

RE: 27210-27210A Aberdeen Loft Vlt Mst Trenco 818 Soundside Rd Edenton, NC 27932

 Site Information:

 Customer: D.R. HORTON - RAL - 055
 Project Name: 27210-27210A

 Lot/Block:
 Model: 1906 / ABERDEEN / 2ND FLOOR/ CATH MSTR

 Address:
 Subdivision:

 City: ROCKY MOUNT
 State: NC

### General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 17 individual, dated Truss Design Drawings and 0 Additional Drawings.

| No. | Seal#     | Truss Name | Date      |
|-----|-----------|------------|-----------|
| 1   | 146497202 | A          | 6/10/2021 |
| 2   | 146497203 | A1         | 6/10/2021 |
|     |           |            |           |
| 3   | 146497204 | A2         | 6/10/2021 |
| 4   | 146497205 | A3         | 6/10/2021 |
| 5   | 146497206 | A4         | 6/10/2021 |
| 6   | 146497207 | A4GR       | 6/10/2021 |
| 7   | 146497208 | A5         | 6/10/2021 |
| 8   | 146497209 | A5E        | 6/10/2021 |
| 9   | l46497210 | A5GR       | 6/10/2021 |
| 10  | l46497211 | AE         | 6/10/2021 |
| 11  | l46497212 | В          | 6/10/2021 |
| 12  | l46497213 | B1         | 6/10/2021 |
| 13  | 146497214 | С          | 6/10/2021 |
| 14  | l46497215 | D          | 6/10/2021 |
| 15  | l46497216 | PB1        | 6/10/2021 |
| 16  | 146497217 | PB2        | 6/10/2021 |
| 17  | 146497218 | V1         | 6/10/2021 |

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by 84 Components - #2383.

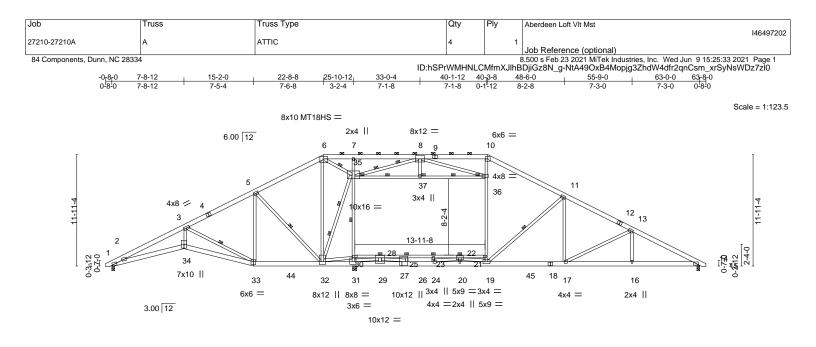
Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of North Carolina is December 31, 2021

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





 26-0-4
 40-3-8

 7-8-12
 15-2-0
 22-8-8
 125-10-12
 28-8-15
 31-7-3
 34-5-6
 37-3-9
 40-1-12
 48-6-0
 55-9-0
 63-0-0

 7-8-12
 7-5-4
 7-6-8
 32-24
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 2-10-3
 2-10-3
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 8-2-8
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| Plate Offsets (X,Y)  | [2:0-0-12,Edge], [6:0-5-0,0-3-0], [27:0-5   | -0,0-2-12], [28:0-6-0,0-3-0]                       | , [31:0-1-8,0-3-4], [32:0                      | 0-3-8,0-4-0], [33:0-3-0,0-3-8  | ], [35:0-8-0,0-3-8]   |
|--|---|--|--|--|---|
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0   | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.95<br>BC 0.89<br>WB 0.98<br>Matrix-MS | Vert(LL) -0.4<br>Vert(CT) -0.9<br>Horz(CT) 0.3 | in (loc) l/defl L/d<br>i5 21-22 >990 240<br>i1 21-22 >487 180<br>i2 14 n/a n/a<br>i3 21-30 742 360 | PLATES         GRIP           MT20         197/144           MT18HS         197/144           Weight: 552 lb         FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 SP No.2<br>BOT CHORD 2x6 SP No.2 *Except*<br>27-33,18-27: 2x6 SP DSS, 21-30: 2x4 SP No.2 or 2x4 SPF No.2<br>WEBS 2x4 SP No.3 *Except*<br>6-32,6-35,35-36,8-35,8-36: 2x4 SP No.2 or 2x4 SPF No.2<br>7-31,10-19: 2x4 SP DSS, 32-35: 2x4 SP No.1 |   |  | BRACING-<br>TOP CHORD<br>BOT CHORD             | 2-0-0 oc purlins (2-2-0 m  | lied or 10-0-0 oc bracing, Except:  |
| 7 01,10 10. 2xt 01 200, 02 00. 2xt 01 10.1   |   |  | WEBS   | 1 Row at midpt<br>2 Rows at 1/3 pts<br>1 Brace at Jt(s): 35, 36, 3                                 | 3-33, 5-32, 6-35, 30-35, 11-19, 35-37,<br>36-37<br>8-35, 32-35<br>37  |
|  |   |  |  | MiTek recommends that<br>be installed during trus<br>Installation guide.                           | at Stabilizers and required cross bracing<br>s erection, in accordance with Stabilizer  |

REACTIONS. (lb/size) 2=1390/0-3-8 (min. 0-2-3), 31=2564/0-5-8 (min. 0-4-8), 14=1882/0-3-8 (min. 0-3-4) Max Horz 2=194(LC 16) Max Uplift 2=-7(LC 12), 14=-119(LC 13) Max Grav 2=1414(LC 2), 31=2862(LC 2), 14=2067(LC 27)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-4626/516, 3-4=-2003/238, 4-5=-1955/264, 5-6=-1403/191, 6-7=-190/3209,
  - 7-8=-201/3365, 8-9=-2567/234, 9-10=-2567/234, 10-11=-2803/193, 11-12=-3384/298, 12-13=-3530/262, 13-14=-3994/366
- BOT CHORD
   2-34=-369/4192, 33-34=-361/4050, 33-44=0/1739, 32-44=0/1739, 31-32=0/2265, 29-31=0/2437, 27-29=0/2437, 26-27=0/2437, 24-26=0/4428, 20-24=0/4001, 19-20=0/4001, 19-45=-12/3097, 18-45=-12/3097, 17-18=-12/3097, 16-17=-223/3507, 14-16=-223/3507, 28-30=0/573, 25-28=-2515/0, 23-25=-2515/0, 22-23=-2515/0

   WEBS
   3-34=-102/2083, 3-33=-2484/429, 5-33=-34/542, 5-32=-919/344, 6-32=-197/3053, 6-35=-5040/480, 30-31=-1749/420, 30-35=-1720/529, 7-35=-447/153, 19-21=0/1214, 21-36=0/1443, 10-36=0/910, 11-19=-978/366, 11-17=-31/545, 13-17=-566/237,
  - 13-16=0/262, 35-37=-1807/0, 36-37=-1805/0, 28-29=-706/0, 25-26=-304/0, 23-24=-281/0, 8-35=-3988/486, 8-36=0/2112, 32-35=-3123/0, 28-31=-1062/156, 26-28=0/2089, 19-22=-1893/0, 22-24=-150/637

NOTES-

nued on page

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

# SEAL 044925 MGINEEP, HE



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

| Job                           | Truss | Truss Type | Qty | Ply | Aberdeen Loft VIt Mst   |
|-------------------------------|-------|------------|-----|-----|---|
| 07010 070104                  |       |            |     |     | 146497202   |
| 27210-27210A                  | A     | ATTIC      | 4   | 1   | Job Reference (optional)  |
| 84 Components, Dunn, NC 28334 |       |            |     |     | 8.500 s Feb 23 2021 MiTek Industries, Inc. Wed Jun 9 15:25:33 2021 Page 2 |

#### 8.500 s Feb 23 2021 MiTek Industries, Inc. Wed Jun 9 15:25:33 2021 Page 2 ID:hSPrWMHNLCMfmXJlhBDjiGz8N\_g-NtA49OxB4Mopjg3ZhdW4dfr2qnCsm\_xrSyNsWDz7zl0

#### NOTES-

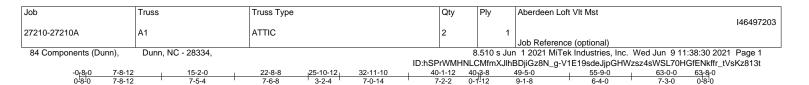
- 3) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 4x6 MT20 unless otherwise indicated.
- r) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Ceiling dead load (5.0 psf) on member(s). 35-37, 36-37; Wall dead load (5.0 psf) on member(s). 30-35, 21-36
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 28-30, 25-28, 23-25, 22-23, 21-22
- 11) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 2 and 119 lb uplift at joint 14.
- 13) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

16) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





9-1-8

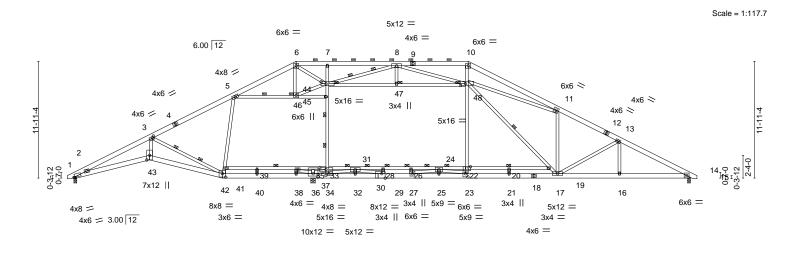




Plate Offsets (X,Y)-- [2:0-1-0,0-0-5], [3:0-2-4,0-2-0], [23:0-3-0,0-4-0], [30:0-4-4,Edge], [33:0-7-8,0-2-4], [34:0-3-8,0-2-0], [35:0-6-0,0-3-0], [42:0-4-0,0-4-4], [43:0-5-11,0-3-8],

|                   |         | [44:0-6-0,0-2-8], [48:0-6-                                       | 0,0-2-0]       | -,,,         | , .3.1,1 |          | ·       |             |               |                       | ,. ,,                   | ,              |
|-------------------|---------|--|----------------|--------------|----------|----------|---------|-------------|---------------|-----------------------|-------------------------|----------------|
| LOADING (ps       | f)      | SPACING-   | 2-0-0          | CSI.         |          | DEFL.    | in      | (loc)       | l/defl        | L/d                   | PLATES                  | GRIP           |
| TCLL 20.          | ,       | Plate Grip DOL   | 1.15           | тс           | 0.73     | Vert(LL) |         | 24-26       | >830          | 240                   | MT20                    | 197/144        |
| TCDL 10.          | 0       | Lumber DOL   | 1.15           | BC           | 0.98     | Vert(CT) | -1.09   | 24          | >425          | 180                   |                         |                |
| BCLL 0.           | 0 *     | Rep Stress Incr  | YES            | WB           | 0.97     | Horz(CT) | 0.34    | 14          | n/a           | n/a                   |                         |                |
| BCDL 10.          | 0       | Code IRC2015/T   | PI2014         | Matrix       | -MS      | Attic    | -0.24   | 22-33       | 703           | 360                   | Weight: 576 lb          | FT = 20%       |
| LUMBER-           |         |  |                |              | •        | BRACING  | -       |             |               |                       |                         |                |
| TOP CHORD         | 2x6 SF  | No.2 *Except*  |                |              |          | TOP CHO  | RD      | Structu     | Iral wood     | sheathing di          | rectly applied or 2-10- | 14 oc purlins, |
| 10-12: 2x6 SP DSS |         |  |                | except       |          |          |         |             |               |                       |                         |                |
| BOT CHORD         | 2x6 SF  | No.2 *Except*  |                |              |          |          |         | 2-0-0 0     | oc purlins    | (2-4-4 max.)          | : 6-10.                 |                |
|                   | 30-42,  | 0-42,18-30: 2x6 SP DSS, 22-33,19-22: 2x4 SP No.2 or 2x4 SPF No.2 |                |              | BOT CHO  | RD       | Rigid o | eiling dire | ectly applied | or 10-0-0 oc bracing. | Except:                 |                |
|                   | 33-41:  | 33-41: 2x4 SP No.1   |                |              |          |          | 2-6-0 0 | oc bracing  | 1: 22-33      |                       |                         |                |
| NEBS              | 2x4 SF  | No.3 *Except*  |                |              |          |          |         | 6-0-0 0     | oc bracing    | j: 19-22              |                         |                |
|                   | 7-34,10 | 0-23: 2x4 SP DSS   |                |              |          |          |         | 10-0-0      | oc bracir     | ng: 33-41             |                         |                |
|                   | 44-48,  | 5-42,8-44,8-48,17-48: 2x   | 4 SP No.2 or 2 | x4 SPF No.2  |          | WEBS     |         | 1 Row       | at midpt      |                       | 5-46, 17-48             |                |
|                   |         |  |                |              |          |          |         | 2 Row       | s at 1/3 p    | ts 3                  | 3-42, 33-44, 8-44       |                |
|                   |         |  |                |              |          | JOINTS   |         | 1 Brac      | e at Jt(s):   | 44, 46, 47, 4         | 48                      |                |
| REACTIONS.        | (siz    | e) 2=0-3-8, 36=0-5-8, 1  | 4=0-3-8 (req.  | 0-3-9)       |          |          |         |             |               |                       |                         |                |
|                   | Max H   | lorz 2=-194(LC 17)   |                |              |          |          |         |             |               |                       |                         |                |
|                   | Max U   | plift 14=-32(LC 13)  |                |              |          |          |         |             |               |                       |                         |                |
|                   | Max G   | rav 2=1490(LC 2), 36=2   | 864(LC 2), 14= | =2257(LC 27) |          |          |         |             |               |                       |                         |                |

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-4937/224, 3-5=-2150/58, 5-6=-1015/493, 6-7=-1167/2100, 7-8=-1195/2157, 8-10=-5992/0, 10-11=-6327/0, 11-13=-4015/126, 13-14=-4387/194 BOT CHORD 2-43=-106/4476, 42-43=-109/4327, 40-42=0/1473, 38-40=0/1473, 36-38=0/1473, 34-36=-134/3013, 32-34=0/4564, 29-32=0/4564, 27-29=0/5371, 25-27=0/4298, 23-25=0/4298, 21-23=0/2356, 17-21=0/2356, 16-17=-66/3854, 14-16=-66/3854, 31-33=-966/474, 28-31=-3513/0, 26-28=-3513/0, 24-26=-3513/0, 22-24=-507/5, 39-41=0/431, 37-39=0/431, 35-37=0/2043, 33-35=0/2043, 20-22=-262/5, 19-20=-262/5 WEBS 3-43=0/2233, 3-42=-2639/382, 33-34=-309/475, 33-45=-2037/536, 44-45=-1955/561, 7-44=-455/167, 22-23=0/905, 22-48=0/1213, 10-48=0/2467, 11-17=-934/197, 13-17=-448/220, 44-47=-338/1730, 47-48=-338/1730, 41-42=0/549, 5-41=0/526, 5-46=-1775/518, 45-46=-16/523, 6-46=-106/842, 44-46=-2074/365, 8-44=-4017/541, 8-48=0/2626, 6-44=-1986/757, 37-38=0/611, 33-36=-3614/253, 36-37=-1880/0, 28-29=-312/0, 26-27=-301/0, 31-34=-2250/0, 29-31=0/1168, 23-24=-2047/0, 24-27=0/1313, 19-48=-218/2029, 17-19=-236/1815, 11-48=0/2088

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



#### Continued on page 2

ᄊ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a duss system: plantieter and property incorporate dust using in the overlain of the optimization opt



| [ | Job                   | Truss             | Truss Type | Qty | Ply        | Aberdeen Loft VIt Mst  |
|---|-----------------------|-------------------|------------|-----|------------|--|
|   |                       |                   |            |     |            | 146497203  |
|   | 27210-27210A          | A1                | ATTIC      | 2   | 1          |  |
|   |                       |                   |            |     |            | Job Reference (optional)                                     |
|   | 84 Components (Dunn), | Dunn, NC - 28334, |            | 8   | .510 s Jur | 1 2021 MiTek Industries, Inc. Wed Jun 9 11:38:37 2021 Page 2 |

8.510 s Jun 1 2021 MiTek Industries, Inc. Wed Jun 9 11:38:37 2021 Page 2 ID:hSPrWMHNLCMfmXJIhBDjiGz8N\_g-oN9hdFi1fsiHdb?Ct2U9EpvCW42sWuOhSa3McQz813m

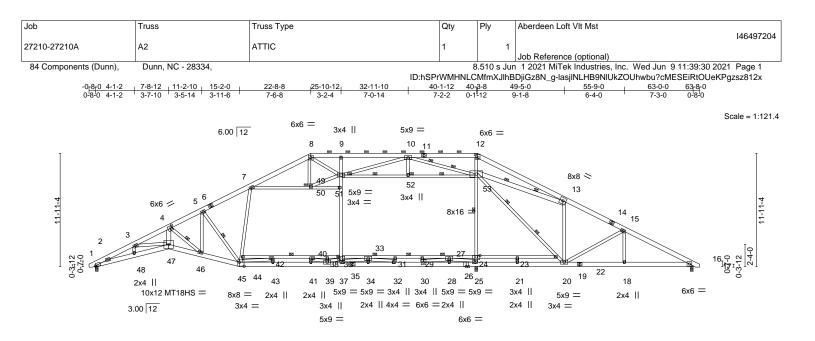
#### NOTES-

- 3) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (5.0 psf) on member(s). 44-47, 47-48, 5-46, 45-46; Wall dead load (5.0 psf) on member(s). 33-45, 44-45, 22-48
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 31-33, 28-31, 26-28, 24-26, 22-24
- 10) WARNING: Required bearing size at joint(s) 14 greater than input bearing size.
- 11) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 36, and 14. This connection is for uplift only and does not consider lateral forces.
- 13) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

15) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **MSIVTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





 4-1-2
 7-8-12
 11-2-10
 15-2-0
 18-8-8
 22-8-8
 24-5-4
 28-8-15
 31-7-2
 34-5-6
 37-3-9
 40-1-12
 44-5-10
 49-5-0
 55-9-0
 63-0-0

 4-1-2
 3-7-10
 3-5-14
 3-11-6
 3-6-8
 40-0
 1-8-12
 2-10-3
 2-10-3
 2-10-3
 2-10-3
 2-10-3
 42-2
 4-11-6
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|--|--|--|---|---|---|
| LOADING(psf)TCLL20.0TCDL10.0BCLL0.0BCDL10.0  | SPACING-         1-7-3           Plate Grip DOL         1.15           Lumber DOL         1.15           *         Rep Stress Incr         YES           Code         IRC2015/TPI2014  | <b>CSI.</b><br>TC 0.34<br>BC 0.63<br>WB 0.91<br>Matrix-MS  | Vert(LL) -0.43<br>Vert(CT) -1.06<br>Horz(CT) 0.32                       | 45 >453 180   | PLATES         GRIP           MT20         197/144           MT18HS         244/190           Weight: 606 lb         FT = 20% |
| BOT CHORD 2  | x6 SP DSS<br>x6 SP DSS *Except*<br>4 26 26 44 22 24: 244 SP No 2 or 244 SPE N  | 2.2  | BRACING-<br>TOP CHORD   | Structural wood sheathing<br>except<br>2-0-0 oc purlins (5-1-9 max  | directly applied or 2-10-8 oc purlins,  |
| 24-36,36-44,22-24: 2x4 SP No.2 or 2x4 SPF No.2<br>WEBS 2x4 SP No.3 *Except*<br>4-47,13-53: 2x6 SP No.2<br>9-37,12-25,49-53,7-50,20-53: 2x4 SP No.2 or 2x4 SPF No.2 |  |  | BOT CHORD   |   | d or 10-0-0 oc bracing. Except:   |
|  |  |  | WEBS  | 1 Row at midpt<br>2 Rows at 1/3 pts<br>1 Brace at Jt(s): 49, 51, 52 | 24-53, 49-52, 52-53, 4-46, 5-45<br>10-53, 20-53, 13-53  |
| Ν  | (size) 2=0-3-8, 25=0-3-8, 16=0-3-8<br>Max Horz 2=-155(LC 13)<br>Max Uplift 2=-32(LC 12)<br>Max Grav 2=2121(LC 24), 25=1600(LC 27), 1   | 6=1802(LC 1)   |   |   |   |
| TOP CHORD  | Max. Comp./Max. Ten All forces 250 (lb) or<br>2-3=-6970/540, 3-4=-7425/499, 4-5=-4691/3<br>8-9=-2720/0, 9-10=-2701/0, 10-12=0/1778, 1<br>15-16=-3466/412   | 36, 5-7=-3659/277, 7-8=-24   |   |   |   |
| BOT CHORD  | 2-48=-438/6281, 47-48=-445/6353, 46-47=-3<br>41-43=0/2933, 39-41=0/2933, 37-39=-85/200<br>30-32=0/4096, 28-30=0/2924, 25-28=0/2924<br>18-20=-281/3034, 16-18=-281/3034, 33-36=<br>27-29=-1531/0, 24-27=0/533, 40-42=-200/31  | 5, 34-37=0/3200, 32-34=0<br>21-25=0/2825, 20-21=0/2<br>/1020, 31-33=-1531/0, 29-   | /3200,<br>825,<br>31=-1531/0,   |   |   |
| WEBS   | 21-253-155/3227, 36-37-0/745, 36-50-0/1078<br>24-53=-848/249, 12-53=-1177/179, 13-20=0/<br>52-53=-1827/316, 44-45=-40/551, 7-44=-31/<br>49-51=-1029/341, 10-49=0/1455, 10-53=-326<br>39-40=-956/0, 27-28=-415/0, 33-37=-1456/0<br>27-30-0/1244, 22-53=-827/56, 20-22=-691/4<br>5-46=-108/1628, 3-47=0/531, 42-45=-195/35 | , 49-50=0/1088, 24-25=-9;<br>742, 15-20=-413/163, 49-5<br>117, 7-51=-1179/389, 8-51<br>8/0, 8-49=0/942, 40-41=0/<br>32-33=0/1061, 25-27=-77<br>3, 13-53=-4395/452, 4-46= | 36/130,<br>;2=-1825/316,<br>=-101/450,<br>/254, 36-39=0/594,<br>94/313, |   | SEAL  |

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Plate Offsets (X,Y)-- [2:0-0-12,Edge], [4:0-2-4,0-3-0], [12:0-3-0,0-3-12], [36:0-2-12,0-2-8], [45:0-6-0,0-5-0]

#### Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





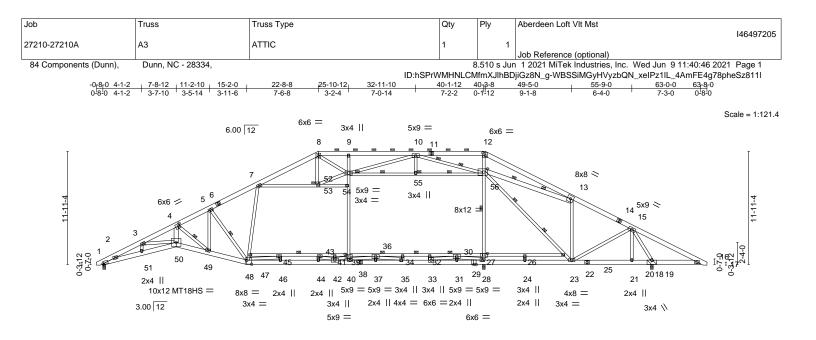
| Job                   | Truss             | Truss Type | Qty | Ply         | Aberdeen Loft VIt Mst  |
|-----------------------|-------------------|------------|-----|-------------|--|
|                       |                   |            |     |             | 146497204  |
| 27210-27210A          | A2                | ATTIC      | 1   | 1           |  |
|                       |                   |            |     |             | Job Reference (optional)                                     |
| 84 Components (Dunn), | Dunn, NC - 28334, |            |     | 3.510 s Jur | 1 2021 MiTek Industries, Inc. Wed Jun 9 11:39:31 2021 Page 2 |

#### NOTES-

- ID:hSPrWMHNLCMfmXJIhBDjiGz8N\_g-DmQ5ziMvxSVc5u8a2OSqQC8X\_saxAKedtz8DVIz812w
- 3) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 4x6 MT20 unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Ceiling dead load (5.0 psf) on member(s). 49-52, 52-53, 7-51, 50-51; Wall dead load (5.0 psf) on member(s).36-50, 49-50, 24-53
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 33-36, 31-33, 29-31, 27-29, 24-27 11) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Dearing at joint(s) 2 considers parallel to grain value using ANS/TET Fargle to grain formula. Building designer should verify capacity of bearing surface.
   One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 16. This connection is for uplift only and does not consider lateral forces.
- 13) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 15) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





25-10-12 40-3-8 40-1-12 44-5-10 63-0-0 5-0-0 4-1-2 7-8-12 11-2-10 15-2-0 18-8-8 4-1-2 3-7-10 3-5-14 3-11-6 3-6-8 22-8-8 24-5-4 28-8-15 31-7-2 34-5-6 37-3-9 -8-12 2-10-3 2-10-3 2-10-4 2-10-3 0-1-12 1-5-8

Plate Offsets (X,Y)--[2:0-0-12,Edge], [4:0-2-4,0-3-0], [12:0-3-0,0-3-8], [39:0-2-12,0-2-8], [48:0-6-0,0-5-0], [56:0-4-12,0-4-0] LOADING (psf) SPACING-1-7-3 CSI DEFL in (loc) l/def L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 тс 0.35 Vert(LL) -0.41 48 >999 240 197/144 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.61 Vert(CT) -1.01 48 >477 180 MT18HS 244/190 BCLL 0.0 Rep Stress Incr YES WB 0.98 0.30 19 Horz(CT) n/a n/a Code IRC2015/TPI2014 BCDL 10.0 Matrix-MS -0.15 27-39 1103 360 FT = 20% Attic Weight: 614 lb LUMBER-BRACING-2x6 SP DSS TOP CHORD TOP CHORD Structural wood sheathing directly applied or 2-10-3 oc purlins, BOT CHORD 2x6 SP DSS \*Except\* except 27-39,39-47,25-27: 2x4 SP No.2 or 2x4 SPF No.2 2-0-0 oc purlins (5-0-14 max.): 8-12. WEBS BOT CHORD 2x4 SP No.3 \*Except\* Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 4-50,13-56: 2x6 SP No.2 6-0-0 oc bracing: 16-19. 9-40,12-28,52-56,7-53: 2x4 SP No.2 or 2x4 SPF No.2 4-6-0 oc bracing: 27-39 23-56: 2x4 SP DSS 6-0-0 oc bracing: 39-47 WEBS 27-56, 52-55, 55-56, 4-49, 5-48 1 Row at midpt 10-56, 23-56, 13-56 2 Rows at 1/3 pts JOINTS 1 Brace at Jt(s): 52, 54, 55 REACTIONS. (size) 2=0-3-8, 28=0-3-8, 19=(0-3-8 + bearing block) (reg. 0-3-12) Max Horz 2=-155(LC 17) Max Uplift 2=-31(LC 12) Max Grav 2=2144(LC 24), 28=1094(LC 27), 19=2374(LC 1) FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-7049/556, 3-4=-7515/518, 4-5=-4749/348, 5-7=-3712/287, 7-8=-2476/0, 8-9=-2771/0, 9-10=-2763/0, 10-12=0/1557, 12-13=0/1606, 13-15=-2402/284, 15-16=-115/395 BOT CHORD 2-51=-453/6352, 50-51=-459/6424, 49-50=-363/6908, 48-49=-154/4416, 46-48=0/2945, 44-46=0/2945, 42-44=0/2945, 40-42=-89/2019, 37-40=0/3199, 35-37=0/3199, 33-35=0/4116, 31-33=0/2958, 28-31=0/2958, 24-28=0/2786, 23-24=0/2786, 21-23=-15/976, 19-21=-15/976, 16-19=-289/170, 36-39=0/1081, 34-36=-1500/14, and a state of the 32-34=-1500/14, 30-32=-1500/14, 27-30=0/608, 43-45=-166/355, 41-43=-28/1062, 39-41=-28/1062, 26-27=0/326, 25-26=0/326 WEBS 4-50=-173/3266, 39-40=0/747, 39-53=0/1081, 52-53=0/1091, 27-28=-443/208, 27-56=-354/363, 12-56=-1062/161, 13-23=0/616, 15-23=-71/1303, 52-55=-1752/308, 55-56=-1753/308, 47-48=-39/548, 7-47=-30/611, 7-54=-1197/391, 8-54=-103/464, 52-54=-1069/346, 10-52=0/1401, 10-56=-3191/0, 8-52=0/967, 43-44=0/253, 39-42=0/598, 42-43=-950/0, 30-31=-390/0, 36-40=-1455/0, 35-36=0/1079, 28-30=-839/248, 30-33=0/1232, 25-56=-1609/0, 23-25=-1402/0, 13-56=-3578/340, 4-49=-3239/274, 044925 5-49=-111/1638, 3-50=0/543, 45-48=-186/391, 5-48=-1854/239, 15-19=-2422/319 NOTES-1) 2x6 SP DSS bearing block 12" long at jt. 19 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity= 425psi. Unbalanced roof live loads have been considered for this design. 3) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope)

gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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Continued on page 2
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932

| Job                   | Truss             | Truss Type | Qty | Ply         | Aberdeen Loft VIt Mst  |
|-----------------------|-------------------|------------|-----|-------------|--|
|                       |                   |            |     |             | 146497205  |
| 27210-27210A          | A3                | ATTIC      | 1   | 1           |  |
|                       |                   |            |     |             | Job Reference (optional)                                     |
| 84 Components (Dunn), | Dunn, NC - 28334, |            |     | 3.510 s Jur | 1 2021 MiTek Industries, Inc. Wed Jun 9 11:40:46 2021 Page 2 |

8.510 s Jun 1 2021 MiTek Industries, Inc. Wed Jun 9 11:40:46 2021 Page 2 ID:hSPrWMHNLCMfmXJlhBDjiGz8N\_g-WBSSiMGyHVyzbQN\_xeIPz1IL\_4AmFE4g78pheSz8111

#### NOTES-

4) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

5) Provide adequate drainage to prevent water ponding.

6) All plates are MT20 plates unless otherwise indicated.

7) All plates are 4x6 MT20 unless otherwise indicated.

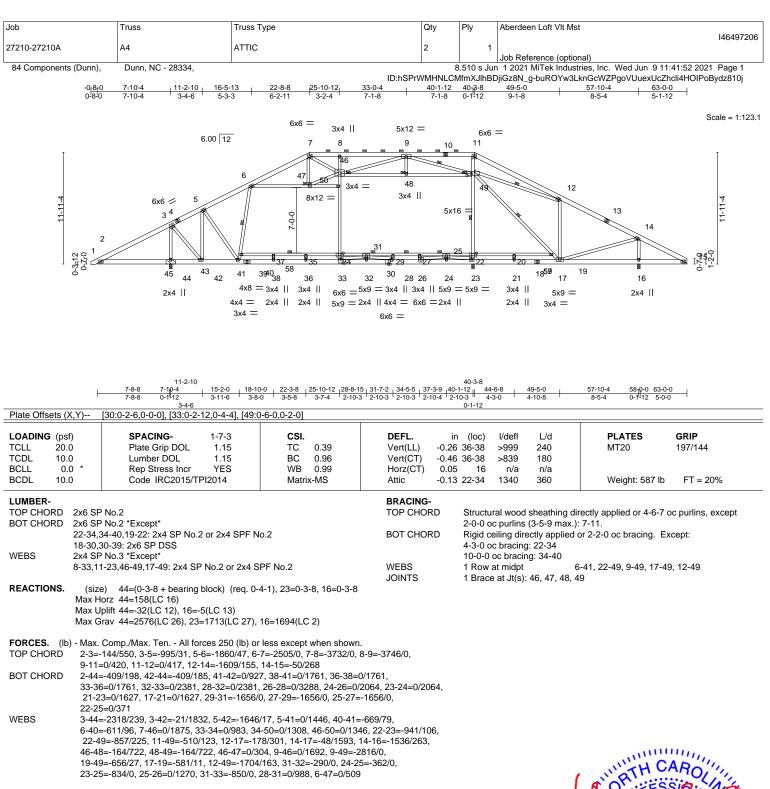
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Ceiling dead load (5.0 psf) on member(s). 52-55, 55-56, 7-54, 53-54; Wall dead load (5.0 psf) on member(s). 39-53, 52-53, 27-56
- 11) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 36-39, 34-36, 32-34, 30-32, 27-30
- 12) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 14) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

16) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **MSIVTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





#### NOTES-

1) 2x6 SP No.2 bearing block 12" long at jt. 44 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity= 425psi.

2) Unbalanced roof live loads have been considered for this design.

3) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

4) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

5) Provide adequate drainage to prevent water ponding.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





| Job                   | Truss             | Truss Type | Qty | Ply        | Aberdeen Loft VIt Mst  |
|-----------------------|-------------------|------------|-----|------------|--|
|                       |                   |            |     |            | 146497206  |
| 27210-27210A          | A4                | ATTIC      | 2   | 1          |  |
|                       |                   |            |     |            | Job Reference (optional)                                     |
| 84 Components (Dunn), | Dunn, NC - 28334, |            | 8   | .510 s Jur | 1 2021 MiTek Industries, Inc. Wed Jun 9 11:41:58 2021 Page 2 |

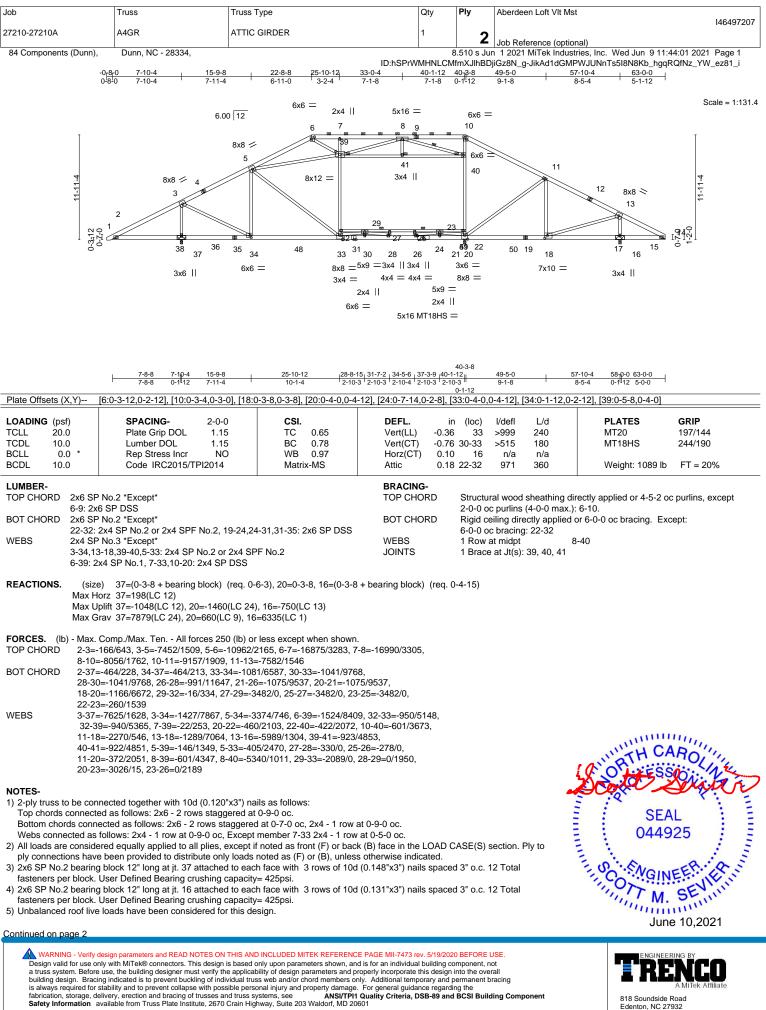
ID:hSPrWMHNLCMfmXJlhBDjiGz8N\_g-P2ogpz86Kd0mEUtq9maluCke26f96omGgLFVAHz810d

#### NOTES-

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 9) Ceiling dead load (5.0psf) on member(s). 46-48, 48-49, 6-47, 47-50; Wall dead load (5.0psf) on member(s).34-50, 46-50, 22-49
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 31-34, 29-31, 27-29, 25-27, 22-25
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 44 and 16. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Edenton, NC 27932

| Job                   | Truss             | Truss Type   | Qty | Ply         | Aberdeen Loft VIt Mst  |
|-----------------------|-------------------|--------------|-----|-------------|--|
|                       |                   |              |     |             | 146497207  |
| 27210-27210A          | A4GR              | ATTIC GIRDER | 1   | 2           |  |
|                       |                   |              |     | <b>_</b>    | Job Reference (optional)                                     |
| 84 Components (Dunn), | Dunn, NC - 28334, |              | 8   | 3.510 s Jur | 1 2021 MiTek Industries, Inc. Wed Jun 9 11:44:02 2021 Page 2 |

8.510 s Jun 1 2021 MiTek Industries, Inc. Wed Jun 9 11:44:02 2021 Page 2 ID:hSPrWMHNLCMfmXJlhBDjiGz8N\_g-nvlYrNeu7jeA6XMfQppNvMtmk503AtvWBel3W5z81\_h

#### NOTES-

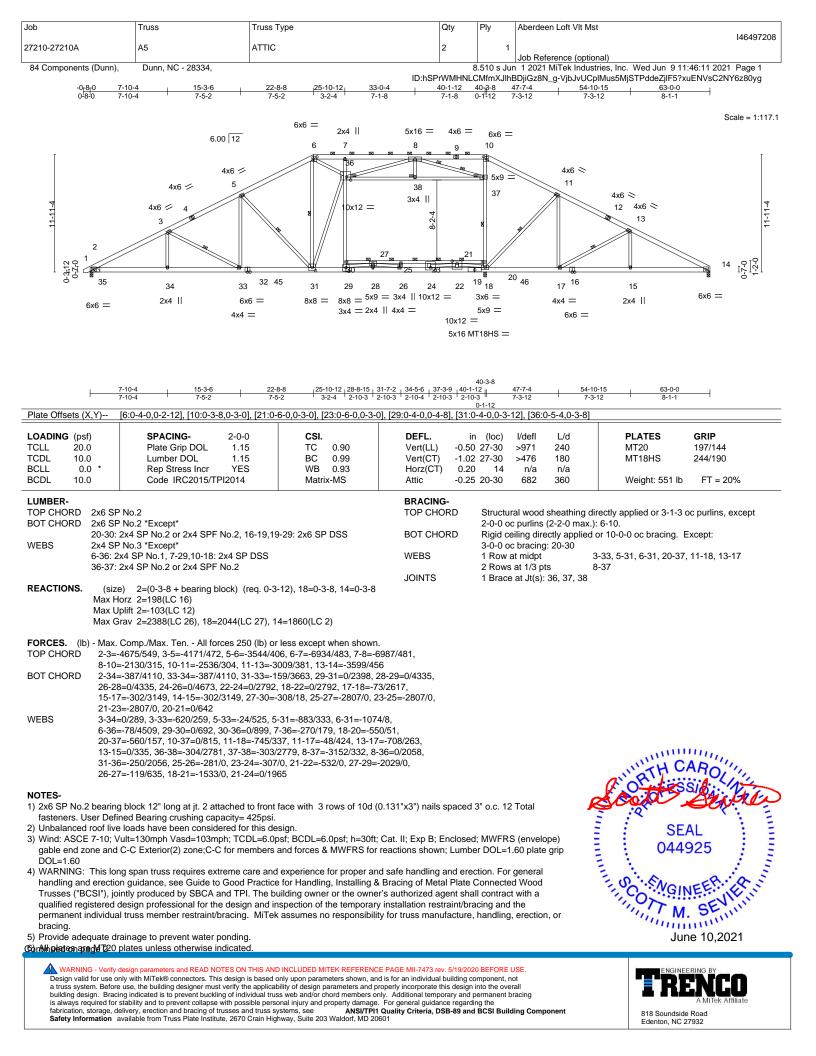
- 6) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 8) Provide adequate drainage to prevent water ponding.
- 9) All plates are MT20 plates unless otherwise indicated.
- 10) All plates are 4x6 MT20 unless otherwise indicated.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 13) Ceiling dead load (5.0 psf) on member(s). 39-41, 40-41; Wall dead load (5.0 psf) on member(s). 32-39, 22-40
- 14) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 29-32, 27-29, 25-27, 23-25, 22-23
- 15) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 750 lb uplift at joint 16.
- 16) LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 37 and 20. This connection is for uplift only and does not consider lateral forces.
- 17) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 18) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 19) This truss has large uplift reaction(s) from gravity load case(s). Proper connection is required to secure truss against upward movement at the bearings. Building designer must provide for uplift reactions indicated.
- 20) LGT2 Hurricane ties must have two studs in line below the truss.
- 21) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 6100 lb down and 1423 lb up at 25-10-12, and 1200 lb down and 280 lb up at 36-7-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 22) Attic room checked for L/360 deflection.

#### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
- Vert: 1-6=-60, 6-10=-60, 10-14=-60, 42-45=-20, 22-32=-30, 39-40=-10
  - Drag: 32-39=-10, 22-40=-10
- Concentrated Loads (lb)

Vert: 33=-6100(F) 49=-1200(F)





| Job                   | Truss             | Truss Type | Qty | Ply        | Aberdeen Loft VIt Mst  |
|-----------------------|-------------------|------------|-----|------------|--|
|                       |                   |            |     |            | 146497208  |
| 27210-27210A          | A5                | ATTIC      | 2   | 1          |  |
|                       |                   |            |     |            | Job Reference (optional)                                     |
| 84 Components (Dunn), | Dunn, NC - 28334, |            | 8   | .510 s Jur | 1 2021 MiTek Industries, Inc. Wed Jun 9 11:46:26 2021 Page 2 |

ID:hSPrWMHNLCMfmXJlhBDjiGz8N\_g-Zc?\_3cNDDznkOgMKs2O8ljgJS87Sv04jJ2Agakz80yR

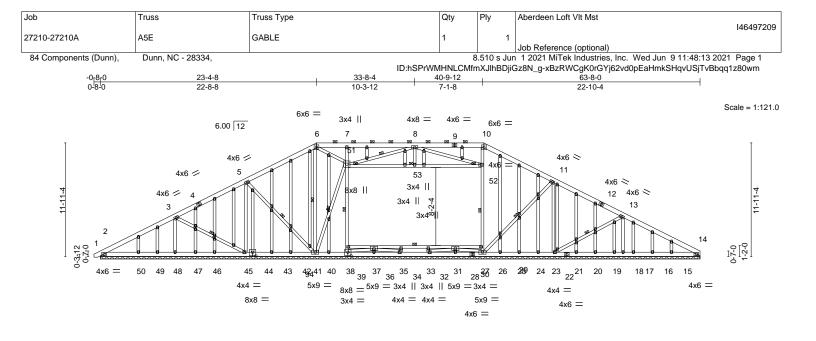
#### NOTES-

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
  9) Ceiling dead load (5.0 psf) on member(s). 36-38, 37-38; Wall dead load (5.0psf) on member(s).30-36, 20-37
  10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 27-30, 25-27, 23-25, 21-23, 20-21

- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 18, and 14. This connection is for uplift only and does not consider lateral forces.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





| 0-8-0<br>0-8-0  | 23-4-8<br>22-8-8  | 6-0-7 2                               | 2-3-2 35-1-6 37-11-9<br>2-10-3 2-10-4 2-10-3                | 63-8-<br>25-8-        |  |  |  |
|---|---|---------------------------------------|---|-----------------------|--|--|--|
| Plate Offsets (X,Y)   | [38:0-4-0,0-4-8], [44:0-4-0,0-4-8], [56:0-  | <u>1-15,0-1-0], [58:0-1-15,0-</u>     | 1-0]  |                       |  |  |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0  | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES   | CSI.<br>TC 0.33<br>BC 0.19<br>WB 0.79 | DEFL. ir<br>Vert(LL) 0.00<br>Vert(CT) 0.00<br>Horz(CT) 0.01 | 1 n/r 120<br>1 n/r 90 | PLATES         GRIP           MT20         197/144 |  |  |
| BCDL 10.0   | Code IRC2015/TPI2014  | Matrix-S                              |   |                       | Weight: 736 lb FT = 20%                            |  |  |
| LUMBER-<br>TOP CHORD         2x6 SP No.2         BRACING-<br>EXCOLUTION         BRACING-<br>TOP CHORD         TOP CHORD         Structural wood sheathing directly applied or 6-0-0 oc purlins, except<br>2-0-0 oc purlins (6-0-0 max.): 6-10.           BOT CHORD         2x4 SP No.2 or 2x4 SPF No.2 or 2x4 SPF No.2<br>7-38,10-27,51-52: 2x4 SP No.2 or 2x4 SPF No.2         BOT CHORD         Rigid ceiling directly applied or 10-0-0 oc bracing, Except:<br>6-0-0 oc bracing: 29-39           OTHERS         2x4 SP No.3         WEBS         1 Row at midpt         3-45, 5-41, 6-41, 39-51, 29-52, 11-27,<br>13-23, 8-51, 41-51   |   |                                       |   |                       |  |  |  |
| (Ib) - Max H<br>Max U   | 13-23, 8-51, 41-51<br>JOINTS 1 Brace at Jt(s): 51, 52, 53   |                                       |   |                       |  |  |  |
| TOP CHORD         11-13           BOT CHORD         40-43           WEBS         3-483           27-29         27-29  | BOT CHORD 40-41=-44/258, 38-40=-44/258  |                                       |   |                       |  |  |  |
| <ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads have been considered for this design.</li> <li>2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.</li> <li>4) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing &amp; Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.</li> </ul> |   |                                       |   |                       |  |  |  |
| <ul><li>qualified registered permanent individua<br/>bracing.</li><li>5) Provide adequate di<br/>6) All plates are 2x4 M</li></ul>  | design professional for the design and ir<br>al truss member restraint/bracing. MiTek<br>rainage to prevent water ponding.<br>T20 unless otherwise indicated. | spection of the temporary             | / installation restraint/bra                                | icing and the         | June 10,2021                                       |  |  |
| WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.       Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601       ANSITPH Quality Criteria, DSB-89 and BCSI Building Component       Bit Soundside Road Edenton, NC 27932  |   |                                       |   |                       |  |  |  |

| Job                   | Truss             | Truss Type | Qty      | Ply         | Aberdeen Loft VIt Mst  |
|-----------------------|-------------------|------------|----------|-------------|--|
|                       |                   |            |          |             | 146497209  |
| 27210-27210A          | A5E               | GABLE      | 1        | 1           |  |
|                       |                   |            |          |             | Job Reference (optional)                                     |
| 84 Components (Dunn), | Dunn, NC - 28334, |            | 6        | 8.510 s Jur | 1 2021 MiTek Industries, Inc. Wed Jun 9 11:48:14 2021 Page 2 |
|                       |                   | ID:hSPrWM  | /HNLCMfr | nXJIhBDjiC  | Gz8N_g-PNWpkYhzn8OPLGc5AkKT7VJvCgA8Dvzc8rLOMUz80wl           |

#### NOTES-

8) Gable studs spaced at 2-0-0 oc.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 11) Ceiling dead load (5.0 psf) on member(s). 51-53, 52-53; Wall dead load (5.0 psf) on member(s).39-51, 29-52
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 38, 27, 50, 49, 16, 15 except (jt=lb) 48=181, 45=151,
- 41=145, 23=163, 18=177. 13) n/a

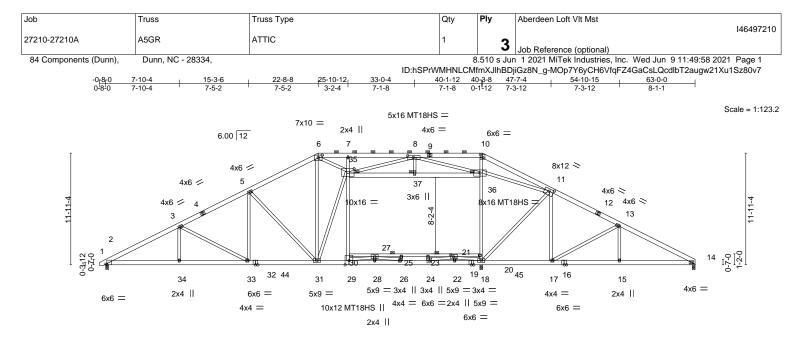
14) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

16) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





40-3-8 7-10-4 7-10-4 54-10-15 7-3-12 63-0-0 8-1-1 15-3-6 7-5-2 22-8-8 7-5-2 47-7-4 7-3-12

| Plate Offsets (X,Y) [2:0-1-6,Edge], [6:0-5-0,0-3-0], [8:0-7-4,0-2-4], [10:0-3-0,0-3-8], [11:0-6-0,0-3-0], [21:0-3-8,0-2-8], [35:0-7-12,0-5-0] |
|---|
|---|

| TCLL     20.0     Plate Grip DOL     1.15     TC     0.60     Vert(LL)     -0.65     29     >746     240     MT20     197/144       TCDL     10.0     Lumber DOL     1.15     BC     0.99     Vert(CT)     -1.00     29     >484     180     MT18HS     197/144       BCLL     0.0 *     Rep Stress Incr     YES     WB     0.99     Horz(CT)     0.23     14     n/a     n/a       BCDL     10.0     Code IRC2015/TPI2014     Matrix-MS     Attic     -0.32     20-30     535     360     Weight: 1712 lb     FT = 20%  | Plate Offsets (X,   | Y) [2:0-1-6,Edge], [6:0-5-0,0-3-0], [8:0-7-4  | ,0-2-4], [10:0-3-0,0-3-8], [11  |  | 8,0-2-8],            | 35:0-7-12                  | ,0-5-0]               |                |                    |
|--|---|---|---|--|----------------------|----------------------------|-----------------------|----------------|--------------------|
| TOP CHORD       2x6 SP No.2 "Except"       TOP CHORD       Structural wood sheathing directly applied or 6-0-0 oc purlins, except         2x0-00 purlins (6-3-11) max; b-10.       2x0-00 purlins (6-3-11) max; b-10.       2x0-00 purlins (6-3-11) max; b-10.         BOT CHORD       2x6 SP DSS "Except"       BOT CHORD       Structural wood sheathing directly applied or 10-0-0 oc bracing; 2x3.         WEBS       2x3 ZH x-16; xx6 SP No.2 * 20:02: 2x4 SP No.2 or 2x4 SP DSS       JOINTS       1 Brace at Jt(s): 35, 36, 37         REACTIONS       (size) 2=(0-3+LCT3-SDS2.5 Simpson Strong-Tie) (req. 0-4-0), 18=0-3.8, 14=0-3.8       h=0-3.8, 14=0-3.8         Wark log: 2=705(LC 20), 18=-3476(LC 2), 14=-423(LC 13)       Max Grav 2=7705(LC 20), 18=-3476(LC 2), 14=-5022(LC 2)         FORCES.       (b) - Max: Comp.Max: Ten All forces 250 (b) or less except when shown.       TOP CHORD       2:3=-1614/22208, 3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1687/1432, 3:3:4=-1369/022, 8:36=-1187/16212, 2:3:3=-1367/12011, 2:2:3:3=-1369/102, 2:2:-1:500(17271, 3:3=-3394)(1:1:3=-1369/020, 2:6:3=-1209/236, 1:1:2:-1:209/236, 1:1:1:2:-2:2:-1:2:0:1:1:1:2:2:-2:-1:2:0:1:1:1:2:2:-2:-1:  | TCDL 10.0<br>BCLL 0.0   | Plate Grip DOL         1.15           Lumber DOL         1.15           *         Rep Stress Incr         YES   | TC 0.60<br>BC 0.99<br>WB 0.99   | Vert(LL) -0.65<br>Vert(CT) -1.00<br>Horz(CT) 0.23                          | 5 29<br>0 29<br>3 14 | >746<br>>484<br>n/a        | 240<br>180<br>n/a     | MT20<br>MT18HS | 197/144<br>197/144 |
| BOT CHORD       2x8 SP DSS *Except*       BOT CHORD       Rigid caling diredity applied or 10-0-0 oc bracing; 2x4         WEBS       2x4 SP No.3 *Except*       800 C 20:30: 2x4 SP No.2       2x-0 SP No.2         Setting 1, 729, 10-18,8-36,8-35,11-36: 2x4 SP DSS       JOINTS       1 Brace at J(s): 35, 36, 37         REACTIONS       (size x4 SP No.2       Size x4 SP No.2       800 C 20: 2x4 SP No.2         REACTIONS       (size) 2=(0-3+LCT3-SDS2.5 Simpson Strong-Tie) (req. 0-4-0), 18=-0-3-6, 14=0-3-8  | TOP CHORD 2   | •   | II  |  |                      |                            |                       |                | purlins, except    |
| 6-35: 224 SP No.1, 7-29, 10-18, 8-36, 8-35, 11-36: 224 SP DSS       JOINTS       1 Brace at Jt(s): 35, 36, 37         REACTIONS.       (size) 2=(0-3+ LCT3-SDS2.5 Simpson Strong-Tie) (req. 0-4-0), 18=0-3-8, 14=0-3-8         Max Horz 2=198(LC 16)       Max Grav 2=7705(LC 20), 18=3476(LC 2), 14=5022(LC 2)         FORCES.       (b) - Max. Comp. AMax. Ten All forces 250 (b) or less except when shown.         TOP CHORD       2-34=1612(2028, 35=-18871/12167, 56=-18405/2116, 67=-27338/03119, 74=27608/3447, 8-10=763/6704, 10-11=872/6835, 11-13=9339/1414, 13-14=-103/31475         BOT CHORD       2-34=-1612(32028, 35=-187405/216, 67=-72338/0319, 17-3=-938/9862, 15-17=-1209/9236, 14-15=-1209/9236, 14-15=-1209/9236, 14-15=-1209/9236, 22-4=-560/1033, 17-18=-938/9862, 15-17=-1209/9236, 14-15=-1209/9236, 22-4=-560/1033, 17-18=-938/9862, 15-17=-1209/9236, 14-15=-1209/9236, 23-34=-01047/1002, 30-35=-1014/10118, 7-35=-44/764, 18-20=02/21=-57/917         WEBS       2-33=-4300/728, 0-32=7=-32640, 22-35=-32640, 21-2=-57/917         WEBS       2-33=-4300/728, 0-33=-1044/0101, 7-35=-44/764, 18-20=02/21=-57/917         .2-33=-4300/728, 0-32=7=-32640/22, 3-37=-1364/202, 3-36=-1807/22/72, 3-35=-1910/17271, 3-35=-418/291, 72-2=-1360/30, 21-2=-710, 22-4=-3803, 0, 1-2=-710/221         .2-38=-1007/98, 1, 2-2==-382/625, 18-21=-682/753, 21-24=-139/4464, 11-36=-18614/2477         NOTES-       1) 3-pb/ trus to be connected to sfollows: 2A - 1 row staggered at 0-9-0 oc.         Reta considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to pro-conscons have been considered for thi   | BOT CHORD 2<br>2  | 2x6 SP DSS *Except*<br>2-32,14-16: 2x6 SP No.2, 20-30: 2x4 SP No.2  | or 2x4 SPF No.2   | BOT CHORD  | Rigid c<br>2-2-0 c   | eiling direo<br>c bracing: | ctly applied of 2-34. | /              | kcept:             |
| Max Horz 2-198(LC 16)       Max Uplit 2727(LC 12), 14423(LC 13)         Max Grav 2-77705(LC 20), 18=3476(LC 2), 14=5022(LC 2)         FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3-16142/2208, 3-5-15871/2167, 5-6-15405(2116, 6-7=2738)6/3419,         -78-27608(1432), 33-34-1865/14432, 31-331678/14337, 29-31=-1315/12911,         15-24-56108(1432), 33-34-1865/14432, 31-331678/14337, 29-31=-1315/12911,         15-22-5611093, 17-18-9908/962, 1-157=-1209/9236,         27-30-2170/280, 25-27=32580, 23-25-32580, 20-2157917         WEBS       -3-33=-4302/13, 5-330148/19, 6-311476(149, 6-351480/126, 2-361480/126, 2-361487/22, 2-35-1910/17271,         31-35-418/2981, 27-28=-110/480, 25-26=2710, 2-36-280, 21-22=-1582/150,         27-29-1877/0, 26-27-882/525, 18-21=682/753, 21-24=-380/0, 21-22=-1582/150,         27-29-1877/0, 26-27-882/525, 18-21=682/753, 21-24=-380/0, 21-22=-1582/150,         27-29-1877/0, 26-27-882/525, 18-21=682/753, 21-24=-380/0, 21-22=-1582/150,         27-29-1877/0, 26-27-882/525, 18-21=602/753, 21-24=-380/0, 21-22=-1582/150,         27-39-1877/0, 26-27-882/525, 18-21=682/753, 21-24=-380/0, 21-22=-1582/150,         27-9-1877/0, 26-27-882/525, 18-21=602/753, 21-24=-380/0, 21-22=-1582/150,         27-9-1877/0, 26-27-882/525, 18-21=602/753, 21-24=-380/0, 21-22=-1582/150,         27-9-1877/0, 26-27-882/525, 18-21=602/753, 21-24=-380/0, 21-22/75   | 6   | 6-35: 2x4 SP No.1, 7-29,10-18,8-36,8-35,11-3  | 6: 2x4 SP DSS   | JOINTS   |                      | 0                          |                       |                |                    |
| TOP CHORD       2-3-161422208, 3-5-16871/12/67, 5-6-15405/2116, 6-7-2738/6/319,<br>7-8-27608/3447, 8-10-763/6704, 10-11-872/6935, 11-13-9939/1414,<br>13-14-10433/1475         BOT CHORD       2-341865/14432, 33-341865/14432, 31-331678/14337, 29-311315/12911,<br>28-291195/16235, 25-28-1195/16235, 22-26-569/11093,<br>18-22566/11093, 17-18998/8962, 15-171209/0236, 14-151209/0236,<br>27-30-2170/280, 25-273258/0, 23-25-3258/0, 20-21-57/917         WEBS       3-3340/445, 5-31624/318, 6-351684/15612,<br>29-301048/10002, 30-351014/10118, 7-35447/64, 18-205093/653,<br>20-36-51007/69, 10-36344/6439, 11-17124/280, 13-17614/243,<br>13-15=0/319, 35-371366/202, 36-371364/202, 8-3618572/2472, 8-351910/17271,<br>31-35418/2981, 27-281101480, 25-26-277/10, 23-24363/0, 21-221592/150,<br>27-291877/0, 26-27882/525, 18-21682/753, 21-24139/4464, 11-36518614/2477         NOTES-       1) 3-ply truss to be connected together with 10d (0.120*x3*) nails as follows:<br>Top chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 cc,<br>2x4 - 1 row at 0-9-0 cc.       SEAL<br>044925         Vontes-<br>1) 3-ply truss to be connected as follows: 2x6 - 2 rows staggered at 0-7-0 cc, 2x4 - 1 row at 0-9-0 cc.       SEAL<br>044925         Verb connected as follows: 2x4 - 1 row at 0-9-0 cc.       Webs connected as follows: 2x4 - 1 row at 0-9-0 cc, Except member 7-29 2x4 - 2 rows staggered at 0-4-0 cc, 2x6 - 2 rows staggered<br>at 0-9-0 cc.         2) All loads are considered for bil ball plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to<br>ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.         2) Unbalanced rord live loads have been considered for this design. </td <td>ז<br/>ז</td> <td>Max Horz 2=198(LC 16)<br/>Max Uplift 2=-727(LC 12), 14=-423(LC 13)</td> <td></td> <td>8=0-3-8, 14=0-3-8</td> <td></td> <td></td> <td></td> <td></td> <td></td> | ז<br>ז  | Max Horz 2=198(LC 16)<br>Max Uplift 2=-727(LC 12), 14=-423(LC 13)   |   | 8=0-3-8, 14=0-3-8  |                      |                            |                       |                |                    |
| BOT CHORD       2-34=-1865/14432, 33-34=-1865/14432, 31-33=-1678/14337, 29-31=-1315/12911,<br>28-29=-1195/16235, 26-28=-1195/16235, 24-26=-690/15395, 22-24=-556/11093,<br>18-22=-556/11093, 17-18=-998/8692, 15-171209/9236,<br>27-30=-2170/280, 25-27=-3258/0, 21-23=-3258/0, 21-23=-3258/0, 20-21=-57/917         WEBS       3-33=-430/213, 5-33=-0/449, 5-31=-824/318, 6-31=-1476/89, 6-35=-1684/15612,<br>29-30=-1048/10002, 30-35=-1014/10118, 7-35=-44/764, 18-20=-5093/653,<br>20-36=-5100/769, 10-36=-3444/564, 11-18=-556/4939, 11-17124/243,<br>13-15=-013/019, 35-37=-1366/202, 36-37=-1364/202, 8-36=-18572/2472, 8-35=-1910/17271,<br>31-35=-418/2981, 27-28=-110/480, 25-26=-271/0, 23-24=-363/0, 21-22=-1592/150,<br>27-29=-1877/0, 26-27=-882/525, 18-21=-682/753, 21-24=-139/4464, 11-36=-18614/2477         NOTES-         N1 3-ply truss to be connected together with 10d (0.120°x3°) nails as follows:<br>Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 cc.<br>Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 cc.<br>Webs connected as follows: 2x6 - 1 rows staggered at 0-9-0 cc.<br>Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 cc.         2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to<br>ply connections have been provided to distribute only loads noted as (F) or tack. (B) face in the LOAD CASE(S) section. Ply to<br>gable end cone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip<br>DCL=1.60         Continued on page 2   | ( )   | 2-3=-16142/2208, 3-5=-15871/2167, 5-6=-15<br>7-8=-27608/3447, 8-10=-763/6704, 10-11=-8  | 405/2116, 6-7=-27336/341  | ,  |                      |                            |                       |                |                    |
| <ul> <li>WEBS 3-33=-430/213, 5-33=-0/449, 5-31=-824/318, 6-31=-1476/89, 6-35=-1684/15612, 29-30=-1048/10002, 30-35=-1014/10118, 7-35=-44/764, 18-20=-5093/653, 20-36=-5100/769, 10-36=-3484/584, 11-18=-556/4939, 11-17=-614/243, 13-15=0/319, 35-37=-1366/202, 36-37=-1364/202, 8-36=-18572/2472, 8-35=-1910/17271, 31-35=-418/2981, 27-28=-110/480, 25-26=-271/0, 23-24=-363/0, 21-22=-1592/150, 27-29=-1877/0, 26-27=-882/525, 18-21=-682/753, 21-24=-139/4464, 11-36=-18614/2477</li> <li>NOTES- NOTES- 1) 3-ply truss to be connected together with 10d (0.120°x3°) nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 1 row at 0-9-0 oc, Except member 7-29 2x4 - 2 rows staggered at 0-4-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated. 3) Unbalanced roof live loads have been considered for this design. 4) Wind: ASCE 7-10; Vult=130mph tr;DCL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Continued on page 2</li></ul>   | BOT CHORD   | 2-34=-1865/14432, 33-34=-1865/14432, 31-<br>28-29=-1195/16235, 26-28=-1195/16235, 24<br>18-22=-556/11093, 17-18=-998/8962, 15-17=   | -26=-690/15395, 22-24=-55<br>-1209/9236, 14-15=-1209/   | 56/11093,<br>′9236,  |                      |                            |                       |                |                    |
| <ul> <li>a) Unbalanced roof live loads have been considered for this design.</li> <li>b) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>Continued on page 2</li> </ul>  | WEBS  | 3-33=-430/213, 5-33=-0/449, 5-31=-824/318,<br>29-30=-1048/10002, 30-35=-1014/10118, 7-5<br>20-36=-5100/769, 10-36=-3484/584, 11-18=-<br>13-15=0/319, 35-37=-1366/202, 36-37=-136<br>31-35=-418/2981, 27-28=-110/480, 25-26=-2 | 6-31=-1476/89, 6-35=-168<br>35=-44/764, 18-20=-5093/6<br>556/4939, 11-17=-124/280<br>4/202, 8-36=-18572/2472, 8<br>71/0, 23-24=-363/0, 21-22= | 34/15612,<br>53,<br>1, 13-17=-614/243,<br>3-35=-1910/17271,<br>=-1592/150, |                      |                            |                       | OR OFESS       | ROLIN              |
| <ul> <li>a) Unbalanced roof live loads have been considered for this design.</li> <li>b) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>Continued on page 2</li> </ul>  | NOTES-  |   |   |  |                      |                            |                       | Deat 2         | some               |
| <ul> <li>a) Unbalanced roof live loads have been considered for this design.</li> <li>b) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>Continued on page 2</li> </ul>  | Top chords co<br>Bottom chords<br>Webs connect                                | onnected as follows: 2x6 - 2 rows staggered at<br>s connected as follows: 2x6 - 2 rows staggered  | 0-9-0 oc.<br>d at 0-7-0 oc, 2x4 - 1 row at  |  | , 2x6 - 2 r          | ows stagg                  | jered                 | SEA<br>0449    | L<br>25            |
| Continued on page 2  | <ol> <li>All loads are c<br/>ply connection</li> <li>Unbalanced ro</li> </ol> | ns have been provided to distribute only loads<br>oof live loads have been considered for this de   | noted as (F) or (B), unless<br>esign.   | otherwise indicated.   |                      |                            |                       | 2 Congrin      | EEP. P. A.         |
|  | gable end zon<br>DOL=1.60   | e and C-C Exterior(2) zone;C-C for members  |   |  |                      |                            |                       | June           | 10,2021            |
|  | Continued on pag  | je 2  |   |  |                      |                            |                       |                |                    |



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-74/3 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

| Job                   | Truss             | Truss Type | Qty | Plv        | Aberdeen Loft VIt Mst  |
|-----------------------|-------------------|------------|-----|------------|--|
| 005                   | 11000             |            | aly | ,          | 146497210  |
| 27210-27210A          | A5GR              | ATTIC      | 1   | -          | 10101210   |
| 21210-21210A          | AJOR              | ATTIC      | 1   | 3          | Job Reference (optional)                                     |
|                       |                   |            |     | -          | Job Reference (optional)                                     |
| 84 Components (Dunn), | Dunn, NC - 28334, |            |     | 3.510 s Ju | 1 2021 MiTek Industries, Inc. Wed Jun 9 11:49:59 2021 Page 2 |

NOTES-

ID:hSPrWMHNLCMfmXJIhBDjiGz8N\_g-qaMVmRyq2PdWRP8GqHj5te9oV?pHJLw3HhGRZuz80v6

- 5) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) The Fabrication Tolerance at joint 29 = 16%, joint 29 = 16%
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Ceiling dead load (5.0 psf) on member(s). 35-37, 36-37; Wall dead load (5.0 psf) on member(s). 30-35, 20-36
- 12) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 27-30, 25-27, 23-25, 21-23, 20-21
- 13) LGT3-SDS2.5 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 14) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 18. This connection is for uplift only and does not consider lateral forces.
- 15) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.
- 16) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 17) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 18) LGT3 Hurricane ties must have three studs in line below the truss.
- 19) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 10231 lb down and 1423 lb up at 25-10-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
   20) Attic room checked for L/360 deflection.

#### LOAD CASE(S) Standard

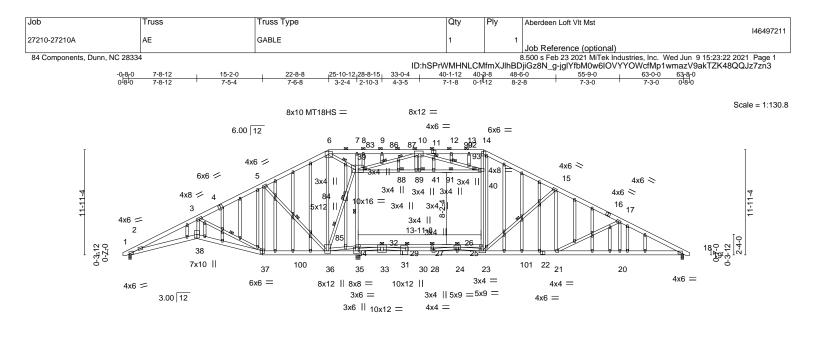
- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
  - Uniform Loads (plf) Vert: 1-6=-60, 6-10=-60, 10-14=-60, 38-41=-20, 20-30=-30, 35-36=-10
  - Drag: 30-35=-10, 20-36=-10

Concentrated Loads (lb)

Vert: 29=-6100(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





 26-0-4
 40-3-8

 7-8-12
 15-2-0
 22-8-8
 25-10-12 (28-8-15) 31-7-3 (34-5-6) (37-3-9) (40-1-12) (34-5-6) (37-3-9) (40-1-12) (34-5-6) (3

Plate Offsets (X,Y)-- [2:0-0-12,Edge], [4:0-3-0,0-4-4], [6:0-5-0,0-3-0], [10:0-5-8,0-3-12], [31:0-5-0,0-2-12], [32:0-6-0,0-3-0], [35:0-1-8,0-3-4], [36:0-3-4,0-2-0], [37:0-3-0,0-3-8], [39:0-5-8,0-3-8], [54:0-1-15,0-1-0], [76:0-1-14,0-1-0]

| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | * SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>* Rep Stress Incr YES<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.46<br>BC 0.89<br>WB 0.99<br>Matrix-MS | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT)<br>Attic | -0.46<br>-0.93<br>0.31 | (loc)<br>25-26<br>25-26<br>18<br>25-34 | l/defl<br>>973<br>>479<br>n/a<br>732 | L/d<br>240<br>180<br>n/a<br>360 | PLATES<br>MT20<br>MT18HS<br>Weight: 725 lb | <b>GRIP</b><br>197/144<br>197/144<br>FT = 20% |
|--|---|--|--|------------------------|--|--------------------------------------|---------------------------------|--|---|
| UMBER-   | 1   |  | BRACING-   |                        |  |                                      |                                 | 1  |   |
| OP CHORD 2   | x6 SP No.2 *Except*   |  | TOP CHOR   | RD                     | Structu                                | ral wood                             | sheathing di                    | rectly applied or 3-1-2 of                 | oc purlins, except                            |
| 6  | -11: 2x6 SP DSS   |  |  |                        | 2-0-0 c                                | c purlins                            | (4-3-12 max.                    | ): 6-14.                                   |   |
| DT CHORD 2   | x6 SP No.2 *Except*   |  | BOT CHOR   | RD                     | Rigid c                                | eiling dire                          | ectly applied                   | or 10-0-0 oc bracing, I                    | Except:                                       |
|  | 1-37,22-31: 2x6 SP DSS, 25-34: 2x4 SP No.2  | or 2x4 SPF No.2                                    |  |                        |  |                                      | ng: 35-36                       |  |   |
|  | x4 SP No.3 *Except*   |  |  |                        |  |                                      | ng: 33-35.                      |  |   |
|  | -36,6-39,39-40,10-39,10-40,36-39: 2x4 SP No   | 0.2 or 2x4 SPF No.2                                |  |                        |  | c bracing                            | j: 25-34                        |  |   |
|  | '-35,14-23: 2x4 SP DSS  |  | WEBS   |                        | 1 Row                                  | at midpt                             |                                 | 8-37, 5-36, 6-39, 34-39,                   | 15-23, 40-41,                                 |
| THERS 2  | x4 SP No.3  |  |  |                        |  |                                      |                                 | 0-86, 36-84                                |   |
|  |   |  | JOINTS   |                        | 1 Brac                                 | e at Jt(s):                          | 39, 40, 41, 8                   | 4, 86, 88, 90, 92                          |   |
|  |   |  |  |                        | MiTe                                   | k recomn                             | nends that Sta                  | abilizers and required o                   | cross bracing                                 |
|  |   |  |  |                        | be in:                                 | stalled du                           | iring truss ere                 | ection, in accordance w                    | ith Stabilizer                                |
|  |   |  |  |                        | Instal                                 | lation gu                            | de.                             |  |   |
|  | lb/size) 2=1373/0-3-8 (min. 0-2-3), 35=259  | 2/0-5-8 (min. 0-4-8), 18=1                         | 870/0-3-8 (min. 0                                  | )-3-4)                 |  |                                      |                                 |  |   |
|  | Max Horz 2=-194(LC 13)  |  |  |                        |  |                                      |                                 |  |   |
| 1  | Max Uplift 2=-4(LC 12), 18=-122(LC 13)  |  |  |                        |  |                                      |                                 |  |   |

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Max Grav 2=1400(LC 2), 35=2885(LC 2), 18=2061(LC 27)

- TOP CHORD
   2-3=-4571/510, 3-4=-1970/234, 4-5=-1922/260, 5-6=-1388/188, 6-7=-227/3203, 7-8=-221/3328, 8-9=-174/3156, 9-10=-174/3156, 10-11=-2555/233, 11-12=-2555/233, 12-13=-2555/233, 13-14=-2555/233, 14-15=-2789/191, 15-16=-3373/294, 16-17=-3519/259,
- 17-18=-3982/363 BOT CHORD 2-38=-363/4142, 37-38=-355/4001, 37-100=0/1709, 36-100=0/1709, 35-36=0/1447, 33-35=0/2423, 31-33=0/2423, 30-31=0/2423, 28-30=0/4419, 24-28=0/4006, 23-24=0/4006, 23-101=-9/3088, 22-101=-9/3088, 21-22=-9/3088, 20-21=-220/3496, 18-20=-220/3496, 32-34=0/607, 29-32=-2522/0, 27-29=-2522/0, 26-27=-2522/0 WEBS 3-38=-99/2060, 3-37=-2463/427, 5-37=-35/541, 5-36=-911/342, 6-36=-216/3219, 6-83=-5108/532, 39-83=-4796/497, 34-35=-1919/375, 34-39=-1892/485, 23-25=0/1212, 25-40=0/1443, 14-40=0/887, 15-23=-983/364, 15-21=-31/546, 17-21=-565/238, 17-20=0/262, 39-88=-1775/0, 88-89=-1773/0, 41-89=-1799/0, 41-91=-1799/0, 40-91=-1799/0, 32-33=-698/0, 29-30=-305/0, 27-28=-280/0, 39-86=-3728/443, 86-87=-3798/444, 10-87=-3801/453, 10-90=0/2129, 90-92=0/2113, 92-93=0/2080, 40-93=0/2118, 36-85=0/809, 35-85=0/853, 36-84=-3436/0, 39-84=-2405/0, 32-35=-1089/120, 30-32=0/2095, 23-26=-1901/0, 26-28=-163/626, 83-84=-618/71,

84-85=0/550, 8-39=-767/199, 9-86=-1/271

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

#### ontinued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **MISI/TPII Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





| Job                     | Truss | Truss Type | Qty | Ply | Aberdeen Loft VIt Mst  |
|-------------------------|-------|------------|-----|-----|--|
| 27210-27210A            | A.F.  | GABLE      | 1   |     | I46497211  |
| 27210-27210A            | AE    | GABLE      | 1   | '   | Job Reference (optional)   |
| 84 Components Dunn NC 3 | 28334 | •          |     |     | 8 500 s Feb 23 2021 MiTek Industries Inc. Wed Jun 9 15:23:22 2021 Page 2 |

ID:hSPrWMHNLCMfmXJlhBDjiGz8N\_g-jglYfbM0w6IOVYYOWcfMp1wmazV9akTZK48QQJz7zn3

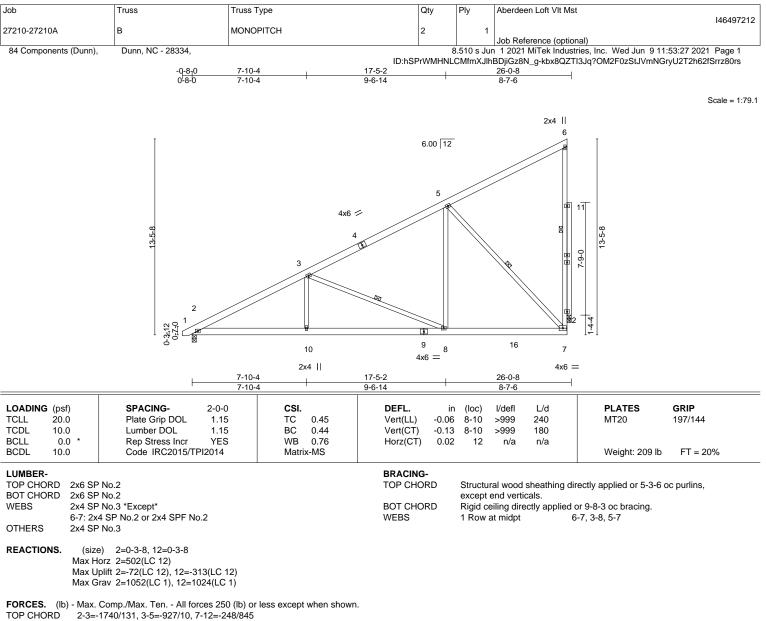
#### NOTES-

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are MT20 plates unless otherwise indicated. 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Ceiling dead load (5.0 psf) on member(s). 39-88, 88-89, 41-89, 41-91, 40-91; Wall dead load (5.0 psf) on member(s).34-39, 25-40
   Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 32-34, 29-32, 27-29, 26-27, 25-26
- 13) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 2 and 122 lb uplift at joint 18.
- 15) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 17) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

18) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard





- BOT CHORD 2-3=-1740/131, 3-3=-927/10, 7-12=-246/645 BOT CHORD 2-10=-566/1507, 8-10=-566/1507, 7-8=-265/719
- BOT CHORD 2-10=-566/1507, 8-10=-566/1507, 7-8=-265/719
- WEBS 3-10=0/360, 3-8=-855/326, 5-8=-4/643, 5-7=-1049/389

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) All plates are 3x4 MT20 unless otherwise indicated.

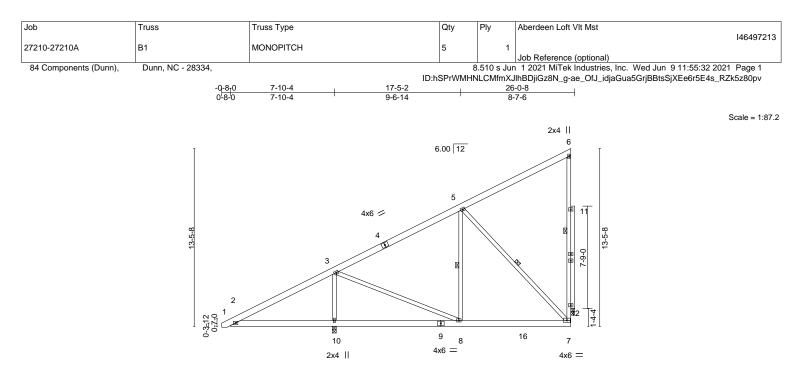
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 12. This connection is for uplift only and does not consider lateral forces.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



ENGINEERING BY **TRENCO** A MITek Affiliate 818 Soundside Road

Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



|  |   | 7-8-8         7-10-4           7-8-8         0-1 <sup>1</sup> -12 | 17-5-2<br>9-6-14 |  | 26-0-8<br>8-7-6                               |                                  |                                    |
|--|---|---|------------------|--|---|----------------------------------|------------------------------------|
| TCLL         20.0         Pla           TCDL         10.0         Lu           BCLL         0.0 *         Re | PACING- 2-0-0<br>ate Grip DOL 1.15<br>Imber DOL 1.15<br>ep Stress Incr YES<br>ode IRC2015/TPI2014 | CSI.<br>TC 0.53<br>BC 0.29<br>WB 0.42<br>Matrix-MS                | Vert(CT) -       | in (loc)<br>-0.05 7-8<br>-0.09 7-8<br>-0.02 12 | l/defl L/d<br>>999 240<br>>999 180<br>n/a n/a | PLATES<br>MT20<br>Weight: 209 lb | <b>GRIP</b><br>197/144<br>FT = 20% |

| LUMBER- |  |
|---------|--|
|---------|--|

| TOP CHORD | 2x6 SP No.2                      |
|-----------|----------------------------------|
| BOT CHORD | 2x6 SP No.2                      |
| WEBS      | 2x4 SP No.3 *Except*             |
|           | 6-7: 2x4 SP No.2 or 2x4 SPF No.2 |
| OTHERS    | 2x4 SP No.3                      |

BRACING-TOP CHORD

WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 6-7, 5-8, 5-7

REACTIONS. (size) 10=0-3-8, 12=0-3-8 Max Horz 10=502(LC 12) Max Uplift 10=-104(LC 12), 12=-281(LC 12) Max Grav 10=1518(LC 1), 12=580(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-178/684, 3-5=-447/0, 7-12=-198/422
- BOT CHORD 2-10=-500/237. 8-10=-534/0. 7-8=-219/303
- WEBS 3-10=-1284/267, 3-8=0/847, 5-7=-436/320

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

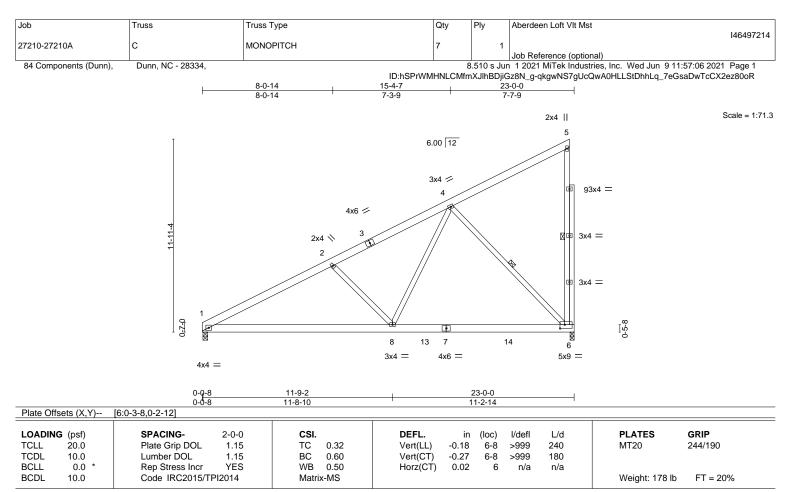
2) All plates are 3x4 MT20 unless otherwise indicated.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 10, 12, and This connection is for uplift only and does not consider lateral forces.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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818 Soundside Road Edenton, NC 27932



BRACING-

TOP CHORD

BOT CHORD

WEBS

#### LUMBER-

| TOP CHORD | 2x6 SP No.2 |
|-----------|-------------|
| BOT CHORD | 2x6 SP No.2 |
| WEBS      | 2x4 SP No.3 |
| OTHERS    | 2x4 SP No.3 |

REACTIONS. (size) 1=0-3-8, 6=0-3-0 Max Horz 1=436(LC 12) Max Uplift 1=-56(LC 12), 6=-273(LC 12) Max Grav 1=908(LC 1), 6=900(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-1404/119, 2-4=-1097/74

BOT CHORD 1-8=-484/1208, 6-8=-250/612

WEBS 2-8=-460/291, 4-8=-78/722, 4-6=-877/365

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

4) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 6. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 5-8-1 oc purlins,

5-6, 4-6

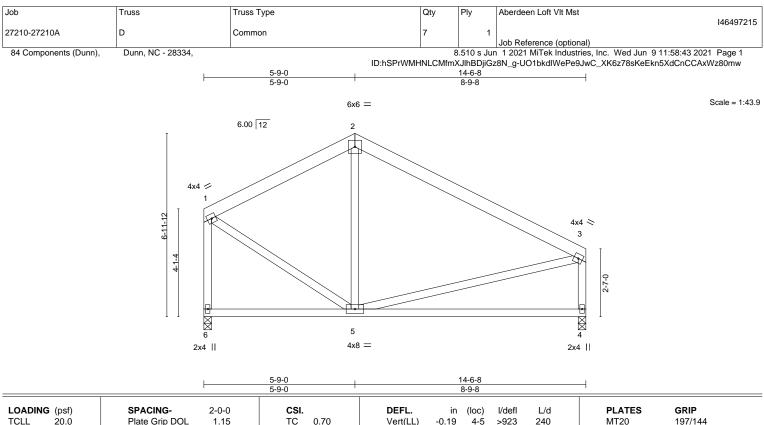
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

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TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.3 WEBS

REACTIONS. 6=0-3-8, 4=0-3-8 (size) Max Horz 6=-106(LC 13) Max Uplift 6=-59(LC 13), 4=-57(LC 13) Max Grav 6=570(LC 1), 4=570(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-414/138, 2-3=-446/106, 1-6=-539/182, 3-4=-500/177 TOP CHORD

WEBS 1-5=-60/381, 3-5=0/327

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 4. This connection is for uplift only and does not consider lateral forces.



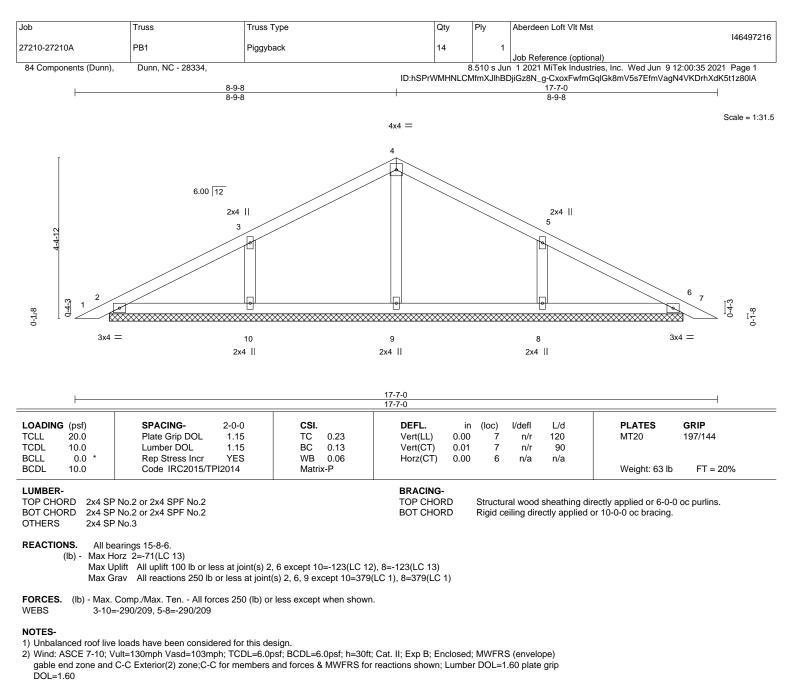
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

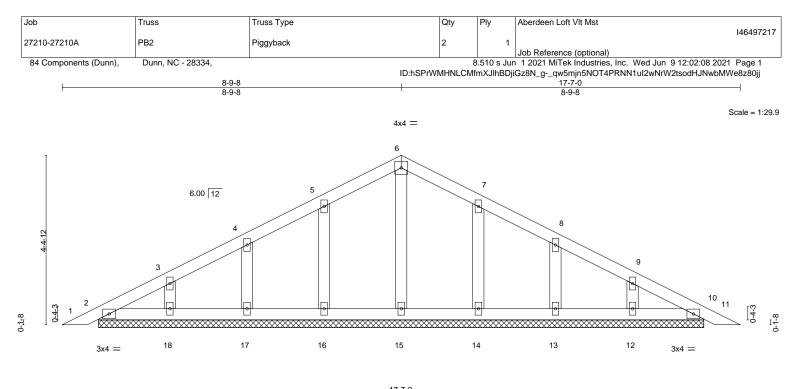
6) n/a

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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| OADING | (psf) | SPACING-        | 2-0-0  | CSI.   |      | DEFL.    | in   | (loc) | l/defl | L/d | PLATES        | GRIP     |
|--------|-------|-----------------|--------|--------|------|----------|------|-------|--------|-----|---------------|----------|
| TCLL   | 20.0  | Plate Grip DOL  | 1.15   | тс     | 0.05 | Vert(LL) | 0.00 | 10    | n/r    | 120 | MT20          | 197/144  |
| TCDL   | 10.0  | Lumber DOL      | 1.15   | BC     | 0.03 | Vert(CT) | 0.00 | 10    | n/r    | 90  |               |          |
| BCLL   | 0.0 * | Rep Stress Incr | YES    | WB     | 0.03 | Horz(CT) | 0.00 | 10    | n/a    | n/a |               |          |
| BCDL   | 10.0  | Code IRC2015/T  | PI2014 | Matrix | ĸ-P  |          |      |       |        |     | Weight: 73 lb | FT = 20% |

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

**REACTIONS.** All bearings 15-8-6.

(lb) - Max Horz 2=71(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 17, 13, 18, 16, 14, 12

Max Grav All reactions 250 lb or less at joint(s) 2, 10, 15, 17, 13, 18, 16, 14, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

7) n/a

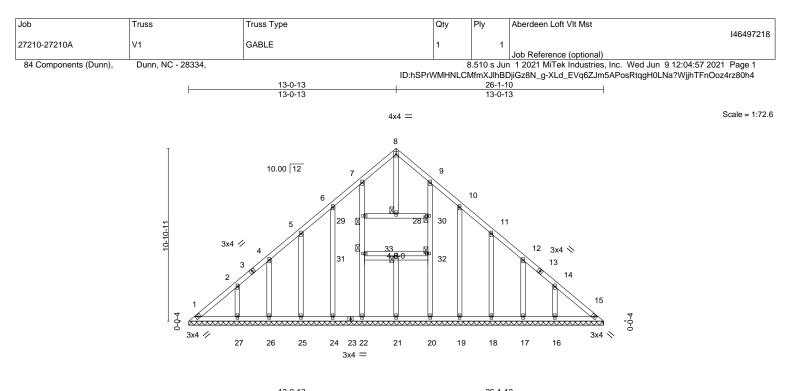
8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

## SEAL 044925 VGINEE June 10,2021

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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



|   |                                 | <u>13-0-13</u><br>13-0-13 |                |  |  |       |        |     |                |             |
|---|---------------------------------|---------------------------|----------------|--|--|-------|--------|-----|----------------|-------------|
|   |                                 |                           |                |  | 13-0-13  |       |        |     |                |             |
| OADING (psf)  | SPACING-                        | 2-0-0 CSI.                |                | DEFL.                                      | in   | (loc) | l/defl | L/d | PLATES         | GRIP        |
| CLL 20.0  | Plate Grip DOL                  | 1.15 TC                   | 0.10           | Vert(LL)                                   | n/a  | -     | n/a    | 999 | MT20           | 197/144     |
| CDL 10.0  | Lumber DOL                      | 1.15 BC                   | 0.07           | Vert(CT)                                   | n/a  | -     | n/a    | 999 |                |             |
| BCLL 0.0 *  | Rep Stress Incr                 | YES WB                    | 0.14           | Horz(CT)                                   | 0.01   | 15    | n/a    | n/a |                |             |
| BCDL 10.0   | Code IRC2015/TPI                | 2014 Matri                | ix-S           |  |  |       |        |     | Weight: 190 lb | FT = 20%    |
| LUMBER-           TOP CHORD         2x4 SP No.2 or 2x4 SPF No.2           BOT CHORD         2x4 SP No.2 or 2x4 SPF No.2           WEBS         2x4 SP No.3           OTHERS         2x4 SP No.3 |                                 |                           |                | BRACING-<br>TOP CHOR<br>BOT CHOR<br>JOINTS | RD Structural wood sheathing directly applied or 6-0-0 o |       |        |     |                | oc purlins. |
|   | earings 26-1-10.                |                           |                |  |  |       |        |     |                |             |
| ( )   | lorz 1=261(LC 9)                |                           |                |  |  |       |        |     |                |             |
| Movil   | blift All unlift 100 lb or loor | a at inimt(a) 1 00 01 05  | 00 00 40 40 47 | 4E aveant 0                                | 7 445/   |       |        |     |                |             |

Max Uplift All uplift 100 lb or less at joint(s) 1, 22, 24, 25, 26, 20, 19, 18, 17, 15 except 27=-115(LC 12), 16=-115(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 22, 24, 25, 26, 27, 20, 19, 18, 17, 16, 15, 21

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-385/241, 2-4=-275/145, 12-14=-259/145, 14-15=-369/241

BOT CHORD 1-27=-200/315, 26-27=-200/315, 25-26=-200/315, 24-25=-200/315, 22-24=-200/315, 21-22=-200/316, 20-21=-200/316, 19-20=-200/315, 18-19=-200/315, 17-18=-200/315, 16-17=-200/315, 15-16=-200/315

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



