

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 2000456-2000456A Wellons Reser

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I47996617 thru I47996661

My license renewal date for the state of North Carolina is December 31, 2021.

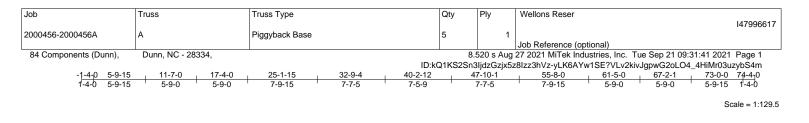
North Carolina COA: C-0844

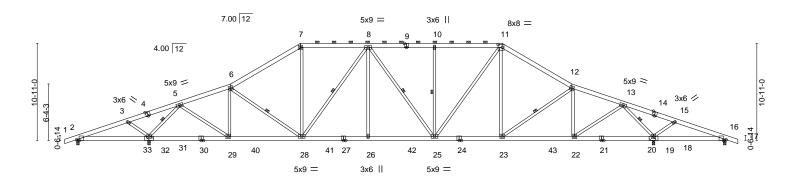


September 22,2021

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





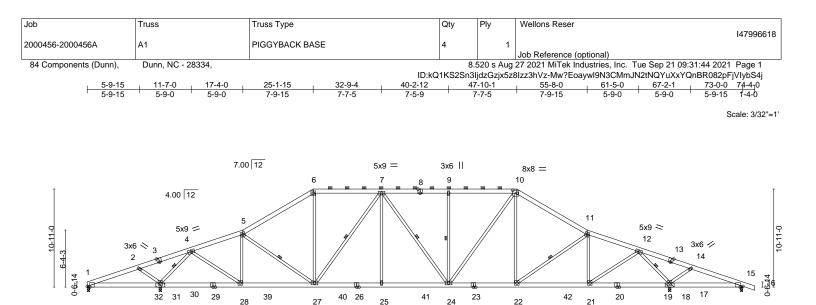
F		5-1-15 <u>32-9-4</u> -9-15 7-7-5	<u>+ 40-2-12</u> + 47-10-1 7-5-9 7-7-5	55-8-0	<u>64-10-4</u> 73-0-0 9-2-4 8-1-12
Plate Offsets (X,Y)	[2:0-3-6,Edge], [4:0-3-0,Edge], [9:0-3-0				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.51 BC 0.62 WB 0.82 Matrix-MS	- ()	l/defl L/d >999 240 >999 180 n/a n/a	PLATES GRIP MT20 197/144 Weight: 566 lb FT = 20%
WEBS 2x4 SI 8-28,8 REACTIONS. All b bear (lb) - Max H	P No.2 P No.2 P No.3 *Except* I-25,11-25: 2x4 SP No.2 or 2x4 SPF No earings 0-3-8 except (jt=length) 32=0-4 ing block). Horz 2=180(LC 12) Jplift All uplift 100 lb or less at joint(s) (25)	14 (input: 0-3-8 + bearing b	2-0-0 oc BOT CHORD Rigid ce 6-0-0 oc WEBS 1 Row a Nock), 19=0-4-14 (input: 0-3-8 +	: purlins (4-2-6 max.): iling directly applied c : bracing: 2-32,16-19. It midpt 5	ectly applied or 3-9-1 oc purlins, except 7-11. or 10-0-0 oc bracing, Except: -32, 6-28, 8-28, 8-25, 10-25, 12-23, 13-19
FORCES. (lb) - Max. TOP CHORD 2-3= 8-10 15- BOT CHORD 2-32 23-2 WEBS 3-32 8-28	Grav All reactions 250 lb or less at join . Comp./Max. Ten All forces 250 (lb) c 183/1261, 3-5=-296/1594, 5-6=-2477/5 =-2773/700, 10-11=-2773/700, 11-12=- 16=-170/1257 =-1136/225, 29-32=-120/798, 28-29=-2 5=-182/2302, 22-23=-287/2340, 19-22= =-492/237, 5-32=-3452/757, 5-29=-233 =-955/221, 8-26=0/413, 10-25=-478/22 =2=-878/245, 13-22=-230/1866, 13-19=-	r less except when shown. 42, 6-7=-2741/636, 7-8=-22 2757/637, 12-13=-2490/546 34/2327, 26-28=-269/2795, -93/805, 16-19=-1132/222 1860, 6-29=-880/246, 7-28 2, 11-25=-218/899, 11-23=-	298/614, 5, 13-15=-285/1590, 25-26=-269/2795, =-110/949,		
 fasteners. User Def 2x6 SP No.2 bearin fasteners. User Def 3) Unbalanced roof liv 4) Wind: ASCE 7-10; ' gable end zone and forces & MWFRS fc 5) WARNING: This lo handling and erectit Trusses ("BCSI"), jo qualified registered permanent individua bracing. 6) Provide adequate d 7) All plates are 6x6 M 	ng block 12" long at jt. 32 attached to fro fined Bearing crushing capacity= 425psi ng block 12" long at jt. 19 attached to fro fined Bearing crushing capacity= 425psi e loads have been considered for this d Vult=130mph Vasd=103mph; TCDL=6.0 d C-C Exterior(2) zone; cantilever left an for reactions shown; Lumber DOL=1.60 p ing span truss requires extreme care an on guidance, see Guide to Good Practic onty produced by SBCA and TPI. The I design professional for the design and al truss member restraint/bracing. MiTe trainage to prevent water ponding. IT20 unless otherwise indicated. In designed for a 10.0 psf bottom chord li	nt face with 3 rows of 10d (esign. psf; BCDL=6.0psf; h=30ft; d d right exposed ; end vertic late grip DOL=1.60 d experience for proper and e for Handling, Installing & uilding owner or the owner inspection of the temporary k assumes no responsibility	0.131"x3") nails spaced 3" o.c. 12 Cat. II; Exp B; Enclosed; MWFRS al left and right exposed;C-C for r I safe handling and erection. For Bracing of Metal Plate Connected s authorized agent shall contract installation restraint/bracing and of truss manufacture, handling,	2 Total	SEAL 044925 September 22,2021
Design valid for use of a truss system. Befor building design. Brac is always required for fabrication, storage, of	r design parameters and READ NOTES ON THIS AP only with MiTek® connectors. This design is based re use, the building designer must verify the applic- cing indicated is to prevent buckling of individual tr r stability and to prevent collapse with possible per delivery, erection and bracing of trusses and truss available from Truss Plate Institute, 2670 Crain Hi	only upon parameters shown, and bility of design parameters and pro iss web and/or chord members onl onal injury and property damage. systems, see ANS/TP11 (is for an individual building component, no pperly incorporate this design into the over y. Additional temporary and permanent br For general guidance regarding the Quality Criteria, DSB-89 and BCSI Build	ot rall racing	ENGINEERING BY REENCO AMITek Atfiliate 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Wellons Reser		
					147996617		
2000456-2000456A	A	Piggyback Base	5	1			
					Job Reference (optional)		
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 21 09:31:41 2021 Page 2		
		ID:kQ1KS2Sn3ljdzGzix5z8lzz3hVz-yLK6AYw1SE?VLv2kivJqpwG2oLO4 4HiMr03uzybS4m					

- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 16. This connection is for uplift only and does not consider lateral forces.
- 11) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 32 and 19. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3x6 ||

5x9 =

5x9 =

		1-15 <u>32-9-4</u> 9-15 7-7-5		10-1 55-8-0 7-5 7-9-15	64-10-4 73-0-0 9-2-4 8-1-12
	[1:0-3-6,Edge], [3:0-3-0,Edge], [8:0-3-0				9-2-4 0-1-12
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.51 BC 0.62 WB 0.82 Matrix-MS	DEFL. in Vert(LL) -0.19 Vert(CT) -0.37 Horz(CT) 0.11		PLATES GRIP MT20 197/144 Weight: 562 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP 7-27,7- REACTIONS. All be beari (lb) - Max H Max U Max G FORCES. (lb) - Max. TOP CHORD 1-2=- 7-9=- 14-15 BOT CHORD 1-31= 22-22 WEBS 2-31=	P No.2	2 14 (input: 0-3-8 + bearing xcept 1=-299(LC 26), 31= (s) 1, 15 except 31=3123 r less except when shown 42, 5-6=-2741/636, 6-7=-2 7/637, 11-12=-2490/546, 94/2326, 25-27=-269/2795 -93/805, 15-18=-1132/222 1860, 5-28=-881/247, 6-2	BOT CHORD WEBS block), 18=0-4-14 (input: (=-302(LC 12), 18=-262(LC (LC 1), 18=3111(LC 1) 1. 2297/614, 12-14=-285/1590, 5, 24-25=-269/2795, 2 (7=-110/948,	2-0-0 oc purlins (4-2-6 max Rigid ceiling directly applie 6-0-0 oc bracing: 1-31,15-1 1 Row at midpt 0-3-8 +	directly applied or 3-9-1 oc purlins, except x.): 6-10. ed or 10-0-0 oc bracing, Except:
 NOTES- 1) 2x6 SP No.2 bearing fasteners. User Defi 2) 2x6 SP No.2 bearing fasteners. User Defi 3) Unbalanced roof live 4) Wind: ASCE 7-10; V gable end zone and forces & MWFRS fo 5) WARNING: This lor handling and erection Trusses ("BCSI"), jo qualified registered of permanent individual bracing. 6) Provide adequate dr 7) All plates are 6x6 M 	I=-878/245, 12-21=-230/1865, 12-18=-3 g block 12" long at jt. 31 attached to fror ned Bearing crushing capacity= 425psi. g block 12" long at jt. 18 attached to fror ned Bearing crushing capacity= 425psi. e loads have been considered for this de 'ult=130mph Vasd=103mph; TCDL=6.0 C-C Exterior(2) zone; cantilever left and r reactions shown; Lumber DOL=1.60 p rg span truss requires extreme care and n guidance, see Guide to Good Practic intly produced by SBCA and TPI. The b design professional for the design and i I truss member restraint/bracing. MiTel 'ainage to prevent water ponding. T20 unless otherwise indicated. designed for a 10.0 psf bottom chord lin	at face with 3 rows of 10d at face with 3 rows of 10d esign. psf; BCDL=6.0psf; h=30ft d right exposed ; end verti late grip DOL=1.60 d experience for proper ar e for Handling, Installing & uilding owner or the owne nspection of the temporar < assumes no responsibili	I (0.131"x3") nails spaced i (0.131"x3") nails spaced i ; Cat. II; Exp B; Enclosed; ical left and right exposed; nd safe handling and erect B Bracing of Metal Plate C er's authorized agent shall y installation restraint/brac ity for truss manufacture, h	3" o.c. 12 Total	SEAL 044925 WGINEEFFICIENT September 22,2021
Design valid for use of a truss system. Before building design. Braci is always required for fabrication, storage, d	design parameters and READ NOTES ON THIS AN nly with MiTek® connectors. This design is based a use, the building designer must verify the applica ng indicated is to prevent buckling of individual tru stability and to prevent collapse with possible pers elivery, erection and bracing of trusses and truss s available from Truss Plate Institute, 2670 Crain Hig	only upon parameters shown, ar bility of design parameters and p ss web and/or chord members o onal injury and property damage ystems, see ANSI/TPI	nd is for an individual building com properly incorporate this design in only. Additional temporary and pe e. For general guidance regarding 1 Quality Criteria. DSB-89 and E	nponent, not to the overall rmanent bracing g the	TENGINEERING BY AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Wellons Reser
					147996618
2000456-2000456A	A1	PIGGYBACK BASE	4	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 21 09:31:44 2021 Page 2
		ID:kQ	1KS2Sn3	jdzGzjx5z8	Blzz3hVz-Mw?Eoaywl9N3CMmJN2tNQYuXxYQnBR082pFjVlybS4j

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 15. This connection is for uplift only and does not consider lateral forces.

11) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 31 and 18. This connection is for uplift only and does not consider lateral forces.

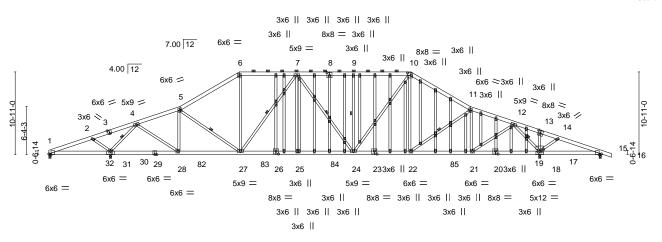
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Job	Truss		Tru	iss Type			Qty	Ply	Wellons Reser	
2000456-2000456A	A1E			BLE			1	1		147996619
2000456-2000456A	AIE		GA	DLE			1	1	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC	- 28334,					8.	520 s Aug	27 2021 MiTek Industries, Inc. T	ue Sep 21 09:31:55 2021 Page 1
						ID:kC	1KS2Sn3	jdzGzjx5z	8lzz3hVz-X1AP6L5p9YmV126Q	XsZyNtrQ9_ALGQlla0PpO9ybS4Y
	5-9-15	11-7-0	17-4-0	25-1-15	32-9-4	40-2-12	47-10	1	55-8-0 61-5-0 67-2-1	73-0-0 74-4 ₁ 0
	5-9-15	5-9-0	5-9-0	7-9-15	7-7-5	7-5-9	7-7-5	5	7-9-15 5-9-0 5-9-0	5-9-15 1-4-0

Scale = 1:152.2



	8-1-12	17-4-0	25-1-15 32-9-4	40-2-12	47-10-1	55-8-0	64-10-4	73-0-0	
	8-1-12	9-2-4	7-9-15 7-7-5	7-5-9	7-7-5	7-9-15	9-2-4	8-1-12	
Plate Offsets (X,Y)	[1:0-3-6,Edge], [3:0-3-	-0,Edge], [8:0-4-0,0	-4-8], [10:0-5-8,0-4-0],	[13:0-4-0,0-4-8],	[15:0-3-6,Edge	e], [18:0-6-0,0-3-	-0], [20:0-4-0,0-	4-8], [23:0-4-0,0-4	-8],
	[26:0-4-0,0-4-8]								

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.51 BC 0.62 WB 0.82 Matrix-MS	Vert(LL) -0.1	n (loc) l/defl L/d 9 24-25 >999 240 7 24-25 >999 180 1 18 n/a n/a	PLATES MT20 Weight: 773 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x6	SP No.2		BRACING- TOP CHORD	Structural wood sheathing d	rectly applied or 3-9-1	oc purlins, except
	SP No.2 SP No.3 *Except*		BOT CHORD	2-0-0 oc purlins (4-2-6 max.) Rigid ceiling directly applied		Except:
	,7-24,10-24: 2x4 SP No.2 or 2x4 SPF No.: SP No.3	2	WEBS	6-0-0 oc bracing: 1-31,15-18 1 Row at midpt	4-31, 5-27, 7-27, 7-24,	9-24, 11-22, 12-18

REACTIONS. All bearings 0-3-8 except (jt=length) 31=0-4-14 (input: 0-3-8 + bearing block), 18=0-4-14 (input: 0-3-8 + bearing block).
 (lb) - Max Horz 1=-188(LC 13)

Max Uplift	All uplift 100 lb or less at joint(s) except 1=-299(LC 26), 31=-302(LC 12),
	18=-262(LC 13), 15=-265(LC 25)
Max Grav	All reactions 250 lb or less at joint(s) 1, 15 except 31=3123(LC 1),
	18=3111(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-173/1254, 2-4=-293/1592, 4-5=-2476/542, 5-6=-2741/636, 6-7=-2297/614, 7-9=-2773/700, 9-10=-2773/700, 10-11=-2757/637, 11-12=-2490/546, 12-14=-285/1590,

	14-15=-169/1257
BOT CHORD	1-31=-1128/221, 28-31=-120/798, 27-28=-284/2326, 25-27=-269/2795, 24-25=-269/2795,
	22-24=-182/2302, 21-22=-287/2340, 18-21=-93/805, 15-18=-1132/222
WEBS	2-31=-502/242, 4-31=-3449/753, 4-28=-233/1860, 5-28=-881/247, 6-27=-110/948,
	7-27=-955/221, 7-25=0/413, 9-24=-478/222, 10-24=-218/899, 10-22=-28/357,
	11-21=-878/245, 12-21=-230/1865, 12-18=-3458/750, 14-18=-492/237

NOTES-

- 1) 2x6 SP No.2 bearing block 12" long at jt. 31 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity= 425psi.
- 2) 2x6 SP No.2 bearing block 12" long at jt. 18 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity= 425psi.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

SEAL 044925 September 22,2021

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Wellons Reser
2000456-2000456A	A1E	GABLE	1	1	147996619
2000430-2000430A			1	· ·	Job Reference (optional)
84 Components (Dunn)	Dunn NC - 28334		. 8	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 21 09:31:55 2021, Page 2

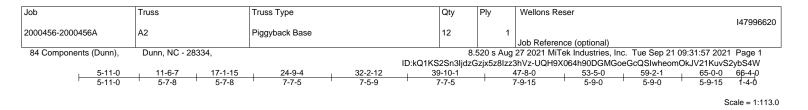
ID:kQ1KS2Sn3ljdzGzjx5z8lzz3hVz-X1AP6L5p9YmV126QXsZyNtrQ9_ALGQlla0PpO9ybS4Y

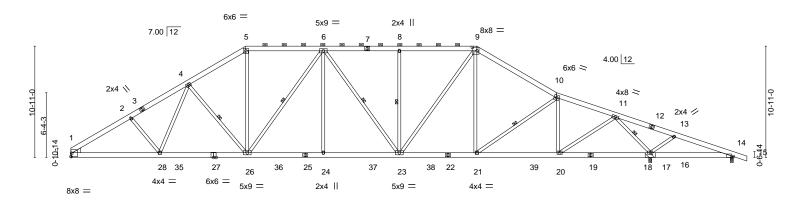
NOTES-

- 6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- Provide adequate drainage to prevent water ponding.
- 8) All plates are 3x4 MT20 unless otherwise indicated.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 15. This connection is for uplift only and does not consider lateral forces.
- 13) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 31 and 18. This connection is for uplift only and does not consider lateral forces.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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	-8-11 17-1-15 -8-11 8-5-3	<u>24-9-4</u> <u>32-2-1</u> 7-7-5 7-5-9		47-8-0 7-9-15	56-10-4 9-2-4	<u>65-0-0</u> 8-1-12
Plate Offsets (X,Y)	[9:0-5-8,0-4-0], [14:0-2-2,Edge]	1-1-5 1-5-8	<i>1-1-</i> 5	7-9-15	9-2-4	0-1-12
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.84 BC 0.97 WB 0.86 Matrix-MS	DEFL. in Vert(LL) -0.23 Vert(CT) -0.43 Horz(CT) 0.15		PLATES MT20 Weight: 512	GRIP 197/144 2 lb FT = 20%
		.2	BOT CHORD	Structural wood sheathing 2-0-0 oc purlins (4-0-10 m Rigid ceiling directly appli 2-2-0 oc bracing: 1-28 6-0-0 oc bracing: 14-17. 1 Row at midpt	nax.): 5-9.	g, Except:
Max H Max L	e) 1=Mechanical, 17=(0-3-8 + bear lorz 1=-290(LC 8) Jplift 1=-168(LC 12), 17=-256(LC 13), Grav 1=2209(LC 2), 17=3255(LC 1), 1	14=-323(LC 25))-3-8			
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-3660/675, 2-4=-3492/689, 4-5=-3111/693, 5-6=-2633/642, 6-8=-2960/718, 8-9=-2960/718, 9-10=-2890/650, 10-11=-2563/551, 11-13=-308/1768, 13-14=-192/1431 BOT CHORD 1-28=-432/3038, 26-28=-336/2902, 24-26=-296/3053, 23-24=-296/3053, 21-23=-193/2416, 20-21=-292/2411, 17-20=-86/771, 14-17=-1296/242 WEBS 4-28=-43/316, 4-26=-586/244, 5-26=-186/1234, 6-26=-831/270, 6-24=0/393, 8-23=-475/221, 9-23=-219/1017, 9-21=-29/336, 10-20=-947/254, 11-20=-245/1995, 11-17=-3660/773, 13-17=-498/238						
 fasteners. User Def 2) Unbalanced roof liv. 3) Wind: ASCE 7-10; W gable end zone and forces & MWFRS fc 4) WARNING: This lo handling and erection Trusses ("BCSI"), jc qualified registered permanent individua bracing. 5) Provide adequate d 6) All plates are 4x6 M 7) This truss has bee will fit between the F 9) Refer to girder(s) for 	g block 12" long at jt. 17 attached to fi ined Bearing crushing capacity= 425p e loads have been considered for this /ult=130mph Vasd=103mph; TCDL=6 I C-C Exterior(2) zone; cantilever left a or reactions shown; Lumber DOL=1.60 ng span truss requires extreme care a on guidance, see Guide to Good Prac inity produced by SBCA and TPI. The design professional for the design and al truss member restraint/bracing. MiT rainage to prevent water ponding. IT20 unless otherwise indicated. designed for a 10.0 psf bottom chord in designed for a live load of 20.0psf of pottom chord and any other members r truss to truss connections. al connection (by others) of truss to be	si. design. .0psf; BCDL=6.0psf; h=30ft nd right exposed ; end verti plate grip DOL=1.60 nd experience for proper ar ice for Handling, Installing & building owner or the owne l inspection of the temporar ek assumes no responsibili	; Cat. II; Exp B; Enclosed; cal left and right exposed; d safe handling and erecti & Bracing of Metal Plate Co r's authorized agent shall y installation restraint/brac ty for truss manufacture, h h any other live loads. eas where a rectangle 3-6-	MWFRS (envelope) C-C for members and ion. For general onnected Wood contract with a ing and the landling, erection, or 0 tall by 2-0-0 wide	SCOTT	CAROL SEAL 14925 M. SEWITHIN mber 22,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Wellons Reser
					147996620
2000456-2000456A	A2	Piggyback Base	12	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 21 09:31:57 2021 Page 2
		ID:kQ1KS	S2Sn3ljdz0	Szjx5z8lzz3	3hVz-UQH9X064h90DGMGoeGcQSIwheomOkJV21KuvS2ybS4W

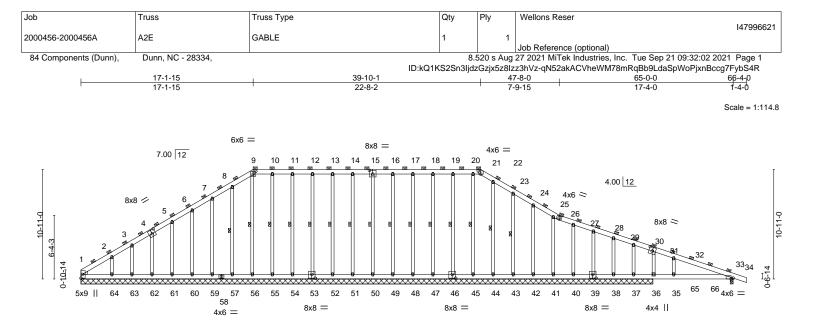
11) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17. This connection is for uplift only and does not consider lateral forces.

12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			65-0-0 65-0-0		
Plate Offsets (X,Y) LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	[4:0-4-0,0-4-8], [9:0-3-0,0-3-4], [15:0-4- SPACING- 2-2-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.70 BC 0.82 WB 0.21 Matrix-S	DEFL. ir Vert(LL) 0.10	n (loc) l/defl L/d 33-35 >964 240 33-35 >494 180	PLATES GRIP MT20 244/190 Weight: 629 lb FT = 20%
(lb) - Max H	P No.2	, 50, 51, 52, 53, 54, 55, 5 (cept 33=-184(LC 9), 64=	-132(LC 35),	2-0-0 oc purlins (6-0-0 m (Switched from sheeted: Rigid ceiling directly appl 1 Row at midpt	Spacing > 2-0-0).
FORCES. (lb) - Max. TOP CHORD 1-2=- 11-12 16-17 21-22	38=-179(LC 13), 37=-537(LC 24), irav All reactions 250 lb or less at joint 59, 60, 61, 62, 63, 49, 48, 47, 46, 4 19), 44=451(LC 24), 43=460(LC 1) 39=417(LC 24), 38=663(LC 1), 36= Comp./Max. Ten All forces 250 (lb) or 287/275, 7-8=-154/260, 8-9=-196/307, 5 2=-182/289, 12-13=-182/289, 13-14=-18 7=-182/289, 17-18=-182/289, 13-19=-18 2=-194/297, 22-23=-176/285 7=-52/305, 31-36=-925/255, 32-35=-158	36=-541(LC 13) (s) 1, 50, 51, 52, 53, 54, 5 45, 37 except 33=710(LC , 42=459(LC 1), 41=464(L :2132(LC 24) less except when shown 9-10=-182/288, 10-11=-18 2/289, 14-15=-182/289, 1 2/289, 19-20=-183/289, 2	55, 56, 57, 1), 64=305(LC .C 1), 40=471(LC 1), 32/289, 15-16=-182/289,		
 Wind: ASCE 7-10; V gable end zone and forces & MWFRS for Truss designed for v Gable End Details a WARNING: This lor handling and erection Trusses ("BCSI"), jo qualified registered of permanent individual bracing. Provide adequate dr All plates are 2x4 M Gable studs spaced 	e loads have been considered for this de (ult=130mph Vasd=103mph; TCDL=6.0) C-C Exterior(2) zone; cantilever left and r reactions shown; Lumber DOL=1.60 p vind loads in the plane of the truss only. s applicable, or consult qualified building g span truss requires extreme care and n guidance, see Guide to Good Practice intly produced by SBCA and TPI. The b design professional for the design and ir il truss member restraint/bracing. MiTel rainage to prevent water ponding. T20 unless otherwise indicated. at 2-0-0 oc. designed for a 10.0 psf bottom chord live	bsf; BCDL=6.0psf; h=30ft; I right exposed ; end verti late grip DOL=1.60 For studs exposed to wii g designer as per ANSI/TI l experience for proper an e for Handling, Installing 8 uilding owner or the owne isspection of the temporary c assumes no responsibili	cal left and right exposed nd (normal to the face), s Pl 1. d safe handling and ered Bracing of Metal Plate (n's authorized agent sha y installation restraint/bra ty for truss manufacture,	d;C-C for members and see Standard Industry ction. For general Connected Wood Il contract with a acing and the	SEAL 044925 MGINEEP, HALL September 22,2021
Design valid for use of a truss system. Before building design. Braci is always required for fabrication, storage, d	design parameters and READ NOTES ON THIS ANI nly with MiTek® connectors. This design is based use, the building designer must verify the applica ng indicated is to prevent buckling of individual tru stability and to prevent collapse with possible pers elivery, erection and bracing of trusses and truss s available from Truss Plate Institute, 2670 Crain Hig	only upon parameters shown, an bility of design parameters and p ss web and/or chord members o onal injury and property damage ystems, see ANSI/TP 11	d is for an individual building co roperly incorporate this design nly. Additional temporary and p . For general guidance regard I Quality Criteria, DSB-89 and	omponent, not into the overall permanent bracing ng the	ENGINEERING BY REENCO AMITEK Attiliate 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Wellons Reser			
					I47996621			
2000456-2000456A	A2E	GABLE	1	1				
					Job Reference (optional)			
84 Components (Dunn),	Dunn, NC - 28334,	8.520 s Aug 27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:04 2021 Page 2						
		ID:kQ1KS2Sn3ljdzGzjx5z8lzz3hVz-nmCo?QCT1JvEcRI9YFE3EmjwycCGtdR4ew5nC8ybS4P						

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) N/A

11) N/A

- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 286 lb down and 97 lb up at 40-11-4, 286 lb down and 97 lb up at 42-11-4, 286 lb down and 97 lb up at 42-11-4, 286 lb down and 97 lb up at 42-11-4, 286 lb down and 97 lb up at 50-11-4, 286 lb down and 97 lb up at 50-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 78 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 78 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb down and 97 lb up at 52-11-4, 286 lb d

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

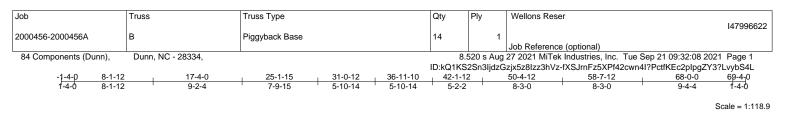
Vert: 1-9=-65, 9-21=-65, 21-26=-65, 26-34=-65, 1-33=-22

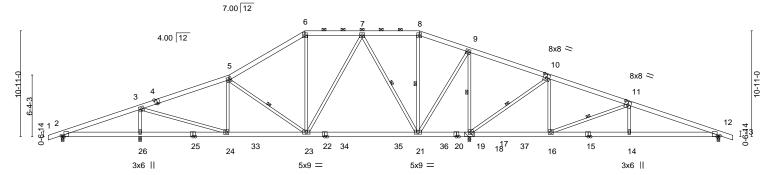
Concentrated Loads (lb)

Vert: 44=-286(B) 43=-286(B) 42=-286(B) 41=-286(B) 40=-286(B) 39=-286(B) 38=-286(B) 37=-286(B) 36=-286(B) 35=-286(B) 65=-290(B) 65=-2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







	1-12 17-4-0 1-12 9-2-4		-11-10 42-1-1 -9-11 5-2-2		<u>58-7-12</u> <u>68-0-0</u> 8-3-0 9-4-4
Plate Offsets (X,Y)	[2:0-3-6,Edge], [4:0-3-0,Edge], [10:0-4	-0,0-4-8], [11:0-4-0,0-4-8],	[12:0-3-6,Edge]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.49 BC 0.58 WB 0.83 Matrix-MS	Vert(LL) -0.19	(loc) l/defl L/d 21-23 >999 240 21-23 >999 180 12 n/a n/a	PLATES GRIP MT20 244/190 Weight: 506 lb FT = 20%
(lb) - Max H Max U	° No.2	except 2=-116(LC 8), 26=-;	267(LC 12), 18=-434(LC	 2-0-0 oc purlins (6-0-0 m Rigid ceiling directly app 1 Row at midpt 9), 12=-197(LC 	
TOP CHORD 3-5=- 11-12 11-12 BOT CHORD 23-24 WEBS 3-26= 7-21= 7-21=	Comp./Max. Ten All forces 250 (b) (-1410/317, 5-6=-1109/312, 6-7=-872/3; 2=-1377/334 4=-151/1252, 21-23=0/635, 18-21=-85; -1406/416, 3-24=-137/1333, 5-24=-25 =-1013/248, 8-21=-284/79, 9-21=-206/ 6=-14/640, 11-16=-1130/342, 11-14=0/	29, 9-10=-74/972, 10-11=-2 //391, 14-16=-217/1253, 12 9/142, 5-23=-508/232, 7-23 1605, 9-18=-2028/465, 10-	299/147, 2-14=-214/1259 3=-65/629,		
 fasteners. User Defi 2) Unbalanced roof live 3) Wind: ASCE 7-10; V gable end zone and forces & MWFRS fo 4) WARNING: This lor handling and erectic Trusses ("BCSI"), jo qualified registered of permanent individual bracing. 5) Provide adequate dr 6) All plates are 6x6 M 7) This truss has been 8) * This truss has been will fit between the b 9) One H2.5A Simpsor Thio connection is for 10) Two H2.5A Simpsor 	g block 12" long at jt. 18 attached to fro ned Bearing crushing capacity= 425ps e loads have been considered for this of /ult=130mph Vasd=103mph; TCDL=6.1 C-C Exterior(2) zone; cantilever left ar r reactions shown; Lumber DOL=1.60 ng span truss requires extreme care ar on guidance, see Guide to Good Practii intly produced by SBCA and TPI. The design professional for the design and al truss member restraint/bracing. MiTe rainage to prevent water ponding. T20 unless otherwise indicated. designed for a 10.0 psf bottom chord I n designed for a 10.0 psf bottom chord I n designed for a 10.0 psf bottom chord I n designed for a no any other members, n Strong-Tie connectors recommended or uplift only and does not consider later plift only and does not consider lateral	i. lesign.)psf; BCDL=6.0psf; h=30ft; dr ight exposed ; end vertii olate grip DOL=1.60 d experience for proper an ze for Handling, Installing & building owner or the owne inspection of the temporary ek assumes no responsibilit ive load nonconcurrent with the bottom chord in all are with BCDL = 10.0psf. to connect truss to bearing ral forces.	cat. II; Exp B; Enclosed cal left and right exposed ad safe handling and ered Bracing of Metal Plate (r's authorized agent sha y installation restraint/bra ty for truss manufacture, h any other live loads. eas where a rectangle 3- g walls due to UPLIFT at	; MWFRS (envelope) i;C-C for members and tion. For general Connected Wood II contract with a ucing and the handling, erection, or 6-0 tall by 2-0-0 wide jt(s) 2, 26, and 12.	SEAL 044925 MGINEEER, HER, MILLING September 22,2021
Design valid for use o a truss system. Before building design. Braci	design parameters and READ NOTES ON THIS Al nly with MITek® connectors. This design is based e use, the building designer must verify the applic ing indicated is to prevent buckling of individual to the build the prevent buckling of individual to	l only upon parameters shown, an ability of design parameters and p uss web and/or chord members or	nd is for an individual building co properly incorporate this design nly. Additional temporary and p	imponent, not into the overall	

a duss system: picture use, the building design. Bracing indicated is to be prevent buckling of individual truss were band/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss were band/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss were band/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss were band/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rection and bracing of trusses and truss systems, see **ADSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

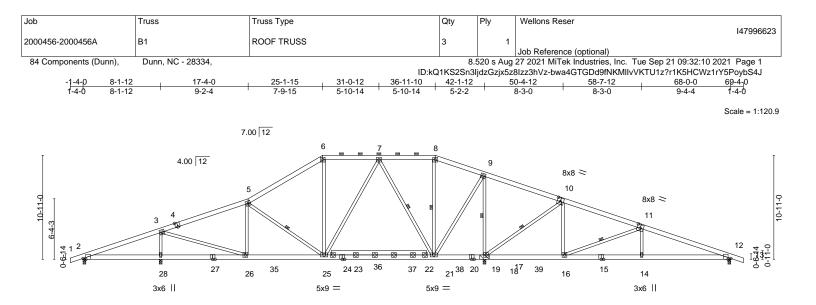


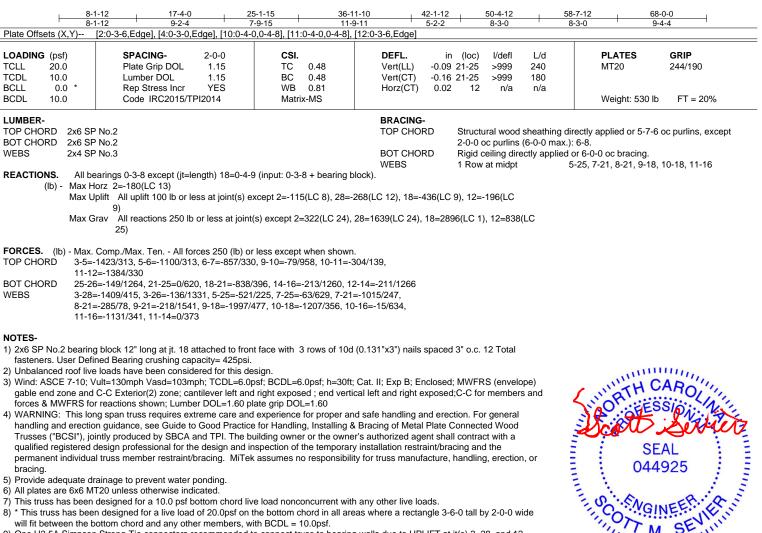
Job	Truss	Truss Type	Qty	Ply	Wellons Reser		
					147996622		
2000456-2000456A	В	Piggyback Base	14	1			
					Job Reference (optional)		
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:08 2021 Page 2		
			ID:kQ1KS2Sn3IjdzGzjx5z8Izz3hVz-fXSJrnFz5XPf42cwn4I?PctfKEc2pIpgZY3?LvybS4L				

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 28, and 12. This connection is for uplift only and does not consider lateral forces.

10) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 18. This Continuentionaisefor uplift only and does not consider lateral forces

ᄊ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORF USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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September 22,2021



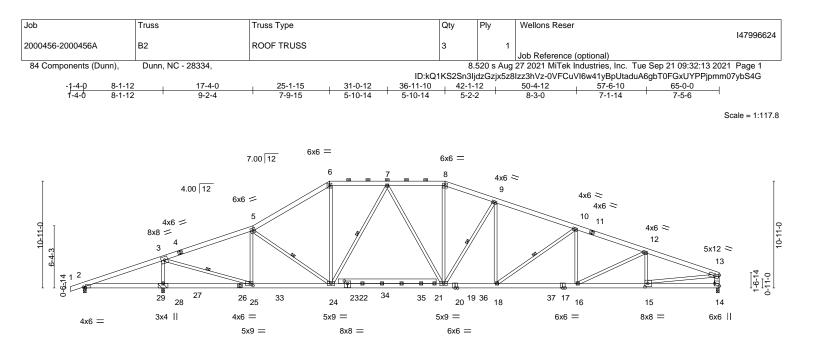
Job	Truss	Truss Type	Qty	Ply	Wellons Reser			
					147996623			
2000456-2000456A	B1	ROOF TRUSS	3	1				
					Job Reference (optional)			
84 Components (Dunn),	Dunn, NC - 28334,	8.520 s Aug 27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:10 2021 Page 2						
		ID:kQ1KS2Sn3ljdzGzjx5z8lzz3hVz-bwa4GTGDd9fNKMIIvVKTU1z?r1K5HCWz1rY5PoybS4J						

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

12) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			<u>-11-10 42-1-</u> 1-9-11 5-2-		<u>57-6-10</u> <u>65-0-0</u> 7-1-14 <u>7-5-6</u>
Plate Offsets (X,Y)	[2:0-2-6,Edge], [14:Edge,0-5-8], [15:0-3				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.68 BC 0.79 WB 0.87 Matrix-MS	Vert(LL) -0.26	i (loc) I/defl L/d 20-24 >999 240 16-18 >999 180 14 n/a n/a	PLATES GRIP MT20 197/144 Weight: 523 lb FT = 20%
		14: 2x6 SP No.2	BRACING- TOP CHORD BOT CHORD WEBS		g directly applied or 3-3-15 oc purlins, 2-0-0 oc purlins (4-1-6 max.): 6-8. ed or 6-0-0 oc bracing. 3-25, 5-24, 7-24, 9-20, 10-18
Max H Max L	e) 2=0-3-8, 28=(0-3-8 + bearing block) Horz 2=190(LC 16) Jplift 2=-152(LC 27), 28=-287(LC 12), 14 Brav 2=114(LC 24), 28=3032(LC 1), 14=	=-356(LC 9)			
TOP CHORD 2-3= 8-9= 8-9= BOT CHORD 2-28 16-1 WEBS 3-28 7-24	Comp./Max. Ten All forces 250 (lb) or -219/958, 3-5=-2754/617, 5-6=-2901/722 -3172/843, 9-10=-3690/914, 10-12=-427 =-833/240, 25-28=-833/240, 24-25=-412 8=-799/4001, 15-16=-836/3893, 14-15=- -2770/738, 3-25=-648/3516, 5-25=-918 =-877/256, 7-20=-104/463, 8-20=-132/74 8=-743/247, 10-16=0/294, 12-16=0/266,	2, 6-7=-2435/687, 7-8=-25 8/1004, 12-13=-4178/940 /2545, 20-24=-417/2785, 106/404 (306, 5-24=-283/187, 6-2: 49, 9-20=-968/316, 9-18=:	971/821, 9, 13-14=-2102/516 18-20=-601/3436, 4=-175/1064, -64/587,		
 fasteners. User Def 2) Unbalanced roof liv 3) Wind: ASCE 7-10; gable end zone and forces & MWFRS fc 4) WARNING: This lo handling and erectit Trusses ("BCSI"), jc qualified registered permanent individua bracing. 5) Provide adequate d 6) All plates are 4x4 M 7) This truss has been 8) * This truss has been will fit between the I 9) One H2.5A Simpso connection is for up 10) Two H2.5A Simpso 	g block 12" long at jt. 28 attached to fron ined Bearing crushing capacity= 425psi. e loads have been considered for this de Vult=130mph Vasd=103mph; TCDL=6.0p I C-C Exterior(2) zone; cantilever left and or reactions shown; Lumber DOL=1.60 pl ng span truss requires extreme care and on guidance, see Guide to Good Practice pintly produced by SBCA and TPI. The bi design professional for the design and ir al truss member restraint/bracing. MiTek rainage to prevent water ponding. IT20 unless otherwise indicated. designed for a 10.0 psf bottom chord liv en designed for a live load of 20.0psf on t bottom chord and any other members, wi n Strong-Tie connectors recommended iff only and does not consider lateral for on Strong-Tie done to accention lateral for	sign. bisf; BCDL=6.0psf; h=30ft; right exposed ; end vertii ate grip DOL=1.60 experience for proper an for Handling, Installing & uilding owner or the owne ispection of the temporary assumes no responsibility e load nonconcurrent with he bottom chord in all are th BCDL = 10.0psf. o connect truss to bearing ces.	Cat. II; Exp B; Enclosed cal left and right exposed d safe handling and ered Bracing of Metal Plate f r's authorized agent sha y installation restraint/bra ty for truss manufacture, h any other live loads. eas where a rectangle 3- g walls due to UPLIFT at	t; MWFRS (envelope) d;C-C for members and ction. For general Connected Wood II contract with a ucing and the handling, erection, or 6-0 tall by 2-0-0 wide jt(s) 2 and 14. This	SEAL 044925 September 22,2021

- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members, with BCDL = 10.0psf. 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This
- connection is for uplift only and does not consider lateral forces. 10) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 28. This

Continuencestionaisefor uplift only and does not consider lateral forces.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



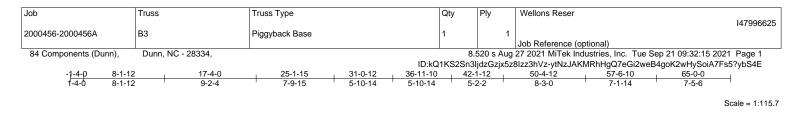
Job	Truss	Truss Type	Qty	Ply	Wellons Reser			
					147996624			
2000456-2000456A	B2	ROOF TRUSS	3	1				
					Job Reference (optional)			
84 Components (Dunn),	Dunn, NC - 28334,	8.520 s Aug 27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:13 2021 Page 2						
		ID:kQ1KS2Sn3IjdzGzjx5z8Izz3hVz-0VFCuVI6w41yBpUtaduA6gbT0FGxUYPPjpmm07ybS4G						

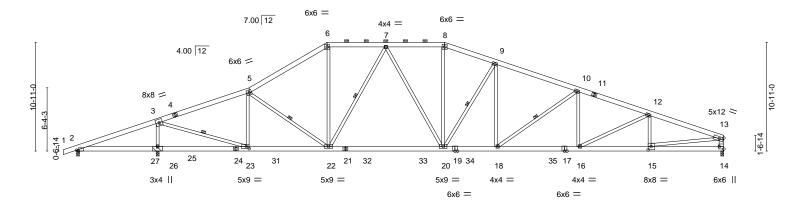
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

12) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







	3-1-12 17-4 3-1-12 9-2-		25-1-15 7-9-15	<u>36-11-10</u> 11-9-11	42-1-12	50-4-12 8-3-0	57-6-10	<u>65-0-0</u> 7-5-6
Plate Offsets (X,Y)	[2:0-2-6,Edge], [14:Edg							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/7	2-0-0 1.15 1.15 YES TPI2014	CSI. TC 0.69 BC 0.92 WB 0.87 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT	in (loc) -0.36 20-22 -0.68 20-22) 0.12 14	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 500	GRIP 197/144 0 lb FT = 20%
3-23,1 REACTIONS. (siz Max H		+ bearing block)	(req. 0-4-13), 14	BRACIN TOP CH BOT CH WEBS =0-3-8	ORD Struct excep ORD Rigid	t end verticals, and	g directly applied or 3- 12-0-0 oc purlins (4-0-1 lied or 2-2-0 oc bracing 3-23, 5-22, 7-22, 9-	15 max.): 6-8. J.
FORCES. (lb) - Max. TOP CHORD 2-3= 8-9= BOT CHORD 2-26 16-1 WEBS 3-26 7-22	Grav 2=97(LC 23), 26=30 . Comp./Max. Ten All for 228/1023, 3-5=-2726/6' 3199/837, 9-10=-3693/9 895/247, 23-26=-895/2 8=-798/3995, 15-16=-83 2790/743, 3-23=-655/3 882/254, 7-20=-103/47 8=-747/245, 10-16=0/30	orces 250 (lb) or l 15, 5-6=-2920/717 914, 10-12=-4272 247, 22-23=-411/2 6/3887, 14-15=-1 3560, 5-23=-937/3 73, 8-20=-130/757	ess except when 7, 6-7=-2451/683, /1003, 12-13=-41 2526, 20-22=-411 06/404 301, 5-22=-261/19 7, 9-20=-949/328,	7-8=-2997/816, 71/939, 13-14=-2099// /2805, 18-20=-601/34: 03, 6-22=-172/1072, 9-18=-76/542,				
 NOTES- 1) 2x6 SP No.2 bearin fasteners. User Def 2) Unbalanced roof liv 3) Wind: ASCE 7-10; ' gable end zone and forces & MWFRS fc 4) WARNING: This loo handling and erecti Trusses ("BCSI"), jg qualified registered permanent individua bracing. 5) Provide adequate d 6) All plates are 4x6 M 7) This truss has been will fit between the 1 9) One H2.5A Simpso connection is for up 10) Two H2.5A Simpso 	Ig block 12" long at jt. 26 ined Bearing crushing cz e loads have been consi vult=130mph Vasd=103r d C-C Exterior(2) zone; c or reactions shown; Lumt ong span truss requires e on guidance, see Guide i pointly produced by SBCA design professional for t al truss member restraint trainage to prevent water 1T20 unless otherwise in n designed for a 10.0 psf en designed for a live loa bottom chord and any ott n Strong-Tie connectors on Strong-Tie connectors on Strong-Tie connectors	attached to front apacity= 425psi. idered for this des mph; TCDL=6.0ps antilever left and r ber DOL=1.60 pla xtreme care and e to Good Practice I to Good Practice I	face with 3 rows ign. sf; BCDL=6.0psf; right exposed ; er te grip DOL=1.6C experience for prot for Handling, Inst ilding owner or the spection of the ter assumes no resp load nonconcurre the bottom chord ir h BCDL = 10.0ps connect truss to es.	of 10d (0.131"x3") nai h=30ft; Cat. II; Exp B; id vertical left and righ oper and safe handling alling & Bracing of Mei e owner's authorized a nporary installation res onsibility for truss man ent with any other live i all areas where a rec i bearing walls due to U	Enclosed; MWFI exposed;C-C fc and erection. Fc al Plate Connec gent shall contra traint/bracing an ufacture, handlir oads. angle 3-6-0 tall I PLIFT at jt(s) 2 a	RS (envelope) or members and or general ted Wood ct with a d the g, erection, or by 2-0-0 wide and 14. This	of the second se	CARO SSI SSI SSI SSI SSI SSI SSI SSI SSI SS

818 Soundside Road Edenton, NC 27932

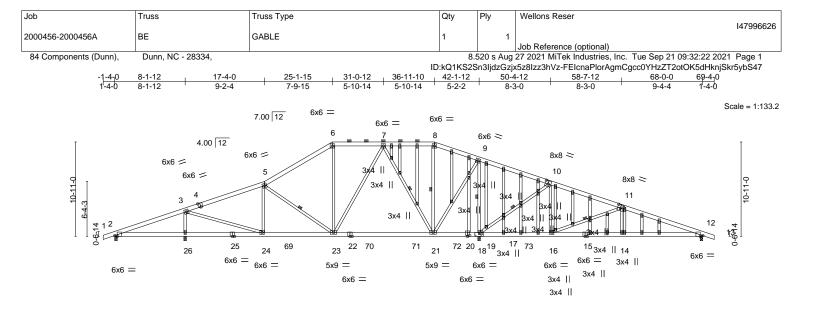
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Wellons Reser		
					147996625		
2000456-2000456A	B3	Piggyback Base	1	1			
					Job Reference (optional)		
84 Components (Dunn),	Dunn, NC - 28334,	8.520 s Aug 27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:15 2021 Page 2					
		ID:kQ1KS2Sn3IjdzGzjx5z8Izz3hVz-ytNzJAKMRhHgQ7eGi2weB4goK2wHySoiA7Fs5?ybS4E					

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





F	8-1-12	17-4-0	25-1-15	36-11-10	42-1-12	50-4-12	58-7-12	68-0-0	
Plate Offsets (X,Y)	8-1-12 [2:0-3-6,Edge], [4:0-	9-2-4 -3-0 Edge] [10:0-4-(7-9-15	<u>11-9-11</u> 0-4-8] [12 [.] 0-3-6 F	5-2-2 dae] [20:0-3-	<u>8-3-0</u>	<u>8-3-0</u> -12 0-1-8] [58·(9-4-4	<u> </u>
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- Plate Grip DC Lumber DOL Rep Stress Ir	2-0-0 DL 1.15 1.15	CSI. TC 0.49 BC 0.58 WB 0.83	DEF Vert Vert	FL. in (LL) -0.19	(loc) l/defl 21-23 >999 21-23 >999	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC20	15/TPI2014	Matrix-MS		. ,			Weight: 617 lb	FT = 20%
(lb) - Max H Max U	P No.2 P No.3	o or less at joint(s) es	xcept 2=-116(LC 8	TOF BOT WEB earing block). 26=-267(LC 12)	, 18=-434(LC	2-0-0 oc purlins Rigid ceiling dir 1 Row at midpt 9), 12=-197(LC	s (6-0-0 max.): 6 rectly applied or 5-2	actly applied or 5-7-8 o 5-8. • 6-0-0 oc bracing. 23, 7-21, 8-21, 9-18, 4	
11-12 BOT CHORD 23-24 WEBS 3-26 7-21:	. Comp./Max. Ten A 1410/317, 5-6=-110: 2=-1377/334 4=-151/1252, 21-23= =-1406/416, 3-24=-1: =-1013/248, 8-21=-2: 6=-14/640, 11-16=-1	9/312, 6-7=-872/329 0/635, 18-21=-851/3 37/1333, 5-24=-259, 84/79, 9-21=-206/16	9, 9-10=-74/972, 1 391, 14-16=-217/1 /142, 5-23=-508/2 605, 9-18=-2028/4	0-11=-299/147, 253, 12-14=-214/ 32, 7-23=-65/629,					
 2) Unbalanced roof live 3) Wind: ASCE 7-10; \ gable end zone and forces & MWFRS for 4) Truss designed for v Gable End Details a 5) WARNING: This loo handling and erectio Trusses ("BCSI"), jo qualified registered 	ined Bearing crushing e loads have been co Vult=130mph Vasd=1 d C-C Exterior(2) zono or reactions shown; L wind loads in the plar as applicable, or cons ing span truss require on guidance, see Gui pintly produced by SE design professional f al truss member restr Irainage to prevent wi IT20 unless otherwise d at 2-0-0 oc. designed for a 10.0 een designed for a liv	g capacity= 425psi. onsidered for this de 103mph; TCDL=6.0p e; cantilever left and umber DOL=1.60 pl ne of the truss only. sult qualified building es extreme care and ide to Good Practice 3CA and TPI. The bu for the design and in raint/bracing. MiTek ater ponding. e indicated. psf bottom chord liv e load of 20.0psf on	sign. sif; BCDL=6.0psf; right exposed; er ate grip DOL=1.60 For studs expose g designer as per / experience for prr e for Handling, Inst uilding owner or th ispection of the ter assumes no resp e load nonconcurr the bottom chord	h=30ft; Cat. II; Ex do vertical left and d to wind (normal ANSI/TPI 1. oper and safe han alling & Bracing o e owner's authoriz mporary installatio onsibility for truss ent with any other in all areas where	p B; Enclosed right exposed to the face), s dling and erec f Metal Plate 0 ted agent shal n restraint/bra manufacture, live loads.	; MWFRS (envel d;C-C for membe eee Standard Indi- ction. For genera Connected Wood II contract with a Licing and the handling, erectic	rs and ustry	MILLIN M	NEER HALL
Design valid for use o a truss system. Befor building design. Brac	design parameters and RE only with MiTek® connecto re use, the building designe cing indicated is to prevent r stability and to prevent co	rs. This design is based of or must verify the applicate buckling of individual trus	only upon parameters s bility of design parameters ss web and/or chord me	hown, and is for an ind ers and properly incorp embers only. Additiona	ividual building co orate this design i I temporary and p	omponent, not into the overall permanent bracing			ENICO A MITek Affiliate

is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Job	Truss	Truss Type	Qty	Ply	Wellons Reser			
					147996626			
2000456-2000456A	BE	GABLE	1	1				
					Job Reference (optional)			
84 Components (Dunn),	Dunn, NC - 28334,	8.520 s Aug 27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:22 2021 Page 2						
		ID:kQ1KS2Sn3IjdzGzjx5z8Izz3hVz-FEIcnaPlorAgmCgcc0YHzZT2otOK5dHknjSkr5ybS47						

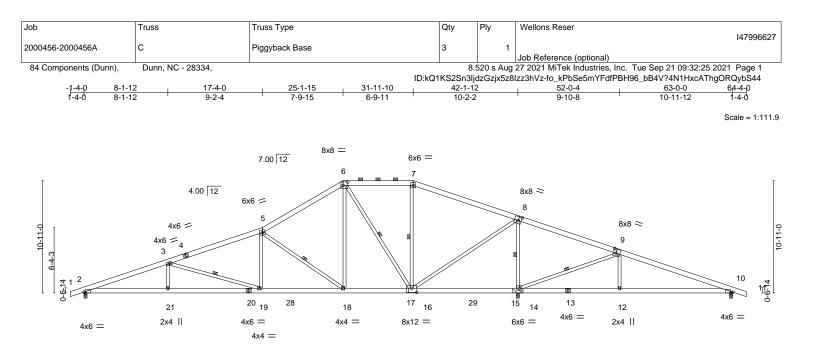
11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 26, and 12. This connection is for uplift only and does not consider lateral forces.

12) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 18. This connection is for uplift only and does not

consider lateral forces. 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





 LOADING (pd) Plate Grp DOL 1.15 LOADING (pd) Plate Grp DOL 1.15 TC 0.74 Vert(T) - 0.18 19.21 Vert(T) - 0.39 19.24 11.24 12.24 12		3-1-12 17-4-0 3-1-12 9-2-4 [2:0-1-6,Edge], [6:0-5-8,0-4-0], [8:0-3-8,	25-1-15 31-1 ⁻ 7-9-15 6-9- 0-4-81 [9:0-3-4 0-4-12] [1	11 10-2-2	9-	2-0-4 10-8	63-0-0 10-11-12			
TOP CHORD 2x6 SP No.2 TOP CHORD Structural wood sheathing diredly applied or 3-7-7 oc purlins, except WEBS 2x4 SP No.3 "Except" BOT CHORD Proceeding directly applied or 5-7-7 oc purlins, except REACTIONS (size) 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD Yeb SP Rigid celling directly applied or 6-0-0 to braing, and the purplet of 5-7-7 oc purlins, except REACTIONS (size) 2x-0-3-8, 14=(0-3+ + bearing block) (req. 0-5-0), 10=0-3-8 WEBS 1 Row at midpt 3-19, 5-18, 6-17, 7-16, 8-14, 9-14 REACTIONS (size) 2x-0-3-8, 14=(0-3+ + bearing block) (req. 0-5-0), 10=0-3-8 WEBS 3-19, 5-18, 6-17, 7-16, 8-14, 9-14 REACTIONS (size) 2x-0-3-8, 14=(0-3+ + bearing block) (req. 0-5-0), 10=0-3-8 WEBS 3-19, 5-18, 6-17, 7-16, 8-14, 9-14 FORCES (b) - Max Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 2-39627/813, 20, 9-10=47073 BOT CHORD 2-39627/813, 20, 9-10=470471 18-19=-317/232, 1, 7-18-36/1112, 14-16=-1263/420, 12-14-473308, 13-21-647338, 13-21-647338, 13-21-647338, 13-21-647338, 13-21-647338, 13-19-317/232, 17-18-36/112, 1/-14=-2479/672, 9-14=-1322/398, 9-12=0/441 Site and the purple of an and t	LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.74 BC 0.70 WB 0.98	DEFL. in Vert(LL) -0.18 Vert(CT) -0.39	(loc) l/defl L/ 19-21 >999 24 19-21 >999 18	0 MT20 0 /a	197/144			
 Max Horz 2=180(IC 13) Max Grav 2=1541(IC 1), 14=381(IC 9), 10=175(IC 9) Max Grav 2=1541(IC 1), 14=3190(IC 1), 10=588(IC 24) FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 2-21=647/3389, 9:10=47/3398, 15:19=-317/2312, 17:18=-36/1112, 14:16=-1263/420, 12:14=423/307, 10:12=447/3398, 19:21=647/3398, 19:21=647/3398, 19:21=647/3398, 19:21=647/3398, 19:19=-317/2312, 17:18=-36/1112, 14:16=-1263/420, 12:14=423/307, 10:12=447/3398, 19:21=647/3398, 9:12=0/441 NOTES- Setter S Ubalanced roof live loads have been consider lateral for this design. Winci: ASCE 7-10; Viller 130mph Vascl=0.60pst; bCDL=6.0pst; bCDL=6.0pst;	TOP CHORD 2x6 SI BOT CHORD 2x6 SI WEBS 2x4 SI	P No.2 P No.3 *Except*		TOP CHORD BOT CHORD	2-0-0 oc purlins (6-0- Rigid ceiling directly	0 max.): 6-7. applied or 6-0-0 oc braci	ing.			
 TOP CHORD 2-33627/819, 3-52541/599, 5-61394/433, 6-7589/348, 7-8734/318, 8-91851/1380, 9-10-470/471 BOT CHORD 2-21647/3389, 19-21-647/3389, 18-19317/2312, 17-1836/1112, 14-161263/420, 12-14-423/370, 10-12407/389, 18-19317/2312, 17-1836/1112, 14-161263/420, 12-14-423/370, 10-12407/389, 18-1936/112, 7-16360/2237, 8-142479/672, 9-141322/388, 9-12=0/441 NOTES- 2x6 SP No.2 bearing block 12" long at jt. 14 attached to front face with 3 rows of 10d (0.131*x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity= 425psi. Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10: Vulti-130mph Vasd=103mph; TOLD=6.0psf; h=-30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; c-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection gridesional for the design and inspection of the temporary installation restrain/bracing and the permanent individual truss member restrain/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing. Provide adequate drainage to prevent water ponding. This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. One H2 5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, and 10. This connector is for uplit only and does not consider lateral forces. This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. O	Max H Max U	Horz 2=-180(LC 13) Jplift 2=-241(LC 12), 14=-381(LC 9), 10=	175(LC 9)							
 2x6 SP No.2 bearing block 12" long at jt. 14 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity= 425psi. Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing. Provide adequate drainage to prevent water ponding. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. One H2:5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, and 10. This connection is for uplift only and does not consider lateral forces. Graphical putfin representation does not depict the size or the orientation of the putfin along the top and/or bottom chord. 	TOP CHORD 2-3= 8-9= BOT CHORD 2-21 12-1 WEBS 3-21 6-16	TOP CHORD 2-3=-3627/819, 3-5=-2541/599, 5-6=-1394/433, 6-7=-589/348, 7-8=-734/318, 8-9=-185/1380, 9-10=-470/471 BOT CHORD 2-21=-647/3389, 19-21=-647/3389, 18-19=-317/2312, 17-18=-36/1112, 14-16=-1263/420, 12-14=-423/370, 10-12=-407/389 WEBS 3-21=0/328, 3-19=-116/354, 5-19=-1/609, 5-18=-1508/430, 6-18=-163/1042, 6-16=-1011/177, 16-17=-36/1112, 7-16=-340/153, 8-16=-362/2237, 8-14=-2479/672,								
	 2x6 SP No.2 bearin fasteners. User Def Unbalanced roof liv Wind: ASCE 7-10; ' gable end zone and forces & MWFRS fc WARNING: This lo handling and erectit Trusses ("BCSI"), jc qualified registered permanent individus bracing. Provide adequate d This truss has beer will fit between the l One H2.5A Simpso This connection is f 	ined Bearing crushing capacity= 425psi. e loads have been considered for this de Vult=130mph Vasd=103mph; TCDL=6.0p I C-C Exterior(2) zone; cantilever left and or reactions shown; Lumber DOL=1.60 pl ng span truss requires extreme care and on guidance, see Guide to Good Practice inity produced by SBCA and TPI. The bi design professional for the design and ir al truss member restraint/bracing. MiTek rainage to prevent water ponding. designed for a 10.0 psf bottom chord liv en designed for a live load of 20.0psf on to bottom chord and any other members, w n Strong-Tie connectors recommended t or uplift only and does not consider laters	sign. st; BCDL=6.0psf; h=30ft; I right exposed ; end vertia ate grip DOL=1.60 experience for proper an a for Handling, Installing & uilding owner or the owne aspection of the temporary assumes no responsibilit the bottom chord in all are the BCDL = 10.0psf. o connect truss to bearing al forces.	Cat. II; Exp B; Enclosed cal left and right exposed d safe handling and erec Bracing of Metal Plate C 's authorized agent shal installation restraint/bra y for truss manufacture, any other live loads. as where a rectangle 3-6 walls due to UPLIFT at	; MWFRS (envelope) ;C-C for members and ttion. For general Connected Wood I contract with a cing and the handling, erection, or 6-0 tall by 2-0-0 wide jt(s) 2, 14, and 10.	Solution Solution	VGINEER HRUNN			

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

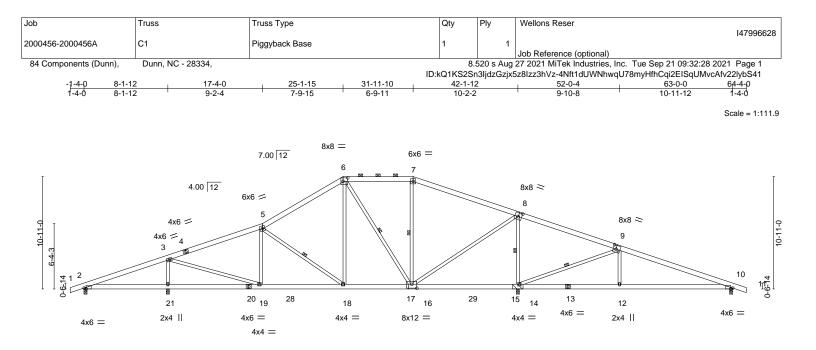
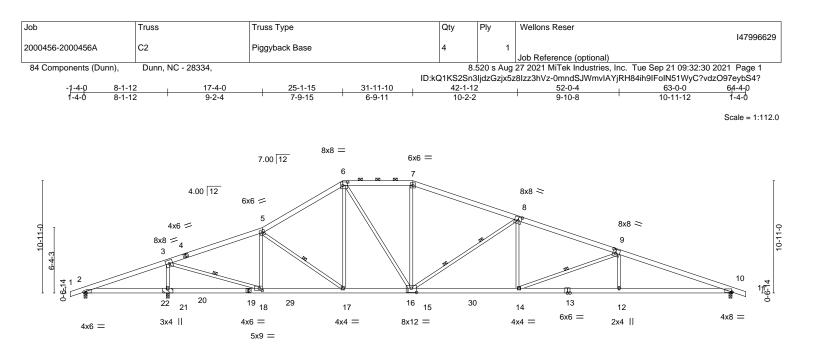


Plate Offsets (X,Y)- [2:0-2-6.Edge], [6:0-5-8,0-4-0], [8:0-4-0,0-4-8], [1:0-2-2,Edge], [1:0-6-0,0-4-8] LOADING (psf) TCLL SPACING- Lumber DOL 1:15 Lomber DOL SPACING- Lumber DOL 1:15 Lomber DOL DCSL Lumber DOL DEFL Lumber DOL in (loc) I/d off L/d MT20 PLATES GRIP MT20 BCLL 0.0 - Rep Stress incn YES WB 0.75 Matrix-MS DFL in (loc) I/d off L/d MT20 MT20 197/144 UMBER- TOP CHORD 2x6 SP No.2 BTACING- Streept* WB 0.75 Matrix-MS BTACING- TOP CHORD Structural wood sheathing directly applied or 5-7.8 oc purlins, except 2-0-0 co purlins (6-0-0 max.); 6-7. Weight: 448 lb FT = 20% LUMBER- TOP CHORD 2x6 SP No.2 or 2x4 SP FN 0.2 BOT CHORD Structural wood sheathing directly applied or 5-7.8 oc purlins, except 2-0-0 co purlins (6-0-0 max.); 6-7. WEES 1.8 wat midpt 5-18, 6-17, 7-16, 8-14, 9-14 REACTIONS. All bearings 0-3-8 except (ji=length) 14=0-3-15 (input: 0-3-8 + bearing block). INO INO 1.8 wat midpt 5-18, 6-17, 7-16, 8-14, 9-14 1.4 wat 0/0/0/0/0/0/0/0/0/0/0/0/0/0/0/0/0/0/0/		8-1-12	17-4-0 9-2-4	25-1-15 7-9-15	<u>31-11-10</u> 6-9-11	42-1-1 10-2-2			52-0-4 9-10-8	63-0-0	
LOADING (pst) TCLL SPACING- 20.0 TCLL 2-0-0 Plate Grip DOL T.15 1.15 Rep Stress Incr CSL VES DEFL Vert(L1) in (loc) Videfl L/d MT20 PLATES GRIP MT20 197/144 BCLL 0.0 - Rep Stress Incr YES WB 0.5 Vert(C1) 0.25 12.27 >996 180 BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Bracing No No Weight: 448 lb FT = 20% LUMBER- TOP CHORD 2x6 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-7-8 oc purlins, except 2-0:0 oc purlins (6-0-0 max): 6-7. Weight: 448 lb FT = 20% LUMBER- TOP CHORD 2x6 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-7-8 oc purlins, except 2-0:0 oc purlins (6-0-0 max): 6-7. Weight: 448 lb FT = 20% WEBS All bearings 0-3-8 except (Belth) 14=0-3-15 (input: 0-3-8 + bearing block). WEBS 1 Row at midpt 5-18, 6-17, 7-16, 8-14, 9-14 REACTIONS. All bearings 0-3-8 except (Menthyling and eling directly applied or 6-0 oc tracing. WEBS 1 Row at midpt 5-18, 6-17, 7-16, 8-14, 9-14 Max Care val Ireactions 250 (Ib or les									9-10-0	10-11-1	2
TOP CHORD BOT CHORD WEBS 2x6 SP No.2 2x6 SP No.3 'Except' 8-16: 2x4 SP No.3 'Except' 8-16: 2x4 SP No.2 or 2x4 SPF No.2 TOP CHORD WEBS Structural wood sheathing directly applied or 5-7-8 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-7. REACTIONS. Max Upit Mail upit 100 b or less at joint(s) except 2=-120(LC 8), 21=-271(LC 12), 14=-357(LC 9), 10=-184(LC 9) Max Corr All reactions 250 lb or less at joint(s) except 2=323(LC 23), 21=1672(LC 1), 14=-397(LC 9), 10=-184(LC 9) Max Grav In excentions 250 (lb or less at joint(s) except 2=323(LC 23), 21=1672(LC 1), 14=-397(LC 9), 10=-184(LC 9) Max Grav FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 9-10=-905/242	TCLL 20.0 TCDL 10.0 BCLL 0.0	SPACING Plate Grip Lumber D Rep Stress	- 2-0-0 DOL 1.15 DL 1.15 s Incr YES	CSI. TC 0.62 BC 0.57 WB 0.75	D V V	EFL. ir ert(LL) 0.12 ert(CT) -0.25	n (loc) 2 12-27 5 12-27	>999 >996	240 180	MT20	197/144
 (lb) - Max Horz 2=-180(LC 13) Max Uplif All uplif 100 b or less at joint(s) except 2=-120(LC 8), 21=-271(LC 12), 14=-357(LC 9), 10=-184(LC 9) Max Grav All reactions 250 b or less at joint(s) except 2=323(LC 23), 21=1672(LC 1), 14=2499(LC 1), 10=731(LC 24) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 3-5=-1454/369, 5-6=-1136/378, 6-7=-641/359, 7-8=-788/329, 8-9=-24/616, 9-10=-905/242 BOT CHORD 18-19=-151/1293, 17-18=0/883, 14-16=-527/264, 12-14=-117/782, 10-12=-110/801 WEBS 3-21=-1443/442, 3-19=-179/1375, 5-19=-257/165, 5-18=-524/221, 6-18=-41/543, 6-16=-496/92, 16-17=0/883, 7-16=-330/152, 8-16=-188/1413, 8-14=-1795/527, 9-14=-1312/394, 9-12=0/436 NOTES- Xot SP No.2 bearing block 12" long at jt. 14 attached to front face with 3 rows of 10d (0.131*x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity= 425psi. Unbalanced root live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph ty3d=-103mph; TCDL=-6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 WaWARNING: This long span truss requires exterme care and experience for proper and safe handling and erection. For general 	TOP CHORD 2x6 S BOT CHORD 2x6 S WEBS 2x4 S 8-16:	SP No.2 SP No.3 *Except* 2x4 SP No.2 or 2x		15 (input: 0.3-8 + be	T(B(W	OP CHORD	2-0-0 oc Rigid ce	c purlins (6- eiling direct	-0-0 max.): ly applied o	6-7. or 6-0-0 oc bracing.	
 NOTES- 1) 2x6 SP No.2 bearing block 12" long at jt. 14 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity= 425psi. 2) Unbalanced roof live loads have been considered for this design. 3) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 4) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general 	Max (Max (Max (FORCES. (lb) - Max TOP CHORD 3-5= 9-10 BOT CHORD 18-1 WEBS 3-21 6-16	Uplift All uplift 10(9) Grav All reactions 10=731(LC 2 k. Comp./Max. Ten. =-1454/369, 5-6=-1 0=-905/242 19=-151/1293, 17-1 1=-1443/442, 3-19= 6=-496/92, 16-17=() lb or less at joint(s) e s 250 lb or less at joint (4) - All forces 250 (lb) o 136/378, 6-7=-641/35 (8=0/883, 14-16=-527/ 179/1375, 5-19=-257 (883, 7-16=-330/152,	(s) except 2=323(LC r less except when s 9, 7-8=-788/329, 8-9 /264, 12-14=-117/78 7/165, 5-18=-524/22	C 23), 21=1672 shown. 9=-24/616, 32, 10-12=-110 1, 6-18=-41/54	(LC 1), 14=249 /801 3,		84(LC			
 bracing. 5) Provide adequate drainage to prevent water ponding. 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 21, 14, and 10. This connection is for uplift only and does not consider lateral forces. 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 	 NOTES- 1) 2x6 SP No.2 bearing fasteners. User De 2) Unbalanced roof ling 3) Wind: ASCE 7-10; gable end zone any forces & MWFRS fill (1998) 4) WARNING: This lid handling and erecting trusses ("BCSI"), j qualified registered permanent individual bracing. 5) Provide adequate to 6) This truss has been will fit between the will fit between the solution of the solution o	ng block 12" long a fined Bearing crust ve loads have beer Vult=130mph Vaso d C-C Exterior(2) z for reactions shown ong span truss required ion guidance, see (jointly produced by d design profession ual truss member re drainage to prevent n designed for a 10 en designed for a 10 bottom chord and on Strong-Tie conn for uplift only and c	t jt. 14 attached to from ning capacity= 425psi. considered for this de 103mph; TCDL=6.0 one; cantilever left and ; Lumber DOL=1.60 p uires extreme care and Guide to Good Practic SBCA and TPI. The b al for the design and in estraint/bracing. MiTel t water ponding. 0.0 psf bottom chord liv ive load of 20.0psf on any other members, w ectors recommended loes not consider later	esign. psf; BCDL=6.0psf; h d right exposed ; end late grip DOL=1.60 d experience for proj e for Handling, Insta uilding owner or the nspection of the term k assumes no respo ve load nonconcurre the bottom chord in ith BCDL = 10.0psf. to connect truss to b al forces.	a=30ft; Cat. II; I d vertical left a per and safe h illing & Bracing owner's autho aporary installa insibility for true ant with any oth all areas wher bearing walls d	Exp B; Enclosed and right exposed of Metal Plate rized agent sha ion restraint/bra ss manufacture, er live loads. e a rectangle 3- ue to UPLIFT at	t; MWFRS d;C-C for r connected Il contract acing and handling, 6-0 tall by jt(s) 2, 21	6 (envelope members a general d Wood with a the erection, o 2-0-0 wide	and or		925 VEER. KAUN

818 Soundside Road Edenton, NC 27932

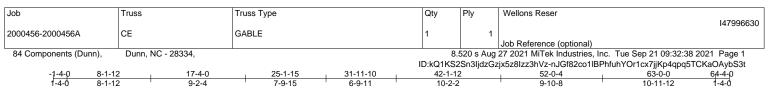
Design valid for use only with MITeK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



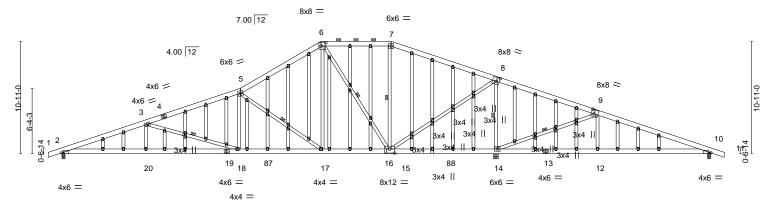
Print Description Parts End offsets (XP) P2-45.6 (PA) P2-41.1 (PA) P2-42.6 (PA) P2-43.6 (PA)		3-1-12 17-4-0	<u>25-1-15</u> <u>31-1</u>		52-0-4	63-0-0
TCLL 20.0 Plate Grip DOL 1.15 TC 0.08 Vert(L1) 0.02 1.15 MT20 197/144 ECLL 0.0 Rep Stress Incr YES WB 0.97 Marix-MS Weight: 448 lb FT = 20% LUMBER- TOP CHORD 2x6 SP No.2 BCAL TOP CHORD 2x6 SP No.2 Structural wood sheathing directly applied, except SDT CHORD 2x6 SP No.2 BCT CHORD 3x18.6-15: 2x4 SP No.2 BCT CHORD 1.8-16, 19: 2x6 SP No.2 BCT CHORD 1.80 xt ating directly applied or 1.4-12 co bracing, 13x16, 5-17, 9-14 3x18.6-15: 2x4 SP No.2 BCT CHORD 1.8 ov at indipt 3x18, 5-17, 9-14 VEES 2x8 SP No.3 Ft copt BCT CHORD 1.8 ov at indipt 3x18, 5-17, 9-14 REACTIONS. (sizp) 2-0-33, 211(-0.3-8 Hearing blockl (reg. 0-4-14), 10=0-3-8 Max Horz 8-15 Max Horz 2-100 Cop Linght Structural wood sheathing directly applied or 1.4-12 to bracing, 1.8 ov at 1/3 pts 8-15 FORCES. (b) - Max. Comp.Max. Ten All forces 250 (b) or less except when show. 1.7 ov at 1/3 pts 8-15 DTO CHORD 221-281/573, 231, 18-21-13/32284, 16-17210/2208, 14-15=-691/3989, 12/1-4=-103/4986, 5-18=-947/301, 6-15=-200/77, 7-8=-2						10-11-12
TOP CHORD 2x6 SP No.2 TOP CHORD Structural wood sheathing directly applied, except BOT CHORD 2x6 SP No.5 Texcept' BOT CHORD Provide and the structural wood sheathing directly applied, except WEBS 2x4 SP No.1 Strengt' BOT CHORD WEBS 13-16,1-10: 2x6 SP No.2 WEBS 2x4 SP No.3 Texcept' BOT CHORD WEBS 3-18,5-17, 9-14 3-18,5-15: 2x4 SP No.2 2x0 wo at midpt' 3-18,5 5-17, 9-14 WEBS 2x0 wo at midpt' 3-18,5 5-17, 9-14 Wax forz 2-49(LC 2a), 21288(LC 12), 10-398(LC 0) Max Grav 2-48(LC 2a), 21289(LC 2a), 21289(LC 1) 8-15 FORCES (b) - Max Corn - Mitorea 250 (b) or less except when shown. TOP CHORD 23-245(130g, 3-5-248/456), 5-6-2673704, 6-7-2809777, 7-9-28537768, 8-9-4184/1020; 9-10-6304/1206 8-13 BOT CHORD 23-248/130g, 3-5-2484/560, 5-6-2673704, 6-15-214/871, 15-691/389, 12-14-103/4896, 10-12-1023/5001 12-14-1031/4896, 10-12-1023/5001 12-14-1031/4896, 10-12-1023/5001 WEBS 3-12-281(LC 2a), 21150/3457, 5-16-9-495, 18,0-1-6-195, 14-001 3-0.5.12 12-14 WEBS 12-14-1031/4896, 10-12-19/366, 6-15=-214/871, 15-169/450, 8-14=0773, 9-14=-19/366, 6-15=-214/871, 15-16=-210/22306, 7-14-19/366, 6-15=-214/871, 15-16 12-14-1031/4896, 10-12-1023/5001 12-14-	TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	TC 0.96 BC 1.00 WB 0.97	Vert(LL) -0.32 14- Vert(CT) -0.68 14-	-15 >999 240 -15 >962 180	MT20 197/144
 Max hoirz 2=-180(LC 13) Max Grav 2=-49(LC 23), 21=-288(LC 12), 10=-386(LC 0) Max Grav 2=-49(LC 23), 21=-3099(LC 1), 10=2196(LC 1) FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 2:-3=-245/1309, 3:-5=:248/586, 5:-6=-2673/704, 6:-7=:2609/777, 7:-8=-2653/768, 8:-9=-4184/1002, 9:-10=:5364/1206 BOT CHORD 2:-21=:1153/323, 18:-21=-1153/323, 17:-18=:313/2284, 16:17=:-210/2208, 14:15=:-691/3898, 12:-14=:103/14986, 10:-12=:-1023/001 WEBS 3:-21=:2615/739, 3:18=-656/3575, 5:18=:-945/313, 6:17==-19/366, 6:15==:214/871, 15:16=:210/2208, 7:15=:0/481, 8:15=:-1559/450, 8:14=:0/773, 9:14=:-1192/366, 9:12=:0/4206 NOTES- Uxbalanced roof live loads have been considered for this design. Wind: Acts F-10: Will=130mph Yasch-103mph; TCDL=6 Opsf; B:CDL=6 Opsf; h=:30ft; Cat. II; Exp B: Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; c: C1 or members and forces & MWFRS for reactions shown; Lumber DOL=:1.60 Opsf; B:CDL=6.0 psf; h=:30ft; Cat. II; Exp B: Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; c: G1 or members and forces & MWFRS for reactions shown; Lumber DOL=:1.60 Opsf; B:CDL=6.0 psf; h=:30ft; Cat. II; Exp B: Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; c: G1 or members and forces & MWFRS for reactions gain truss requires exterme care and experimente for proper and safe handling and erection. For general handling and erection, guidance, see Guide to Good Practice for Plandling, Installing & Bracing of Metal Plate Connectd Wood Trusses (FBSCT), jointly produced by SECA and TPI. The building nower or the owner's suthnized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restrain/bracing and the permanent Individual truss member restrain/bracing. MTek assumes	TOP CHORD 2x6 SF BOT CHORD 2x6 SF 13-16, WEBS 2x4 SF	P DSS *Except* ,16-19: 2x6 SP No.2 P No.3 *Except*		TOP CHORD Str 2-0 BOT CHORD Rig WEBS 1 F	D-0 oc purlins (4-4-10 max.) gid ceiling directly applied o Row at midpt 3-	: 6-7. r 1-4-12 oc bracing. 18, 5-17, 9-14
 TOP CHORD 2-3=-245/1309, 3-5=-248/4586, 5-6=-26737704, 6-7=-2609/777, 7-8=-2853/768, 8-9=-4184/1002, 9-10=-5364/1206 BOT CHORD 2-21=-1153/323, 18-21=-1153/323, 17-18=-313/2284, 16-17=-210/2208, 14-15=-691/3898, 12-14=-1031/4986, 10-12=-1023/5001 WEBS 3-21==2315/739, 3-18=-656/3757, 5-18=-945/313, 6-17=-19/366, 6-15=-214/871, 15-16=-210/2208, 7-15=0/481, 8-15=-1559/450, 8-14=0/773, 9-14=-1192/366, 9-12=-0/406 NOTES- 1) 2x6 SP DSS bearing block 12" long at jt. 21 attached to front face with 3 rows of 10d (0.131*x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity=425psi. 2) Unbalanced roof live loads have been considered for this design. 3) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; -c0 for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 4) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection, goldrance, see Guide to Good Practice for Handling, Installation Restanit/bracing and the permanent Individual truss member restanit/bracing. MTek assumes no responsibility for truss manufacture, handling, erection, or bracing. 6) Provide adequate drainage to prevent water ponding. 6) This truss has been designed for a 10.0 p5 bottom chord live load nonconcurrent with any other live loads. 7) "This truss has been designed for a 10.0 p5 bottom chord live load nonconcurrent with any other live loads. 7) "This truss has been designed for a 10.0 p5 bottom chord live load nonconcurrent with any other live loads. 8) One H2.5A Simpson Strong-Tie connector streommended to connect truss to bearing walls due to UPLIFT at j(s) 2, 21, and 10. This connection is for uplift only and does not consider lateral forces.	Max H Max L	Horz 2=-180(LC 13) Jplift 2=-251(LC 24), 21=-288(LC 12), 10	=-396(LC 9)			
 2x6 SP DSS bearing block 12" long at jt. 21 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity= 425psi. Uhbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing. Provide adequate drainage to prevent water ponding. This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 21, and 10. This connection is for uplit only and does not consider lateral forces. 	TOP CHORD 2-3= 8-9= BOT CHORD 2-21 12-1 12-1 WEBS 3-21 15-1 15-1	-245/1309, 3-5=-2484/586, 5-6=-2673/7(-4184/1002, 9-10=-5364/1206 =-1153/323, 18-21=-1153/323, 17-18=-3 14=-1031/4986, 10-12=-1023/5001 =-2815/739, 3-18=-656/3575, 5-18=-945, 6=-210/2208, 7-15=0/481, 8-15=-1559/4)4, 6-7=-2609/777, 7-8=-2 13/2284, 16-17=-210/220 /313, 6-17=-19/366, 6-15:	2853/768, 8, 14-15=-691/3898, =-214/871,		
	 2x6 SP DSS bearin fasteners. User Def Unbalanced roof livv Wind: ASCE 7-10; V gable end zone and forces & MWFRS fc WARNING: This lo handling and erectic Trusses ("BCSI"), jc qualified registered permanent individua bracing. Provide adequate d This truss has been will fit between the M One H2.5A Simpson This connection is fit 	The Bearing crushing capacity= 425psi. e loads have been considered for this de Vult=130mph Vasd=103mph; TCDL=6.0p d C-C Exterior(2) zone; cantilever left and or reactions shown; Lumber DOL=1.60 pl ng span truss requires extreme care and on guidance, see Guide to Good Practice pintly produced by SBCA and TPI. The bi design professional for the design and in al truss member restraint/bracing. MiTek rainage to prevent water ponding. or designed for a 10.0 psf bottom chord liv bottom chord and any other members, win n Strong-Tie connectors recommended to or uplift only and does not consider latera	sign. sisf; BCDL=6.0psf; h=30ft; right exposed ; end vertii ate grip DOL=1.60 experience for proper an for Handling, Installing & uilding owner or the owne spection of the temporary assumes no responsibilit e load nonconcurrent with he bottom chord in all are th BCDL = 10.0psf. o connect truss to bearing al forces.	Cat. II; Exp B; Enclosed; MV cal left and right exposed;C-C d safe handling and erection Bracing of Metal Plate Conr r's authorized agent shall con v installation restraint/bracing by for truss manufacture, han any other live loads. eas where a rectangle 3-6-0 t g walls due to UPLIFT at jt(s)	WFRS (envelope) C for members and For general nected Wood ntract with a g and the idling, erection, or all by 2-0-0 wide 2, 21, and 10.	M. SEL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Scale = 1:111.9



		8-1-12	9-2-	4	7-9-15	6	5-9-11	10-2-2	1	9-10-8	10-11-1	2
Plate Offset	s (X,Y)	[2:0-1-6,Edg [74:0-1-12,0		-4-0], [8:0-		3-4,0-4-12]	, [10:0-2-2,Edge], [1		2], [43:0-1		:0-1-12,0-1-8], [71:0-1-	
	(psf) 20.0 10.0 0.0 *	Plate Lumb	CING- Grip DOL ber DOL Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.74 0.70 0.98	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.18 18-20 -0.39 18-20 0.07 14	>999 >999	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
	10.0		e IRC2015/TF		Matri		1012(01)	0.07	r n/a	174	Weight: 640 lb	FT = 20%
LUMBER- TOP CHOR BOT CHOR	D 2x6 S	SP No.2 SP No.2					BRACING TOP CHO	RD Struc 2-0-0	oc purlins	(6-0-0 max.):		oc purlins, excep
WEBS OTHERS	8-15:	SP No.3 *Exce 2x4 SP No.2 SP No.3		lo.2			BOT CHO WEBS		w at midpt		or 6-0-0 oc bracing. -18, 5-17, 6-16, 7-15, 8	3-14, 9-14

42-1-12

52-0-4

31-11-10

25-1-15

REACTIONS. (size) 2=0-3-8, 14=0-5-8, 10=0-3-8 Max Horz 2=-180(LC 13) Max Uplift 2=-241(LC 12), 14=-381(LC 9), 10=-175(LC 9) Max Grav 2=1541(LC 1), 14=3190(LC 1), 10=588(LC 24)

8-1-12

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-3627/819, 3-5=-2541/599, 5-6=-1394/433, 6-7=-589/348, 7-8=-734/318, 8-9=-185/1380, 9-10=-470/471

 BOT CHORD
 2-20=-647/3389, 18-20=-647/3389, 17-18=-317/2312, 16-17=-36/1112, 14-15=-1263/420, 12-14=-423/370, 10-12=-407/389

 WEBS
 3-20=0/328, 3-18=-1116/354, 5-18=-1/609, 5-17=-1508/430, 6-17=-163/1042, 6-15=-1011/177, 15-16=-36/1112, 7-15=-340/153, 8-15=-362/2237, 8-14=-2479/672, 9-14=-1332/398, 9-12=0/441

17-4-0

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



63-0-0



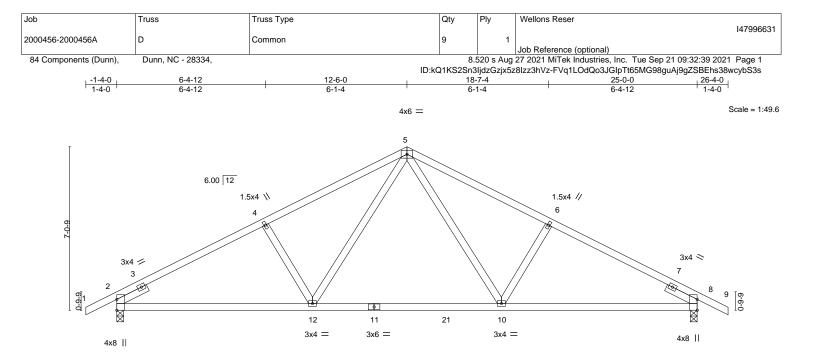
Job	Truss	Truss Type	Qty	Ply	Wellons Reser
					147996630
2000456-2000456A	CE	GABLE	1	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:38 2021 Page 2
		I	D:kQ1KS2	2Sn3ljdzGz	;jx5z8lzz3hVz-nJGf82co1lBPhfuhYOr1cx7jjKp4qpq5TCKaOAybS3t

10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, and 10. This connection is for uplift only and does not consider lateral forces.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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F	8-5-3 8-5-3			16-6-13 8-1-11			25-0-0 8-5-3		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Plate Grip DOL Lumber DOL	2-0-0 CSI 1.15 TC 1.15 BC YES WB	0.76 0.75 0.21	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.23 10-12 -0.38 10-12 0.06 8	l/defl >999 >791 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2	-	rix-MS	BRACING		174	in a	Weight: 122 lb	FT = 20%

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.3 WEBS SLIDER Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-9-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

- REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=115(LC 12) Max Uplift 2=-144(LC 12), 8=-144(LC 13) Max Grav 2=1080(LC 1), 8=1080(LC 1)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-4=-1546/405, 4-5=-1387/425, 5-6=-1387/425, 6-8=-1546/405

BOT CHORD 2-12=-245/1312, 10-12=-79/926, 8-10=-250/1312

WFBS 5-10=-111/495. 6-10=-315/227. 5-12=-111/495. 4-12=-315/227

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

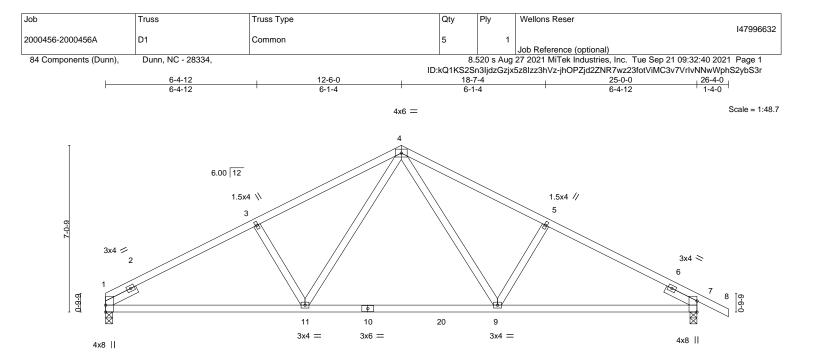
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING (psf) SPACING- 2-0-0 CSI.	DEFL. in (loc	c) l/defl L/d	PLATES GF	
TCLL 20.0 Plate Grip DOL 1.15 TC 0.76 TCDL 10.0 Lumber DOL 1.15 BC 0.75 BCLL 0.0 * Rep Stress Incr YES WB 0.21	Vert(LL) -0.23 9-1 Vert(CT) -0.37 9-1 Horz(CT) 0.06			97/144
BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS	BRACING-		Weight: 120 lb	FT = 20%

TOP CHORD

BOT CHORD

REACTIONS.

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.3 WEBS SLIDER Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0

(size) 1=0-3-8, 7=0-3-8

Max Horz 1=-126(LC 13) Max Uplift 1=-116(LC 12), 7=-145(LC 13) Max Grav 1=998(LC 1), 7=1082(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-3=-1558/411, 3-4=-1399/430, 4-5=-1391/428, 5-7=-1550/408 TOP CHORD

- BOT CHORD 1-11=-256/1325, 9-11=-82/930, 7-9=-253/1316
- WFBS 4-9=-110/495, 5-9=-315/227, 4-11=-115/506, 3-11=-322/229

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 7. This connection is for uplift only and does not consider lateral forces.

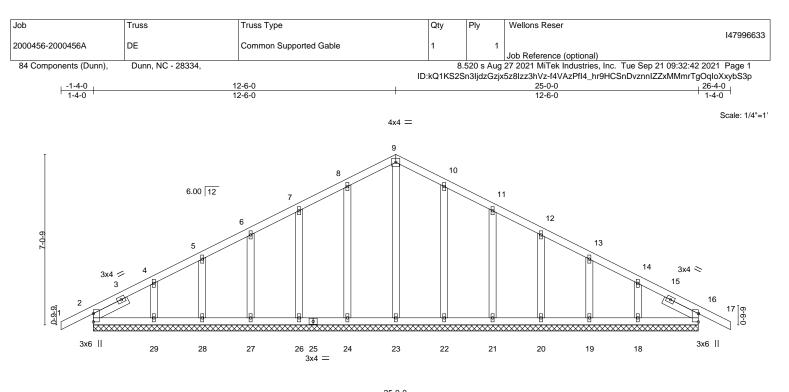


Structural wood sheathing directly applied or 2-9-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road Edenton, NC 27932



			25-0-0 25-0-0					
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.11	Vert(LL) -0.00	· · /	n/r	120	MT20	197/144
CDL 10.0	Lumber DOL 1.15	BC 0.04	Vert(CT) -0.00	17	n/r	90		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.11	Horz(CT) 0.00	16	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 150 lb	FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 OTHERS
 2x4 SP No.3

 SLIDER
 Left 2x4 SP No.3 1-7-8, Right 2x4 SP No.3 1-7-8

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 25-0-0.

(lb) - Max Horz 2=115(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 2, 24, 26, 27, 28, 29, 22, 21, 20, 19, 18, 16

Max Grav All reactions 250 lb or less at joint(s) 2, 23, 24, 26, 27, 28, 29, 22, 21, 20, 19, 18, 16

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

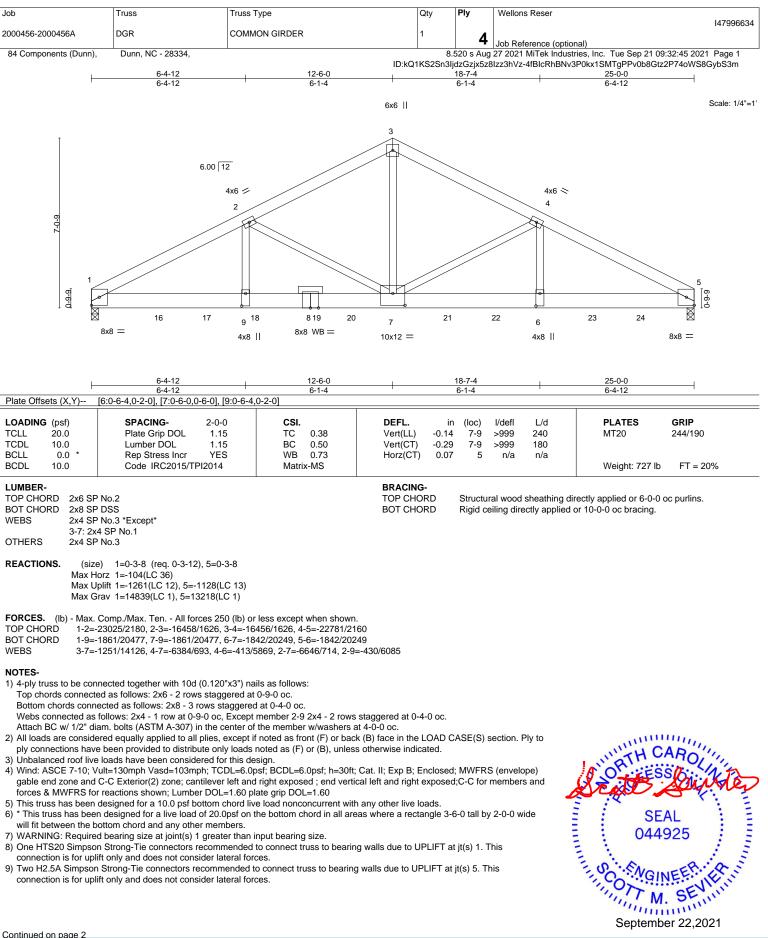
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Wellons Reser
					147996634
2000456-2000456A	DGR	COMMON GIRDER	1	1	
				4	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:46 2021 Page 2
			D:kQ1KS2	Sn3ljdzGz	x5z8lzz3hVz-Yrlhpnip8DBGeuWD03_vxdSALYc6iVfGISG?giybS3l

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2191 lb down and 185 lb up at 0-9-4, 2189 lb down and 188 lb up at 2-9-4, 2189 lb down and 188 lb up at 6-9-4, 2189 lb down and 188 lb up at 8-9-4, 2189 lb down and 188 lb up at 10-9-4, 2189 lb down and 188 lb up at 12-9-4, 2189 lb down and 188 lb up at 14-9-4, 2189 lb down and 188 lb up at 16-9-4, 2189 lb down and 188 lb up at 18-9-4, and 2189 lb down and 188 lb up at 18-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4, and 2189 lb down and 188 lb up at 20-9-4.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

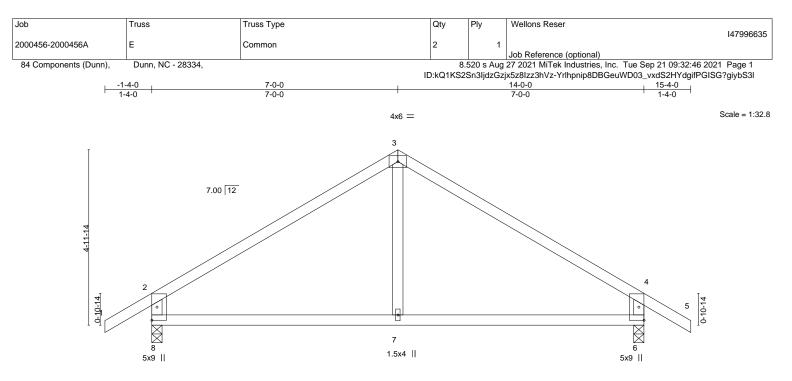
Vert: 1-3=-60, 3-5=-60, 10-13=-20 Concentrated Loads (lb)

Vorte C 2171(E

Vert: 6=-2171(F) 7=-2171(F) 12=-2174(F) 16=-2171(F) 17=-2171(F) 18=-2171(F) 19=-2171(F) 20=-2171(F) 21=-2171(F) 22=-2171(F) 23=-2171(F) 24=-2171(F) 24=-217(F) 24=-217(F) 24=-217(F) 24=-217(F) 24=-217(F) 24=-217(F) 24=-217(F) 24=-217(F) 24=-217(F)

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		7-0-0 7-0-0	14-0-0 7-0-0
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.83 BC 0.40 WB 0.11 Matrix-MR	DEFL. in (loc) l/defl L/d Vert(LL) -0.05 6-7 >999 240 MT20 197/144 Vert(CT) -0.10 6-7 >999 180 MT20 197/144 Horz(CT) 0.01 6 n/a n/a Weight: 58 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF	No.2 or 2x4 SPF No.2	·	BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.3 WEBS

REACTIONS. 8=0-3-8, 6=0-3-8 (size) Max Horz 8=-145(LC 10) Max Uplift 8=-92(LC 12), 6=-92(LC 13) Max Grav 8=637(LC 1), 6=637(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-612/126, 3-4=-612/126, 2-8=-570/208, 4-6=-570/208 TOP CHORD

BOT CHORD 7-8=0/435, 6-7=0/435

WEBS 3-7=0/291

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

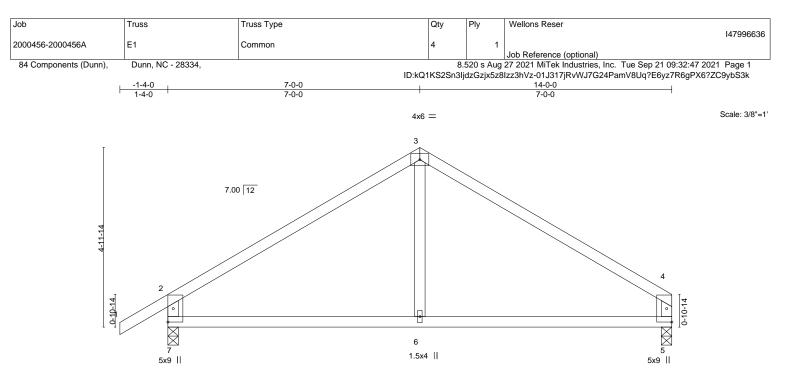
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 6. This connection is for uplift only and does not consider lateral forces.



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		7-0-0 7-0-0				14-0-0 7-0-0		1
TCLL 20.0 Plate TCDL 10.0 Luml	CING- 2-0-0 Grip DOL 1.15 er DOL 1.15	CSI. TC 0.83 BC 0.39	DEFL. in Vert(LL) -0.05 Vert(CT) -0.11	(loc) 6-7 6-7	l/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 197/144
	Stress Incr YES IRC2015/TPI2014	WB 0.11 Matrix-MR	Horz(CT) 0.01	5	n/a	n/a	Weight: 56 lb	FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2
WEBS	2x4 SP No.3

REACTIONS. 7=0-3-8, 5=0-3-8 (size) Max Horz 7=138(LC 9) Max Uplift 7=-92(LC 12), 5=-59(LC 13) Max Grav 7=642(LC 1), 5=544(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-3=-619/129, 3-4=-612/127, 2-7=-572/208, 4-5=-469/137 TOP CHORD

BOT CHORD 6-7=-34/435, 5-6=-34/435

WEBS 3-6=0/285

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.



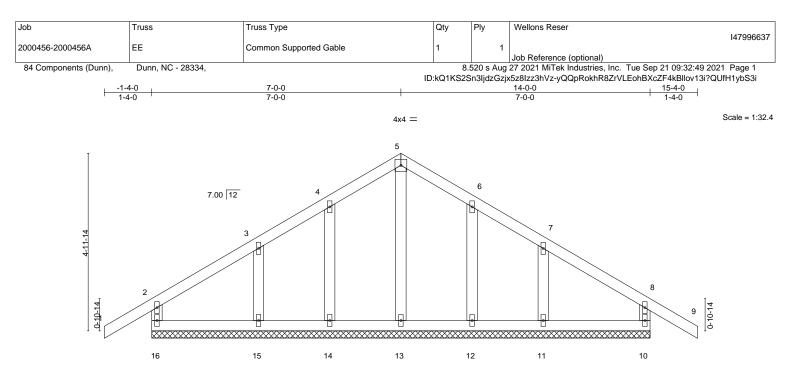
Structural wood sheathing directly applied or 5-3-13 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. it	n (loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL) -0.01	9	n/r	120	MT20	197/144
CDL 10.0	Lumber DOL 1.15	BC 0.06	Vert(CT) -0.01	9	n/r	90		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT) 0.00	10	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-R					Weight: 74 lb	FT = 20%

LU	JM	BE	:R-	•
				-

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 14-0-0.

Max Horz 16=-145(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11

Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

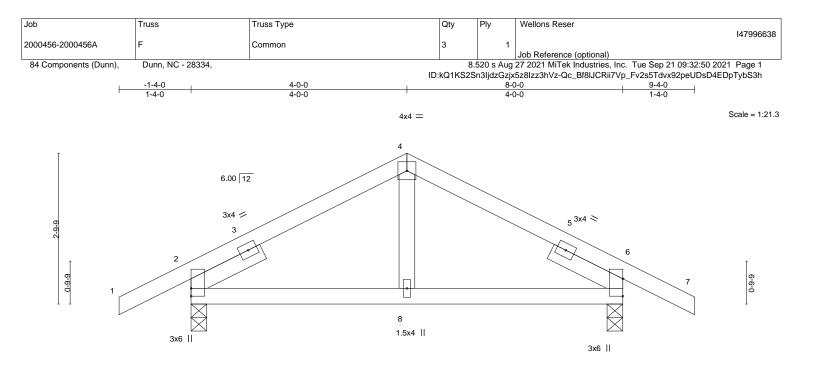
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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			4-0-0 4-0-0			+ <u>8-0-0</u> 4-0-0						
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.15	TC	0.15	Vert(LL)	-0.01	8-15	>999	240	MT20	197/144	
TCDL 10.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	-0.01	8-15	>999	180			
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.06	Horz(CT)	-0.00	2	n/a	n/a			
BCDL 10.0	Code IRC2015	/TPI2014	Matri	x-MP						Weight: 38 lb	FT = 20%	

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

 SLIDER
 Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=-45(LC 13) Max Uplift 2=-65(LC 12), 6=-65(LC 13) Max Grav 2=400(LC 1), 6=400(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-4=-277/128, 4-6=-277/128

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

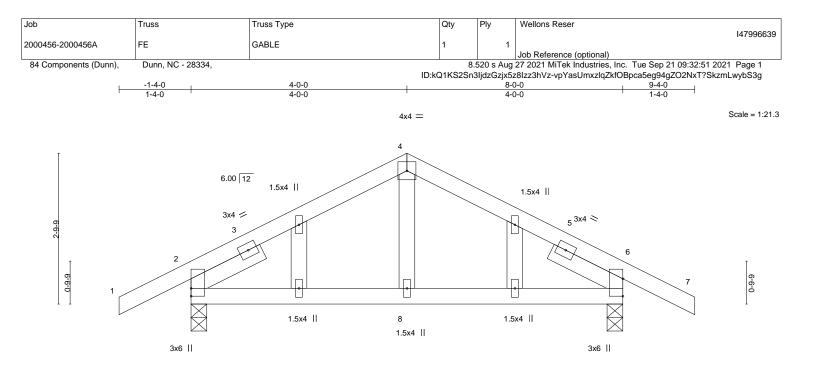
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



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			4-0-0 4-0-0					-0-0 -0-0			
LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC		DEFL. Vert(LL)	in -0.01	(loc)	l/defl >999	L/d 240	PLATES MT20	GRIP 197/144
TCDL 10.0	Lumber DOL	1.15	BC (0.13	Vert(CT)	-0.01		>999	180	WI 20	197/144
BCLL 0.0 * BCDL 10.0	Rep Stress Incr Code IRC2015		WB (Matrix-l		Horz(CT)	-0.00	2	n/a	n/a	Weight: 41 lb	FT = 20%



TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=-45(LC 13) Max Uplift 2=-65(LC 12), 6=-65(LC 13) Max Grav 2=400(LC 1), 6=400(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-277/128, 4-6=-277/128

NOTES-

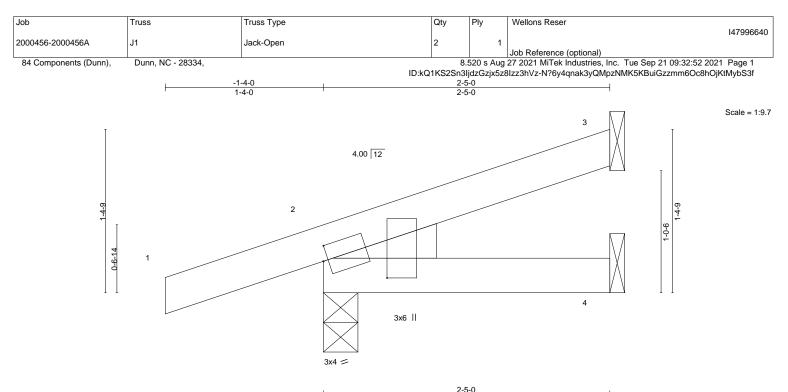
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



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¹⁾ Unbalanced roof live loads have been considered for this design.



			2-5-0	—
Plate Offsets (X,Y)	[2:0-0-8,0-1-8], [2:0-1-11,0-6-7]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.12	Vert(LL) -0.00 7 >999 240	MT20 197/144
TCDL 10.0	Lumber DOL 1.15	BC 0.04	Vert(CT) -0.00 7 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 2 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP		Weight: 11 lb $FT = 20\%$

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEDGE
 2x4 SP No.2 or 2x4 SPF No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-5-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Left: 2x4 SP No.3

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=53(LC 8) Max Uplift 3=-21(LC 12), 2=-76(LC 8) Max Grav 3=47(LC 1), 2=197(LC 1), 4=37(LC 3)

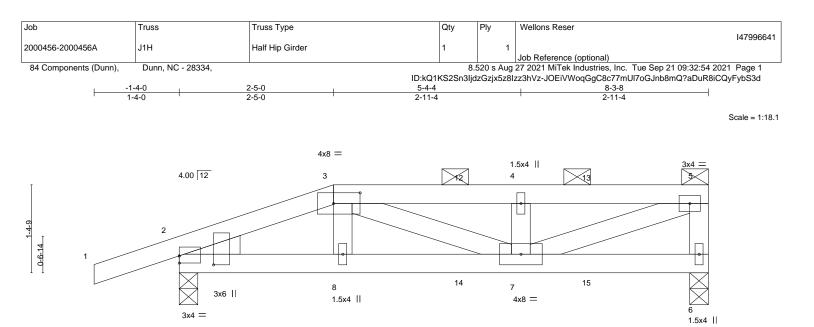
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 4) Relet to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.







	I	2-5-0	I	2-11-4	1		2-11-4	I
Plate Offsets (X,Y)	[2:0-1-11,0-6-7], [2:0-0-0,	0-1-5], [3:0-5-0	,0-2-0]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 NO Pl2014	CSI. TC 0.14 BC 0.18 WB 0.27 Matrix-MP	DEFL. ir Vert(LL) -0.01 Vert(CT) -0.02 Horz(CT) 0.00	7 >999 7-8 >999	L/d 240 180 n/a	PLATES MT20 Weight: 40 lb	GRIP 197/144 FT = 20%
	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 P No.3			BRACING- TOP CHORD BOT CHORD	except end vert	ticals, and 2-0-0	ctly applied or 6-0-0) oc purlins (6-0-0 m · 10-0-0 oc bracing.	

5-4-4

REACTIONS. (size) 6=0-3-8, 2=0-3-8

Max Horz 2=51(LC 11) Max Uplift 6=-66(LC 9), 2=-118(LC 8) Max Grav 6=340(LC 1), 2=422(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-540/181, 3-4=-607/238, 4-5=-607/238, 5-6=-307/137

- BOT CHORD 2-8=-205/492, 7-8=-201/496
- WEBS 5-7=-252/651

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

2-5-0

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 19 lb down and 49 lb up at 2-5-0, and 19 lb down and 46 lb up at 4-5-12, and 22 lb down and 52 lb up at 6-5-12 on top chord, and 8 lb down and 0 lb up at 2-5-12, and 8 lb down and 0 lb up at 4-5-12, and 17 lb down and 16 lb up at 6-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 6-9=-20

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



8-3-8

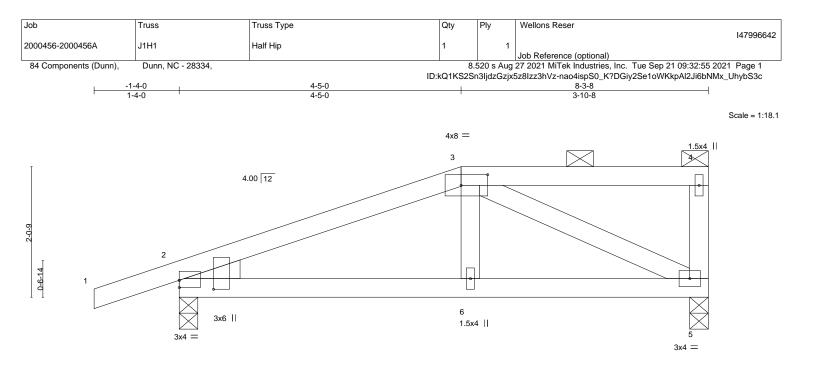


Job	Truss	Truss Type	Qty	Ply	Wellons Reser
					147996641
2000456-2000456A	J1H	Half Hip Girder	1	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:54 2021 Page 2
		ID:kQ1	KS2Sn3ljc	IzGzjx5z8l	zz3hVz-JOEiVWoqGgC8c77mUl7oGJnb8mQ?aDuR8iCQyFybS3d

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 8=-5(B) 13=-3(B) 14=-5(B) 15=-17(B)





		<u>4-5-0</u> 4-5-0		<u>8-3-8</u> 3-10-8	
Plate Offsets (X,Y)	[2:0-0-0,0-1-5], [2:0-1-11,0-6-7], [3:0-5-	-0,0-2-0]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.27 BC 0.19 WB 0.14 Matrix-MP	DEFL. in Vert(LL) -0.01 Vert(CT) -0.02 Horz(CT) 0.00		PLATES GRIP MT20 197/144 Weight: 38 lb FT = 20%
	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 P No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals, and 2-0 Rigid ceiling directly applied	•
Max L	ee) 2=0-3-8, 5=0-3-8 Horz 2=79(LC 11) Jplift 2=-113(LC 8), 5=-59(LC 8) Grav 2=412(LC 1), 5=319(LC 1)				
TOP CHORD 2-3=	. Comp./Max. Ten All forces 250 (lb) o -436/144 -177/379. 5-6=-174/388	r less except when shown.			

BOT CHORD 2-6=-177/379, 5-6=-174/388

WEBS 3-5=-429/170

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

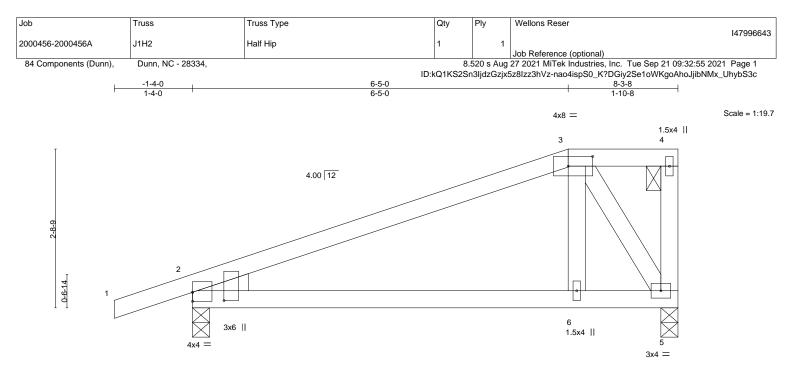
6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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	(nsf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.53	Vert(LL)	0.06	6-9	>999	240	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.47	Vert(CT)	-0.11	6-9	>931	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.11	Horz(CT)	0.02	2	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matrix	K-MP						Weight: 39 lb	FT = 20%

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2		except end verticals, and 2-0-0 oc purlins: 3-4.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEDGE			

Left: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 5=0-3-8 Max Horz 2=107(LC 11)

Max Uplift 2=-110(LC 8), 5=-62(LC 8) Max Grav 2=412(LC 1), 5=319(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-299/92

 WEBS
 3-6=-11/276, 3-5=-420/168

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and

forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Provide adequate drainage to prevent water ponding.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

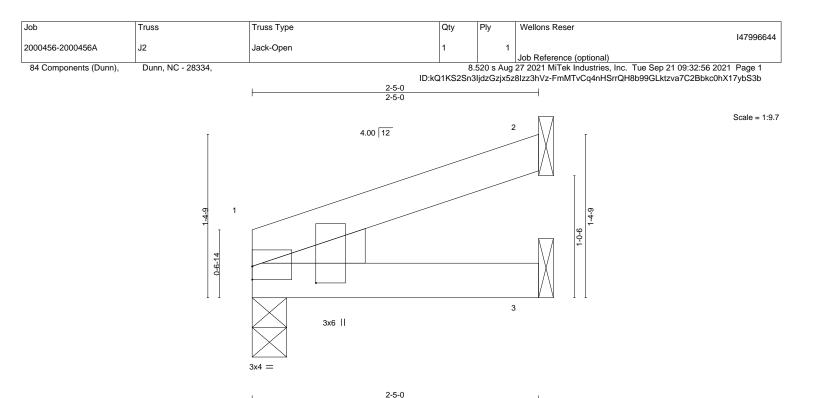
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







2-5-0

LUMBER	۹-		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-MP
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00
TCDL	10.0	Lumber DOL 1.15	BC 0.07
TCLL	20.0	Plate Grip DOL 1.15	TC 0.06

2-0-0

[1:0-0-0,0-1-5], [1:0-1-11,0-6-7]

SPACING-

BRACING-TOP CHORD BOT CHORD

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

(loc)

6 >999

6 >999

1

l/defl

n/a

L/d

240

180

n/a

in

-0.00

-0.00

0.00

Structural wood sheathing directly applied or 2-5-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

PLATES

Weight: 9 lb

MT20

GRIP

197/144

FT = 20%

REACTIONS. (size) 2=Mechanical, 3=Mechanical, 1=0-3-8 Max Horz 1=30(LC 8) Max Uplift 2=-24(LC 8), 3=-1(LC 8), 1=-9(LC 8)

Max Grav 2=57(LC 1), 3=41(LC 3), 1=94(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

WEDGE Left: 2x4 SP No.3

Plate Offsets (X,Y)--

LOADING (psf)

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

CSI.

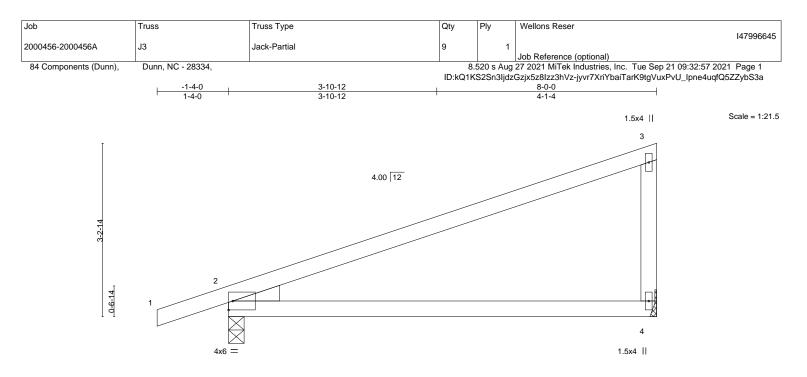
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 3) will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.

BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This
- connection is for uplift only and does not consider lateral forces.



818 Soundside Road Edenton, NC 27932



	8-0-0 8-0-0										
LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC	0.96	DEFL. Vert(LL)	in 0.20	(loc) 4-7	l/defl >483	L/d 240	PLATES MT20	GRIP 197/144
TCDL 10.0	Lumber DOL	1.15	BC	0.75	Vert(CT)	-0.39	4-7	>243	180	WI 20	137/144
BCLL 0.0 * BCDL 10.0	Rep Stress Incr Code IRC2015/TP	YES 912014	WB Matri	0.05 x-MP	Horz(CT)	0.05	2	n/a	n/a	Weight: 32 lb	FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

 WEDGE
 Left: 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=Mechanical Max Horz 2=124(LC 8) Max Uplift 2=-94(LC 8), 4=-76(LC 12) Max Grav 2=401(LC 1), 4=307(LC 1)

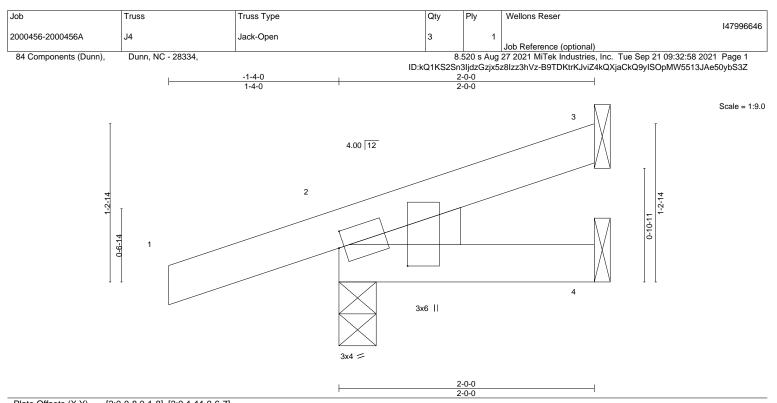
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces.







OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.12	Vert(LL) -0.00	7 >999	240	MT20	197/144
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) -0.00	7 >999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00	2 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP				Weight: 10 lb	FT = 20%

BOT CHORD

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEDGE

Left: 2x4 SP No.3

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=48(LC 8) Max Uplift 3=-15(LC 12), 2=-76(LC 8) Max Grav 3=34(LC 1), 2=186(LC 1), 4=30(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

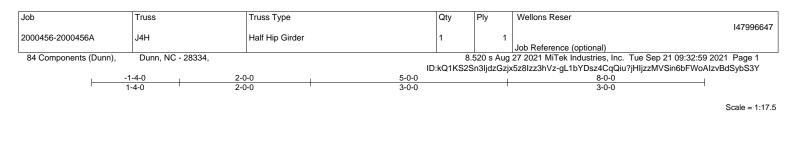
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





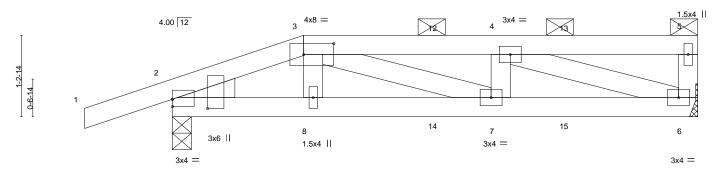


Plate Offsets (X,Y)			4-10-4 2-10-4		8-0-0 3-1-12	———————————————————————————————————————
Plate Offsets (X, Y)	[2:0-1-11,0-6-7], [2:0-0-0,0-1-5], [3:0-5-8	3,0-2-0]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO	CSI. TC 0.15 BC 0.22 WB 0.16	DEFL. in Vert(LL) -0.01 Vert(CT) -0.02 Horz(CT) 0.01	7 >999 2 7-8 >999 1	L/d PLATES 140 MT20 80 n/a	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP	. ,		Weight: 38 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP WEDGE Left: 2x4 SP No.3	No.2 or 2x4 SPF No.2	I	BRACING- TOP CHORD BOT CHORD	except end vertical	eathing directly applied or 6-0- s, and 2-0-0 oc purlins (6-0-0 r y applied or 10-0-0 oc bracing.	nax.): 3-5.
Max He Max U	e) 2=0-3-8, 6=Mechanical orz 2=45(LC 11) plift 2=-114(LC 8), 6=-56(LC 9) rav 2=405(LC 1), 6=312(LC 1)					
TOP CHORD 2-3=- BOT CHORD 2-8=-	Comp./Max. Ten All forces 250 (lb) or 506/173, 3-4=-629/244 191/461, 7-8=-187/465, 6-7=-254/629 663/256	less except when shown.				
 2) Wind: ASCE 7-10; V gable end zone and forces & MWFRS for 3) Provide adequate dr. 4) This truss has been 5) * This truss has been 6) Refer to girder(s) for 7) Provide mechanical 8) One H2.5A Simpson connection is for upli 9) Graphical purlin repr 10) Hanger(s) or other 2-0-0, and 13 lb do lb down at 4-0-12, responsibility of oth 	e loads have been considered for this de ult=130mph Vasd=103mph; TCDL=6.0p C-C Exterior(2) zone; cantilever left and r reactions shown; Lumber DOL=1.60 pl ainage to prevent water ponding. designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on to ottom chord and any other members. truss to truss connections. connection (by others) of truss to bearin Strong-Tie connectors recommended t ift only and does not consider lateral for esentation does not depict the size or th connection device(s) shall be provided a wn and 38 lb up at 4-0-12, and 13 lb dc and 5 lb down at 6-0-12 on bottom cho ers. (S) section, loads applied to the face of	bsf; BCDL=6.0psf; h=30ft; C I right exposed ; end vertica ate grip DOL=1.60 e load nonconcurrent with a he bottom chord in all areas g plate capable of withstam o connect truss to bearing w ces. the orientation of the purlin a sufficient to support concen wm and 38 lb up at 6-0-12 rd. The design/selection of	I left and right exposed any other live loads. s where a rectangle 3-1 ding 100 lb uplift at joir walls due to UPLIFT at long the top and/or boi trated load(s) 13 lb doo on top chord, and 5 lb such connection device	t;C-C for members at 6-0 tall by 2-0-0 wide nt(s) 6. jt(s) 2. This ttom chord. wn and 41 lb up at down at 2-0-12, and	nd ORTHE S	CAROLINE SSIGN EAL 4925
LOAD CASE(S) Stand 1) Dead + Roof Live (ba	dard alanced): Lumber Increase=1.15, Plate	Increase=1.15			in the N	M. SEVILLI

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



September 22,2021

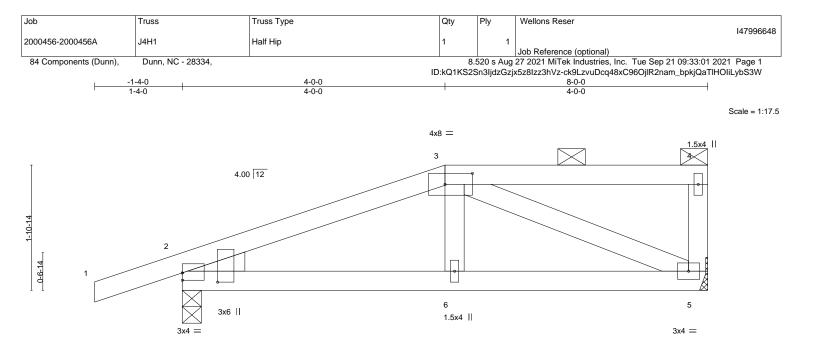
Job	Truss	Truss Type	Qty	Ply	Wellons Reser
					147996647
2000456-2000456A	J4H	Half Hip Girder	1	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 21 09:32:59 2021 Page 2

ID:kQ1KS2Sn3ljdzGzjx5z8lzz3hVz-gL1bYDsz4CqQiu?jHIjzzMVSin6bFWoAlzvBdSybS3Y

LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 6-9=-20 Concentrated Loads (lb) Vert: 8=-3(B) 14=-3(B) 15=-3(B)





	I.		4-0-0		1			8-0-0		1
			4-0-0		1			4-0-0		
Plate Offsets (X,Y)	[2:0-0-0,0-1-5], [2:0-1-11	,0-6-7], [3:0-5-0),0-2-0]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	2-0-0 1.15 1.15 YES PI2014	CSI. TC 0.29 BC 0.17 WB 0.14 Matrix-MP	DEFL Vert(L Vert(C Horz(C	Ý) -0.02	5-6 5-6	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 37 lb	GRIP 197/144 FT = 20%
BOT CHORD 2x4 S WEBS 2x4 S WEDGE Left: 2x4 SP No.3 REACTIONS. (siz Max I Max I	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 P No.3 ze) 2=0-3-8, 5=Mechani Horz 2=73(LC 11) Uplift 2=-112(LC 8), 5=-56 Grav 2=401(LC 1), 5=307	(LC 8)		BRAC TOP C BOT C	HORD	except	end vert	cals, and 2-	directly applied or 6-0-0 -0-0 oc purlins: 3-4. d or 10-0-0 oc bracing.) oc purlins,
TOP CHORD 2-3= BOT CHORD 2-6=	:. Comp./Max. Ten All fo 433/144 176/379, 5-6=-173/388 421/167	rces 250 (lb) or	less except when s	hown.						
gable end zone and	Vult=130mph Vasd=103m d C-C Exterior(2) zone; ca or reactions shown; Lumb	ntilever left and	right exposed ; end							

2) Provide adequate drainage to prevent water ponding.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.

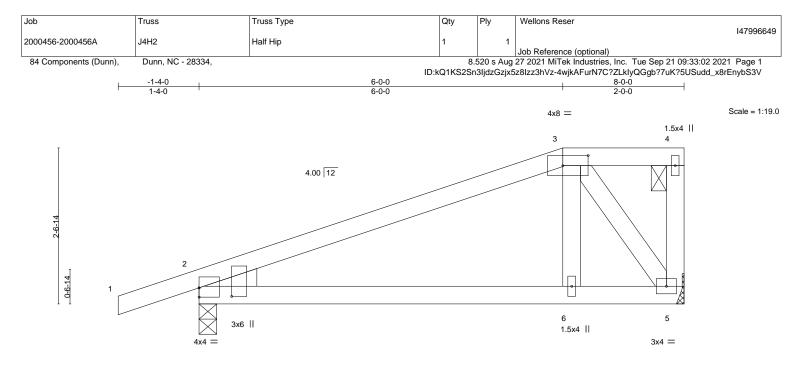
One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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A MiTek Affiliate B18 Soundside Road Edenton, NC 27932



	(nsf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.45	Vert(LL)	-0.04	6-9	>999	240	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.40	Vert(CT)	-0.08	6-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.01	2	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matrix	k-MP	. ,					Weight: 37 lb	FT = 20%

TOP CHORD2x4 SP No.2 or 2x4 SPF No.2TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins,
except end verticals, and 2-0-0 oc purlins: 3-4.BOT CHORD2x4 SP No.3BOT CHORDRigid ceiling directly applied or 10-0-0 oc bracing.WEDGE

Left: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 5=Mechanical

Max Horz 2=101(LC 11) Max Uplift 2=-109(LC 8), 5=-59(LC 8) Max Grav 2=401(LC 1), 5=307(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-299/95

 WEBS
 3-5=-392/161

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Provide adequate drainage to prevent water ponding.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.

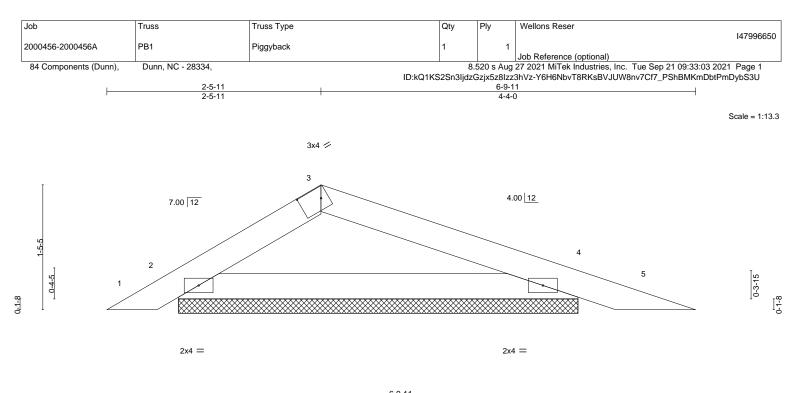
7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This

connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







LOADING (psf) TCLL 20.0 TCDL 10.0	Plate Grip DOL 1 Lumber DOL 1	-0-0 CSI. 1.15 TC 1.15 BC	0.13 0.27	DEFL. Vert(LL) Vert(CT)	0.00 0.01	(loc) 5 5	l/defl n/r n/r	L/d 120 90	PLATES MT20	GRIP 197/144
3CLL 0.0 * 3CDL 10.0	Rep Stress Incr Y Code IRC2015/TPI20	YES WB 14 Matrix	0.00 x-P	Horz(CT)	0.00	4	n/a	n/a	Weight: 18 lb	FT = 20%

BOT CHORD

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

REACTIONS. (size) 2=4-7-7, 4=4-7-7 Max Horz 2=-38(LC 10) Max Uplift 2=-28(LC 12), 4=-66(LC 9) Max Grav 2=214(LC 1), 4=242(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5)

will fit between the bottom chord and any other members. 6) N/A

7) N/A

8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



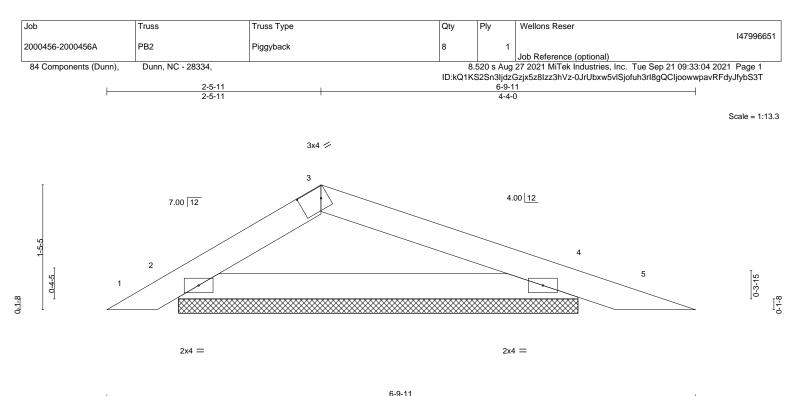


Plate Offsets (X,Y)-	[3:0-3-0.0-1-8]		6-9-11					-
	[3:0-3-0;0-1-0]							
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.13	Vert(LL) 0.00	5	n/r	120	MT20	197/144
CDL 10.0	Lumber DOL 1.15	BC 0.27	Vert(CT) 0.01	5	n/r	90		
SCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00	4	n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 18 lb	FT = 20%

BOT CHORD

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

REACTIONS. 2=4-7-7, 4=4-7-7 (size) Max Horz 2=-38(LC 10) Max Uplift 2=-28(LC 12), 4=-66(LC 9) Max Grav 2=214(LC 1), 4=242(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members.

6) N/A

7) N/A

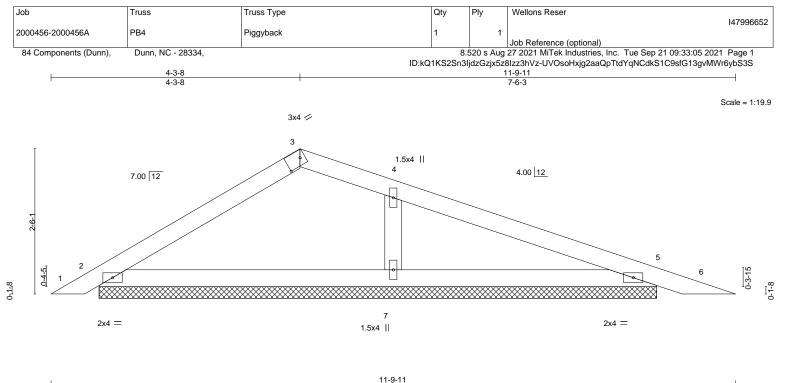
8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





DADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
LL 2	20.0	Plate Grip DOL	1.15	тс	0.22	Vert(LL)	0.01	6	n/r	120	MT20	197/144
DL 1	10.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	0.02	6	n/r	90		
CLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	5	n/a	n/a		
DL 1	10.0	Code IRC2015/TF	PI2014	Matrix	k-S						Weight: 35 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 OTHERS
 2x4 SP No.3

REACTIONS. (size) 2=9-7-7, 5=9-7-7, 7=9-7-7

Max Horz 2=-69(LC 10) Max Uplift 2=-55(LC 12), 5=-62(LC 9), 7=-64(LC 13)

Max Grav 2=244(LC 23), 5=241(LC 1), 7=382(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 4-7=-255/173

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

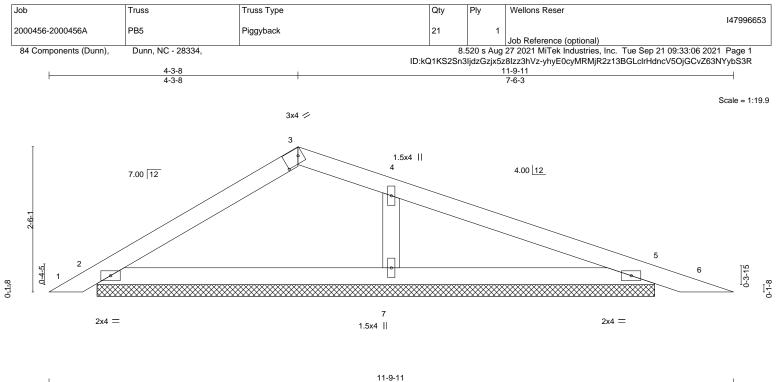
6) N/A

7) N/A

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







ADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
LL 20.0	Plate Grip DOL 1.15	TC 0.22	Vert(LL) 0.	.01 6	n/r	120	MT20	197/144
DL 10.0	Lumber DOL 1.15	BC 0.23		.02 6	n/r	90	-	
LL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT) 0.	.00 5	n/a	n/a		
DL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 35 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 OTHERS
 2x4 SP No.3

REACTIONS. (size) 2=9-7-7, 5=9-7-7, 7=9-7-7

Max Horz 2=-69(LC 10) Max Uplift 2=-55(LC 12), 5=-62(LC 9), 7=-64(LC 13)

Max Grav 2=244(LC 23), 5=241(LC 1), 7=382(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 4-7=-255/173

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

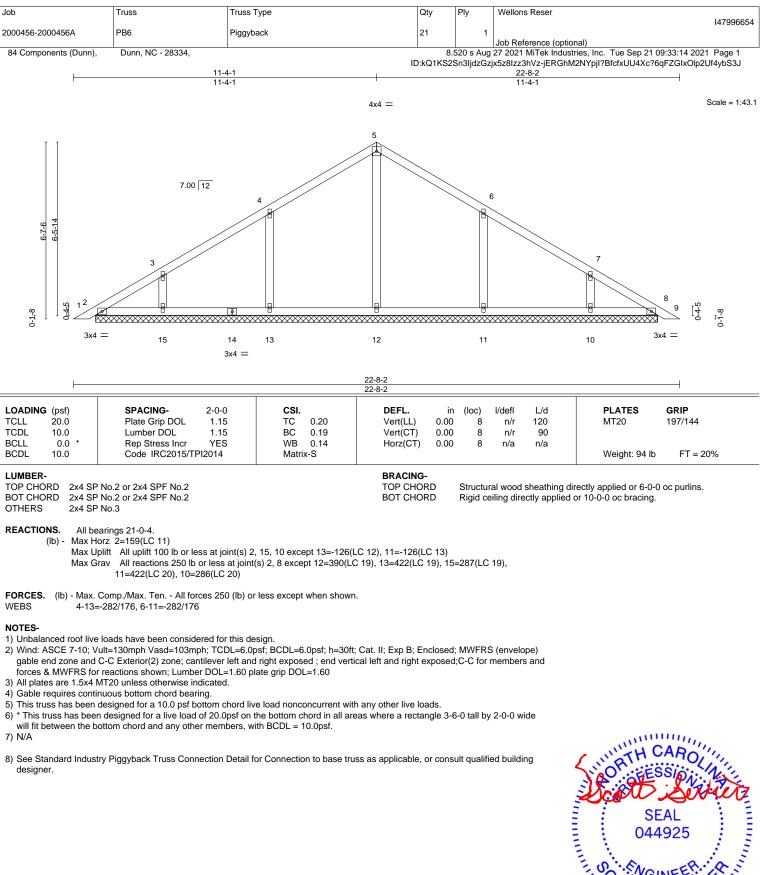
6) N/A

7) N/A

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

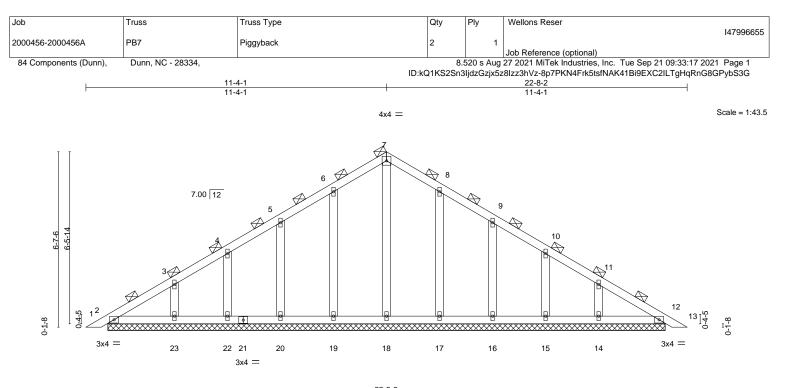












			22-8-2 22-8-2		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-2-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.06 WB 0.10 Matrix-S	DEFL. Vert(LL) 0.0 Vert(CT) 0.0 Horz(CT) 0.0	0 13 n/r 90	PLATES GRIP MT20 197/144 Weight: 115 lb FT = 20%
	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 P No.3	11	BRACING- TOP CHORD BOT CHORD	2-0-0 oc purlins (6-0-0 max.) (Switched from sheeted: Spa Rigid ceiling directly applied	, acing > 2-0-0).

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

REACTIONS. All bearings 21-0-4.

Max Horz 2=-173(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 23, 16, 14, 22, 19, 17, 15

Max Grav All reactions 250 lb or less at joint(s) 2, 18, 20, 23, 16, 14, 12, 22, 19, 17, 15

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 1.5x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

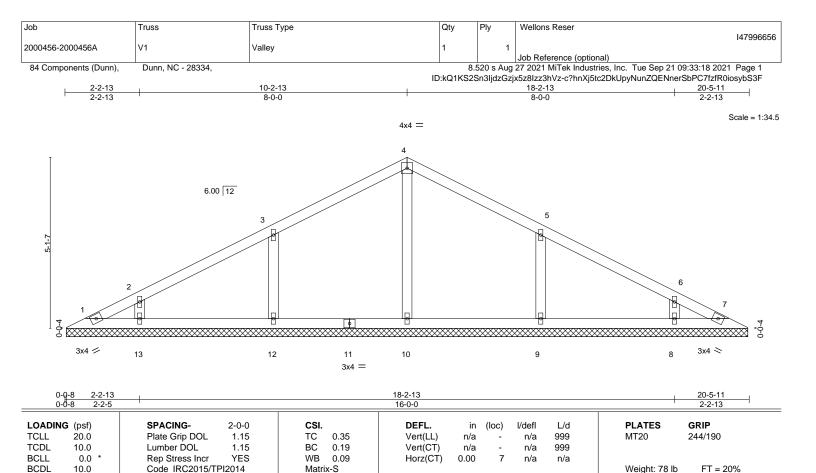
7) N/A

8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD 2x4 SP No.3 WEBS OTHERS 2x4 SP No.3

10.0

REACTIONS. All bearings 20-4-11.

Max Horz 1=80(LC 16) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 8 except 12=-112(LC 12), 9=-112(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=271(LC 1), 12=351(LC 23), 9=351(LC 24), 13=261(LC 1), 8=261(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-12=-268/180, 5-9=-268/180

NOTES-

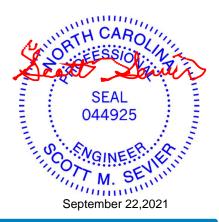
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 1.5x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

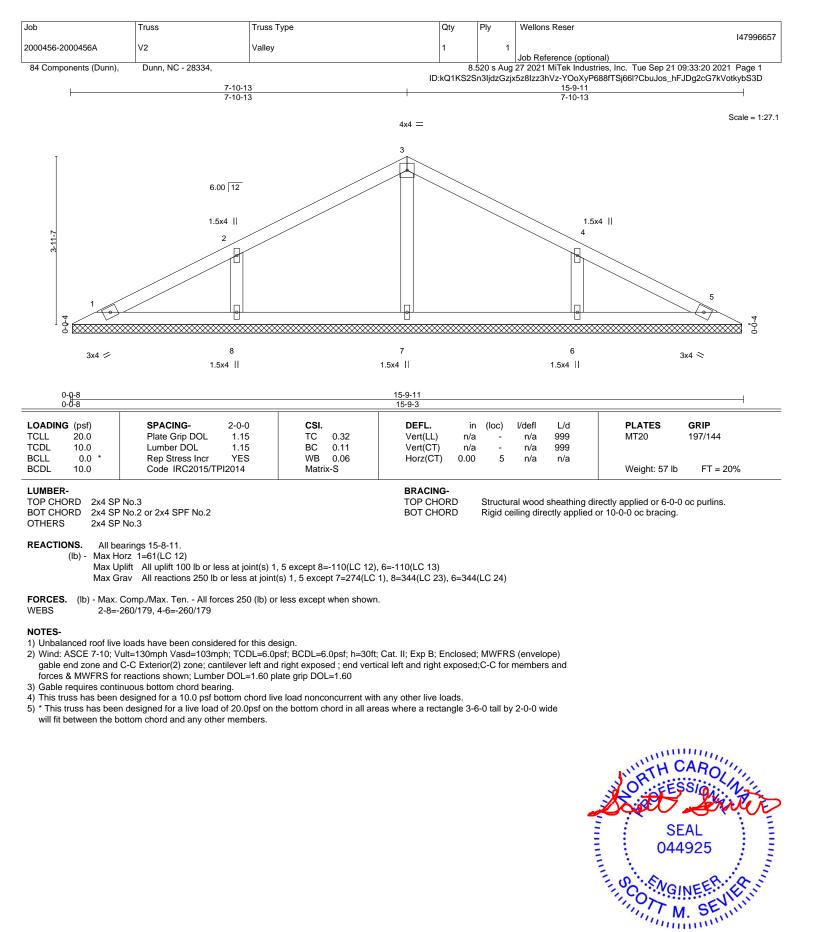
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



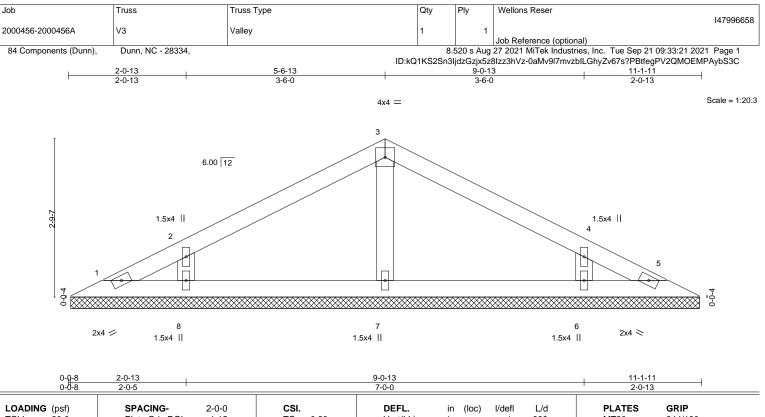


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September 22,2021





TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI	1.15 1.15 YES	TC 0.23 BC 0.16 WB 0.05 Matrix-S	Vert(LL) Vert(CT) Horz(CT)	n/a n/a 0.00	(10C) - - 5	n/a n/a n/a n/a	999 999 n/a	Weight: 38 lb	244/190 FT = 20%
BOT CHORD 2x	4 SP No.3 4 SP No.3 1 SP No.3			BRACING- TOP CHOR BOT CHOR					rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins.

2x4 SP No.3 VEBS OTHERS 2x4 SP No.3

REACTIONS. All bearings 11-0-11. (lb) -

Max Horz 1=41(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 1, 6, 8

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=252(LC 1), 6=258(LC 24), 8=258(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

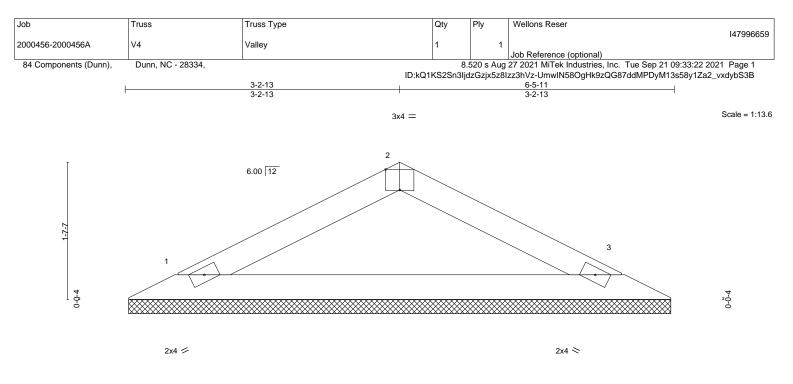
3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







	0- <u>0-8</u> 0-0-8				6-5-11 6-5-3						
Plate Offsets (X,Y)	[2:0-2-0,Edge]										
OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC	0.59	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matrix	к-Р						Weight: 18 lb	FT = 20%

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (size) 1=6-4-11, 3=6-4-11 Max Horz 1=-22(LC 13) Max Uplift 1=-25(LC 12), 3=-25(LC 13) Max Grav 1=208(LC 1), 3=-208(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

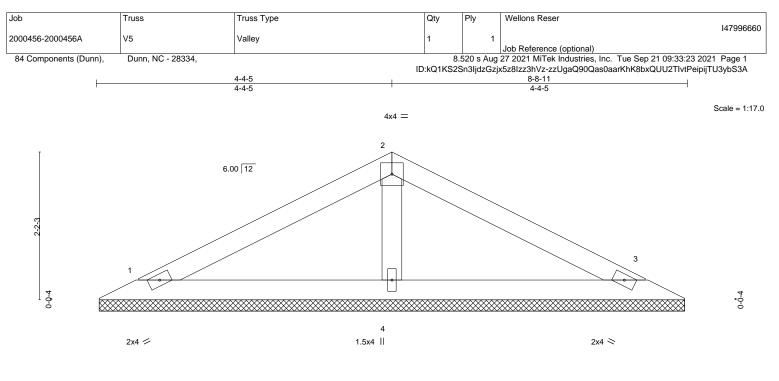
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





	0- <u>0-8</u> 0-0-8				8-8-11 8-8-3						
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip Do		TC	0.44	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL		BC	0.24	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 * BCDL 10.0	Rep Stress In Code IRC20		WB Matrix	0.04 -P	Horz(CT)	0.00	3	n/a	n/a	Weight: 28 lb	FT = 20%
LUMBER-	I		1		BRACING-					1	

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

2x4 SP No.3 OTHERS

REACTIONS. 1=8-7-11, 3=8-7-11, 4=8-7-11 (size) Max Horz 1=31(LC 12) Max Uplift 1=-35(LC 12), 3=-41(LC 13), 4=-1(LC 12) Max Grav 1=151(LC 1), 3=151(LC 1), 4=296(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

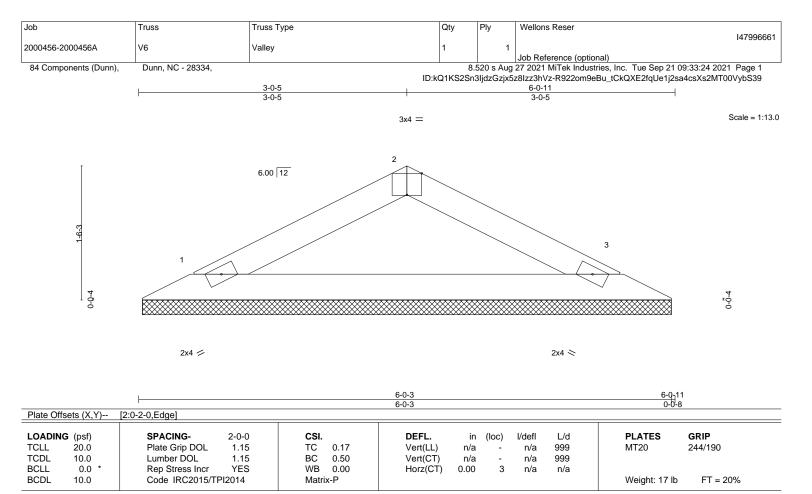
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (size) 1=5-11-11, 3=5-11-11 Max Horz 1=-20(LC 13) Max Uplift 1=-23(LC 12), 3=-23(LC 13) Max Grav 1=192(LC 1), 3=192(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



