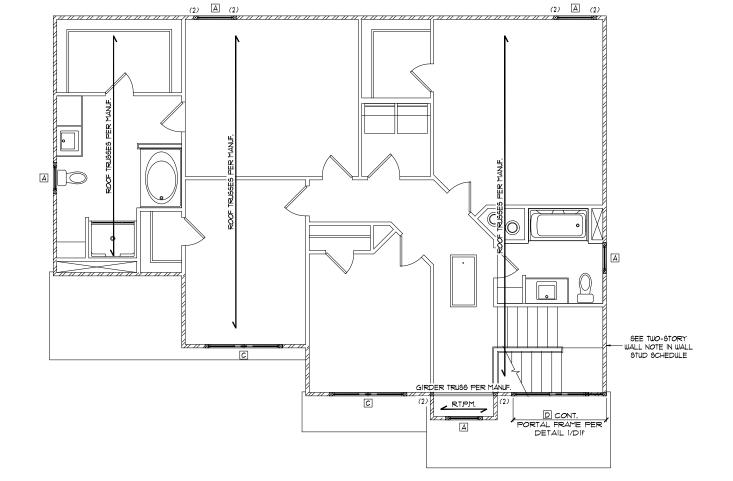
ORIGINAL DRAWING

PROJECT # DATE 10/04/19 24616

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

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ALL ELEVATIONS

ROOF TRUSS UPLIFT CONNECTOR SCHEDULE					
ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND			
H2.5A	PER WALL SHEATHIN	G & FASTENERS			
(2) H2.5A	CSI6 (END = 11")	DTT2Z			
HT52Ø	C\$16 (END = 11")	DTT2Z			
(2) MT62Ø	(2) CS16 (END = 11")	DTT2Z			
(2) HT62Ø	(2) CS16 (END = 11")	HTT4			
LGT3-SDS2.5	MSTC52	HTT4			
	ROOF TO WALL H2.5A (2) H2.5A HT62Ø (2) MT62Ø (2) HT62Ø	ROOF TO WALL FLOOR TO FLOOR H2.5A PER WALL SHEATHIN (2) H2.5A CSI6 (END = ") HTS2Ø CSI6 (END = ") (2) MTS2Ø (2) CSI6 (END = ") (2) HTS2Ø (2) CSI6 (END = ")			

1. ALL PRODUCTS LISTED ARE SIMPSON STRONG-TIE, EQUIVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS. 2. UPLIFT VALUES LISTED ARE FOR SYP 12 GRADE MEMBERS. 3. REFER TO TRUSS LAYOUT PER MANUF, FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS, CONNECTIONS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE. 4. CONTACT SUMMIT FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

NOTE: 1ST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

ROOF TRUSS AND FLOOR JOIST LAYOUTS, AND THEIR ROOF TRUSS AND FLOOR JOIST LAYOUTS, AND THEIR CORRESPONDING LOCADING DETAILS, IJERE NOT PROVIDED TO SUMMIT ENGINEERING, LABORATORY 4 TESTING, INC. (SUMMIT) PRIOR TO THE INITIAL DESIGN. THEREFORE, TRUSS AND JOIST DIRECTIONS WERE ASSUMED BASED ON THE INFORMATION PROVIDED BY LEFT HOMES, SUBSECUENT PLAN REVISIONS BASED ON ROOF TRUSS AND FLOOR JOIST LAYOUTS SHALL BE NOTED IN THE REVISION LIST, INDICATING THE DATE THE LAYOUTS WERE PROVIDED. SHOULD ANY DISCREPANCIES BECOME APPARENT, THE CONTRACTOR SHALL NOTIFY SUMMIT IMMEDIATELY.

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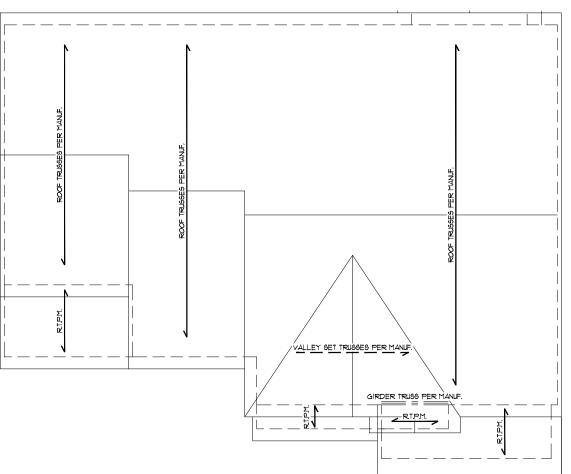
SUMMIT CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

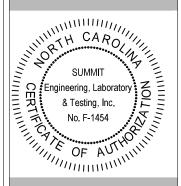
ROOF FRAMING PLAN SCALE: 1/8"=1"



ALL ELEVATIONS



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14 Suite Road, LGI Homes 7201 Creedmoor F Raleigh, NC 27613

CURRENT DRAWING

Plan

Framing

Roof

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DATE: 10/14/2021

SCALE: 1/8"=1'-0"

PROJECT #: 1203-08R: 24616R

DRAWN BY: BAF

CHECKED BY: CTB

ORIGINAL DRAWING

DATE 10/04/19

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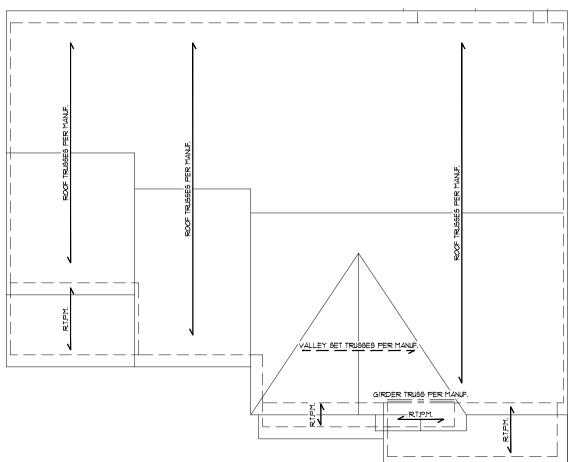
STRUCTURAL MEMBERS ONLY

PROJECT # 24616

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

S5.0



REQUIRED BRACED WALL PANEL CONNECTIONS						
		MIN.	REQUIRED CONNECTION			
METHOD	MATERIAL	THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS		
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS: 9 6" O.C.	6d COMMON NAILS* 9 12" O.C.		
GB	GYP9UM BOARD	1/2"	5d COOLER NAILS** # 7" O.C.	5d COOLER NAILS** 9 T" O.C.		
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS! 9 6" O.C.	6d COMMON NAILS+ 9 12" O.C.		
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.6.4	PER FIGURE R602.106.4		
"BASED ON 16" O.C. STUD SPACING. "OR EQUIVALENT PER TABLE RT0235						

- IIIALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R60210 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION R602.10 OF THE 2018 NC RESIDENTIAL CODE.
- 2) WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS OF
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH IRC TABLE R602.104.
 REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES.
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE FER TABLE R602.105.
 THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO).
- FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- OR DEARING WALL DELOW WITHOUT ADDITIONAL ENGINEERING CALCULATION.

 (D) A BRACED WALL LINE.

 (II) THE DISTANCE BETWEEN ADJACENT EDGES OF BRACED WALL PANELS ALONG A
- PRACED WALL LINE SHALL BE NO GREATER THAN 20 FEET.

 ADEQUATE CONTINUOUS LOAD PATHS FOR TRANSFER OF BRACING LOADS AND UPLIFT LOADS SHALL COMPLY WITH IRC SECTION R602.3.5.
- MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602109
- BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.1038 (SEE DETAIL 1/D5) FROM DETAIL PACKAGE).
- 15) BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.8.2 AND FIGURES R602.10.8(1)4(2)4(3).
- (6) CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10.11
- 17) PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.106.4
- 18) ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
- 9) ABBREVIATIONS: GB = GYPSUM BOARD

CS-XXX = CONT. SHEATHED PF = PORTAL FRAME

WSP = WOOD STRUCTURAL PANEL ENG = ENGINEERED SOLUTION ENG-PF = ENGINEERED PORTAL FRAME

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.8 AND FIGURE R602.10.1 OF THE 2015 IRC.

ROOF TRUSS AND FLOOR JOIST LAYOUTS AND THEIR CORRESPONDING LOADING DETAILS, WERE NOT PROVIDED TO SUMMIT ENGINEERING, LABORATORY & TESTING, INC. (SUMMIT) PRIOR TO THE INITIAL DESIGN. THEREFORE, TRUSS AND JOIST DIRECTIONS WERE ASSUMED BASED ON THE INFORMATION PROVIDED BY LGI HOMES SUBSEQUENT PLAN REVISIONS BASED ON ROOF TRUSS AND FLOOR JOIST LAYOUTS SHALL BE NOTED IN THE REVISION LIST, INDICATING THE DATE THE LAYOUTS WERE PROVIDED. SHOULD ANY DISCREPANCIES BECOME APPARENT THE CONTRACTOR SHALL

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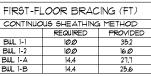
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

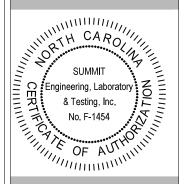
FIRST FLOOR BRACING PLAN SCALE: 1/8"=1"

FIRST-FLOOR BRACING (FT) CONTINUOUS SHEATHING METHOD 10.0 10.0 14.4





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CURRENT DRAWING

DATE: 10/14/2021

SCALE: 1/8"=1'-0"

PROJECT # 1203-08R 24616R

DRAWN BY: BAF

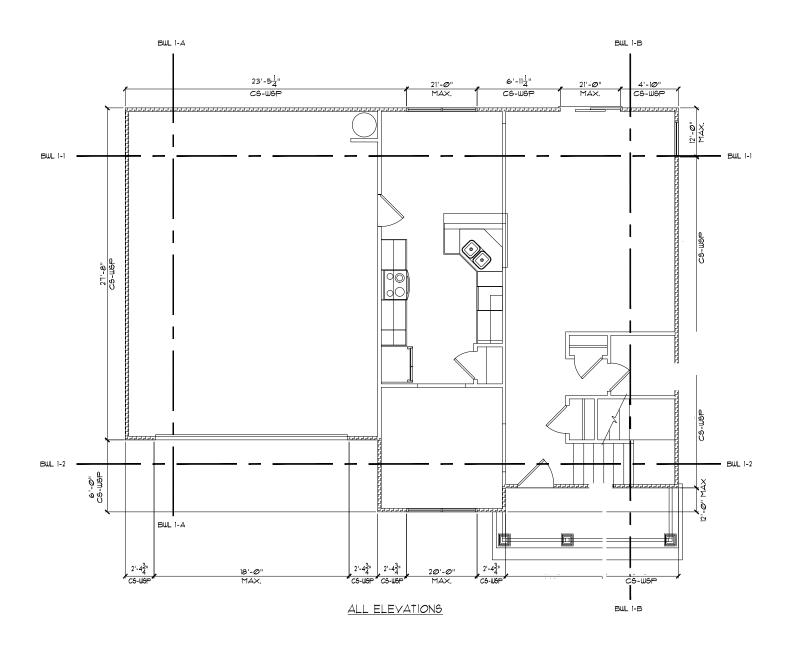
CHECKED BY: CTB

ORIGINAL DRAWING

DATE 10/04/19 PROJECT # 24616

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

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STRUCTURAL MEMBERS ONLY

REQUIRED BRACED WALL PANEL CONNECTIONS						
		MIN.	REQUIRED CONNECTION			
METHOD	MATERIAL	THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS		
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS: 9 6" O.C.	6d COMMON NAILS* 9 12" O.C.		
GB	GYP9UM BOARD	1/2"	5d COOLER NAILS** # 7" O.C.	5d COOLER NAILS** 9 T" O.C.		
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS! 9 6" O.C.	6d COMMON NAILS+ 9 12" O.C.		
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4		
"BASED ON 16" O.C. STUD SPACING "OR EQUIVALENT PER TABLE RT0235						

BRACED WALL NOTES:

- IIIALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R60210 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION R602.10 OF THE 2018 NC RESIDENTIAL CODE.
- 2) WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS OF
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH
- IRC TABLE R602.10.4.
 REFER TO ARCHITECTURAL PLAN FOR DOORWINDOW OPENING SIZES.
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINITUM PANEL LENGTH SHALL BE PER TABLE R603.105.

 THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO).
- FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- OR DEARING WALL DELOW WITHOUT ADDITIONAL ENGINEERING CALCULATION.

 (D) A BRACED WALL LINE.

 (II) THE DISTANCE BETWEEN ADJACENT EDGES OF BRACED WALL PANELS ALONG A
- PRACED WALL LINE SHALL BE NO GREATER THAN 20 FEET.

 ADEQUATE CONTINUOUS LOAD PATHS FOR TRANSFER OF BRACING LOADS AND UPLIFT LOADS SHALL COMPLY WITH IRC SECTION R602.3.5.
- MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602109
- BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.1038 (SEE DETAIL 1/D5) FROM DETAIL PACKAGE).
- 15) BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.8.2 AND FIGURES R602.10.8(1)4(2)4(3).
- (6) CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10.11
- 17) PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.106.4
- 18) ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
- 9) ABBREVIATIONS: GB = GYPSUM BOARD CS-XXX = CONT. SHEATHED PF = PORTAL FRAME

WSP = WOOD STRUCTURAL PANEL ENG = ENGINEERED SOLUTION ENG-PF = ENGINEERED PORTAL FRAME

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.8 AND FIGURE R602.10.1 OF THE 2015 IRC.

ROOF TRUSS AND FLOOR JOIST LAYOUTS AND THEIR CORRESPONDING LOADING DETAILS, WERE NOT PROVIDED TO SUMMIT ENGINEERING, LABORATORY & TESTING, INC. (SUMMIT) PRIOR TO THE INITIAL DESIGN. THEREFORE, TRUSS AND JOIST DIRECTIONS WERE ASSUMED BASED ON THE INFORMATION PROVIDED BY LGI HOMES SUBSEQUENT PLAN REVISIONS BASED ON ROOF TRUSS AND FLOOR JOIST LAYOUTS SHALL BE NOTED IN THE REVISION LIST, INDICATING THE DATE THE LAYOUTS WERE PROVIDED. SHOULD ANY DISCREPANCIES BECOME APPARENT THE CONTRACTOR SHALL NOTIFY SUMMIT IMMEDIATELY.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY LGI HOMES COMPLETED/REVISED ON 12/19/2019. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

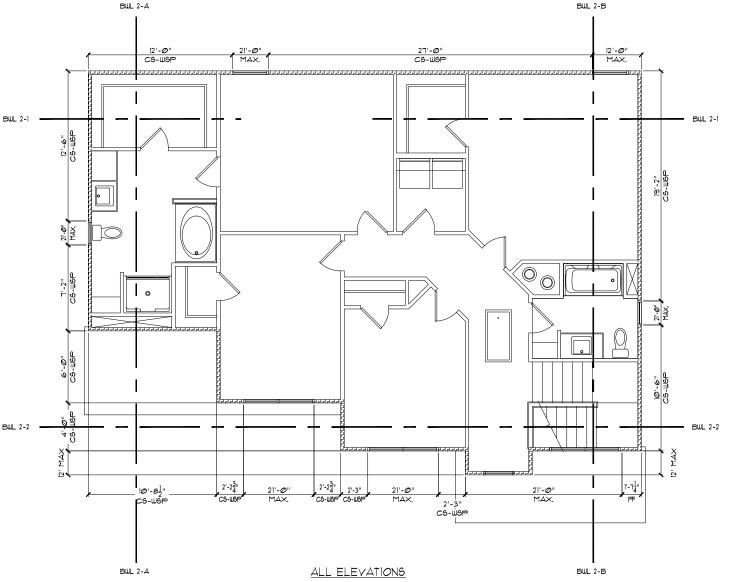
STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS.
ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

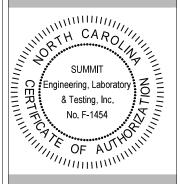
SECOND FLOOR BRACING PLAN

SECOND-FLOOR BRACING (FT) CONTINUOUS SHEATHING METHOD REQUIRED PROVIDED





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uite ิดี Road, Creedmoor | gh, NC 2761 il Homes 01 Cree leigh, N CLIENT LGI 7201 Ralei

4

CURRENT DRAWING

DATE: 10/14/2021

Bracing

Floor

Second

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SCALE: 1/8"=1'-0"

PROJECT # 1203-08R 24616R

DRAWN BY: BAF

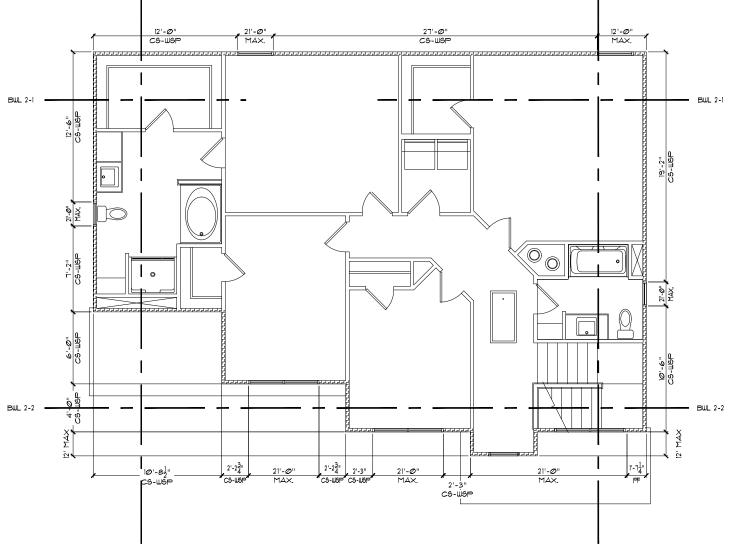
CHECKED BY: CTB

ORIGINAL DRAWING

DATE 10/04/19 PROJECT # 24616

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

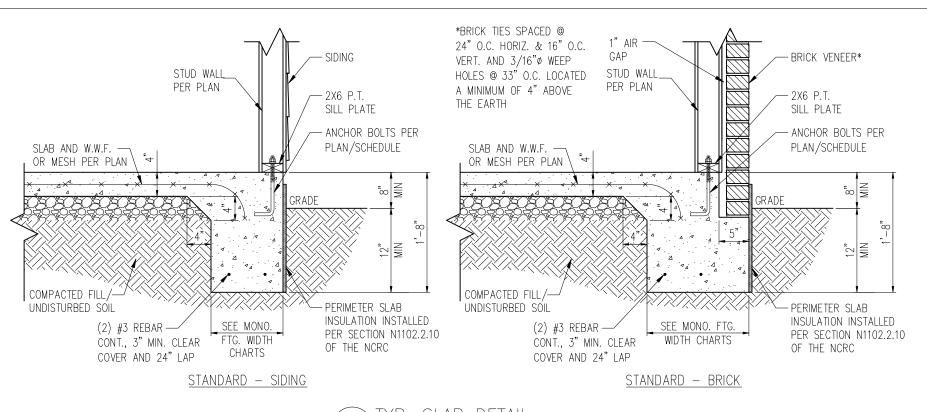
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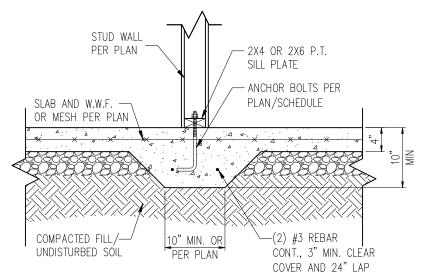




CARO

ORTH CAROLINA





3 TYP. THICKENED SLAB DETAIL
D1m 3/4" = 1'-0"

STUD WALL PER PLAN 2X6 P.T. SLAB TO BE SLOPED -DRIVEWAY SLOPED-SILL PLATE PER BUILDER 1/8" PER FOOT TOWARDS GARAGE ENTRY ANCHOR BOLTS PER PLAN/SCHEDULE 1/D1 SLAB AND W.W.F. -SLAB AND W.W.F. 1/2" EXPANSION OR MESH PER PLAN OR MESH PER PLAN JOINT COMPACTED FILL, 10' TO NEAREST FULL UNDISTURBED SÓIL 16' (2) #3 REBAR WIDTH EXP. JOINT COMPACTED FILL/—/(2) #3 REBAR -CONT., 3" MIN. CLEAR UNDISTURBED SOIL CONT., 3" MIN. CLEAR ONE-POUR COVER AND 24" LAP COVER AND 24" LAP

WALL ANCHOR SCHEDULE

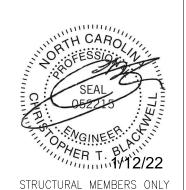
TYPE OF ANCHOR	MIN. CONC.	SPACING	INTERIOR	EXTERIOR
	EMBEDMENT	EMBEDMENT	WALL	WALL
1/2"ø A307 BOLTS w/	7"	6'-0"	YES	YES
STD. 90° BEND				
SST - MASA	4"	6'-0"	NO	YES
1/2"ø THREADED ROD w/	7"	6'-0"	YES	YES
w/ SST SET-XP EPOXY				
1/2"ø SST TITEN HD	4-1/4"	6'-0"	YES	YES

NOTE

- 1) INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.
- 2) EQUIVALENT ANCHORS MAY BE USED. SIZE & SPACING PER MANUF. SPECS.

MONOLITHIC FOOTING WIDTH

# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY				
	1500 PSF	2000 PSF	2500 PSF		
1 STORY - STD.	16"	16"	16"		
1 STORY - BRICK VENEER	21"*	21"*	21"*		
2 STORY — STD.	16"	16"	16"		
2 STORY – BRICK VENEER	21"*	21"*	21"*		
3 STORY — STD.	23"	18"	18"		
3 STORY - BRICK VENEER	32"*	24"*	24"*		
*5" BRICK LEDGE HAS BEEN ADDED TO THE MONOLITHIC					
FOOTING WIDTH FOR BRICK SUPPORT					





SUMMIT Engineering, Laboratory & Testing, Inc.

C-2530

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Standard Details
Monolithic Slab Details
LGI Homes
3037 Sherman Drive
Lancaster, SC 29720

CURRENT DRAWING

DATE: 10/05/21

SCALE: NTS

PROJECT #: 3554.T0040

DRAWN BY: MSB

CHECKED BY: CTB

ORIGINAL DRAWING

DATE 10/01/19 PROJECT # 24512

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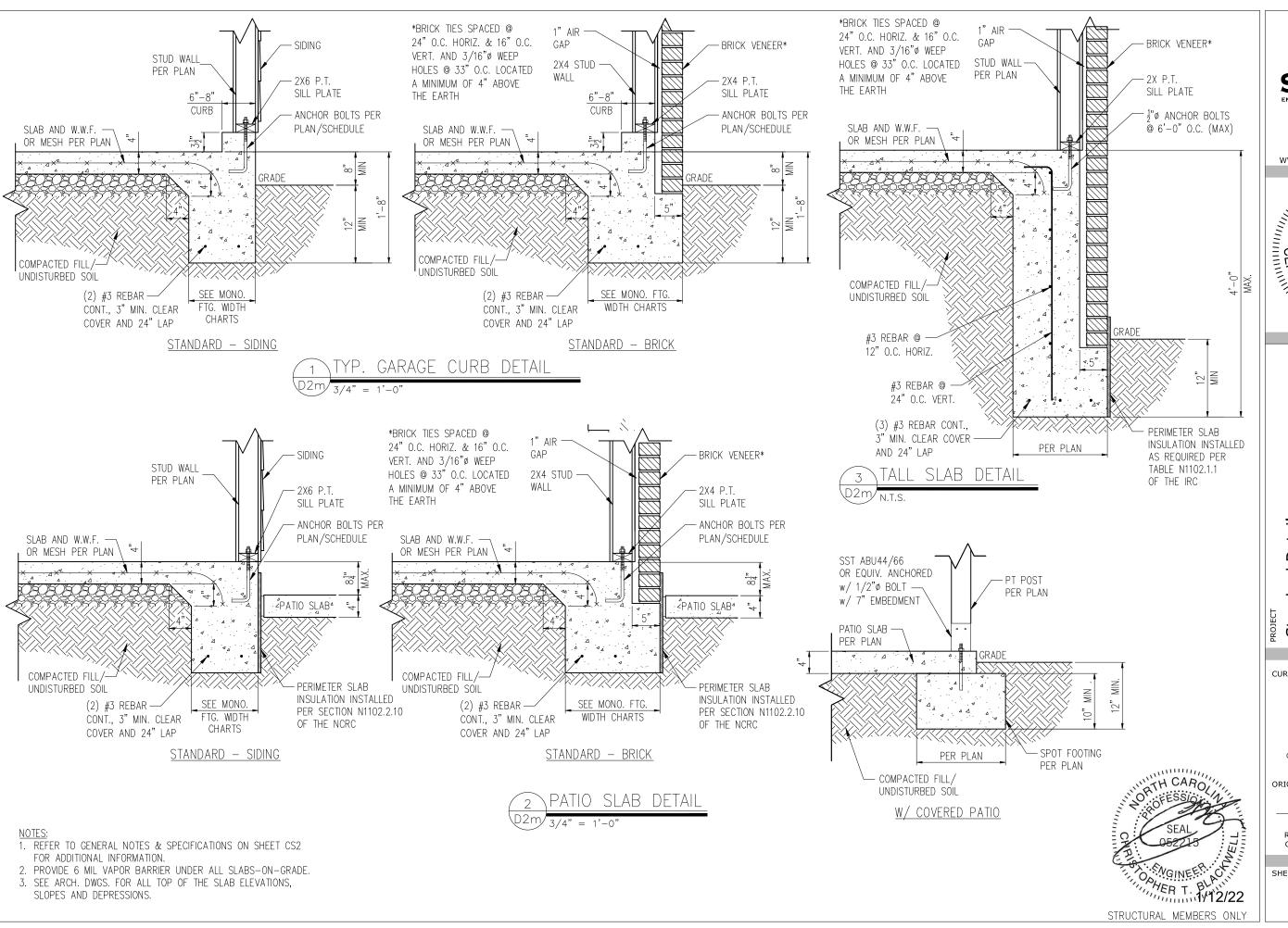


1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.

2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS—ON—GRADE. 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS,

SLAB AT GARAGE DOOR

SLOPES AND DEPRESSIONS.





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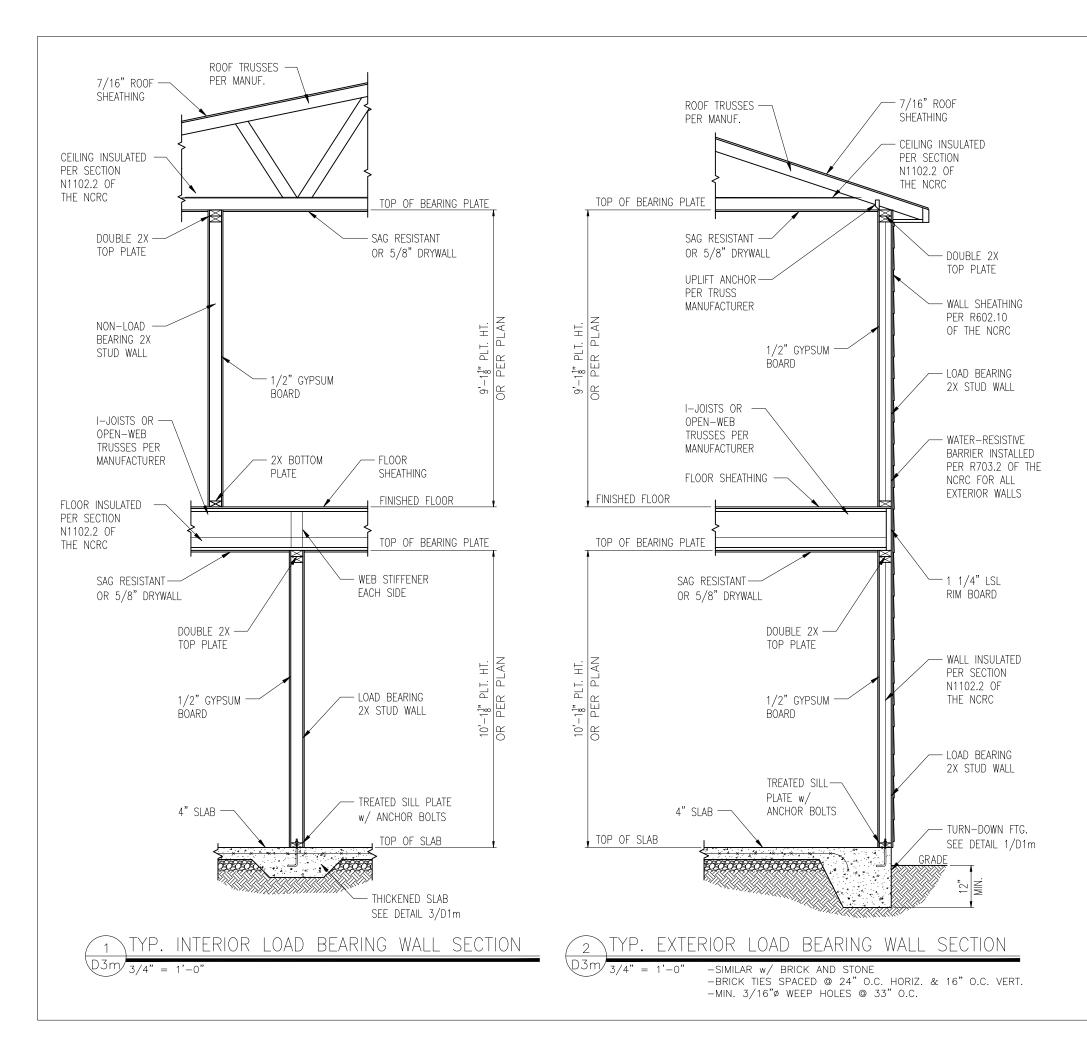
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- NOTES:
 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.



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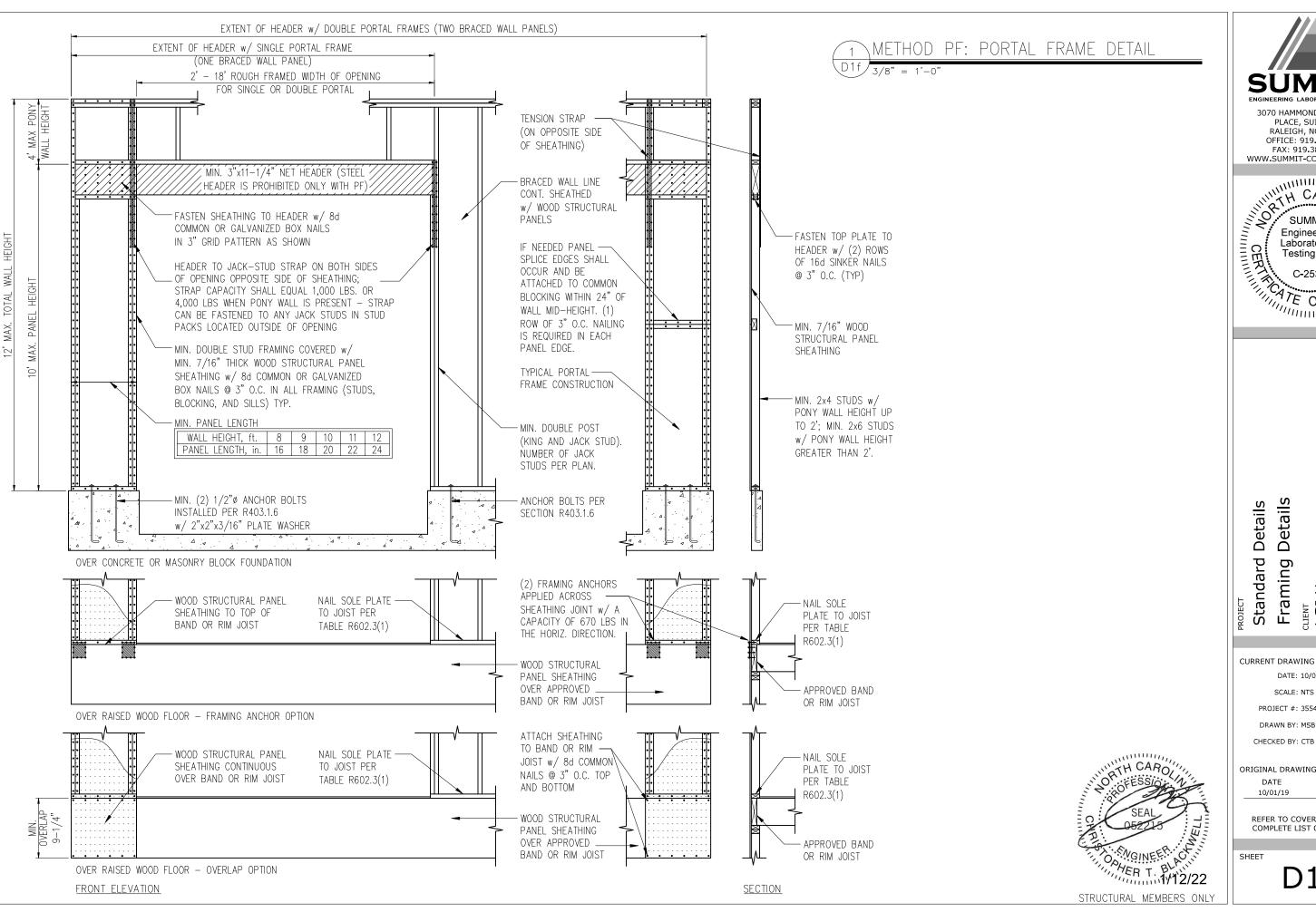
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> Details Details CLGI Homes 3037 Sherma Lancaster, SC Framing Standard

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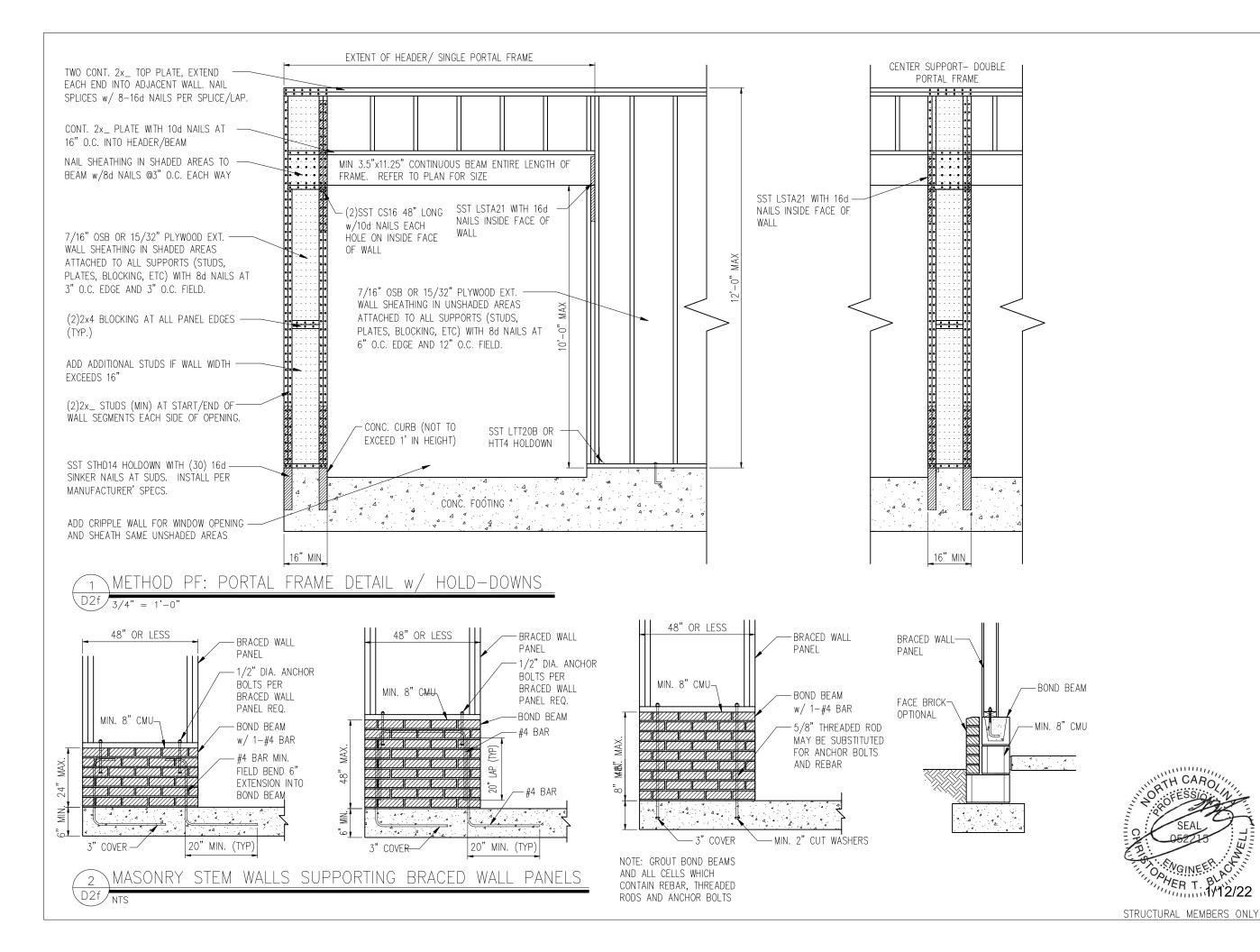
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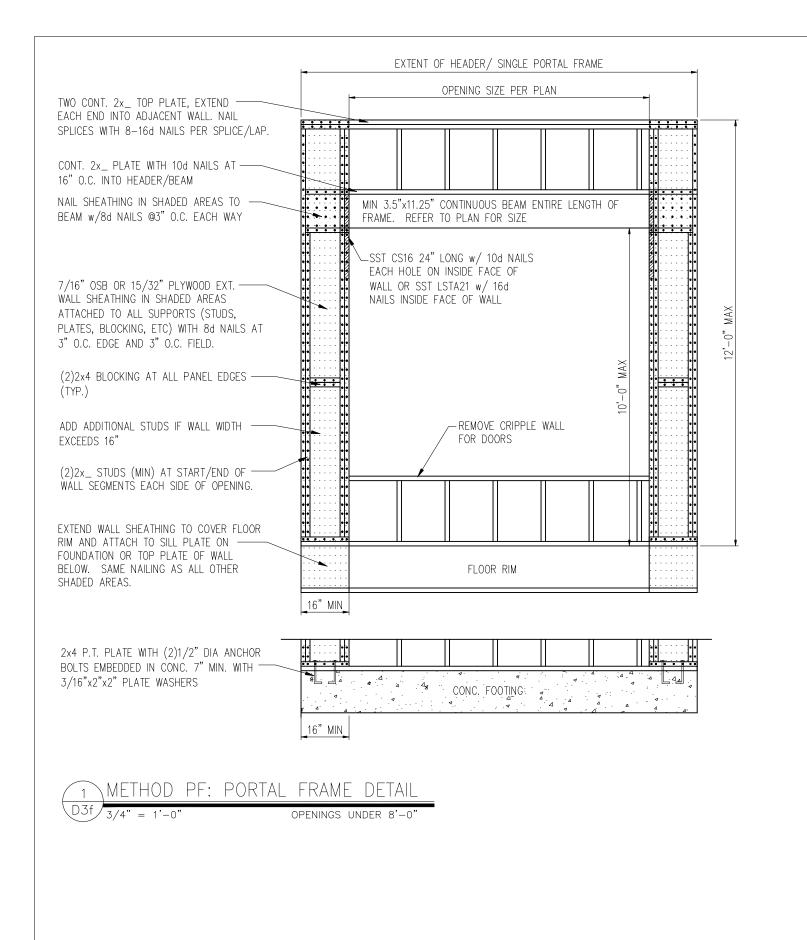
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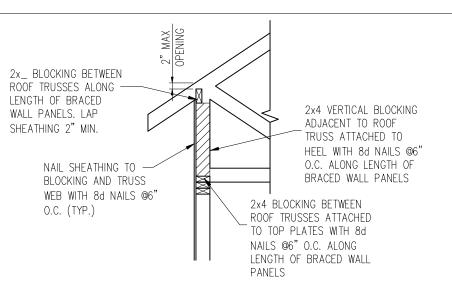
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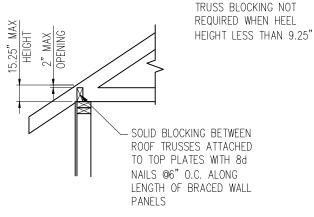
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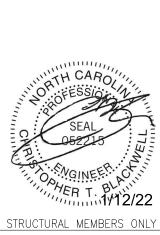


HEEL HEIGHT BETWEEN 15.25" AND 48"



HEEL HEIGHT BETWEEN 9.25" AND 15.25"







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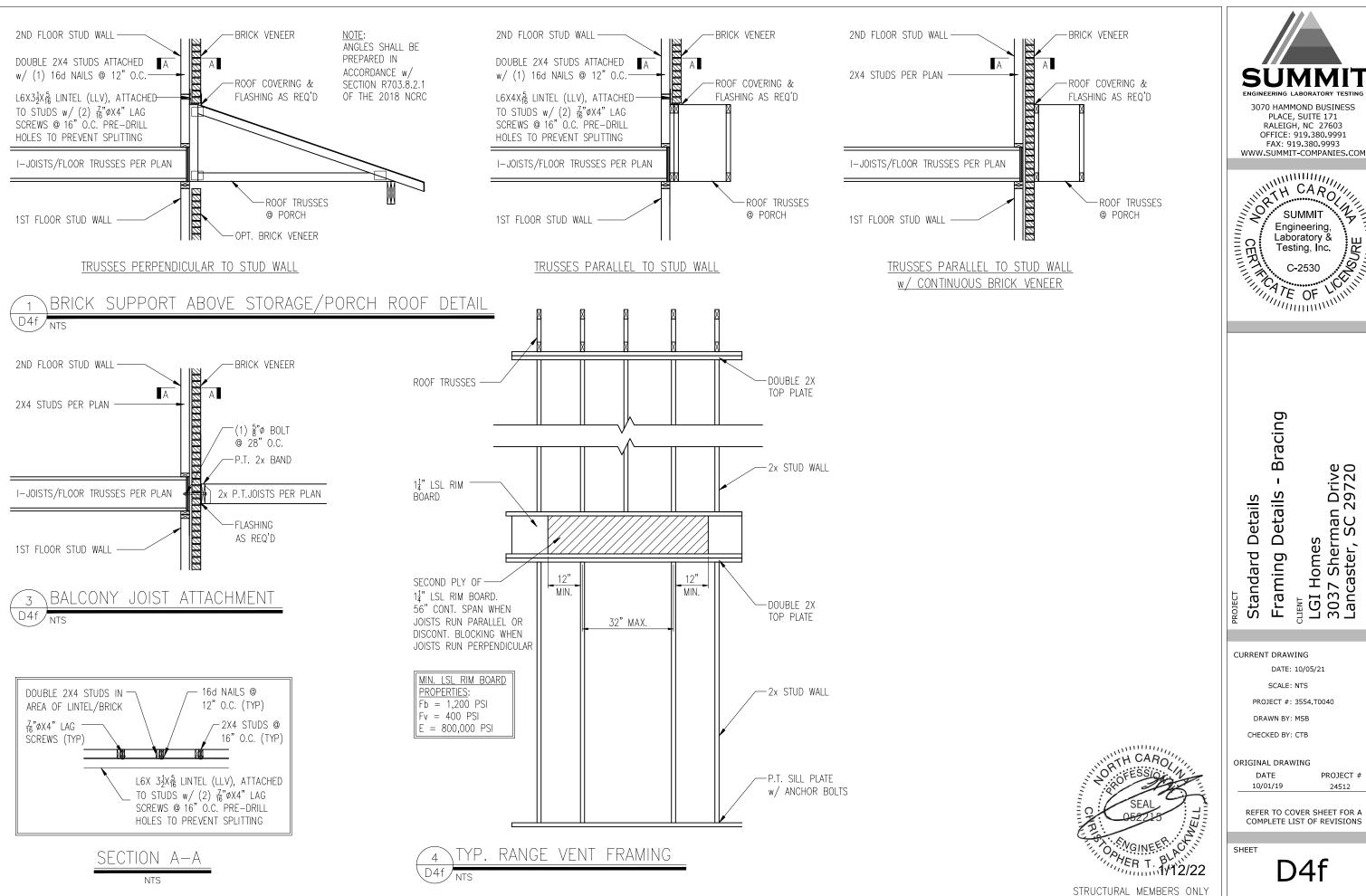
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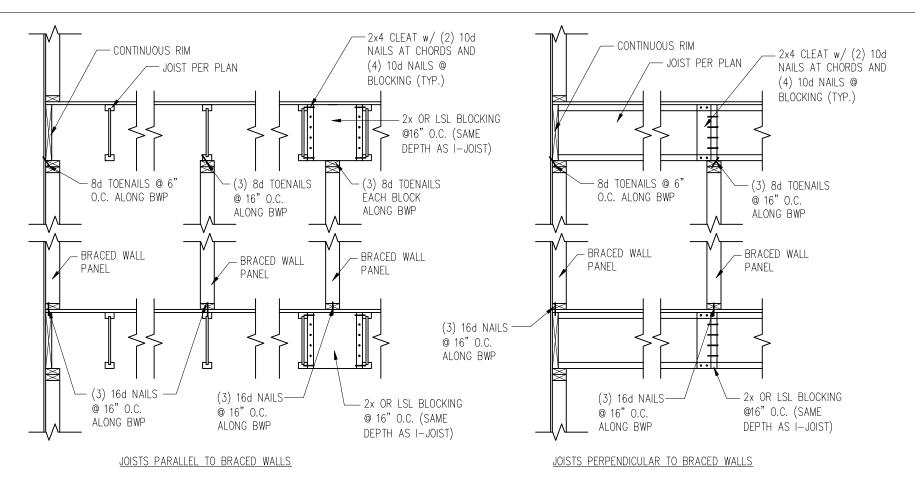
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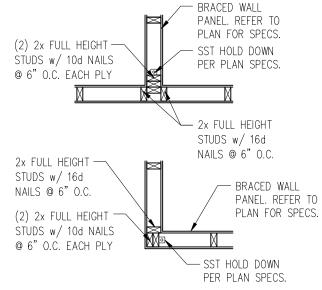
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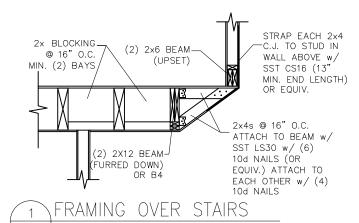
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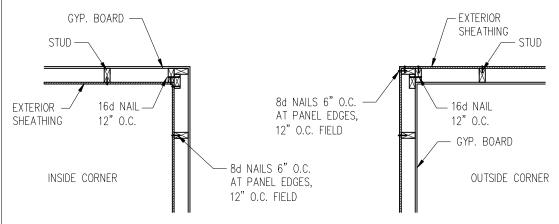


'P. HOLD DOWN DETAIL

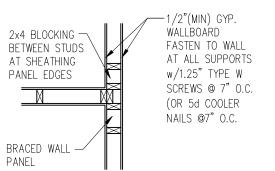


TYP. WALL PANEL TO FLOOR/CEILING CONNECTION

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NTERIOR 3-STUD WALL INTERSECTION



STRUCTURAL MEMBERS ONLY





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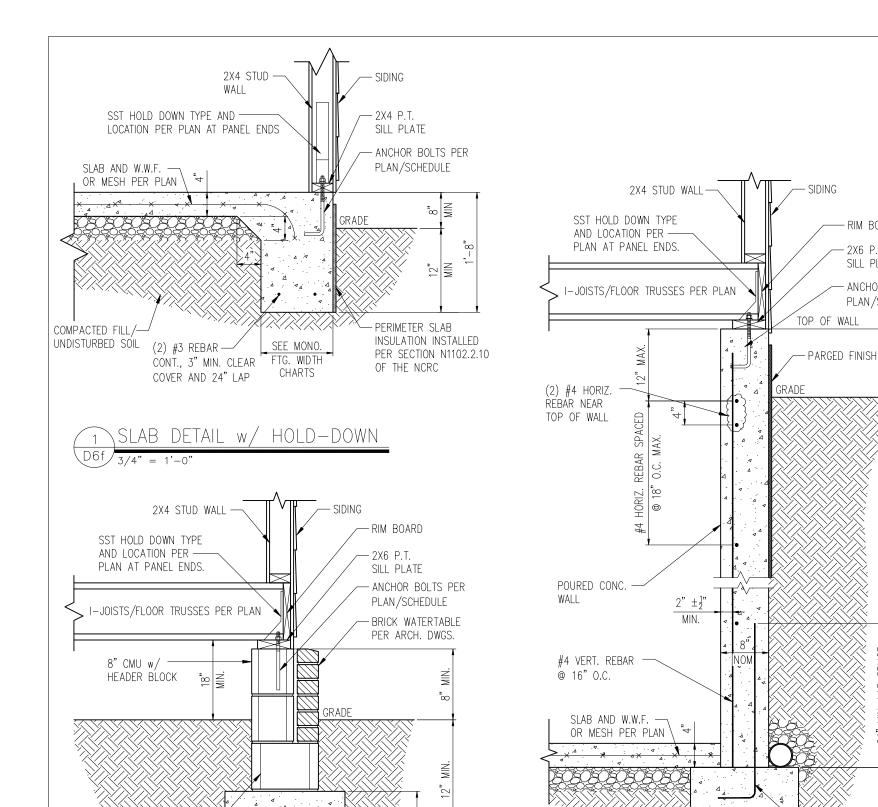
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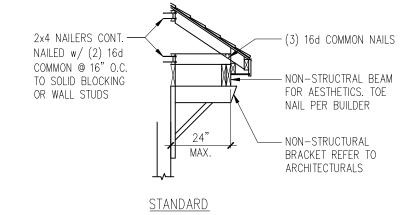
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(2) #3 REBAR

CONT., 3" MIN. CLEAR

COVER AND 24" LAP



RIM BOARD

ANCHOR BOLTS PER

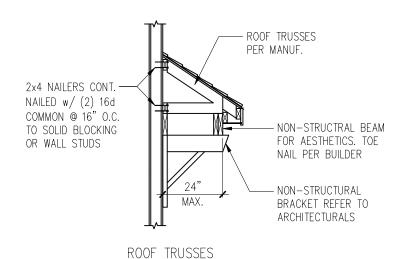
PLAN/SCHEDULE

#4X30" DOWEL

@ 16 " O.C.

BASEMENT FOUNDATION WALL DETAIL W/ HOLD-DOWN

2X6 P.T. SILL PLATE



GABLE ROOF RETURN

ONGINEER ON HER T. BLANNING



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(2) #3 REBAR -

CONT., 3" MIN. CLEAR

COVER AND 24" LAP

SEE BASEMENT

FTG. WIDTH CHARTS

STANDARD - SIDING

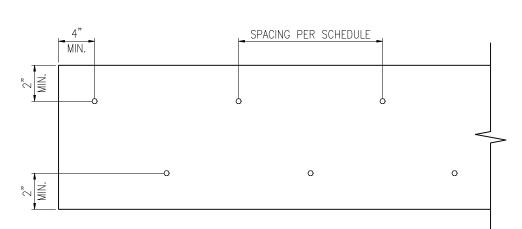
CRAWL FOUNDATION WALL DETAIL W/ H-D

SEE CRAWL SPACE

FTG. WIDTH CHARTS

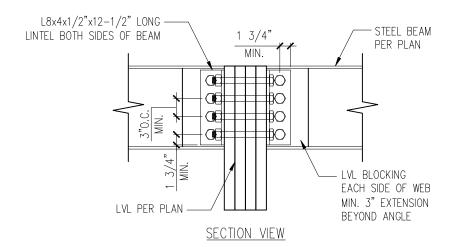
12" CMU

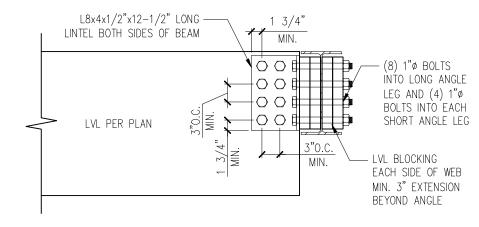
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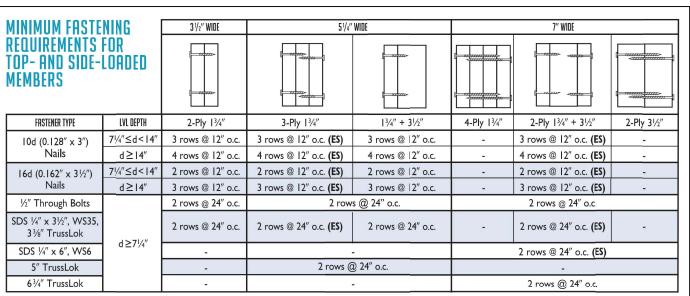
ELEVATION VIEW

MULTI-PLY BEAM CONNECTION DETAIL





ELEVATION VIEW

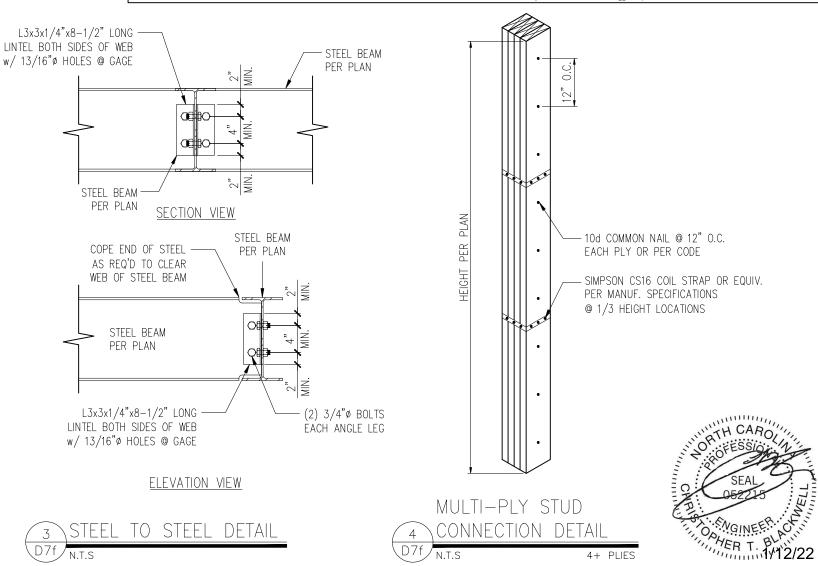


- $\label{lambda} \textbf{I.All fasteners must meet the minimum requirements in the table above. Side-loaded}$ multiple-ply members must meet the minimum fastening and side-loading capacity requirements given on page 48.
- 2. Minimum fastening requirements for depths less than $7\frac{1}{4}$ require special consideration. Please contact your technical representative.
- 3. Three general rules for staggering or offsetting for a certain fastener schedule:

4+ PLIES

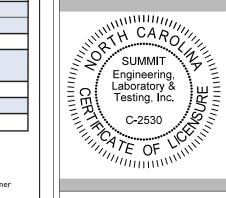
clearances above; and

- (I) if staggering or offsetting is not referenced, then none is required; (2) if staggering is referenced, then fasteners installed in adjacent rows on the front side are to be staggered up to one-half the o.c. spacing, but maintaining the fastener
- (3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).





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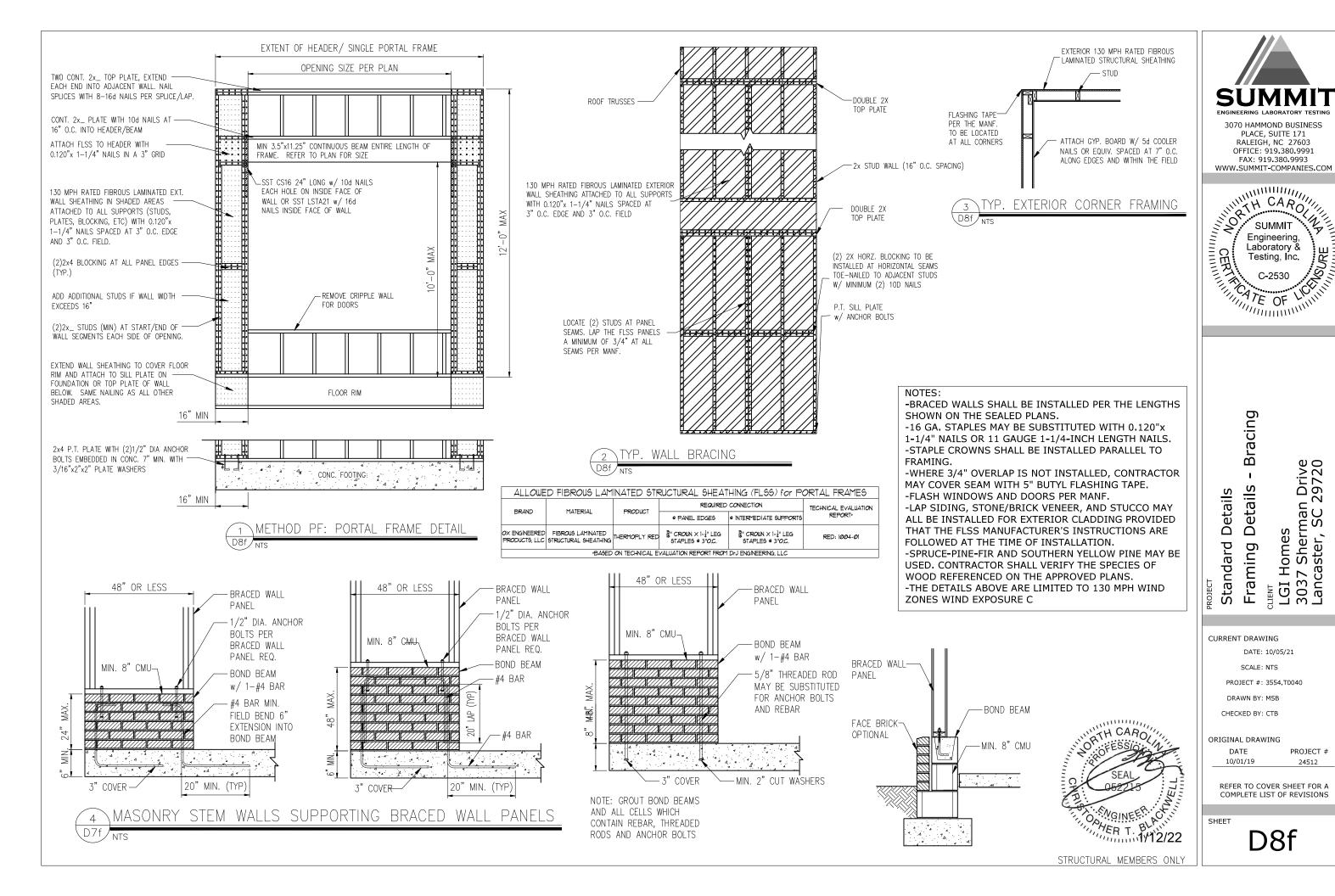
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