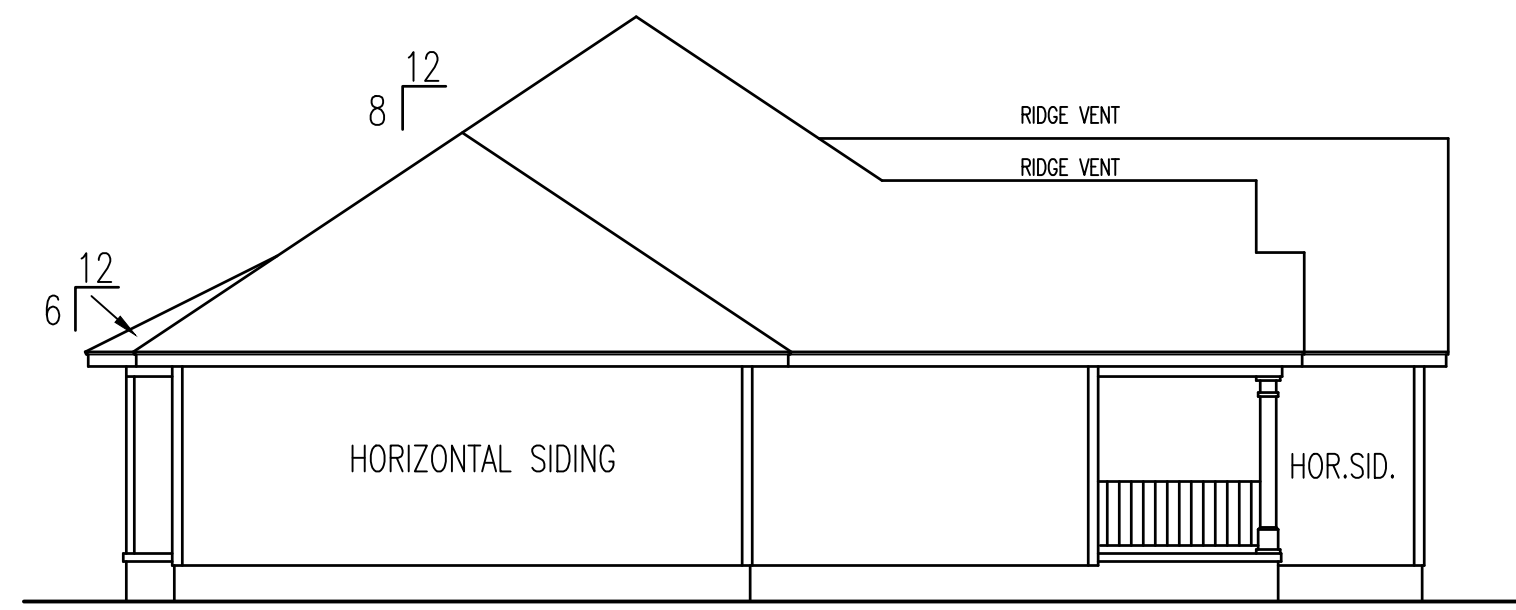
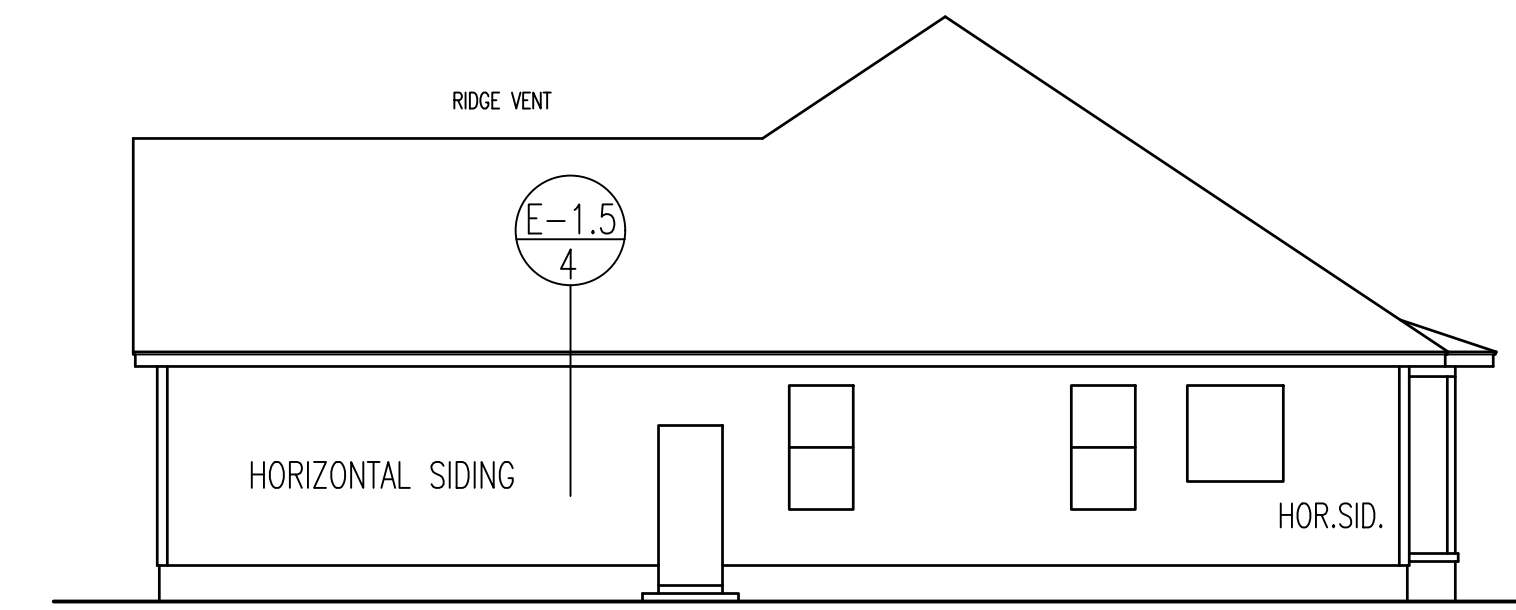


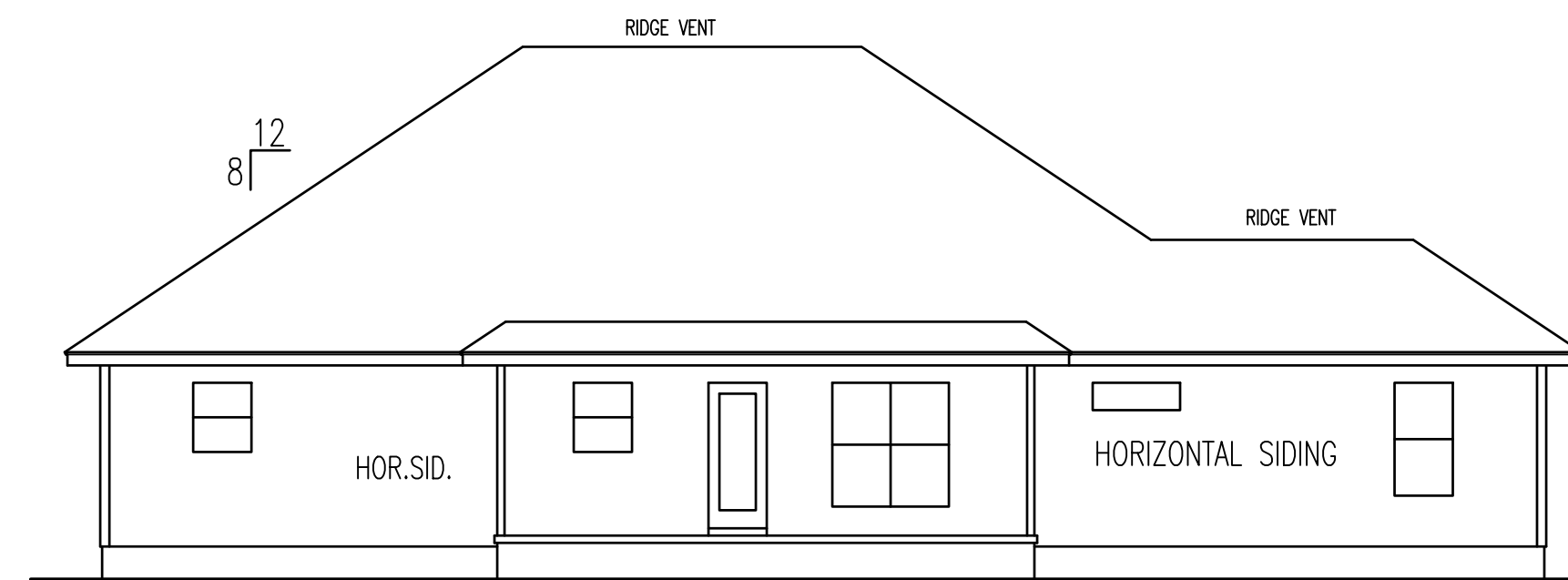
FRONT ELEVATION  
SCALE: 1/4" = 1'-0"



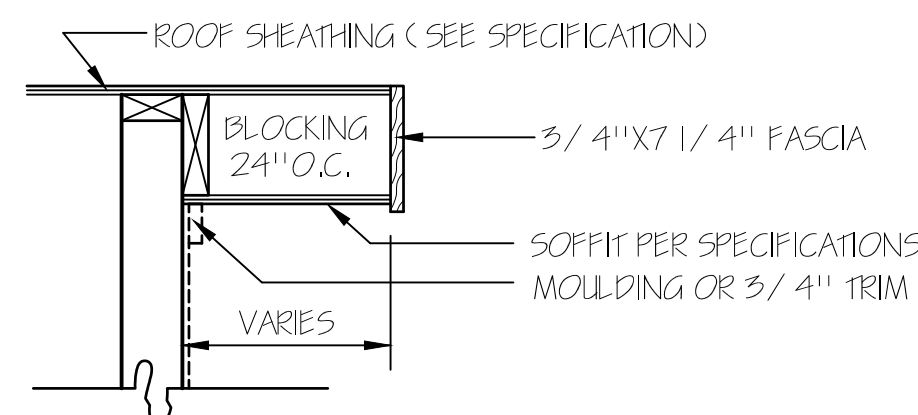
LEFT ELEVATION



RIGHT ELEVATION



REAR ELEVATION  
SCALE: 1/8" = 1'-0"



RAKE DETAIL FOR GABLE ENDS

**T M DESIGNS**  
RESIDENTIAL PLANS BY TINA MCFADDEN  
(910) 354-4736 TMDDESIGNS2016@GMAIL.COM

**WATERMARK HOMES**  
EXCLUSIVE RESIDENCE DESIGN FOR:  
LOT: 162 BALLARD WOODS  
NAME: EUCALYPTUS

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T M DESIGNS WILL NOT BE LIABLE FOR ANY ERRORS NOT BROUGHT TO THEIR ATTENTION PRIOR TO THE START OF CONSTRUCTION. WHILE EVERY EFFORT WAS MADE IN THE PREPARATION OF THESE DRAWINGS AND DIMENSIONS TO AVOID ERRORS THE OWNER AND / OR BUILDER SHALL VERIFY ALL DIMENSIONS, DETAILS, LOCAL AND STATE CODES.

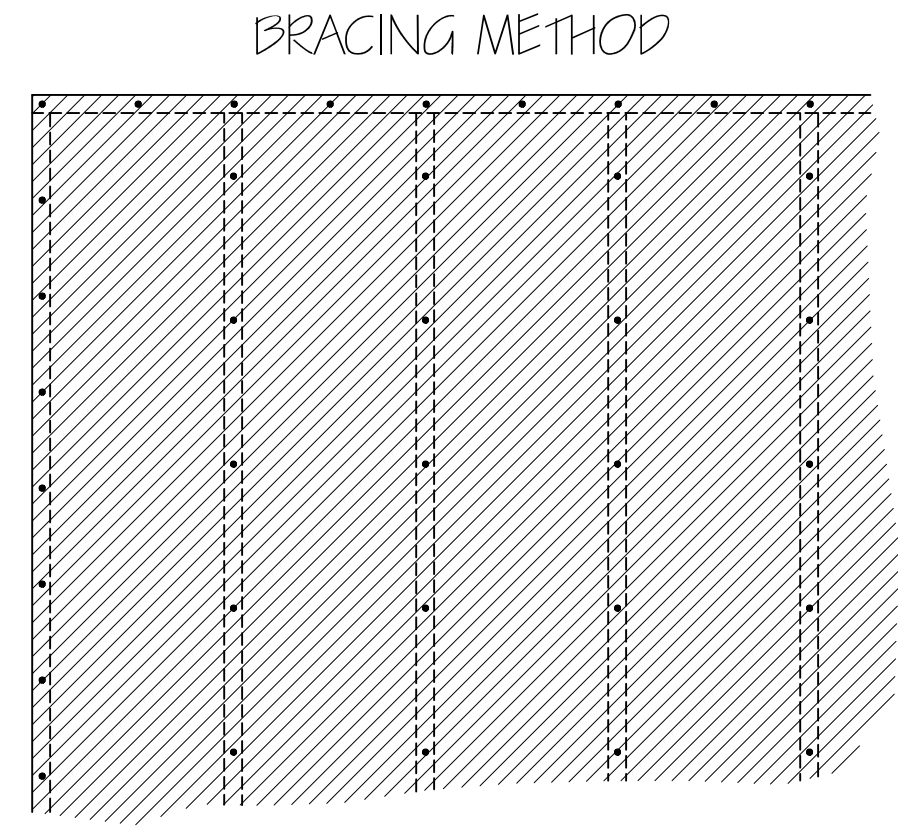
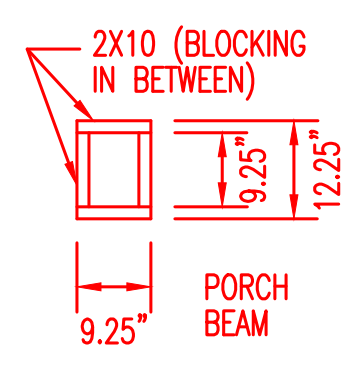
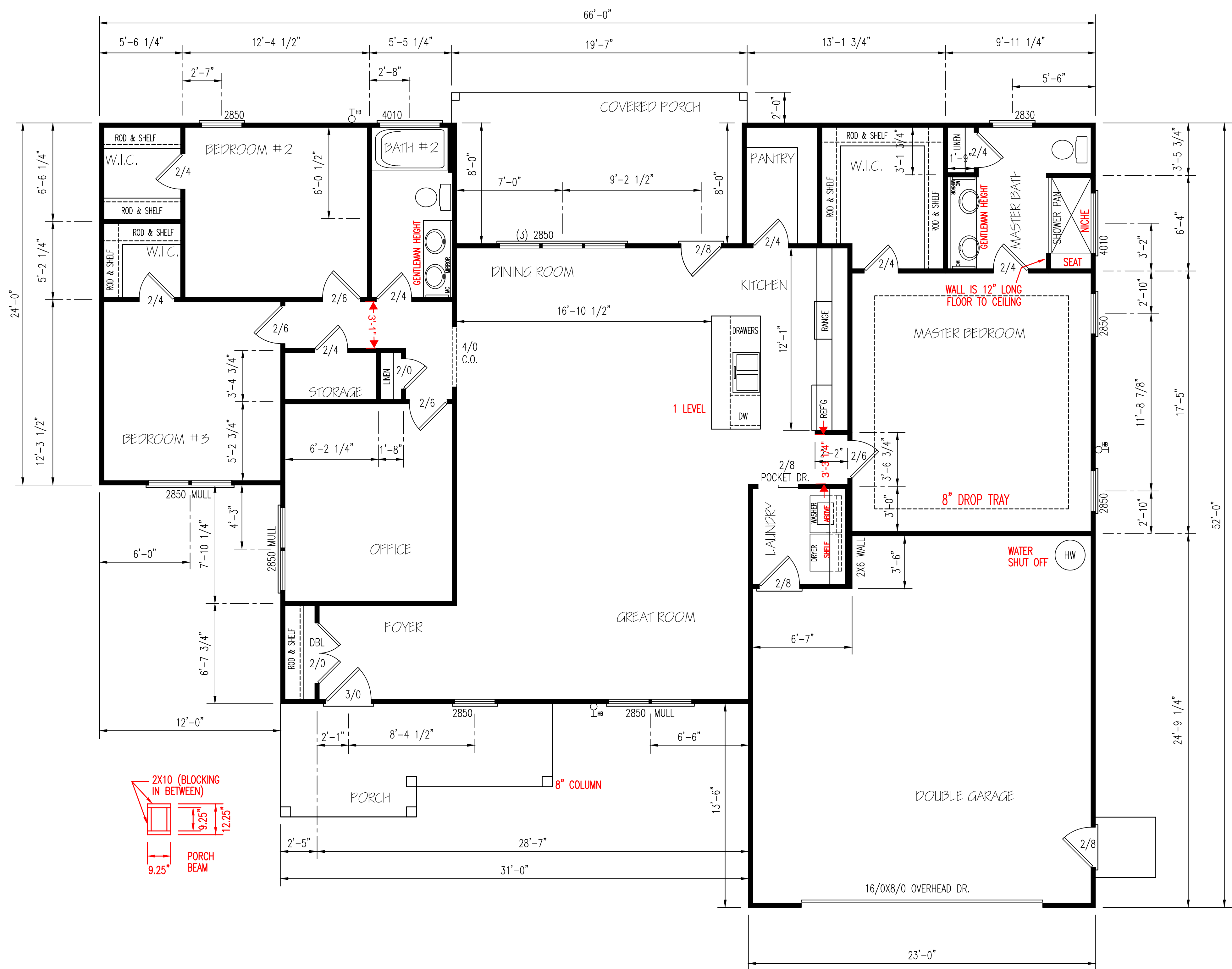
I HEREBY CERTIFY THAT THIS DRAWING MEETS LOCAL CODES, 2012 INTERNATIONAL BUILDING CODES

THIS IS FOR THE CONSTRUCTION OF ONE HOUSE ON A SINGLE LOT. NOT TO BE REUSED

PLAN NUMBER  
**RG18-A01**

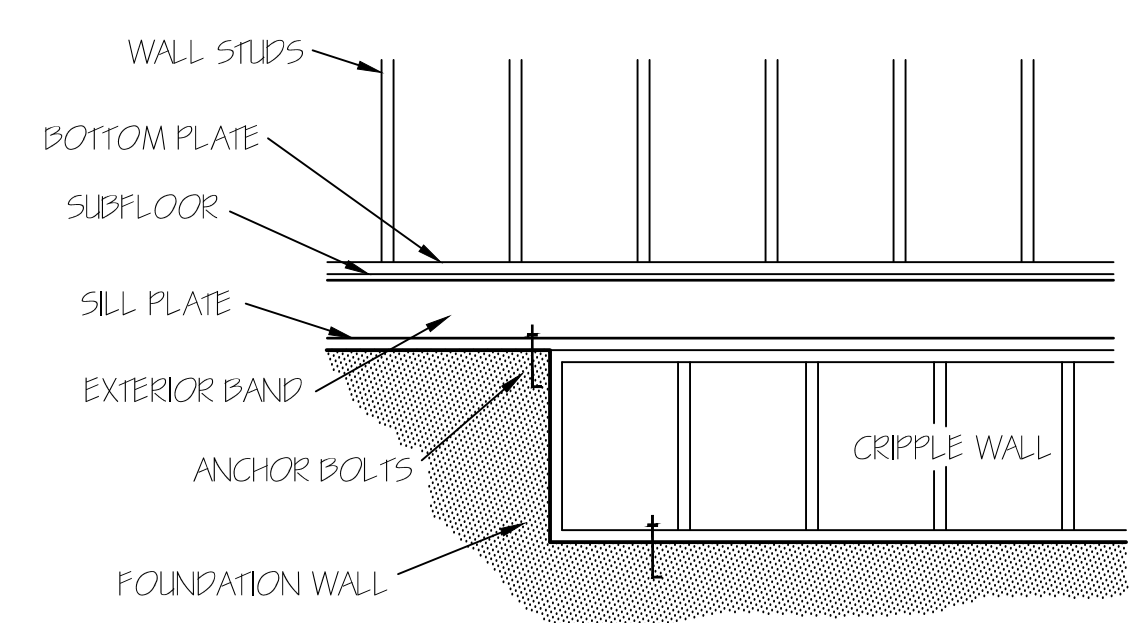
OPTION #1

<b>1</b>	GARAGE	R	F
	DATE:	11/4/20	



EXTERIOR WALL TO BE FULLY SHEATHED WITH 7/16\"/>

**ENERGY TABLE**  
 FACTOR OF WINDOWS .30  
 CLIMATE ZONE 3  
 INSULATION: WALLS 15  
 CEILING 38  
 FLOORS 19



FOUNDATION CRIPPLE WALLS SHALL BE FRAMED OF STUDS NOT SMALLER THAN THE STUDDING ABOVE. WHEN EXCEEDING 4 FT. IN HEIGHT, SUCH WALLS SHALL BE FRAMED OF STUDS HAVING THE SIZE REQUIRED FOR AN ADDITIONAL STORY. CRIPPLE WALLS WITH A STUD HEIGHT LESS THAN 14 INCHES SHALL BE CONTINUOUSLY SHEATHED ON ONE SIDE WITH WOOD STRUCTURAL PANELS FASTENED TO BOTH THE TOP AND BOTTOM PLATES IN ACCORDANCE WITH TABLE R602.3X(D). OR CRIPPLE WALLS SHALL BE CONSTRUCTED OF SOLID BLOCKING.

**NOTE:**  
 CEILINGS ARE 9'-0\"/>

(2) 2X10 HEADERS	
CLEAR SPAN FOR HEADER	NUMBER OF JACK STUDS
ALL DOOR & C.O. WIDTH 5'-0\"/>	1
ALL DOOR & C.O. WIDTH ABOVE 5'-0\"/>	2
3/0 DOOR W/ SIDE LITES	2
ALL SINGLE WINDOWS	1
ALL TWIN WINDOWS	2
ALL TRIPLE WINDOWS	3
<b>**UNLESS NOTED OTHER WISE**</b>	

**FLOOR PLAN**  
 SCALE: 1/4\"/>

HEATED AREA  
 1986 SQ FT

OTHER AREAS  
 GARAGE 540 SQ FT  
 F.PORCH 117 SQ FT  
 R.PORCH 191 SQ FT  
 TOTAL 878 SQ FT

**GARAGE PANEL WALL**

GARAGE PANEL WALLS UNDER 24\"/>

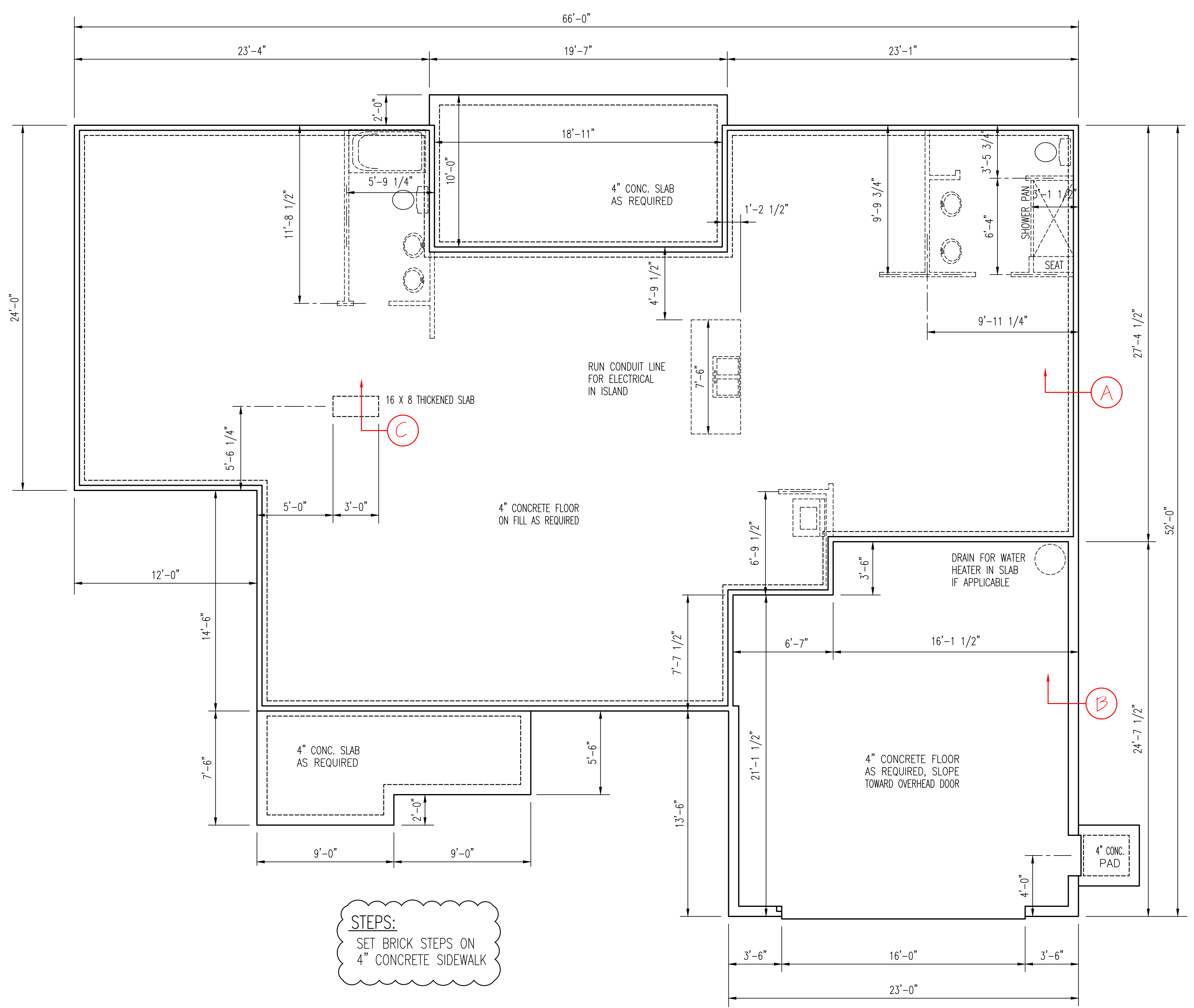
**TM DESIGNS**  
 RESIDENTIAL PLANS BY TINA MCFADDEN  
 (910) 354-4736 TMDDESIGNS2016@GMAIL.COM

**WATERMARK HOMES**  
 LOT: 162 BALLARD WOODS  
 NAME: EUCALYPTUS

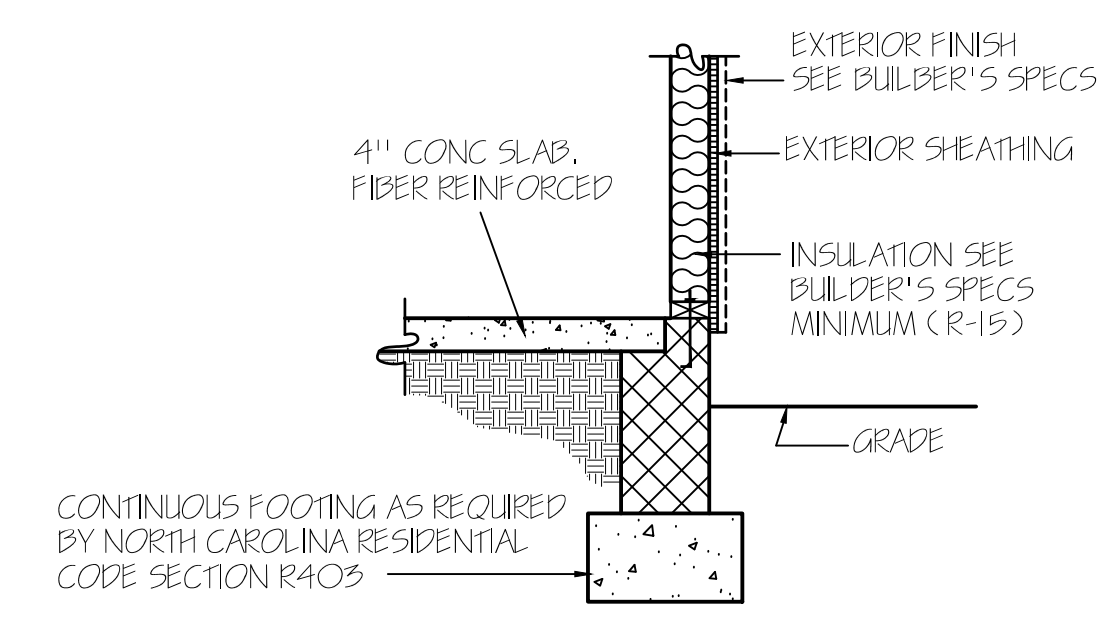
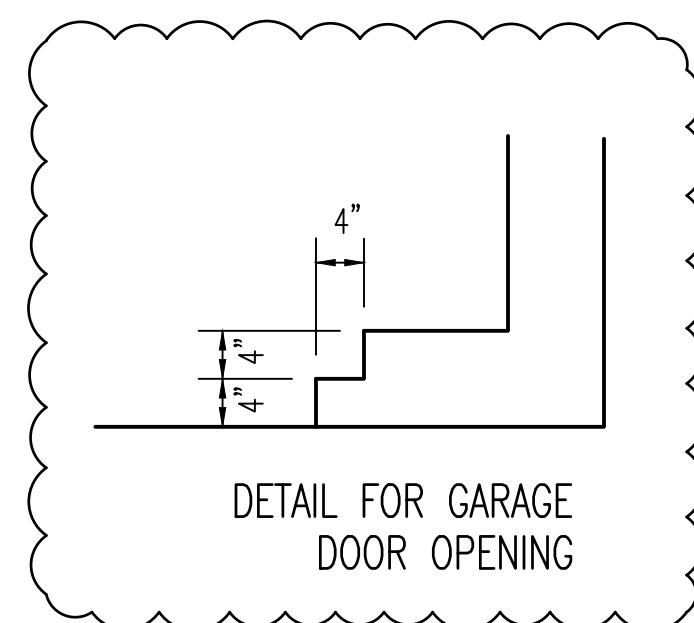
EXCLUSIVE RESIDENCE DESIGN FOR:

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 TM DESIGNS WILL NOT BE LIABLE FOR ANY ERRORS NOT BROUGHT TO THEIR ATTENTION PRIOR TO THE START OF CONSTRUCTION. WHILE EVERY EFFORT WAS MADE IN THE PREPARATION OF THESE DRAWINGS AND DIMENSIONS TO AVOID ERRORS THE OWNER AND/ OR BUILDER SHALL VERIFY ALL DIMENSIONS, DETAILS, LOCAL AND STATE CODES.  
 I HEREBY CERTIFY THAT THIS DRAWING MEETS LOCAL CODES, 2012 INTERNATIONAL BUILDING CODES.  
 THIS IS FOR THE CONSTRUCTION OF ONE HOUSE ON A SINGLE LOT, NOT TO BE REUSED.

**PLAN NUMBER**  
 RA18-AOI  
**OPTION #1**  
**2** GARAGE R F  
 DATE: 11/4/20

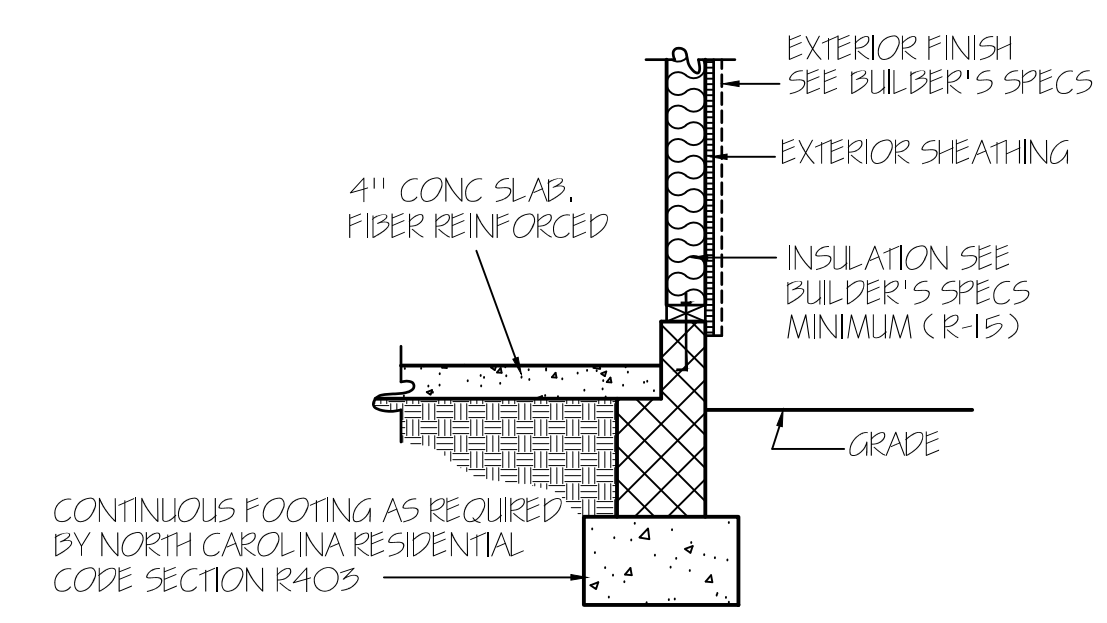


**STEPS:**  
SET BRICK STEPS ON 4" CONCRETE SIDEWALK



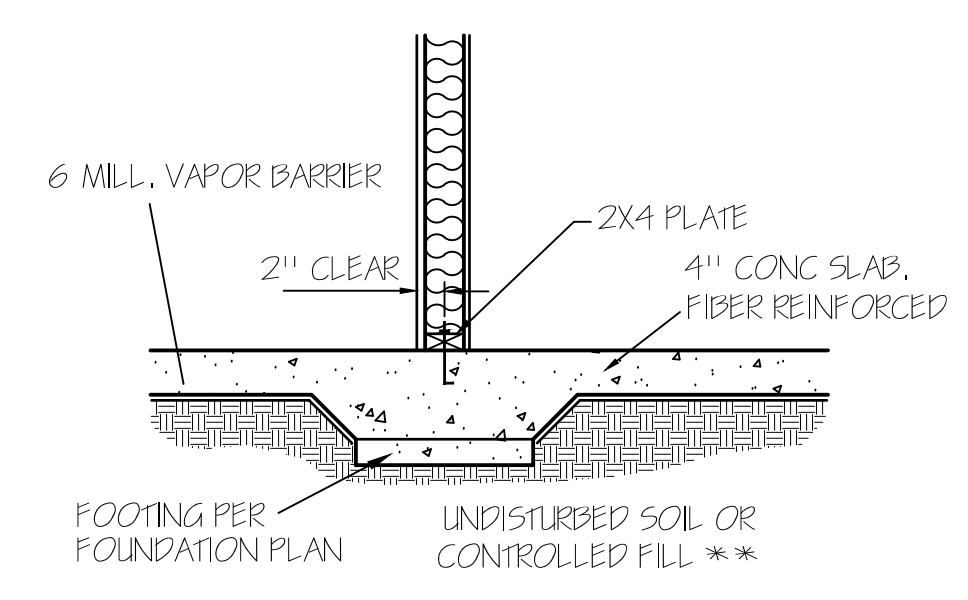
NOTE:  
PERIMETER INSUL. MAY EXTEND HORIZ. UNDER SLAB 24" OR VERTICAL 24" BELOW SLAB FLOOR

CONCRETE SLAB FLOOR — (A)



NOTE:  
PERIMETER INSUL. MAY EXTEND HORIZ. UNDER SLAB 24" OR VERTICAL 24" BELOW SLAB FLOOR

GARAGE WALL — (B)



LOAD BEARING WALL THICKENED SLAB — (C)

**WALL ANCHOR OPTIONS**  
USE ANCHOR BOLTS  
ANCHOR BOLTS: 1/2" DIA. BOLTS AT 6'-0" O.C. AND NOT MORE THAN 12" FROM CORNERS, EMBEDDED MIN. 7" INTO FOUNDATION. USE A MIN. OF 2 BOLTS PER EACH STUD WALL.

NOTE:  
FOUNDATION DETAILS SHOWN ARE BASED ON ASSUMED SOIL BEARING CAPACITY OF 2000 PSF. LOCAL SITE CONDITIONS MUST BE INVESTIGATED. ALL FOOTING TO BE LOCATED BELOW FROST DEPTH.

**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"



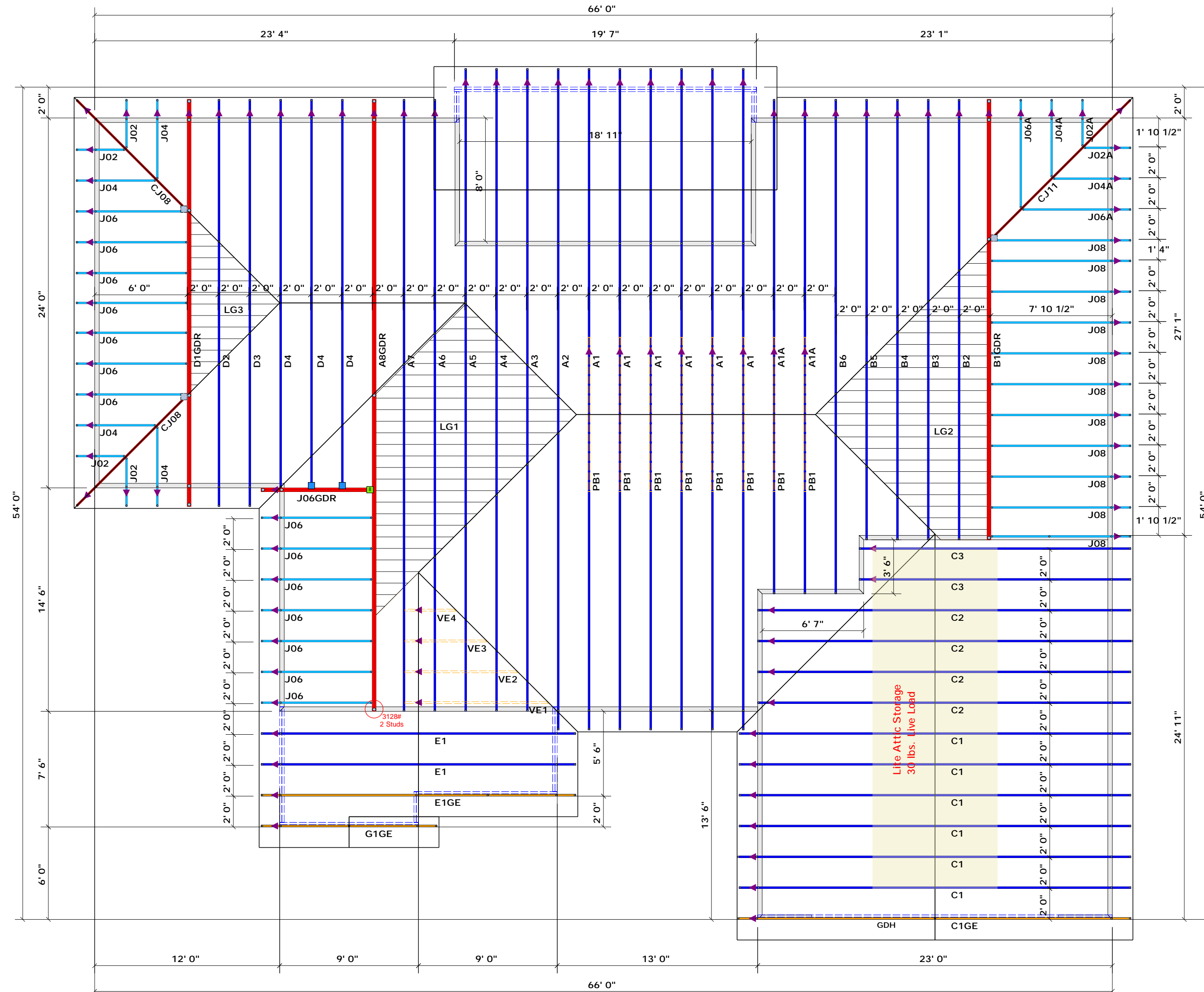
# ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park  
Fayetteville, N.C. 28309  
Phone: (910) 864-8787  
Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature **Curtis Quick**  
Curtis Quick

LOAD CHART FOR JACK STUDS			
(BASED ON TABLES ROU511, 512)			
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADERS/STROPS			
END REACTION (IP / O)	REQ. STUDS FOR 16D/3" HEADER	END REACTION (IP / O)	REQ. STUDS FOR 10D/3" BEAM
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		



▲ = Denotes Left End of Truss  
(Reference Engineered Truss Drawing)  
Do Not Erect Trusses Backwards

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.  
○ -- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan  
SCALE: 3/16" = 1"

Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
■	HJC26	USP	3	Varies	16d/3-1/2"	10d/3"
■	HUS26	USP	2	Varies	16d/3-1/2"	16d/3-1/2"
■	THD28-2	USP	1	Varies	16d/3-1/2"	10d/3"

Beam Legend					
PlotID	Length	Product	Plies	Net Qty	Fab Type
GDH	23' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF

BUILDER	Watermark Homes	Lillington / Harnett
JOB NAME	Lot 162 Ballard Woods	Lot 162 Ballard Woods
PLAN	The Eucalyptus	Roof
SEAL DATE	11/4/20	09/20/22
QUOTE #	Quote #	Curtis Quick
JOB #	J0922-4792	Anthony Williams
CITY / CO.	Lillington / Harnett	
ADDRESS	Lot 162 Ballard Woods	
MODEL	Roof	
DATE REV.	09/20/22	
DRAWN BY	Curtis Quick	
SALES REP.	Anthony Williams	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com.



**ROOF & FLOOR TRUSSES & BEAMS**

Reilly Road Industrial Park  
Fayetteville, N.C. 28309  
Phone: (910) 864-8787  
Fax: (910) 864-4444

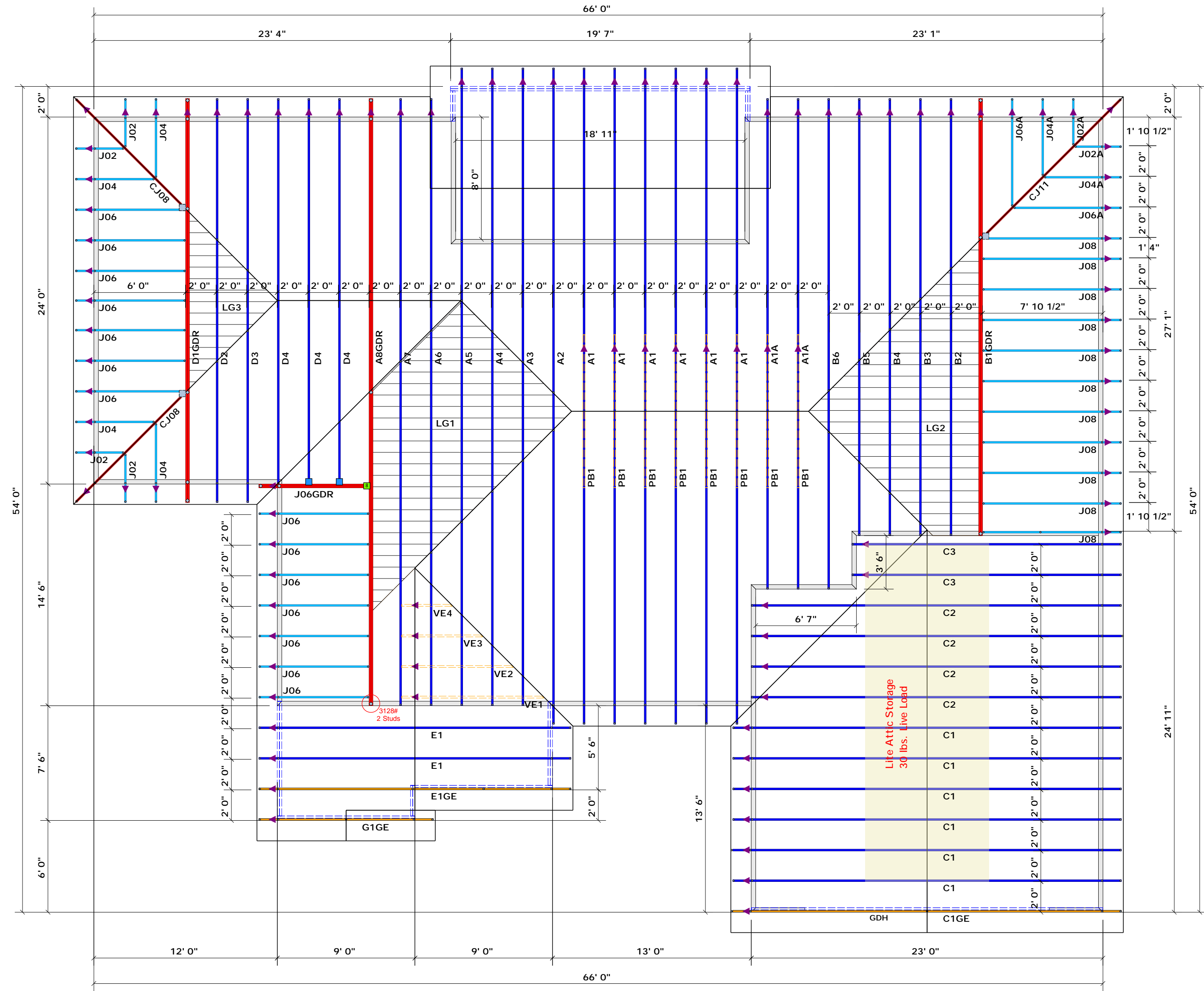
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature **Curtis Quick**  
**Curtis Quick**

**LOAD CHART FOR JACK STUDS**

(BASED ON TABLES ROU11C & 11D)  
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADERS/STRIPS

END REACTION (IP/TS)	REQ'D STUDS FOR 10' PLATE	END REACTION (IP/TS)	REQ'D STUDS FOR 10' PLATE
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		



▲ = Denotes Left End of Truss  
(Reference Engineered Truss Drawing)  
Do Not Erect Trusses Backwards

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.  
○ -- Denotes Reaction Greater than 3,000 lbs.

**Truss Placement Plan**  
SCALE: 3/16" = 1'

Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
■	HJC26	USP	3	Varies	16d/3-1/2"	10d/3"
■	HUS26	USP	2	Varies	16d/3-1/2"	16d/3-1/2"
■	THD28-2	USP	1	Varies	16d/3-1/2"	10d/3"

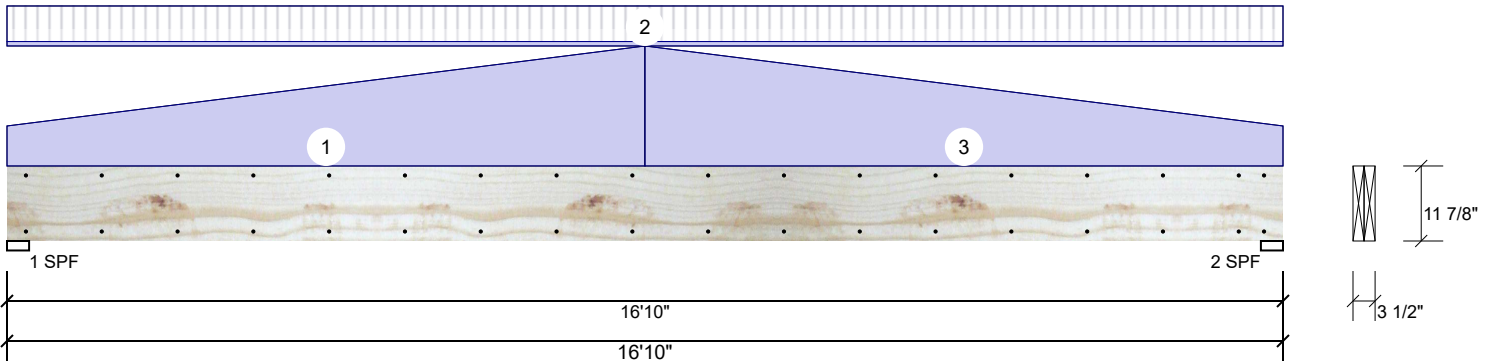
Beam Legend						
PlotID	Length	Product	Plies	Net Qty	Fab Type	
GDH	23' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF	

BUILDER	Watermark Homes	Lillington / Harnett
JOB NAME	Lot 162 Ballard Woods	Lot 162 Ballard Woods
PLAN	The Eucalyptus	Roof
SEAL DATE	11/4/20	09/20/22
QUOTE #	Quote #	Curtis Quick
JOB #	J0922-4792	Anthony Williams
CITY / CO.	Lillington / Harnett	
ADDRESS	Lot 162 Ballard Woods	
MODEL	Roof	
DATE REV.	09/20/22	
DRAWN BY	Curtis Quick	
SALES REP.	Anthony Williams	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com.

**GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED**

Level: Level



**Member Information**

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

**Reactions UNPATTERNED lb (Uplift)**

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	337	877	0	0	0
2	Vertical	337	877	0	0	0

**Bearings**

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	23%	877 / 337	1214	L	D+L
2 - SPF	3.500"	Vert	23%	877 / 337	1214	L	D+L

**Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5365 ft-lb	8'5"	19911 ft-lb	0.269 (27%)	D+L	L
Unbraced	5365 ft-lb	8'5"	6063 ft-lb	0.885 (88%)	D+L	L
Shear	1087 lb	15'6 5/8"	8867 lb	0.123 (12%)	D+L	L
LL Defl inch	0.070 (L/2809)	8'5 1/16"	0.409 (L/480)	0.171 (17%)	L	L
TL Defl inch	0.276 (L/712)	8'5 1/16"	0.546 (L/360)	0.506 (51%)	D+L	L

**Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Tapered Start	0-0-0		Top	45 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Gable
	End	8-5-0		Top	135 PLF	0 PLF	0 PLF	0 PLF	0 PLF	
2	Tie-In	0-0-0 to 16-10-0	1-0-0	Top	5 PSF	40 PSF	0 PSF	0 PSF	0 PSF	Roof
3	Tapered Start	8-5-0		Top	135 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Gable
	End	16-10-0		Top	45 PLF	0 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				9 PLF					

**Notes**  
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

**Lumber**  
 1. Dry service conditions, unless noted otherwise  
 2. LVL not to be treated with fire retardant or corrosive

**Handling & Installation**  
 1. LVL beams must not be cut or drilled  
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals  
 3. Damaged Beams must not be used  
 4. Design assumes top edge is laterally restrained  
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 5/24/2024

**Manufacturer Info**

Metsä Wood  
 301 Merritt 7 Building, 2nd Floor  
 Norwalk, CT 06851  
 (800) 622-5850  
[www.metsawood.com/us](http://www.metsawood.com/us)

Comtech, Inc.  
 1001 S. Reilly Road, Suite #639  
 Fayetteville, NC  
 USA  
 28314  
 910-864-TRUS



**GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED**

Level: Level



**Multi-Ply Analysis**

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

**Notes**

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

**Lumber**

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 5/24/2024

**Manufacturer Info**

Metsä Wood  
 301 Merritt 7 Building, 2nd Floor  
 Norwalk, CT 06851  
 (800) 622-5850  
[www.metsawood.com/us](http://www.metsawood.com/us)

Comtech, Inc.  
 1001 S. Reilly Road, Suite #639  
 Fayetteville, NC  
 USA  
 28314  
 910-864-TRUS

