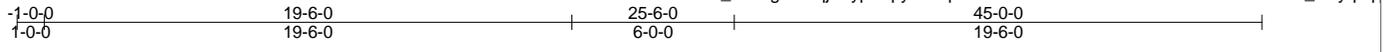


Job 72285969	Truss A1	Truss Type GABLE	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton					Job Reference (optional) 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:52:11 2022 Page 1
ID:w7S_7?4ogSiYqjEuyP5BpyU8dq-11kwxE8UJRvNHLGfxRl8HAsrQT5EQlPrDa_fkoyzqzj					



Scale = 1:84.7

LOADING (psf)		SPACING-		CSL		DEFL.		PLATES		GRIP	
TCLL (roof)	20.0	Plate Grip DOL	2-0-0	TC	0.08	Vert(LL)	0.00	in (loc)	l/def	L/d	
Snow (Ps/Pg)	15.4/20.0**	Lumber DOL	1.15	BC	0.04	Vert(CT)	-0.00	A	n/r	120	MT20
TCDL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(CT)	0.01	AN	n/a	n/a	244/190
BCLL	0.0 *	Code IRC2015/TPI2014		Matrix-SH							
BCDL	10.0										Weight: 493 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
OTHERS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-6-7, Right 2x4 SP No.3 1-11-4

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.); S-X.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt U-BF, T-BG, R-BH, Q-BI, P-BJ, O-CL, V-BE, W-BD, Y-BC, Z-BB, AA-BA, AB-AY

REACTIONS. (lb/size) B=131/45-0-0 (min. 0-6-4), BF=108/45-0-0 (min. 0-6-4), BG=105/45-0-0 (min. 0-6-4), BH=94/45-0-0 (min. 0-6-4), BI=94/45-0-0 (min. 0-6-4), BJ=94/45-0-0 (min. 0-6-4), BL=94/45-0-0 (min. 0-6-4), BM=94/45-0-0 (min. 0-6-4), BN=94/45-0-0 (min. 0-6-4), BO=94/45-0-0 (min. 0-6-4), BP=94/45-0-0 (min. 0-6-4), BQ=94/45-0-0 (min. 0-6-4), BR=94/45-0-0 (min. 0-6-4), BS=94/45-0-0 (min. 0-6-4), BT=95/45-0-0 (min. 0-6-4), BU=92/45-0-0 (min. 0-6-4), BV=104/45-0-0 (min. 0-6-4), BE=108/45-0-0 (min. 0-6-4), BD=105/45-0-0 (min. 0-6-4), BC=94/45-0-0 (min. 0-6-4), BB=94/45-0-0 (min. 0-6-4), BA=94/45-0-0 (min. 0-6-4), AN=63/45-0-0 (min. 0-6-4), AY=94/45-0-0 (min. 0-6-4), AX=94/45-0-0 (min. 0-6-4), AW=94/45-0-0 (min. 0-6-4), AV=94/45-0-0 (min. 0-6-4), AU=94/45-0-0 (min. 0-6-4), AT=94/45-0-0 (min. 0-6-4), AS=95/45-0-0 (min. 0-6-4), AR=94/45-0-0 (min. 0-6-4), AQ=97/45-0-0 (min. 0-6-4), AP=85/45-0-0 (min. 0-6-4), AO=127/45-0-0 (min. 0-6-4)

Max Horz B=221(LC 13)
Max Uplift B=-56(LC 10), BF=-27(LC 10), BG=-10(LC 11), BI=-39(LC 14), BJ=-41(LC 14), BL=-37(LC 14), BM=-38(LC 14), BN=-38(LC 14), BO=-38(LC 14), BP=-38(LC 14), BQ=-38(LC 14), BR=-38(LC 14), BS=-37(LC 14), BT=-41(LC 14), BU=-22(LC 14), BV=-125(LC 14), BE=-26(LC 10), BD=-8(LC 11), BB=-40(LC 15), BA=-41(LC 15), AN=-12(LC 13), AY=-37(LC 15), AX=-38(LC 15), AW=-38(LC 15), AV=-38(LC 15), AU=-38(LC 15), AT=-38(LC 15), AR=-37(LC 15), AQ=-38(LC 15), AP=-34(LC 15), AO=-102(LC 15)
Max Grav B=197(LC 30), BF=148(LC 36), BG=139(LC 36), BH=132(LC 51), BI=148(LC 37), BJ=146(LC 37), BL=145(LC 37), BM=145(LC 37), BN=145(LC 37), BO=145(LC 37), BP=147(LC 37), BQ=130(LC 37), BR=107(LC 52), BS=107(LC 2), BT=107(LC 52), BU=105(LC 2), BV=148(LC 28), BE=148(LC 36), BD=139(LC 36), BC=117(LC 51), BB=148(LC 37), BA=146(LC 37), AN=136(LC 15), AY=145(LC 37), AX=145(LC 37), AW=145(LC 37), AV=145(LC 37), AU=147(LC 37), AT=130(LC 37), AS=107(LC 53), AR=106(LC 53), AQ=109(LC 2), AP=97(LC 53), AO=152(LC 29)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/6, B-C=303/157, C-D=288/159, D-E=226/135, E-F=197/131, F-G=163/125, G-H=130/118, H-BW=-114/109, I-BW=103/112, I-J=104/112, J-K=93/116, K-L=96/135, L-BX=91/150, M-BX=79/154, M-N=117/173, N-O=143/192, O-P=169/210, P-Q=196/230, Q-R=223/258, R-S=202/233, S-T=-206/244, T-U=-206/244, U-V=-206/244, V-W=-206/244, W-X=-206/244, X-Y=-202/233, Y-Z=-223/258, Z-AA=-196/194, AA-AB=-169/194, AB-AC=-143/162, AC-AD=-117/131, AD-BY=-85/100, AE-BY=-91/96, AE-AF=-65/74, AF-AG=-27/55, AG-AH=-50/51, AH-BZ=-33/43, AI-BZ=-52/39, AI-AJ=-69/48, AJ-AK=-102/55, AK-AL=-135/78, AL-AM=-169/103, AM-AN=-242/162
BOT CHORD B-BV=-142/217, BU-BV=-142/217, BT-BU=-142/217, BS-BT=-142/217, BR-BS=-142/217, BQ-BR=-142/217, BP-BQ=-142/217, BO-BP=-142/217, BN-BO=-142/217, BM-BN=-142/217, BL-BM=-142/217, BK-BL=-142/217, BJ-BK=-142/217, BI-BJ=-142/217, BH-BI=-142/217, BG-BH=-142/217, BF-BG=-142/217, BE-BF=-142/217, BD-BE=-142/217, BC-BD=-142/217, BB-BC=-142/217, BA-BB=-142/217, AZ-BA=-142/217, AY-AZ=-142/217, AX-AY=-142/217, AW-AX=-142/217, AV-AW=-142/217, AU-AV=-142/217, AT-AU=-142/217, AS-AT=-142/217, AR-AS=-142/217, AQ-AR=-142/217, AP-AQ=-142/217, AO-AP=-142/217
WEBS U-BF=-121/47, T-BG=-112/26, R-BH=-106/13, Q-BI=-122/55, P-BJ=-120/57, O-CL=-118/53, N-BM=-119/54, M-BN=-119/54, L-BO=-119/54, K-BP=-121/54, I-BQ=-103/54, H-BR=-81/54, G-BS=-81/53, F-BT=-82/55, E-BU=-80/44, D-BV=-132/122, V-BE=-121/47, W-BD=-112/24, Y-BC=-90/13, Z-BB=-122/56, AA-BA=-120/57, AB-AY=-118/53, AC-AX=-119/54, AD-AW=-119/54, AE-AV=-119/54, AF-AU=-121/54, AH-AT=-103/54, AI-AS=-81/54, AJ-AR=-81/54, AK-AQ=-81/53, AL-AP=-80/54, AM-AO=-129/108

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCCL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only.
4) ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps= varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
5) Roof design snow load has been reduced to account for slope.



Job 72285969	Truss A1	Truss Type GABLE	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton

Job Reference (optional)

8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:52:12 2022 Page 2
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NOTES-

- 6) Unbalanced snow loads have been considered for this design.
- 7) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 8) Provide adequate drainage to prevent water ponding.
- 9) All plates are 2x5 MT20 unless otherwise indicated.
- 10) Gable requires continuous bottom chord bearing.
- 11) Gable studs spaced at 1-4-0 oc.
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint B, 27 lb uplift at joint BF, 10 lb uplift at joint BG, 39 lb uplift at joint BI, 41 lb uplift at joint BJ, 37 lb uplift at joint BL, 38 lb uplift at joint BM, 38 lb uplift at joint BN, 38 lb uplift at joint BO, 38 lb uplift at joint BP, 38 lb uplift at joint BQ, 38 lb uplift at joint BR, 37 lb uplift at joint BS, 41 lb uplift at joint BT, 22 lb uplift at joint BU, 125 lb uplift at joint BV, 26 lb uplift at joint BE, 8 lb uplift at joint BD, 40 lb uplift at joint BB, 41 lb uplift at joint BA, 12 lb uplift at joint AN, 37 lb uplift at joint AY, 38 lb uplift at joint AX, 38 lb uplift at joint AW, 38 lb uplift at joint AV, 38 lb uplift at joint AU, 38 lb uplift at joint AT, 38 lb uplift at joint AS, 37 lb uplift at joint AR, 38 lb uplift at joint AQ, 34 lb uplift at joint AP and 102 lb uplift at joint AO.
- 15) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: A-S=51, S-X=61, X-AN=51, B-AN=20



Job 72285969	Truss A2	Truss Type PIGGYBACK BASE	Qty 6	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 Job Reference (optional)
 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:52:19 2022 Page 1

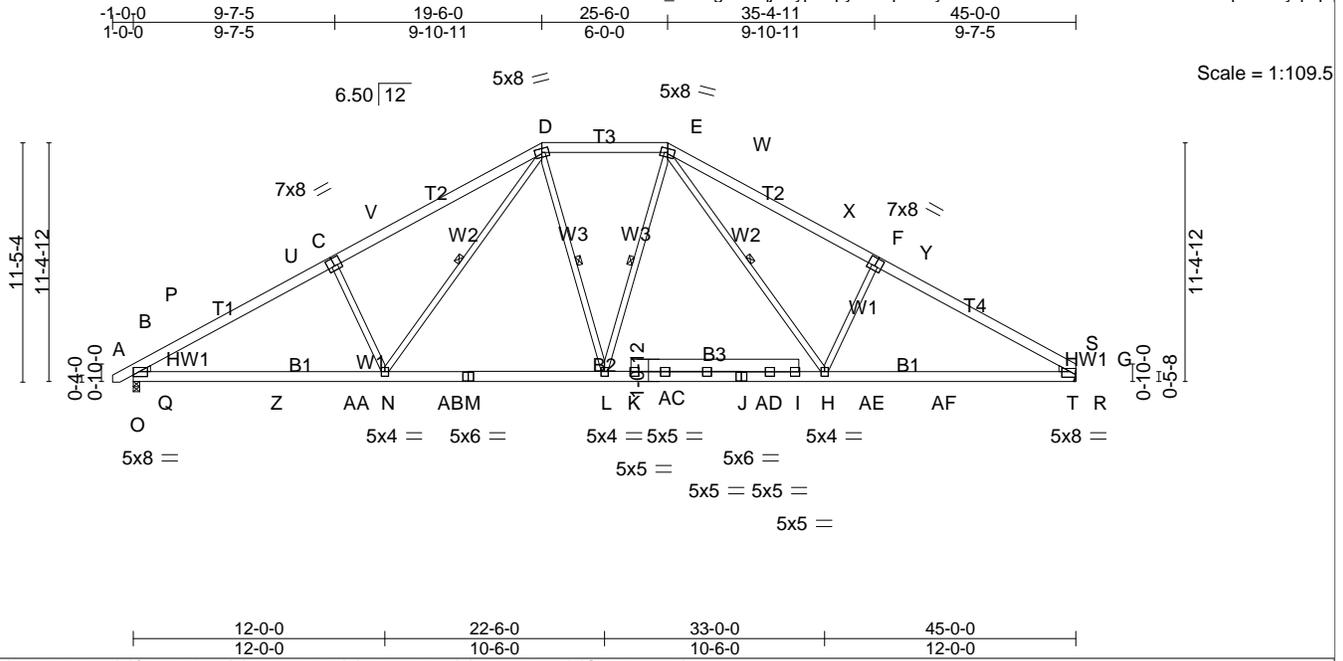


Plate Offsets (X,Y)-- [B:0-0-0,0-0-14], [C:0-4-0,0-4-8], [D:0-4-0,0-2-4], [E:0-4-0,0-2-4], [F:0-4-0,0-4-8], [G:0-0-0,0-1-6]

LOADING (psf)	SPACING	CSL	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.67	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0**	Plate Grip DOL 1.15	BC 0.97	Vert(LL) -0.25 L-N >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.47	Vert(CT) -0.40 L-N >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-MSH	Horz(CT) 0.12 G n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 340 lb	FT = 20%

LUMBER-
 TOP CHORD 2x6 SP No.2 *Except*
 T2: 2x6 SP SS
 BOT CHORD 2x6 SP No.2 *Except*
 B3: 2x8 SP No.2
 WEBS 2x4 SP No.3
 WEDGE
 Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 3-6-5 oc purlins, except 2-0-0 oc purlins (4-7-14 max.): D-E.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
 WEBS 1 Row at midpt D-N, D-L, E-L, E-H

REACTIONS. (lb/size) B=1665/0-3-8 (min. 0-3-3), G=1623/Mechanical
 Max Horz B=219(LC 13)
 Max Uplift B=-238(LC 14), G=-219(LC 15)
 Max Grav B=2028(LC 43), G=1987(LC 43)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/24, B-P=-1373/19, P-U=-3469/602, C-U=-3293/609, C-V=-3300/639, D-V=-3120/691, D-E=-2223/546, E-W=-2940/694, W-X=-3101/692, F-X=-3283/642, F-Y=-3275/613, S-Y=-3451/605, G-S=-1040/0
 BOT CHORD O-Q=-368/945, Q-Z=-415/3020, Z-AA=-415/3020, N-AA=-415/3020, N-AB=-157/2160, M-AB=-157/2160, M-AC=-157/2160, L-AC=-157/2160, K-L=-157/2157, J-K=-158/2158, J-AD=-159/2145, I-AD=-161/2142, H-I=-157/2157, H-AE=-418/2952, AE-AF=-418/2952, T-AF=-418/2952, R-T=-226/763
 WEBS C-N=-681/380, D-N=-254/1120, D-L=-83/375, E-L=-82/386, E-H=-258/1097, F-H=-684/380, B-O=-1071/0, P-Q=-295/508, O-P=-1334/190, B-Q=-59/981, G-R=-971/0, S-T=-587/624, R-S=-1517/265, G-T=-181/1251

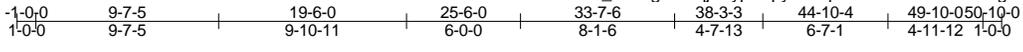
- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps= varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) Unbalanced snow loads have been considered for this design.
 - 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 7) Provide adequate drainage to prevent water ponding.
 - 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint B and 219 lb uplift at joint G.
 - 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard
 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: A-D=-51, D-E=-61, E-G=-51, O-R=-20



Job 72285969	Truss A3	Truss Type PIGGYBACK BASE	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:52:24 2022 Page 1
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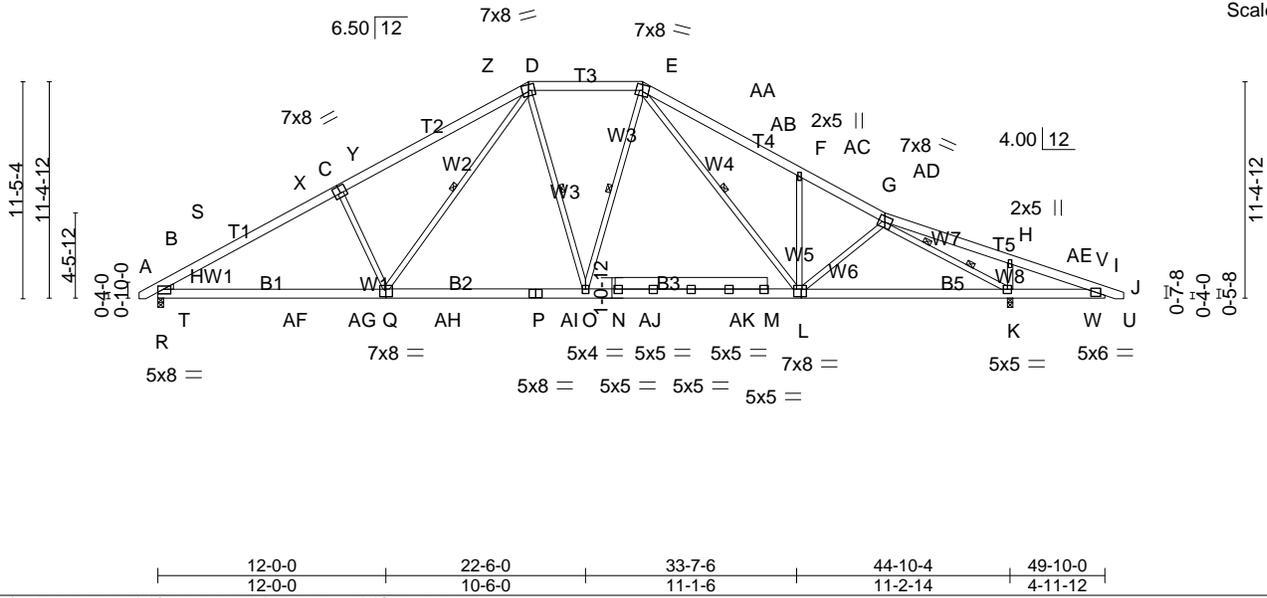


Plate Offsets (X,Y)-- [B:0-0-0,0-1-2], [C:0-4-0,0-4-8], [L:0-3-4,0-4-8], [Q:0-4-0,0-4-8]

LOADING (psf)	SPACING	CSL	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.78	in (loc) l/def L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0**	Plate Grip DOL 1.15	BC 0.93	Vert(LL) -0.24 O-Q >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.88	Vert(CT) -0.39 O-Q >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-MSH	Horz(CT) 0.10 K n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 386 lb	FT = 20%

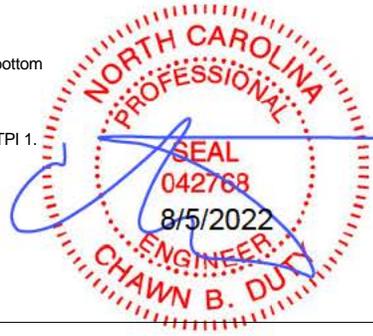
LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2 *Except* T2: 2x6 SP SS	TOP CHORD Structural wood sheathing directly applied or 2-8-1 oc purlins, except 2-0-0 oc purlins (4-9-7 max.): D-E.
BOT CHORD 2x6 SP No.2 *Except* B4: 2x8 SP No.2	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt D-Q, D-O, E-O, E-L 2 Rows at 1/3 pts G-K
WEDGE Left: 2x4 SP No.2	

REACTIONS. (lb/size) B=1636/0-3-8 (min. 0-3-2), K=2034/0-3-8 (req. 0-3-9)
 Max Horz B=234(LC 12)
 Max Uplift B=238(LC 14), K=323(LC 15)
 Max Grav B=1984(LC 47), K=2266(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/24, B-S=-1340/6, S-X=-3383/567, C-X=-3207/574, C-Y=-3215/605, Y-Z=-3034/631, D-Z=-2920/656, D-E=-2129/508, E-AA=-2862/664, AA-AB=-2992/662, AB-AC=-3148/630, F-AC=-3173/617, F-AD=-2873/473, G-AD=-2982/455, G-H=-342/496, H-AE=-441/532, V-AE=-455/522, I-V=-137/185, I-J=0/17
 BOT CHORD R-T=-360/942, T-AF=-390/2968, AF-AG=-390/2968, Q-AG=-390/2968, Q-AH=-88/2082, P-AH=-88/2082, P-AI=-88/2082, O-AI=-88/2082, O-AJ=-64/2040, N-AJ=-64/2040, N-AK=-64/2040, M-AK=-64/2040, L-M=-64/2040, K-L=-210/2532, K-W=-431/463, U-W=-152/149
 WEBS C-Q=-681/380, D-Q=-255/1124, D-O=-121/323, E-O=-47/469, G-K=-3097/787, H-K=-534/308, E-L=-311/1064, F-L=-724/317, G-L=-8/162, B-R=-1049/0, S-T=-294/508, R-S=-1301/180, B-T=-46/956, I-U=-25/38, V-W=-98/3, U-V=-98/99, I-W=-99/114

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps= varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) Unbalanced snow loads have been considered for this design.
 - 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 7) Provide adequate drainage to prevent water ponding.
 - 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 10) WARNING: Required bearing size at joint(s) K greater than input bearing size.
 - 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint B and 323 lb uplift at joint K.
 - 12) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard
 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: A-D=-51, D-E=-61, E-G=-51, G-J=-51, R-U=-20



Job 72285969	Truss A3T	Truss Type PIGGYBACK BASE	Qty 4	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:52:29 2022 Page 1
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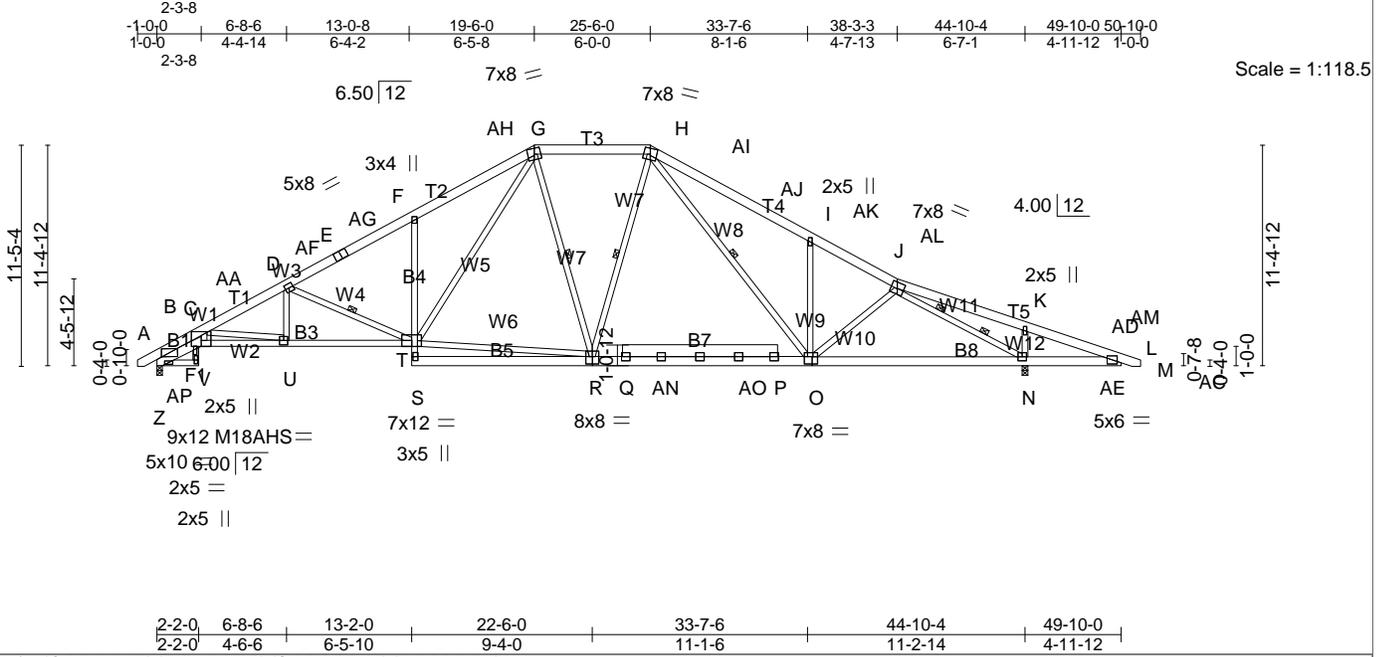


Plate Offsets (X,Y)-- [O:0-3-4,0-4-8], [R:0-4-0,0-4-8], [S:0-2-8,0-0-12], [Y:0-2-6,0-0-4]

LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.77	in (loc) l/def l/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0**	Plate Grip DOL 1.15	BC 0.74	Vert(LL) -0.25 F >999 240	M18AHS	186/179
TCDL 10.0	Lumber DOL 1.15	WB 0.97	Vert(CT) -0.48 T-U >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-MSH	Horz(CT) 0.26 N n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 415 lb	FT = 20%

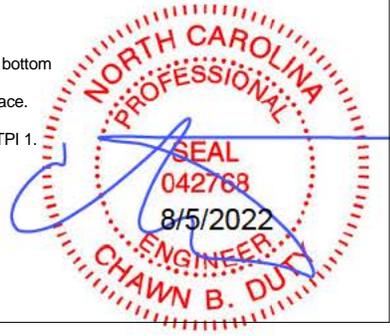
LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except
BOT CHORD 2x6 SP No.2 *Except*	2-0-0 oc purlins (4-11-14 max.): G-H.
WEBS 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
	WEBS 1 Row at midpt G-R, H-R, H-O, D-T
	2 Rows at 1/3 pts J-N

REACTIONS. (lb/size) N=2034/0-3-8 (min. 0-2-11), B=1626/0-3-8 (min. 0-2-0)
 Max Horz B=-234(LC 12)
 Max Uplift N=323(LC 15), B=-243(LC 14)
 Max Grav N=2266(LC 2), B=1959(LC 39)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/24, B-AA=-2622/418, C-AA=-6254/924, C-D=-4291/666, D-AF=-3396/551, E-AF=-3304/563, E-AG=-3285/565, F-AG=-3175/584, F-AH=-3408/721, G-AH=-3200/738, G-H=-2008/504, H-AI=-2819/663, AI-AJ=-2982/661, AJ-AK=-3145/629, I-AK=-3171/616, I-AL=-2838/473, J-AL=-2972/455, J-K=-342/497, K-AM=-441/532, AD-AM=-455/522, L-AD=-137/185, L-M=0/17
 BOT CHORD U-V=-859/4906, T-U=-535/3817, S-T=0/166, F-T=-595/277, R-S=-4/256, R-AN=-64/1971, Q-AN=-64/1971, P-AO=-64/1971, O-P=-64/1971, N-O=-210/2530, N-AE=-431/463, AC-AE=-152/149, Z-AP=-408/1719, AB-AP=-405/1726, V-AB=-965/5476
 WEBS C-V=-283/1679, D-U=0/439, R-T=-83/1788, G-T=-370/1573, G-R=-244/178, H-R=-42/327, H-O=-311/1085, I-O=-719/316, J-O=-8/151, K-N=-534/308, J-N=-3099/788, C-U=-1130/329, D-T=-1022/275, B-Z=-1351/250, AA-AB=-824/137, Z-AA=-1739/257, B-AB=-288/1836, L-AC=-25/38, AD-AE=-98/3, AC-AD=-98/99, L-AE=-100/114

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps= varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 unless otherwise indicated.
 - All plates are 5x5 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Bearing at joint(s) B considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 lb uplift at joint N and 243 lb uplift at joint B.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard
 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: A-G=-51, G-H=-61, H-J=-51, J-M=-51, T-V=-20, S-AC=-20, V-AP=-20



Job 72285969	Truss A5	Truss Type PIGGYBACK BASE	Qty 2	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton

Job Reference (optional)
8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:52:38 2022 Page 1

ID:w7S_774ogSiYqjEuyP5BpyU8dq-IDs8btTQykIY0Uqr0ckTtsTGM9RFOaqQH1bAkyqzpt

-1-0-0	9-7-5	19-6-0	25-6-0	33-7-6	38-3-3	42-4-12	49-10-0	50-10-0
1-0-0	9-7-5	9-10-11	6-0-0	8-1-6	4-7-13	4-1-8	7-5-4	1-0-0

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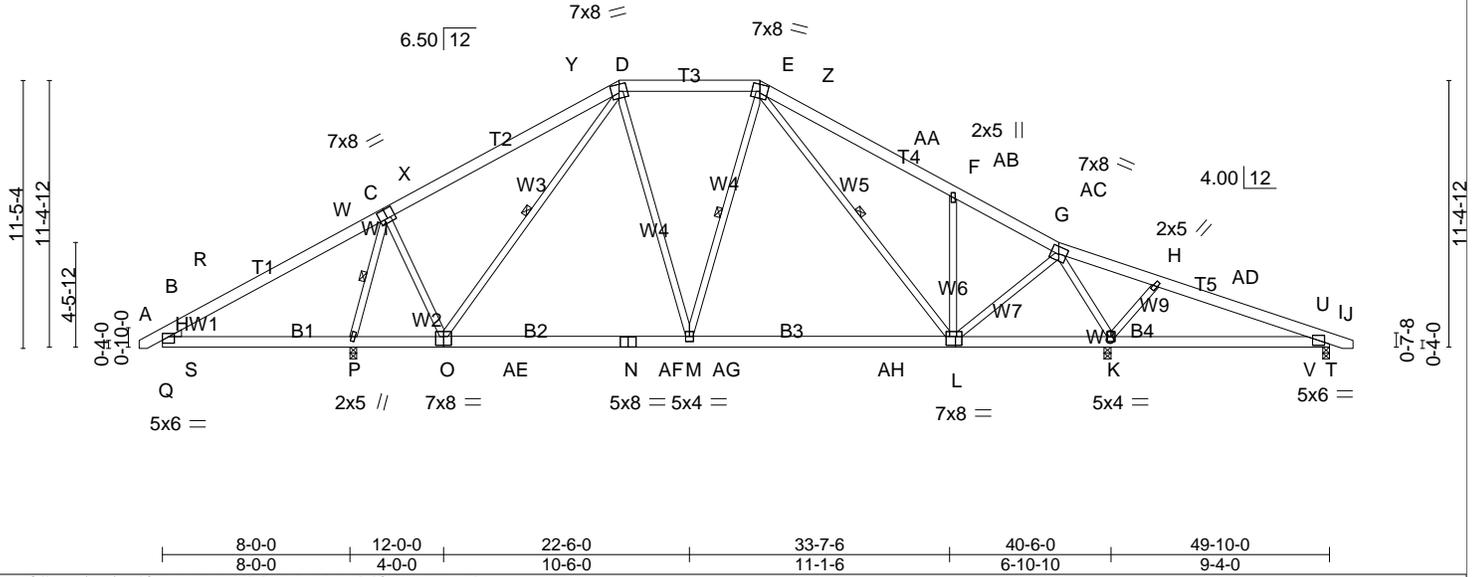


Plate Offsets (X,Y)-- [C:0-4-0,0-4-12], [L:0-3-4,0-4-8], [O:0-4-0,0-4-8]

LOADING (psf)	SPACING-	CSI	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.85	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0**	Plate Grip DOL 1.15	BC 0.68	Vert(LL) -0.15 L-M >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.69	Vert(CT) -0.25 L-M >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-MSH	Horz(CT) 0.02 K n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 367 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 5-3-0 oc purlins, except
BOT CHORD 2x6 SP No.2	2-0-0 oc purlins (6-0-0 max.): D-E.
WEBS 2x4 SP No.3	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEDGE	1 Row at midpt D-O, E-M, E-L, C-P
Left: 2x4 SP No.2	

REACTIONS. (lb/size) K=1606/0-3-8 (min. 0-2-13), I=228/0-3-8 (min. 0-1-8), P=1836/0-3-8 (min. 0-2-10)
 Max Horz P=234(LC 12)
 Max Uplift K=292(LC 15), I=123(LC 11), P=283(LC 14)
 Max Grav K=1804(LC 39), I=372(LC 45), P=2198(LC 39)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/24, B-R=-243/401, R-W=-394/625, C-W=-352/771, C-X=-743/139, X-Y=-550/166, D-Y=-454/191, D-E=-985/258, E-Z=-1122/377, Z-AA=-1250/375, AA-AB=-1384/343, F-AB=-1405/329, F-AC=-1137/184, G-AC=-1246/165, G-H=-102/508, H-AD=-42/443, U-AD=-108/395, I-U=-160/86, I-J=0/17
 BOT CHORD Q-S=-99/56, P-S=-536/436, O-P=-103/327, O-AE=0/928, N-AE=0/928, N-AF=0/928, M-AF=0/928, M-AG=0/976, AG-AH=0/976, L-AH=0/976, K-L=0/386, K-V=-347/106, T-V=-90/133
 WEBS C-O=-10/869, D-O=-744/220, D-M=-22/508, E-M=-141/217, E-L=-210/305, H-K=-530/241, F-L=-717/323, G-L=-78/988, G-K=-1651/267, C-P=-2053/535, B-Q=-326/422, R-S=-518/455, Q-R=-140/181, B-S=-253/184, I-T=-236/53, U-V=-53/229, T-U=-88/88, I-V=-93/64

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps= varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint K, 123 lb uplift at joint I and 283 lb uplift at joint P.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard
 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: A-D=-51, D-E=-61, E-G=-51, G-J=-51, Q-T=-20



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.



Job 72285969	Truss A6	Truss Type PIGGYBACK BASE	Qty 4	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:52:42 2022 Page 1
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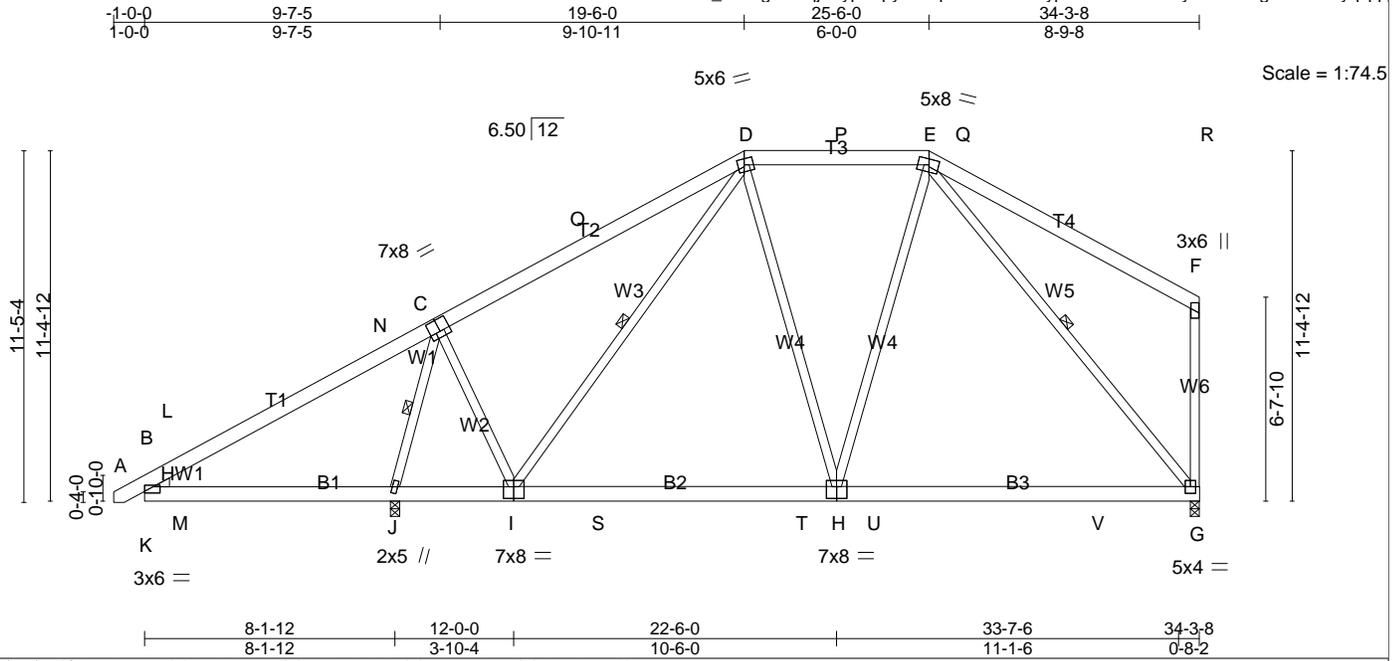


Plate Offsets (X,Y)-- [C:0-4-0,0-4-12], [D:0-2-8,0-2-4], [E:0-3-12,0-2-4], [H:0-4-0,0-4-8], [I:0-4-0,0-4-8]

LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.85	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0**	Plate Grip DOL 1.15	BC 0.74	Vert(LL) -0.21 G-H >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.87	Vert(CT) -0.35 G-H >902 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-MSH	Horz(CT) 0.01 G n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 270 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): D-E.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt D-I, E-G, C-J
WEDGE	
Left: 2x4 SP No.2	

REACTIONS. (lb/size) G=853/0-3-8 (min. 0-1-14), J=1666/0-3-8 (min. 0-2-5)
 Max Horz J=348(LC 13)
 Max Uplift G=112(LC 15), J=277(LC 14)
 Max Grav G=1193(LC 43), J=1972(LC 37)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/24, B-L=242/400, L-N=411/619, C-N=-369/772, C-O=-599/160, D-O=-405/188, D-P=-688/224, E-P=-688/224, E-Q=-170/245, Q-R=-212/243, F-R=-277/201, F-G=-391/216
 BOT CHORD K-M=100/64, J-M=-537/451, I-J=-342/270, I-S=-171/673, S-T=-171/673, H-T=-171/673, H-U=-116/603, U-V=-116/603, G-V=-116/603
 WEBS C-I=0/713, D-I=-593/207, D-H=-33/340, E-H=-22/417, E-G=-909/114, C-J=-1821/523, B-K=-325/420, L-M=-516/454, K-L=-144/181, B-M=-253/187

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) Unbalanced snow loads have been considered for this design.
 - 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 7) Provide adequate drainage to prevent water ponding.
 - 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint G and 277 lb uplift at joint J.
 - 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard
 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: A-D=-51, D-E=-61, E-F=-51, G-K=-20



Job 72285969	Truss A7	Truss Type PIGGYBACK BASE	Qty 6	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton ID:w7S_7?4ogSiYqjEuy5BpyU8dq-WmL9HCZR3BJP_jROUitLBypf5at17w6?GWz0SGyqzpl 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:52:46 2022 Page 1

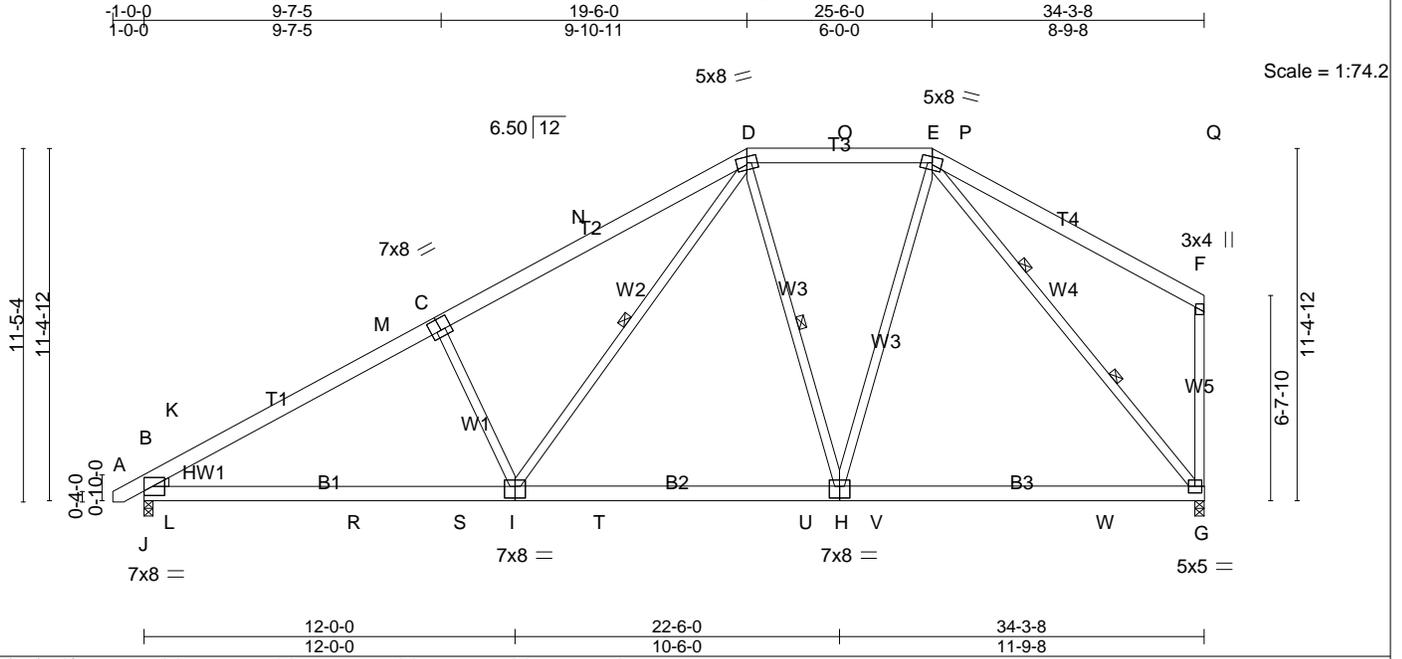


Plate Offsets (X,Y)-- [C:0-4-0,0-4-8], [D:0-4-0,0-2-4], [E:0-3-12,0-2-4], [H:0-4-0,0-4-8], [I:0-4-0,0-4-8]

LOADING (psf)	SPACING	CSL	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.82	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0**	Plate Grip DOL 1.15	BC 0.77	Vert(LL) -0.22 G-H >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.97	Vert(CT) -0.38 G-H >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-MSH	Horz(CT) 0.05 G n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 262 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2 *Except* T2: 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): D-E.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3 *Except* W4: 2x4 SP No.2	WEBS 1 Row at midpt D-I, D-H 2 Rows at 1/3 pts E-G
WEDGE Left: 2x4 SP No.2	

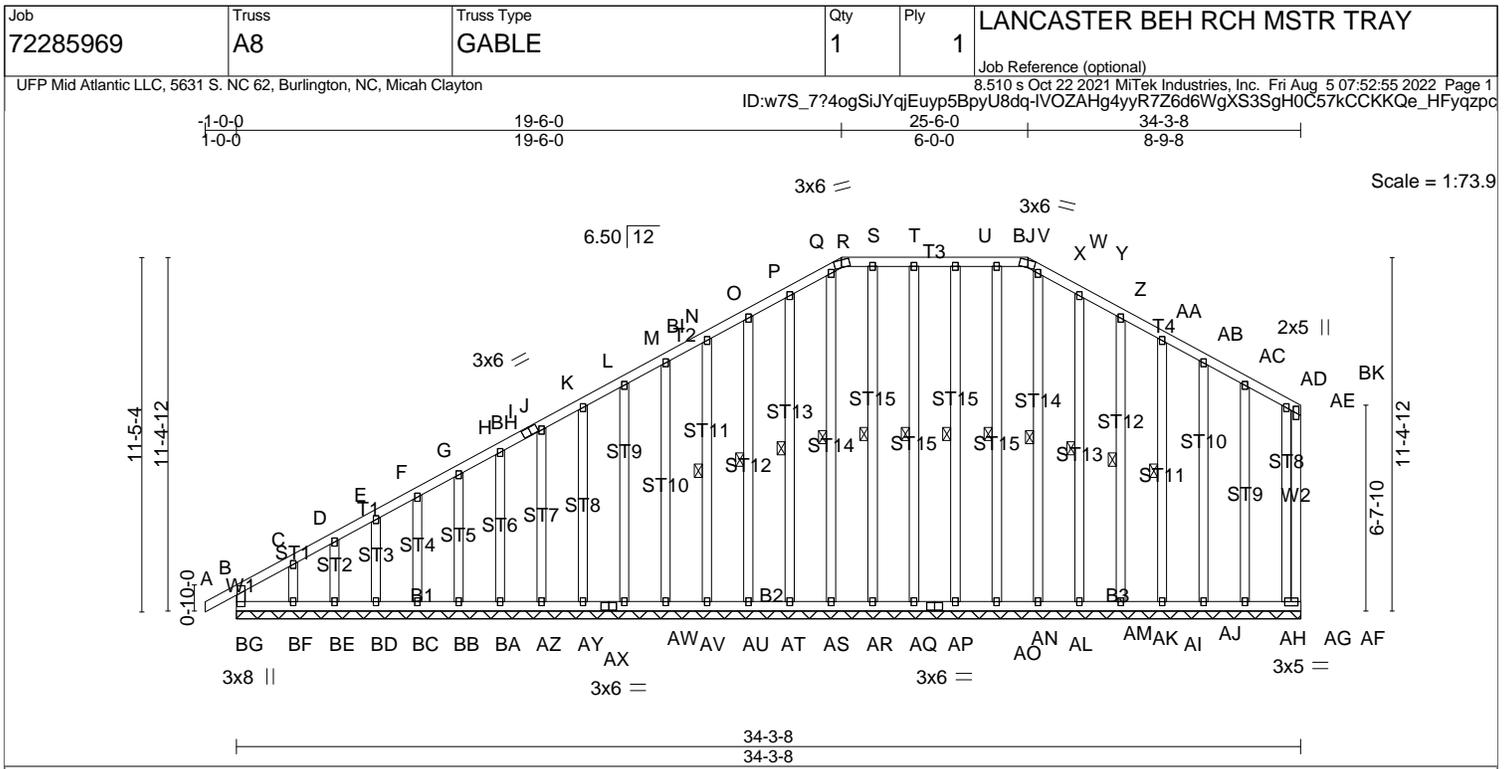
REACTIONS. (lb/size) B=1272/0-3-8 (min. 0-2-6), G=1248/0-3-8 (min. 0-2-10)
 Max Horz B=335(LC 14)
 Max Uplift B=200(LC 14), G=122(LC 14)
 Max Grav B=1527(LC 45), G=1666(LC 43)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/24, B-K=-1169/61, K-M=-2459/407, C-M=-2292/414, C-N=-2304/468, D-N=-2118/496, D-O=-1190/330, E-O=-1190/330, E-P=-65/176,
 P-Q=-109/163, F-Q=-184/90, F-G=-386/168
 BOT CHORD J-L=-343/908, L-R=-470/2168, R-S=-470/2168, I-S=-470/2168, I-T=-196/1264, T-U=-196/1264, H-U=-196/1264, H-V=-146/970, V-W=-146/970,
 G-W=-146/970
 WEBS C-I=-710/377, D-I=-270/1145, D-H=-400/221, E-H=-66/886, E-G=-1507/228, B-J=-945/84, K-L=-186/325, J-K=-990/115, B-L=-166/933

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps= varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 200 lb uplift at joint B and 122 lb uplift at joint G.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard
 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: A-D=-51, D-E=-61, E-F=-51, G-J=-20





LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.42	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0**	Plate Grip DOL 1.15	BC 0.13	Vert(LL) 0.00 B n/r 120		
TCDL 10.0	Lumber DOL 1.15	WB 0.15	Vert(CT) -0.00 A n/r 90		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-R	Horz(CT) -0.01 AF n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 397 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): R-W.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt T-AP, S-AQ, Q-AR, P-AS, O-AT, N-AU, U-AN, V-AM, X-AL, Y-AK, Z-AJ, AA-AI
OTHERS 2x4 SP No.3	

REACTIONS. (lb/size) BG=133/34-3-8 (min. 0-4-14), AF=58/34-3-8 (min. 0-4-14), AP=108/34-3-8 (min. 0-4-14), AQ=107/34-3-8 (min. 0-4-14), AR=98/34-3-8 (min. 0-4-14), AS=94/34-3-8 (min. 0-4-14), AT=94/34-3-8 (min. 0-4-14), AU=94/34-3-8 (min. 0-4-14), AV=94/34-3-8 (min. 0-4-14), AW=94/34-3-8 (min. 0-4-14), AY=94/34-3-8 (min. 0-4-14), AZ=94/34-3-8 (min. 0-4-14), BA=94/34-3-8 (min. 0-4-14), BB=94/34-3-8 (min. 0-4-14), BC=94/34-3-8 (min. 0-4-14), BD=94/34-3-8 (min. 0-4-14), BE=95/34-3-8 (min. 0-4-14), BF=91/34-3-8 (min. 0-4-14), AN=108/34-3-8 (min. 0-4-14), AM=107/34-3-8 (min. 0-4-14), AL=98/34-3-8 (min. 0-4-14), AK=94/34-3-8 (min. 0-4-14), AJ=94/34-3-8 (min. 0-4-14), AI=95/34-3-8 (min. 0-4-14), AH=93/34-3-8 (min. 0-4-14), AG=104/34-3-8 (min. 0-4-14)

Max Horz BG=361(LC 13)
 Max Uplift BG=-131(LC 10), AF=-34(LC 14), AP=-29(LC 10), AQ=-27(LC 11), AR=-26(LC 11), AS=-39(LC 14), AT=-41(LC 14), AU=-37(LC 14), AV=-38(LC 14), AW=-38(LC 14), AY=-38(LC 14), AZ=-38(LC 14), BA=-38(LC 14), BB=-38(LC 14), BC=-35(LC 14), BD=-48(LC 14), BE=-4(LC 10), BF=-194(LC 14), AN=-28(LC 10), AM=-25(LC 11), AL=-2(LC 11), AK=-39(LC 15), AJ=-41(LC 15), AI=-34(LC 15), AH=45(LC 15), AG=37(LC 10)
 Max Grav BG=290(LC 29), AF=95(LC 47), AP=148(LC 36), AQ=141(LC 36), AR=126(LC 45), AS=148(LC 37), AT=146(LC 37), AU=145(LC 37), AV=145(LC 37), AW=145(LC 37), AY=145(LC 37), AZ=147(LC 37), BA=130(LC 37), BB=107(LC 52), BC=107(LC 2), BD=109(LC 28), BE=114(LC 29), BF=184(LC 28), AN=148(LC 36), AM=141(LC 36), AL=119(LC 37), AK=148(LC 37), AJ=146(LC 37), AI=145(LC 37), AH=144(LC 37), AG=173(LC 47)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD B-BG=-234/112, A-B=0/33, B-C=-346/213, C-D=-277/178, D-E=-270/180, E-F=-251/172, F-G=-242/173, G-BH=-232/172, H-BH=-221/176, H-I=-222/176, I-J=-211/179, J-K=-212/183, K-L=-202/186, L-M=-192/189, M-BI=-182/199, N-BI=-173/203, N-O=-206/234, O-P=-234/267, P-Q=-261/298, Q-R=-233/267, R-S=-239/279, S-T=-239/279, T-BJ=-239/279, U-BJ=-239/279, U-V=-239/279, V-W=-239/279, W-X=-233/267, X-Y=-261/298, Y-Z=-234/267, Z-AA=-206/234, AA-AB=-180/203, AB-AC=-151/169, AC-BK=-135/156, AD-BK=-141/149, AD-AE=-143/157, AE-AF=-138/139
 BOT CHORD BF-BG=95/107, BE-BF=95/107, BD-BE=95/107, BC-BD=95/107, BB-BC=95/107, BA-BB=95/107, AZ-BA=95/107, AY-AZ=95/107, AX-AY=95/107, AW-AX=95/107, AV-AW=95/107, AU-AV=95/107, AT-AU=95/107, AS-AT=95/107, AR-AS=95/107, AQ-AR=95/107, AP-AQ=95/107, AO-AP=95/107, AN-AO=95/107, AM-AN=95/107, AL-AM=95/107, AK-AL=95/107, AJ-AK=95/107, AI-AJ=95/107, AH-AI=95/107, AG-AH=95/107, AF-AG=95/107
 WEBS T-AP=-121/48, S-AQ=-114/43, Q-AR=-99/48, P-AS=-122/55, O-AT=-119/57, N-AU=-118/53, M-AV=-119/54, L-AW=-119/54, K-AY=-119/54, J-AZ=-121/54, H-BA=-103/54, G-BB=-81/54, F-BC=-81/53, E-BD=-84/58, D-BE=-82/39, C-BF=-140/143, U-AN=-121/48, V-AM=-114/41, X-AL=-92/26, Y-AK=-122/55, Z-AJ=-119/56, AA-AI=-118/53, AB-AH=-119/55, AC-AG=-127/77, AD-AF=-70/44

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone: cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only.
 - ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps= varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x3 MT20 unless otherwise indicated.
 - Gable eaves require continuous bottom chord bearing.



Job 72285969	Truss A8	Truss Type GABLE	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton

Job Reference (optional)

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NOTES-

- 11) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 12) Gable studs spaced at 1-4-0 oc.
- 13) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 14) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 15) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint BG, 34 lb uplift at joint AF, 29 lb uplift at joint AP, 27 lb uplift at joint AQ, 26 lb uplift at joint AR, 39 lb uplift at joint AS, 41 lb uplift at joint AT, 37 lb uplift at joint AU, 38 lb uplift at joint AV, 38 lb uplift at joint AW, 38 lb uplift at joint AY, 38 lb uplift at joint AZ, 38 lb uplift at joint BA, 38 lb uplift at joint BB, 35 lb uplift at joint BC, 48 lb uplift at joint BD, 4 lb uplift at joint BE, 194 lb uplift at joint BF, 28 lb uplift at joint AN, 25 lb uplift at joint AM, 2 lb uplift at joint AL, 39 lb uplift at joint AK, 41 lb uplift at joint AJ, 34 lb uplift at joint AI, 45 lb uplift at joint AH and 37 lb uplift at joint AG.
- 16) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 17) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: A-B=-51, B-R=-51, R-W=-61, W-AE=-51, AF-BG=-20



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.



Job 72285969	Truss B1	Truss Type GABLE	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 Job Reference (optional) 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:02 2022 Page 1
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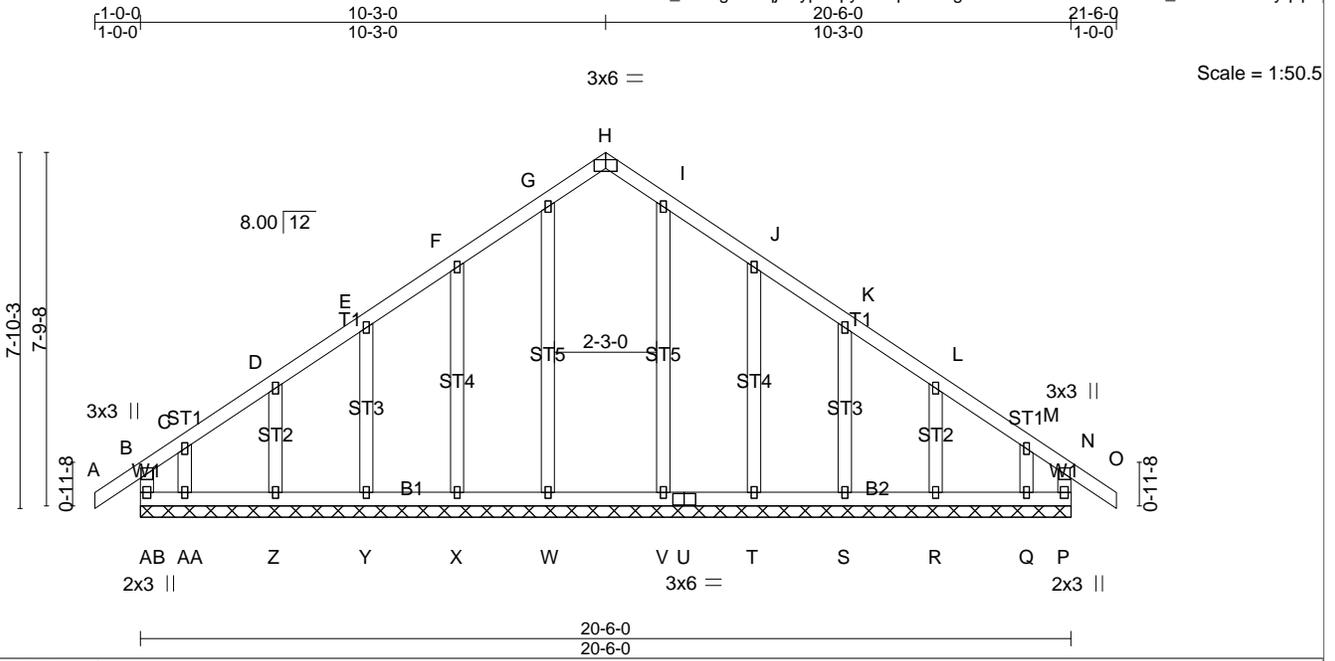


Plate Offsets (X,Y)-- [H:0-3-0,Edge]

LOADING (psf) TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.20 BC 0.11 WB 0.12 Matrix-R	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 O n/r 120 Vert(CT) -0.01 O n/r 90 Horz(CT) 0.01 P n/a n/a	PLATES MT20 GRIP 244/190 Weight: 128 lb FT = 20%
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LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) AB=132/20-6-0 (min. 0-2-12), P=132/20-6-0 (min. 0-2-12), W=157/20-6-0 (min. 0-2-12), X=136/20-6-0 (min. 0-2-12), Y=141/20-6-0 (min. 0-2-12), Z=150/20-6-0 (min. 0-2-12), AA=57/20-6-0 (min. 0-2-12), V=157/20-6-0 (min. 0-2-12), T=136/20-6-0 (min. 0-2-12), S=141/20-6-0 (min. 0-2-12), R=150/20-6-0 (min. 0-2-12), Q=57/20-6-0 (min. 0-2-12)
 Max Horz AB=222(LC 10)
 Max Uplift AB=110(LC 10), P=73(LC 11), X=95(LC 12), Y=66(LC 12), Z=60(LC 12), AA=212(LC 12), T=98(LC 13), S=65(LC 13), R=61(LC 13), Q=201(LC 13)
 Max Grav AB=230(LC 26), P=217(LC 27), W=231(LC 24), X=157(LC 24), Y=174(LC 24), Z=174(LC 24), AA=192(LC 10), V=217(LC 25), T=162(LC 25), S=173(LC 25), R=175(LC 25), Q=169(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD B-AB=-175/108, A-B=0/38, B-C=-224/142, C-D=-138/106, D-E=-102/83, E-F=-88/75, F-G=-117/133, G-H=-120/126, H-I=-120/126, I-J=-117/133, J-K=-69/62, K-L=-82/56, L-M=-125/80, M-N=-208/137, N-O=0/38, O-P=-160/108
 BOT CHORD AA-AB=-102/175, Z-AA=-102/175, Y-Z=-102/175, X-Y=-102/175, W-X=-102/175, V-W=-102/175, U-V=-102/175, T-U=-102/175, S-T=-102/175, R-S=-102/175, Q-R=-102/175, P-Q=-102/175
 WEBS G-W=-139/15, F-X=-156/119, E-Y=-131/88, D-Z=-136/93, C-AA=-163/145, I-V=-130/0, J-T=-156/121, K-S=-130/87, L-R=-137/93, M-Q=-163/139

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only.
 - 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 5) Roof design snow load has been reduced to account for slope.
 - 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 7) All plates are 1.5x3 MT20 unless otherwise indicated.
 - 8) Gable requires continuous bottom chord bearing.
 - 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 10) Gable studs spaced at 2-0-0 oc.
 - 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint AB, 73 lb uplift at joint P, 95 lb uplift at joint X, 66 lb uplift at joint Y, 60 lb uplift at joint Z, 212 lb uplift at joint AA, 98 lb uplift at joint T, 65 lb uplift at joint S, 61 lb uplift at joint R and 201 lb uplift at joint Q.
 - 14) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.



Job 72285969	Truss B2	Truss Type COMMON	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
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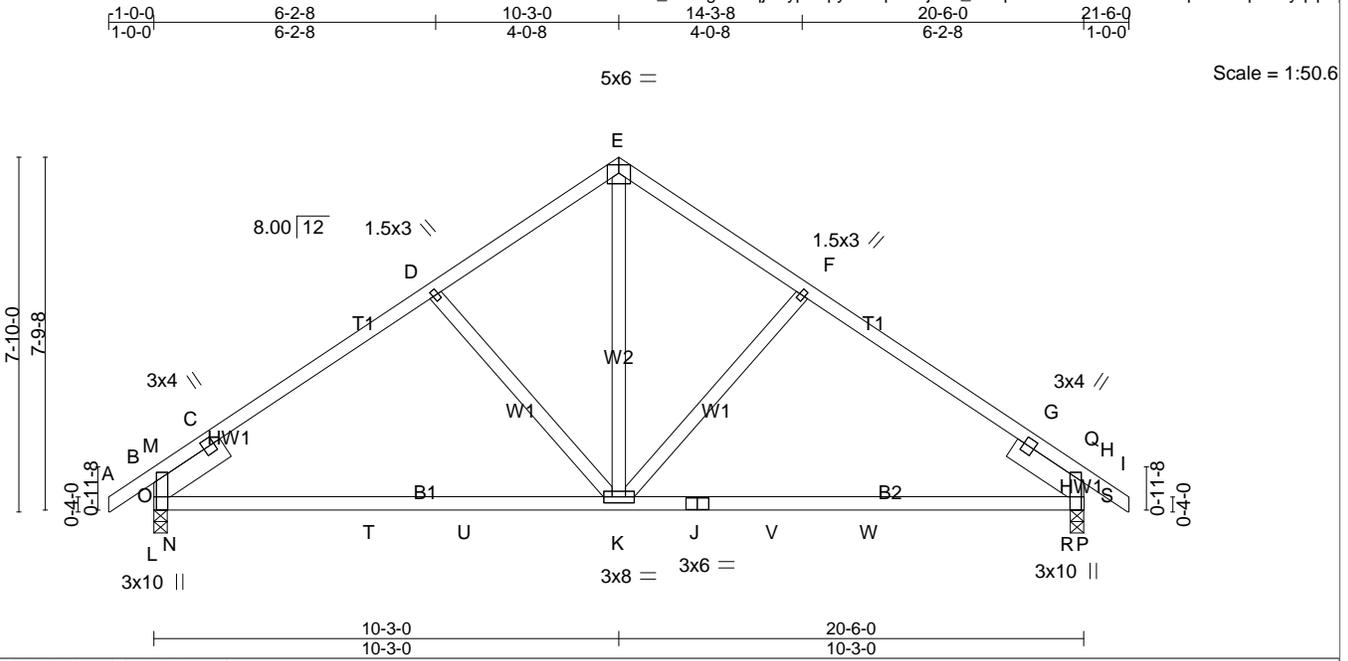


Plate Offsets (X,Y)-- [B:0-7-5,Edge], [H:0-7-5,Edge]

LOADING (psf) TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.29 BC 0.62 WB 0.25 Matrix-MSH	DEFL. in (loc) l/defl L/d Vert(LL) -0.13 K-N >999 240 Vert(CT) -0.26 K-N >958 180 Horz(CT) 0.02 B n/a n/a	PLATES MT20 GRIP 244/190 Weight: 108 lb FT = 20%
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LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.1
 WEBS 2x4 SP No.3
 SLIDER Left 2x6 SP No.2 1-11-0, Right 2x6 SP No.2 1-11-0

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 5-7-2 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) B=776/0-3-8 (min. 0-1-8), H=776/0-3-8 (min. 0-1-8)
 Max Horz B=194(LC 11)
 Max Uplift B=122(LC 12), H=122(LC 13)
 Max Grav B=879(LC 2), H=879(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/33, B-M=618/0, C-M=570/0, C-D=977/233, D-E=783/236, E-F=783/236, F-G=977/233, G-Q=570/0, H-Q=618/0, H-I=0/33
 BOT CHORD L-N=-233/385, N-T=-133/849, T-U=-133/849, K-U=-133/849, J-K=-53/751, J-V=-53/751, V-W=-53/751, R-W=-53/751, P-R=-125/299
 WEBS E-K=-161/652, D-K=-293/213, F-K=-293/213, B-L=-658/0, N-O=0/497, M-O=-41/149, B-O=0/495, L-O=-523/168, C-O=-849/434, H-P=-658/0, R-S=0/497, Q-S=40/148, H-S=0/495, P-S=-523/168, G-S=-849/434

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint B and 122 lb uplift at joint H.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Job 72285969	Truss B3	Truss Type COMMON	Qty 2	Ply 14-3-8	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton Job Reference (optional) 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:10 2022 Page 1

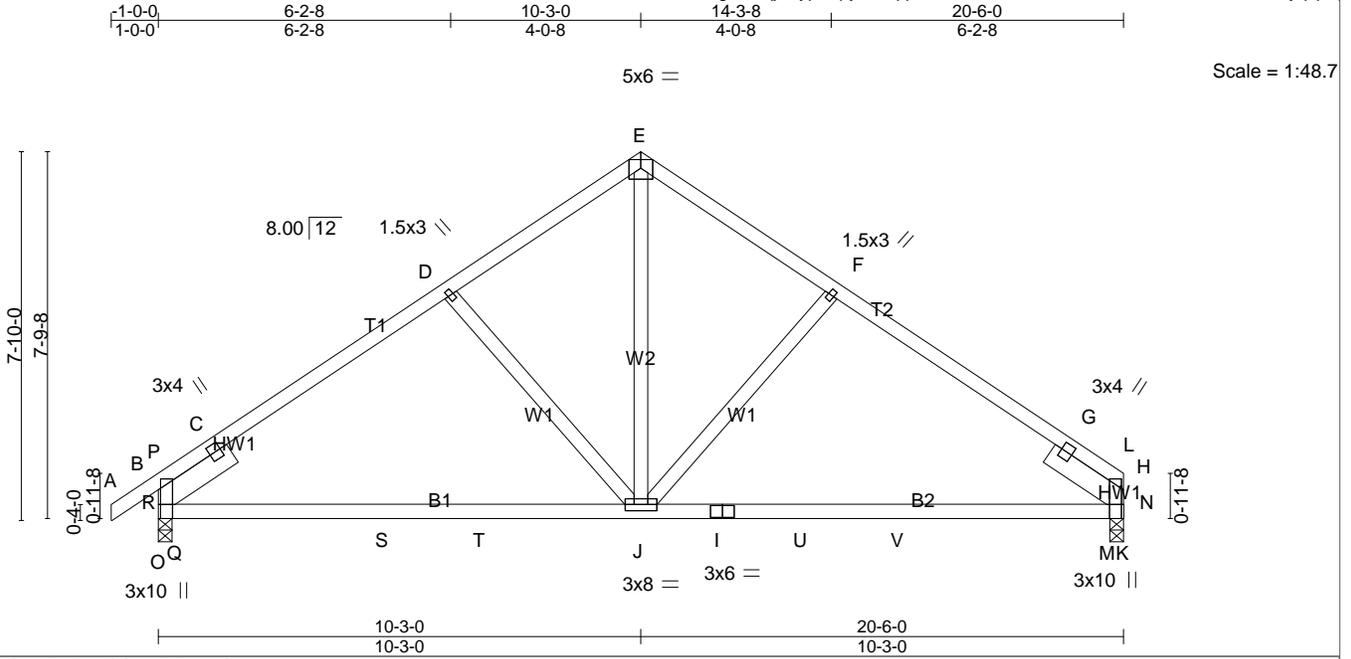


Plate Offsets (X,Y)-- [B:0-7-5,Edge], [H:0-7-5,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.30	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.84	Vert(LL) -0.15 J-Q >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.25	Vert(CT) -0.29 J-Q >841 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-MSH	Horz(CT) 0.02 B n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 107 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B2: 2x4 SP No.1
WEBS 2x4 SP No.3
SLIDER Left 2x6 SP No.2 1-11-0, Right 2x6 SP No.2 1-11-0

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-7-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) H=724/0-3-8 (min. 0-1-8), B=777/0-3-8 (min. 0-1-8)
Max Horz B=188(LC 9)
Max Uplift H=100(LC 13), B=122(LC 12)
Max Grav H=819(LC 2), B=881(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=0/33, B-P=-636/0, C-P=-589/0, C-D=-979/234, D-E=-785/237, E-F=-786/238, F-G=-939/235, G-L=-596/0, H-L=-629/0
BOT CHORD O-Q=-247/364, Q-S=-145/843, S-T=-145/843, J-T=-145/843, I-J=-94/752, I-U=-94/752, U-V=-94/752, M-V=-94/752, K-M=-157/312
WEBS E-J=-163/652, D-J=-295/213, F-J=-298/213, H-K=-638/0, M-N=0/497, L-N=-48/123, H-N=0/512, K-N=-519/186, G-N=-792/415, B-O=-671/0, Q-R=0/496, P-R=-43/148, B-R=0/509, O-R=-515/172, C-R=-849/437

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint H and 122 lb uplift at joint B.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Job 72285969	Truss B4	Truss Type Common	Qty 6	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 ID:w7S_7?4ogSiYqjEuyP5BpyU8dq-h91l9nv?TorRL0am7ANwKSZZ4sSohnJ7itIVSeyqzpj
 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:14 2022 Page 1

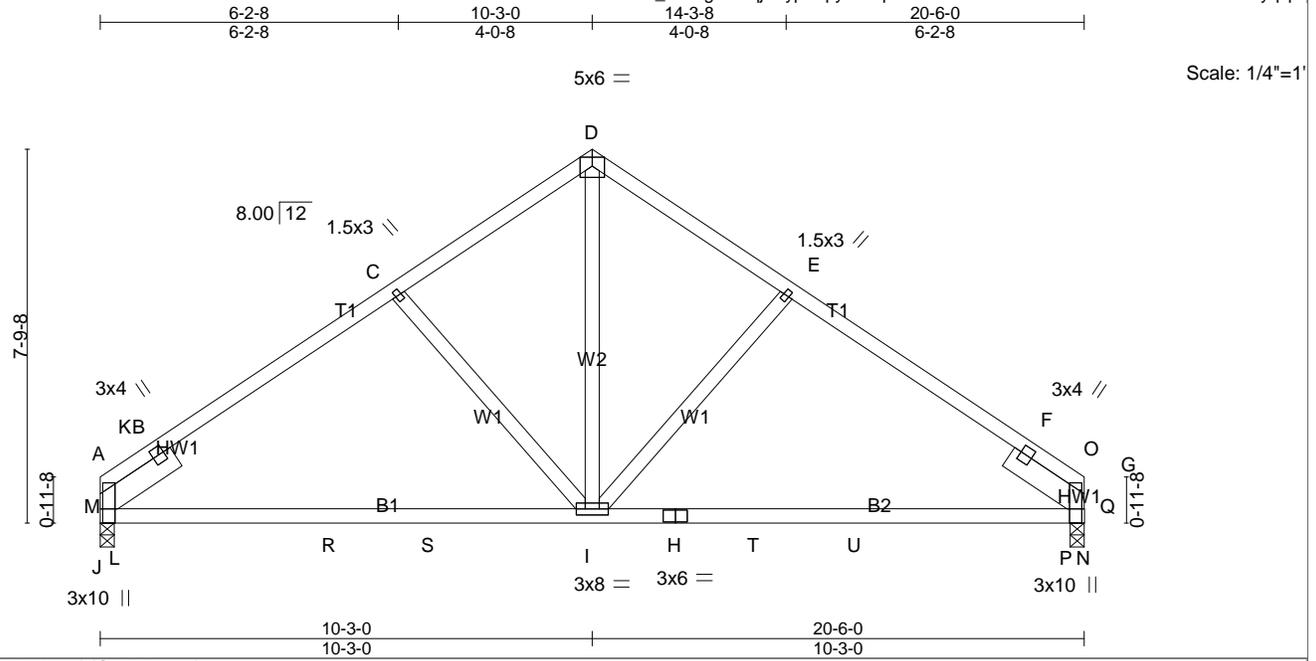


Plate Offsets (X,Y)-- [A:0-3-8,Edge], [G:0-7-5,Edge]

LOADING (psf) TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.30 BC 0.62 WB 0.25 Matrix-MSH	DEFL. in (loc) l/defl L/d Vert(LL) -0.13 I-L >999 240 Vert(CT) -0.26 I-L >953 180 Horz(CT) 0.02 A n/a n/a	PLATES MT20 GRIP 244/190 Weight: 105 lb FT = 20%
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LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.1
 WEBS 2x4 SP No.3
 SLIDER Left 2x6 SP No.2 1-11-0, Right 2x6 SP No.2 1-11-0

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 5-7-10 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=726/0-3-8 (min. 0-1-8), G=726/0-3-8 (min. 0-1-8)
 Max Horz A=177(LC 9)
 Max Uplift A=100(LC 12), G=100(LC 13)
 Max Grav A=820(LC 2), G=820(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-K=622/0, B-K=588/0, B-C=984/236, C-D=789/238, D-E=789/238, E-F=984/236, F-O=588/0, G-O=622/0
 BOT CHORD J-L=240/390, L-R=145/847, R-S=145/847, I-S=145/847, H-I=94/755, H-T=94/755, T-U=94/755, P-U=94/755, N-P=157/315
 WEBS D-I=164/652, C-I=297/213, E-I=297/213, A-J=633/0, L-M=0/492, K-M=47/125, A-M=0/506, J-M=522/186, B-M=801/416, G-N=633/0, P-Q=0/492, O-Q=47/125, G-Q=0/506, N-Q=522/186, F-Q=801/416

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint A and 100 lb uplift at joint G.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Job 72285969	Truss C1	Truss Type Common Supported Gable	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton

Job Reference (optional)
8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:18 2022 Page 1

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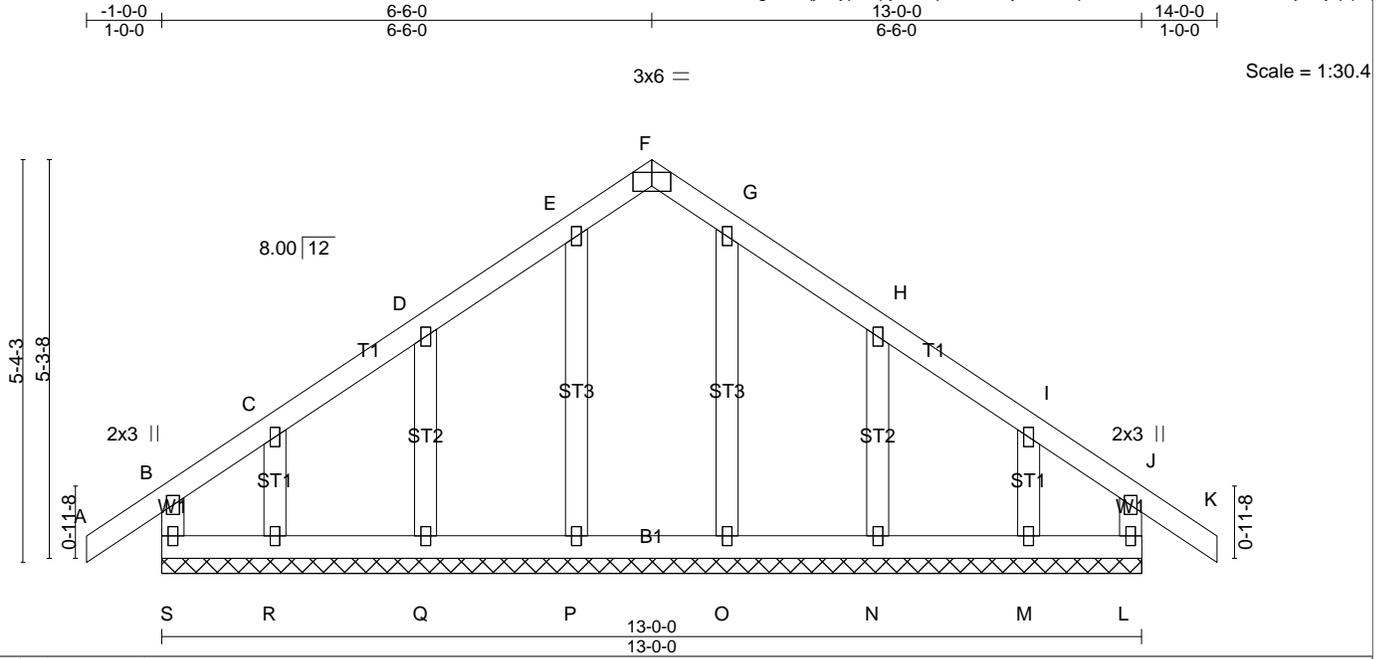


Plate Offsets (X,Y)-- [F:0-3-0,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.14	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.06	Vert(LL) -0.00 K n/r 120		
TCDL 10.0	Lumber DOL 1.15	WB 0.05	Vert(CT) -0.01 K n/r 90		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-R	Horz(CT) 0.00 L n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 73 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) S=119/13-0-0 (min. 0-1-13), L=119/13-0-0 (min. 0-1-13), P=142/13-0-0 (min. 0-1-13), Q=148/13-0-0 (min. 0-1-13), R=99/13-0-0 (min. 0-1-13), O=142/13-0-0 (min. 0-1-13), N=148/13-0-0 (min. 0-1-13), M=99/13-0-0 (min. 0-1-13)
Max Horz S=-157(LC 10)
Max Uplift S=-51(LC 8), L=-36(LC 9), Q=81(LC 12), R=-108(LC 12), N=83(LC 13), M=-104(LC 13)
Max Grav S=155(LC 25), L=142(LC 24), P=168(LC 24), Q=174(LC 24), R=165(LC 24), O=160(LC 2), N=177(LC 25), M=157(LC 25)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD B-S=-125/53, A-B=0/38, B-C=-95/85, C-D=-62/65, D-E=-92/109, E-F=-97/108, F-G=-97/108, G-H=-92/109, H-I=-51/54, I-J=-81/67, J-K=0/38, J-L=-124/53
BOT CHORD R-S=-76/111, Q-R=-76/111, P-Q=-76/111, O-P=-76/111, N-O=-76/111, M-N=-76/111, L-M=-76/111
WEBS E-P=-127/15, D-Q=-153/117, C-R=-138/100, G-O=-121/7, H-N=-153/117, I-M=-138/97

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only.
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - All plates are 1.5x3 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint S, 36 lb uplift at joint L, 81 lb uplift at joint Q, 108 lb uplift at joint R, 83 lb uplift at joint N and 104 lb uplift at joint M.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.

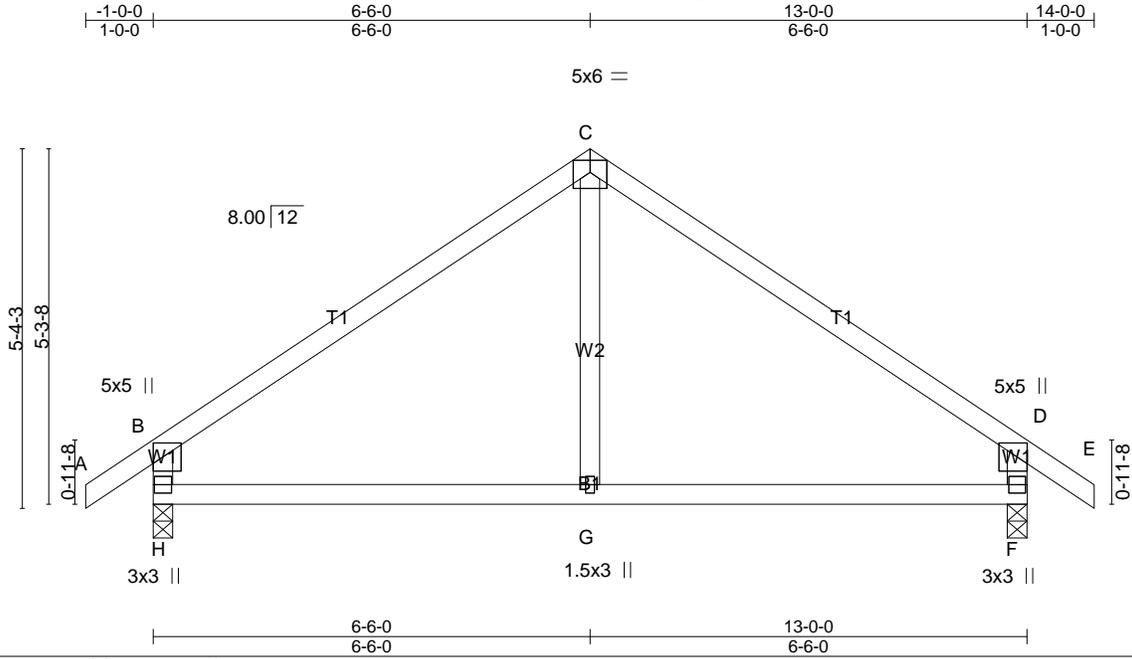


Job 72285969	Truss C2	Truss Type Common	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton

Job Reference (optional)
8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:21 2022 Page 1

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Scale = 1:34.1

Plate Offsets (X,Y)-- [B:0-2-8,0-1-12], [D:0-2-8,0-1-12]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.75	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.34	Vert(LL) -0.04 G-H >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.10	Vert(CT) -0.08 G-H >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-MR	Horz(CT) 0.01 F n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 55 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	

REACTIONS. (lb/size) H=508/0-3-8 (min. 0-1-8), F=508/0-3-8 (min. 0-1-8)
 Max Horz H=157(LC 11)
 Max Uplift H=87(LC 12), F=87(LC 13)
 Max Grav H=577(LC 2), F=577(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/38, B-C=-539/134, C-D=-539/134, D-E=0/38, B-H=-515/203, D-F=-515/203
 BOT CHORD G-H=0/368, F-G=0/368
 WEBS C-G=0/269

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint H and 87 lb uplift at joint F.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Job 72285969	Truss C3	Truss Type Common	Qty 2	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
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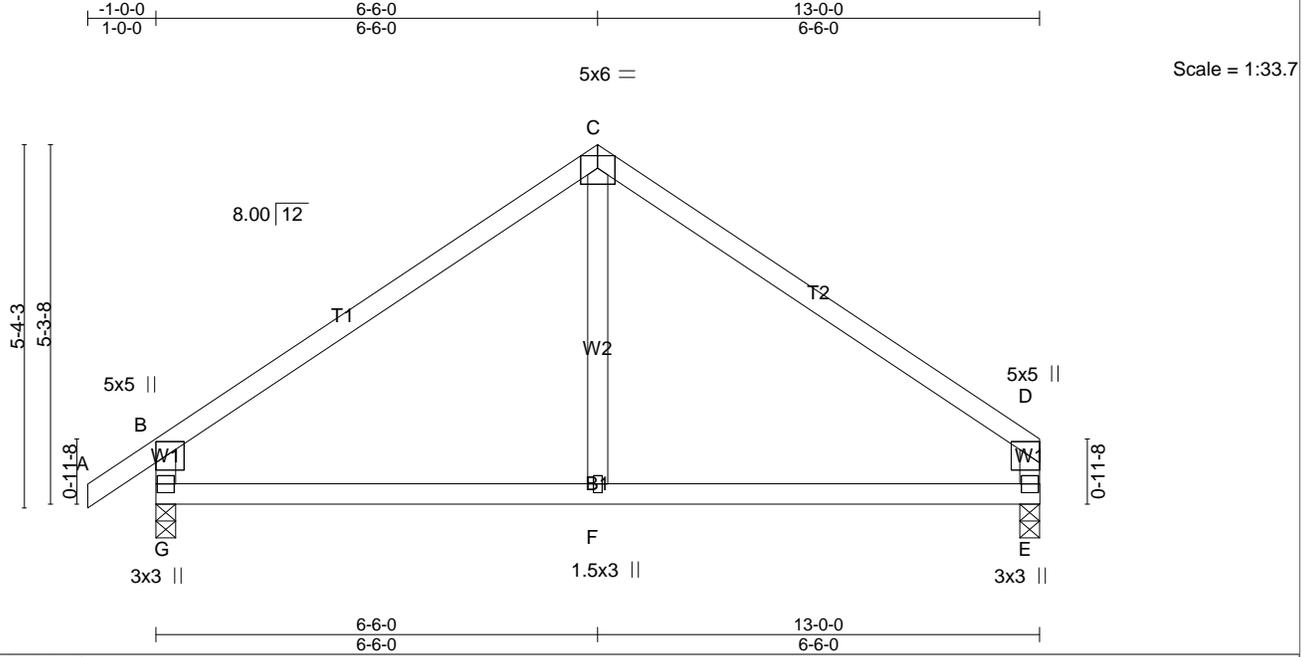


Plate Offsets (X,Y)-- [B:0-2-8,0-1-12]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.74	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.32	Vert(LL) -0.04 F-G >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.10	Vert(CT) -0.08 F-G >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-MR	Horz(CT) 0.01 E n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 53 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

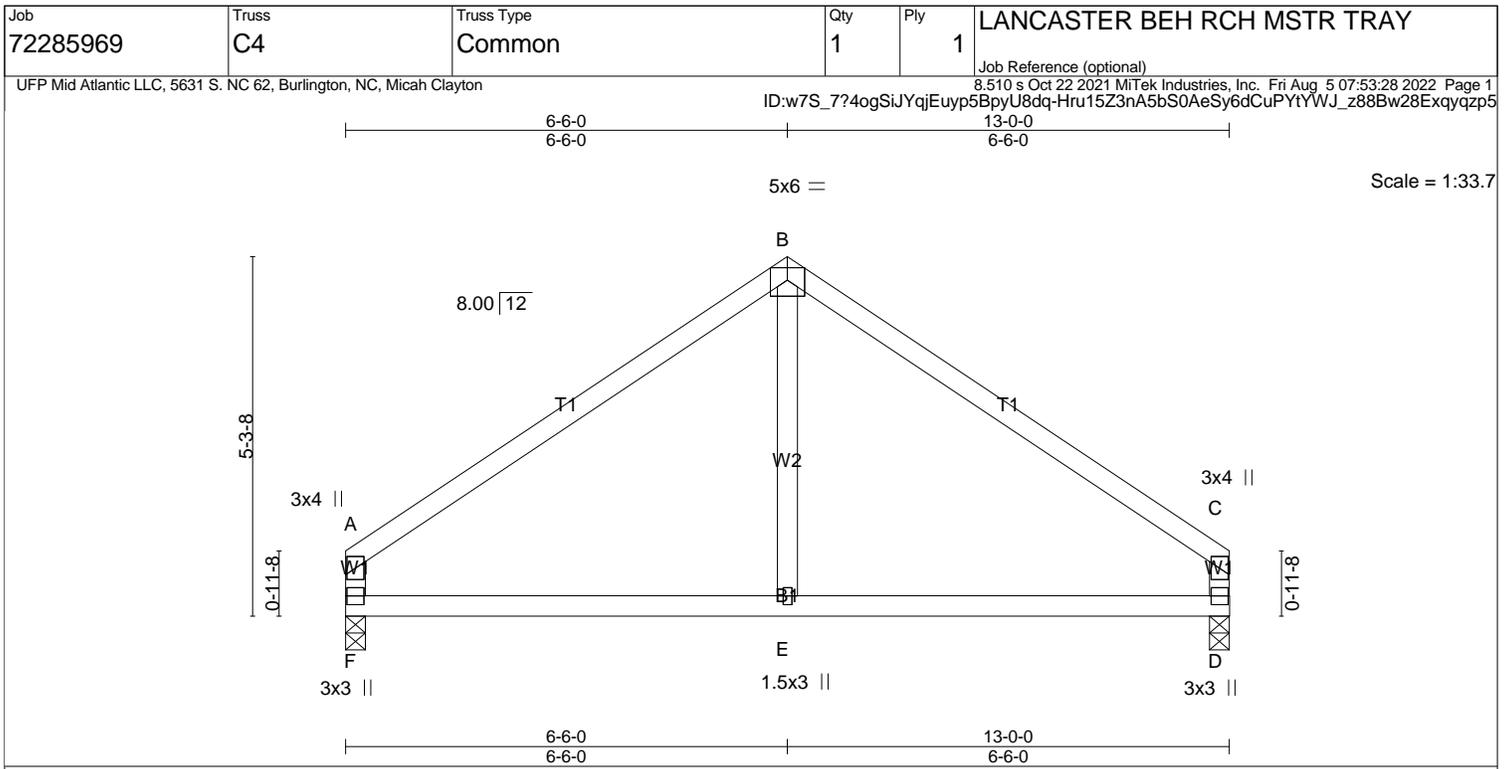
REACTIONS. (lb/size) G=511/0-3-8 (min. 0-1-8), E=447/0-3-8 (min. 0-1-8)
 Max Horz G=150(LC 9)
 Max Uplift G=87(LC 12), E=61(LC 13)
 Max Grav G=580(LC 2), E=505(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/38, B-C=-541/135, C-D=-535/132, B-G=-515/203, D-E=-435/144
 BOT CHORD F-G=-9/360, E-F=-9/360
 WEBS C-F=0/263

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint G and 61 lb uplift at joint E.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard





LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	TC 0.62 BC 0.31 WB 0.10 Matrix-MR	in (loc) l/defl L/d Vert(LL) -0.04 D-E >999 240 Vert(CT) -0.08 D-E >999 180 Horz(CT) 0.01 D n/a n/a	MT20 Weight: 52 lb	244/190 FT = 20%

LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3	BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS. (lb/size) F=450/0-3-8 (min. 0-1-8), D=450/0-3-8 (min. 0-1-8)
 Max Horz F=-137(LC 8)
 Max Uplift F=61(LC 12), D=-61(LC 13)
 Max Grav F=508(LC 2), D=508(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-538/133, B-C=-538/133, A-F=-436/144, C-D=-436/144
 BOT CHORD E-F=-9/363, D-E=-9/363
 WEBS B-E=0/257

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint F and 61 lb uplift at joint D.
 - 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

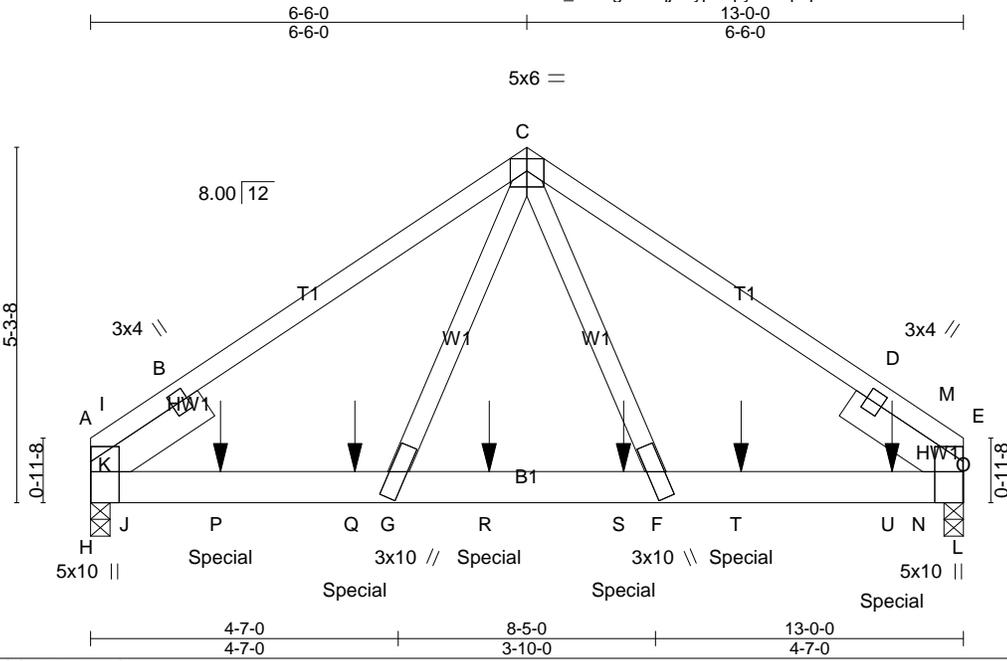


This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.



Job 72285969	Truss C5	Truss Type COMMON	Qty 1	Ply 3	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 Job Reference (optional)
 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:33 2022 Page 1
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Scale = 1:34.1

Plate Offsets (X,Y)-- [A:0-5-8,Edge], [E:0-7-5,Edge]

LOADING (psf) TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.63 BC 0.84 WB 0.60 Matrix-MSH	DEFL. in (loc) l/defl L/d Vert(LL) -0.07 F-G >999 240 Vert(CT) -0.12 F-G >999 180 Horz(CT) 0.03 E n/a n/a	PLATES MT20 GRIP 244/190 Weight: 235 lb FT = 20%
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LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x6 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x6 SP No.2 2-0-0, Right 2x6 SP No.2 2-0-0

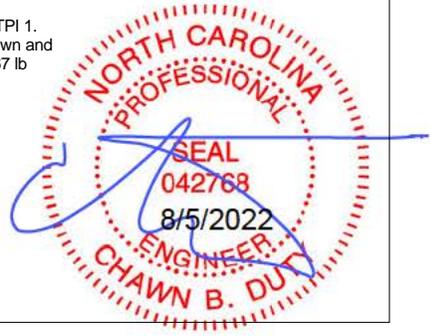
BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=5923/0-3-8 (min. 0-3-2), E=6633/0-3-8 (min. 0-3-8)
 Max Horz A=-112(LC 31)
 Max Uplift A=-714(LC 10), E=-799(LC 11)
 Max Grav A=5952(LC 4), E=6672(LC 4)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-I=-5558/792, B-I=-4972/689, B-C=-7187/902, C-D=-7290/914, D-M=-5120/707, E-M=-5710/811
 BOT CHORD H-J=-489/3431, J-P=-703/5901, P-Q=-703/5901, G-Q=-703/5901, G-R=-507/4334, R-S=-507/4334, F-S=-507/4334, F-T=-676/5989,
 T-U=-676/5989, N-U=-676/5989, L-N=-428/3539
 WEBS C-G=-513/4115, C-F=-540/4345, A-H=-4445/605, J-K=-651/3208, I-K=-199/1032, A-K=-531/3609, H-K=-3823/465, B-K=-2388/591,
 E-L=-4562/620, N-O=-668/3357, M-O=-200/1039, E-O=-543/3708, L-O=-3946/482, D-O=-2342/585

- NOTES-**
- 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-4-0 oc.
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 714 lb uplift at joint A and 799 lb uplift at joint E.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1967 lb down and 239 lb up at 1-11-4, 1967 lb down and 239 lb up at 3-11-4, 1967 lb down and 239 lb up at 5-11-4, 1967 lb down and 239 lb up at 7-11-4, and 1967 lb down and 239 lb up at 9-8-4, and 1967 lb down and 239 lb up at 11-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: H-L=-20, A-C=-51, C-E=-51
 Concentrated Loads (lb)
 Vert: P=-1939(B) Q=-1939(B) R=-1939(B) S=-1939(B) T=-1939(B) U=-1939(B)



Job 72285969	Truss PB1	Truss Type GABLE	Qty 2	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 Job Reference (optional) 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:37 2022 Page 1
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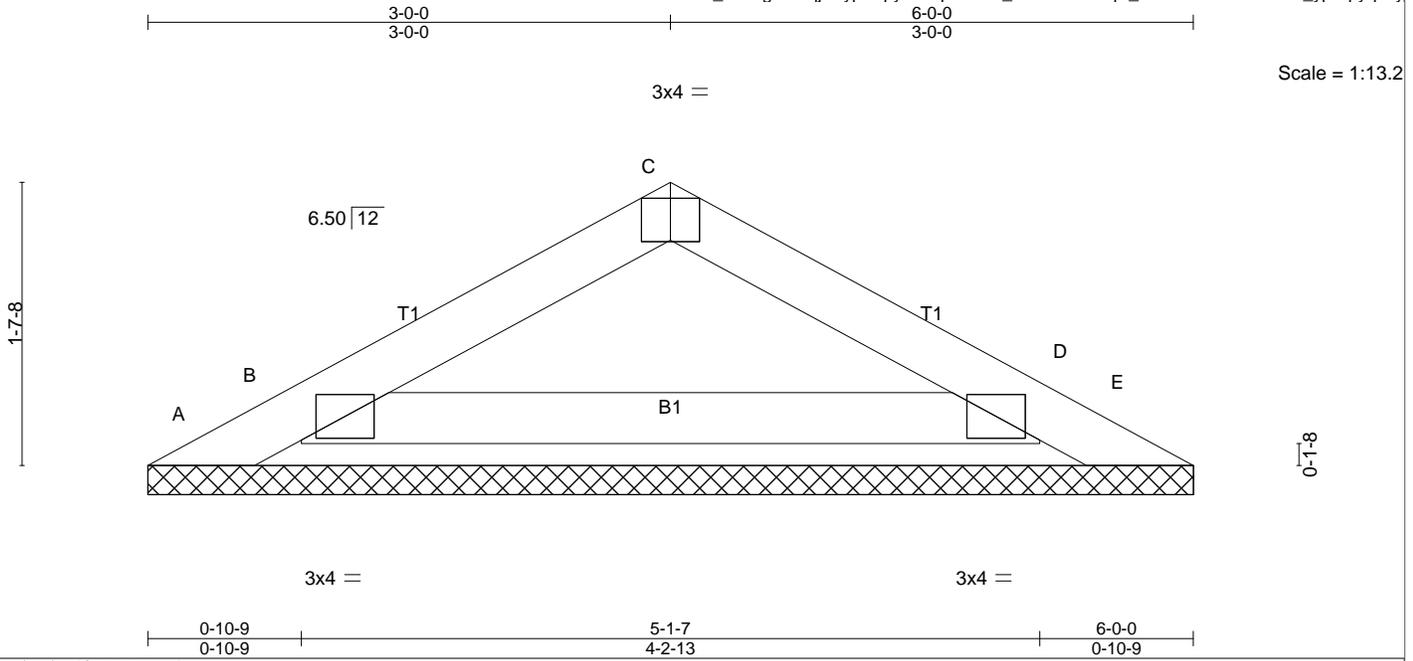


Plate Offsets (X,Y)-- [C:0-2-0,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.07	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.22	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.00	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 E n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 16 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=-23/6-0-0 (min. 0-2-7), E=-23/6-0-0 (min. 0-2-7), B=215/6-0-0 (min. 0-2-7), D=215/6-0-0 (min. 0-2-7)
 Max Horz A=-29(LC 10)
 Max Uplift A=-37(LC 28), E=-28(LC 2), B=-72(LC 14), D=-59(LC 15)
 Max Grav A=38(LC 14), E=26(LC 15), B=245(LC 2), D=245(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-39/53, B-C=-139/60, C-D=-139/60, D-E=-12/32
 BOT CHORD B-D=-11/88

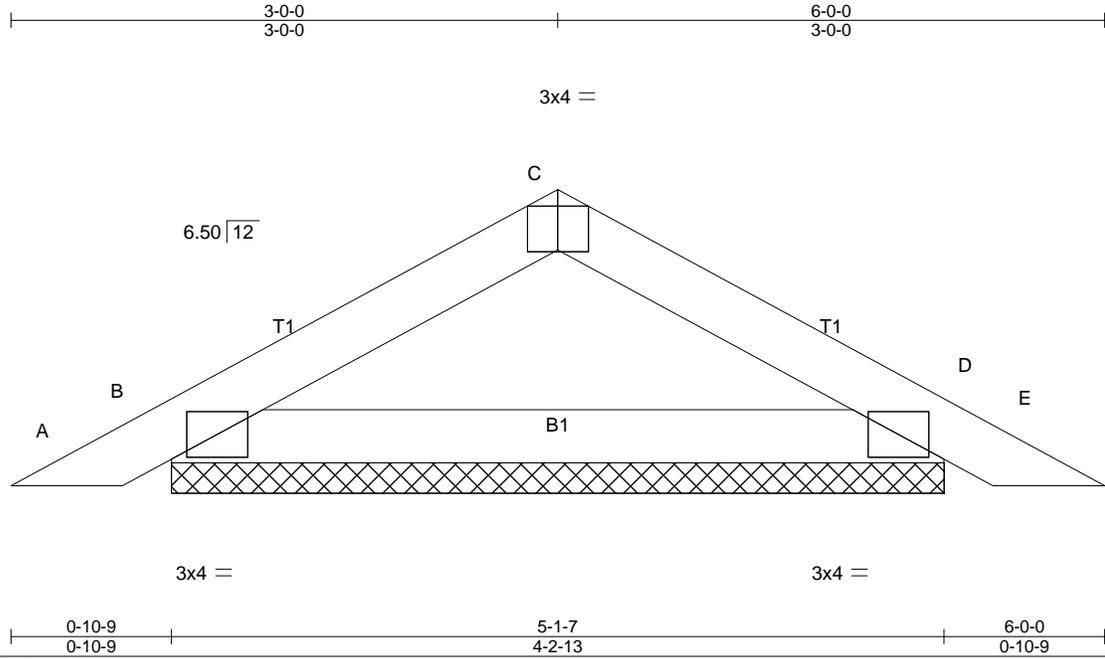
- NOTES-** (12)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone: cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only.
 - 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 5) Roof design snow load has been reduced to account for slope.
 - 6) Unbalanced snow loads have been considered for this design.
 - 7) Gable requires continuous bottom chord bearing.
 - 8) Gable studs spaced at 1-4-0 oc.
 - 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 12) Piggyback cap bottom chord to be attached to 2x4 purlins located at each end of cap bottom chord and at 24" oc max spacing with two 16d nails each.

LOAD CASE(S) Standard



Job 72285969	Truss PB2	Truss Type Piggyback	Qty 24	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 Job Reference (optional)
 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:40 2022 Page 1
 ID:w7S_7?4ogSiYqjEuy5BpyU8dq-w9cacgDJLn6IT0Zmfdro0x23BLQ6nbeygw2tM8yqzov



Scale = 1:12.6

Plate Offsets (X,Y)-- [C:0-2-0,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.07	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.22	Vert(LL) 0.00 E n/r 120		
TCDL 10.0	Lumber DOL 1.15	WB 0.00	Vert(CT) 0.00 E n/r 90		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 D n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 16 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

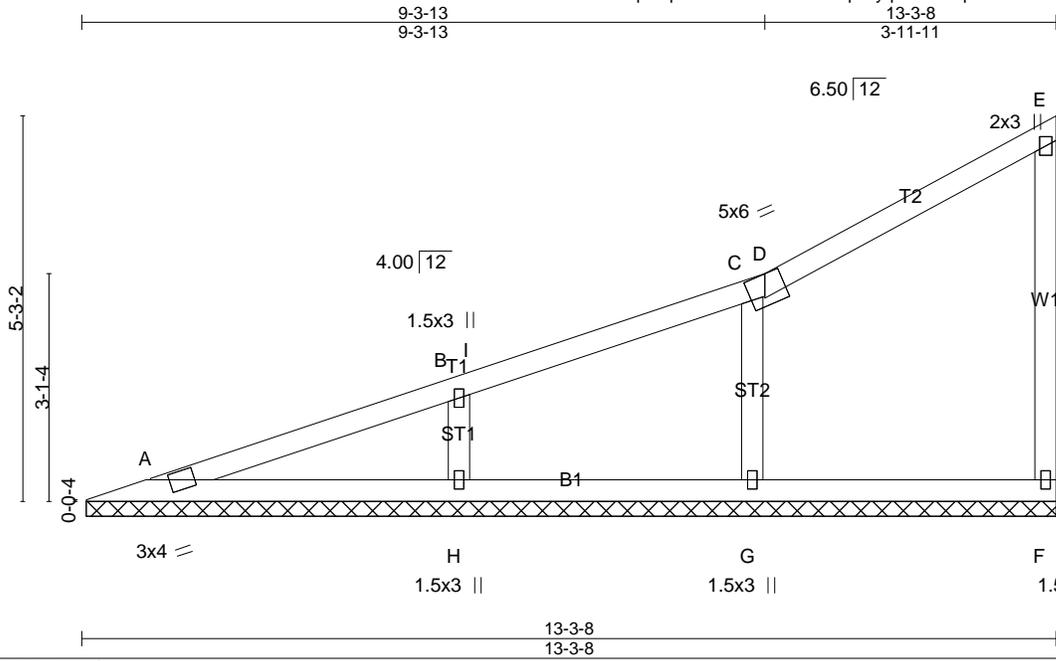
REACTIONS. (lb/size) B=179/4-2-13 (min. 0-1-8), D=179/4-2-13 (min. 0-1-8)
 Max Horz B=-29(LC 12)
 Max Uplift B=-36(LC 14), D=-36(LC 15)
 Max Grav B=204(LC 2), D=204(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/15, B-C=-154/67, C-D=-154/67, D-E=0/15
 BOT CHORD B-D=-21/105

- NOTES-** (11)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) Unbalanced snow loads have been considered for this design.
 - 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 7) Gable requires continuous bottom chord bearing.
 - 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 11) Piggyback cap bottom chord to be attached to 2x4 purlins located at each end of cap bottom chord and at 24" oc max spacing with two 16d nails each.

LOAD CASE(S) Standard





Scale = 1:31.3

Plate Offsets (X,Y)-- [D:0-3-0,0-1-11]

LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.27	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0**	Plate Grip DOL 1.15	BC 0.13	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.06	Lumber DOL n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-SH	Horz(CT) -0.00 F n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 52 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=121/13-2-12 (min. 0-1-9), F=124/13-2-12 (min. 0-1-9), G=289/13-2-12 (min. 0-1-9), H=331/13-2-12 (min. 0-1-9)
 Max Horz A=207(LC 13)
 Max Uplift A=9(LC 10), F=35(LC 11), G=90(LC 14), H=111(LC 10)
 Max Grav A=136(LC 2), F=149(LC 27), G=325(LC 2), H=374(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-162/38, B-I=-126/10, C-I=-123/40, C-D=-109/42, D-E=-119/68, E-F=-111/50
 BOT CHORD A-H=-75/88, G-H=-75/88, F-G=-75/88
 WEBS C-G=-246/157, B-H=-270/184

- NOTES-**
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps= varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10
 - 3) Roof design snow load has been reduced to account for slope.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint A, 35 lb uplift at joint F, 90 lb uplift at joint G and 111 lb uplift at joint H.
 - 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard
 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: A-D=-51, D-E=-51, A-F=-20



Job 72285969	Truss V2	Truss Type Valley	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton

Job Reference (optional)
8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:47 2022 Page 1

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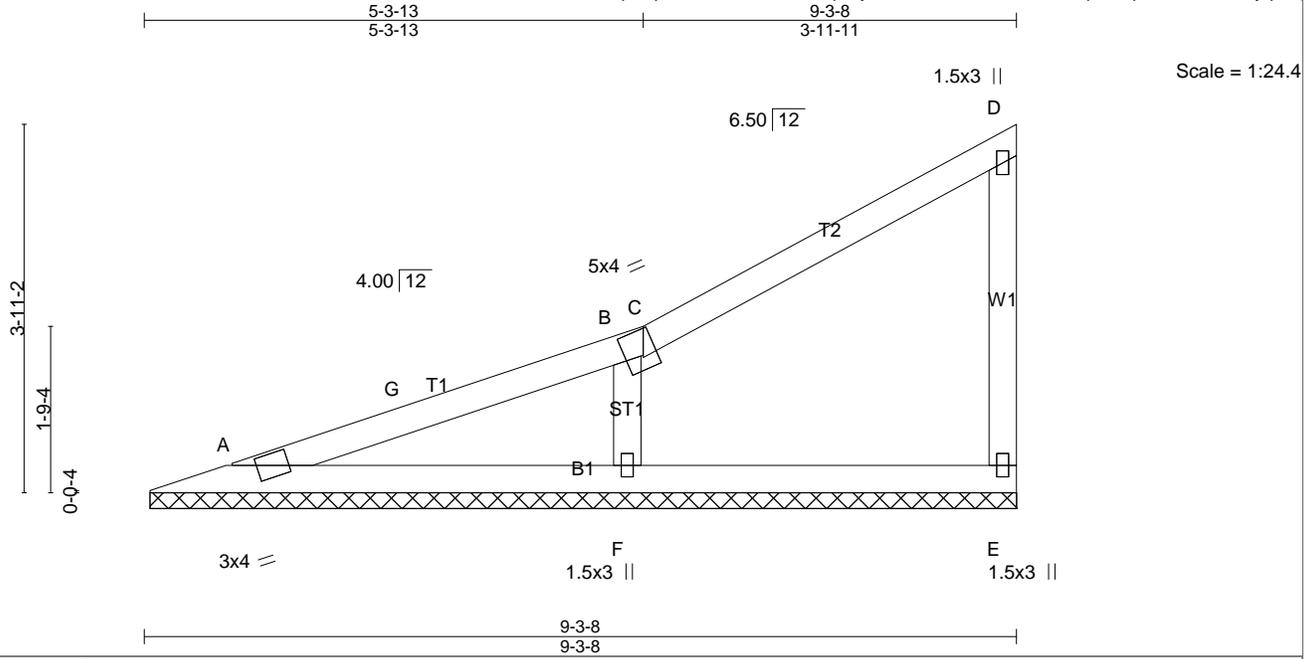


Plate Offsets (X,Y)-- [C:0-1-12,0-3-3]

LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.20	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0**	Plate Grip DOL 1.15	BC 0.16	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.06	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-SH	Horz(CT) -0.00 E n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 34 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	
OTHERS 2x4 SP No.3	

REACTIONS. (lb/size) A=122/9-2-12 (min. 0-1-8), E=114/9-2-12 (min. 0-1-8), F=345/9-2-12 (min. 0-1-8)
 Max Horz A=148(LC 11)
 Max Uplift A=-32(LC 10), E=-30(LC 14), F=-104(LC 14)
 Max Grav A=139(LC 2), E=139(LC 27), F=389(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-G=-111/13, B-G=-103/36, B-C=-97/26, C-D=-108/53, D-E=-106/53
 BOT CHORD A-F=-52/65, E-F=-52/65
 WEBS B-F=-276/190

- NOTES-**
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) ** TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps= varies (min. roof snow=15.4 psf Lumber DOL=1.15 Plate DOL=1.15) see load cases; Category II; Exp B; Partially Exp.; Ct=1.10
 - 3) Roof design snow load has been reduced to account for slope.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint A, 30 lb uplift at joint E and 104 lb uplift at joint F.
 - 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard
 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: A-C=-51, C-D=-51, A-E=-20



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.

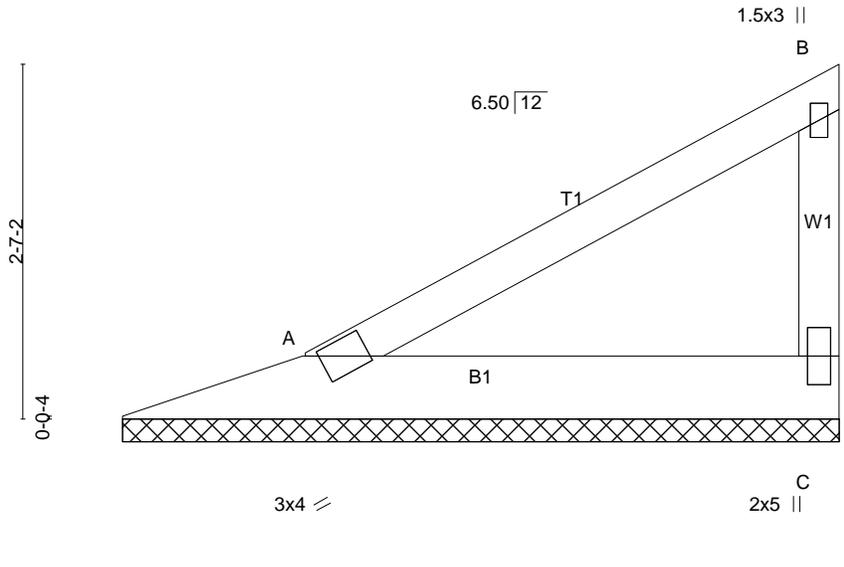


Job 72285969	Truss V3	Truss Type VALLEY	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton

Job Reference (optional)
8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:51 2022 Page 1

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5-3-8
5-3-8



Scale = 1:16.7

LOADING (psf) TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSL TC 0.15 BC 0.08 WB 0.00 Matrix-SH	DEFL Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 C n/a n/a	PLATES GRIP MT20 244/190 Weight: 22 lb FT = 20%
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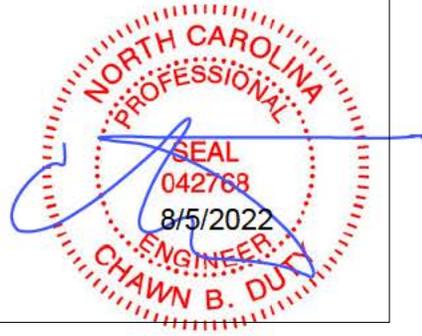
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3	BRACING- TOP CHORD Structural wood sheathing directly applied or 5-3-8 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS. (lb/size) A=133/5-2-12 (min. 0-1-8), C=133/5-2-12 (min. 0-1-8)
Max Horz A=86(LC 14)
Max Uplift A=-7(LC 14), C=-55(LC 14)
Max Grav A=150(LC 2), C=150(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-85/28, B-C=-98/69
BOT CHORD A-C=-12/23

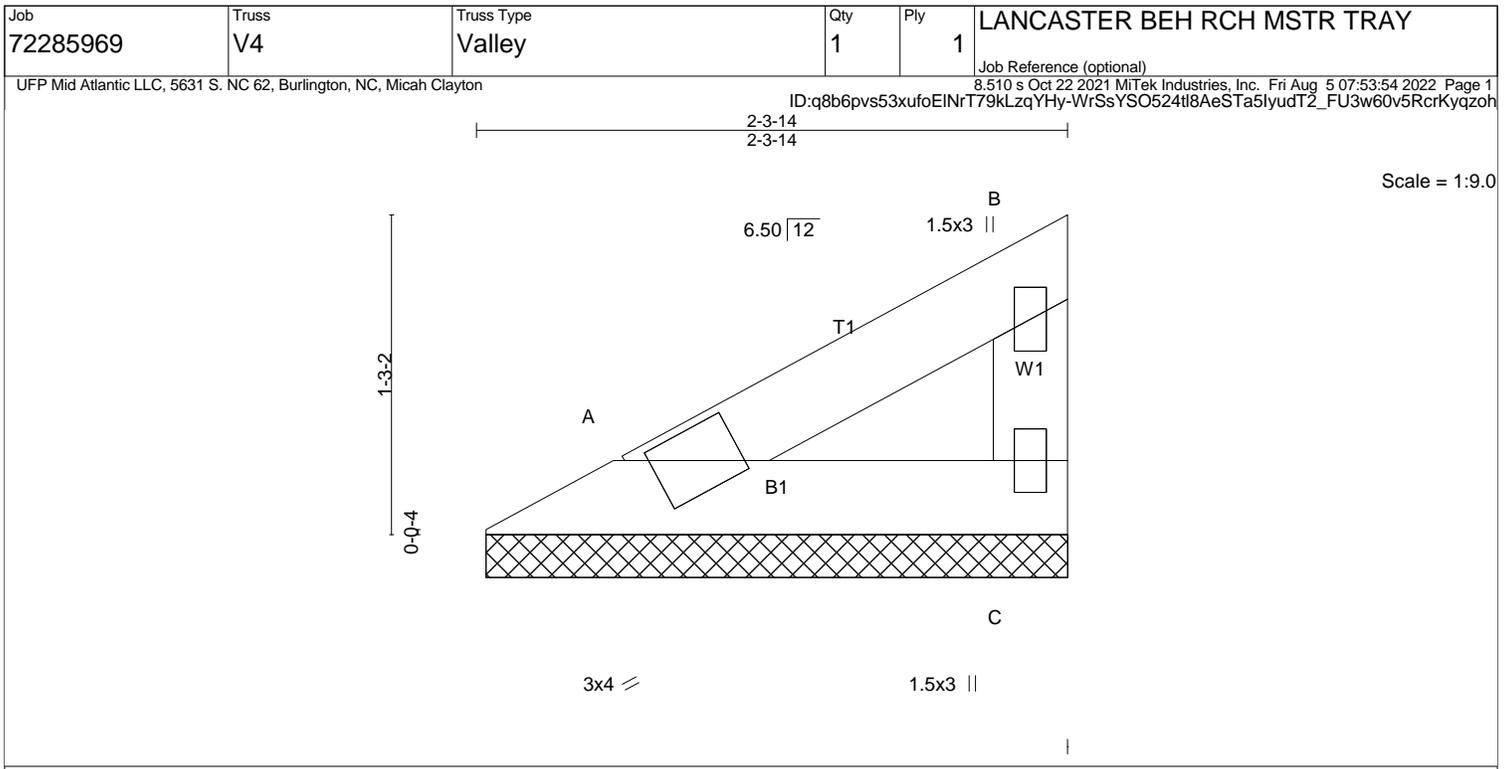
- NOTES-**
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 3) Roof design snow load has been reduced to account for slope.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint A and 55 lb uplift at joint C.
 - 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.





LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.04	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.03	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.00	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 C n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 7 lb	FT = 20%

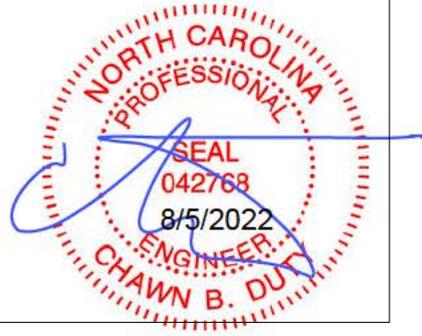
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3	BRACING- TOP CHORD Structural wood sheathing directly applied or 2-3-14 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS. (lb/size) A=56/2-3-7 (min. 0-1-8), C=56/2-3-7 (min. 0-1-8)
 Max Horz A=35(LC 13)
 Max Uplift A=-8(LC 14), C=-18(LC 14)
 Max Grav A=64(LC 2), C=64(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-30/22, B-C=-48/28
 BOT CHORD A-C=-17/18

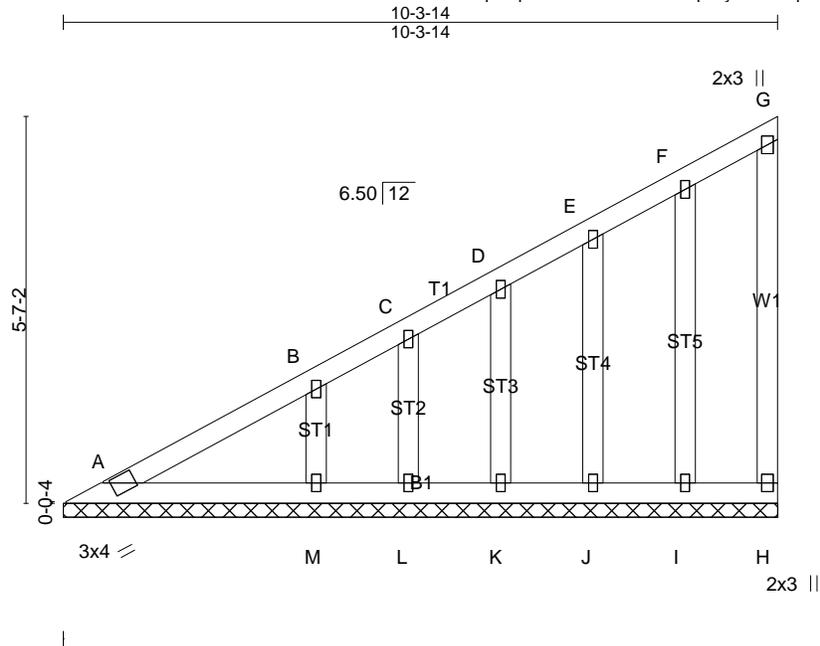
- NOTES-**
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 3) Roof design snow load has been reduced to account for slope.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint A and 18 lb uplift at joint C.
 - 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Job 72285969	Truss V5	Truss Type GABLE	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:53:58 2022 Page 1
 ID:q8b6pvs53xuf0EInRT79kLzqYHy-PciNOqRb6JNBdnxDiP9E6kn43bbN?kVbwpjPq_5yqzod



Scale = 1:33.1

LOADING (psf) TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.30 BC 0.09 WB 0.04 Matrix-SH	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) -0.00 H n/a n/a	PLATES GRIP MT20 244/190 Weight: 61 lb FT = 20%
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LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3	BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS. (lb/size) A=89/10-3-14 (min. 0-1-8), H=35/10-3-14 (min. 0-1-8), I=96/10-3-14 (min. 0-1-8), J=93/10-3-14 (min. 0-1-8), K=107/10-3-14 (min. 0-1-8), L=36/10-3-14 (min. 0-1-8), M=224/10-3-14 (min. 0-1-8)
 Max Horz A=209(LC 11)
 Max Uplift H=33(LC 13), I=41(LC 14), J=34(LC 14), K=43(LC 14), L=15(LC 14), M=89(LC 14)
 Max Grav A=128(LC 28), H=50(LC 27), I=117(LC 20), J=105(LC 2), K=121(LC 2), L=41(LC 2), M=253(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-175/90, B-C=-135/54, C-D=-121/58, D-E=-113/59, E-F=-105/73, F-G=-78/80, G-H=-33/17
 BOT CHORD A-M=-91/99, L-M=-91/99, K-L=-91/99, J-K=-91/99, I-J=-91/99, H-I=-91/99
 WEBS F-I=-89/48, E-J=-90/62, D-K=-91/58, C-L=-44/33, B-M=-180/110

- NOTES-**
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 3) Roof design snow load has been reduced to account for slope.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) All plates are 1.5x3 MT20 unless otherwise indicated.
 - 6) Gable requires continuous bottom chord bearing.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint H, 41 lb uplift at joint I, 34 lb uplift at joint J, 43 lb uplift at joint K, 15 lb uplift at joint L and 89 lb uplift at joint M.
 - 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.

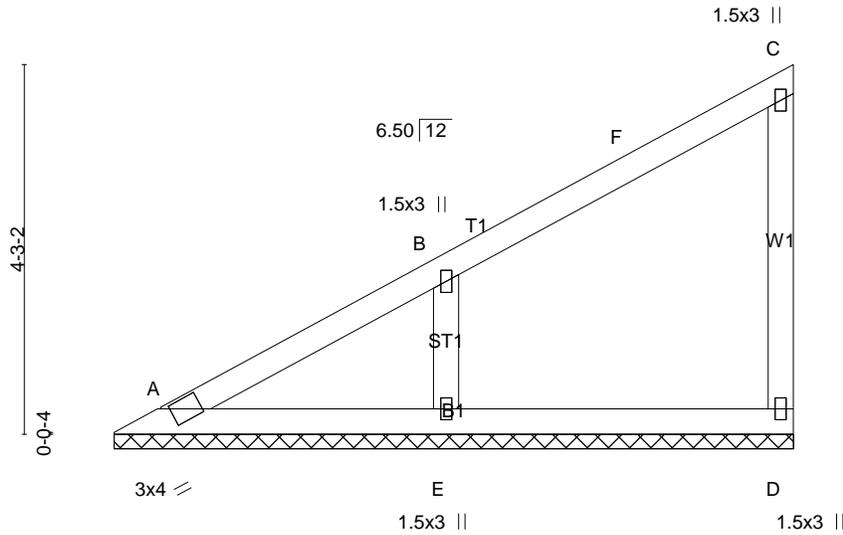


Job 72285969	Truss V6	Truss Type Valley	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton

Job Reference (optional)
8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:54:01 2022 Page 1

ID:q8b6pvs53xuf0EINrT79kLzqYHy-pBNW0sTUPEmmUFgoOYjxkMPcwpdfC411WhdUbQyqzoa
7-10-6
7-10-6



Scale = 1:26.4

LOADING (psf) TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.19 BC 0.12 WB 0.05 Matrix-SH	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) -0.00 D n/a n/a	PLATES GRIP MT20 244/190 Weight: 32 lb FT = 20%
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LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3	BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS. (lb/size) A=87/7-9-14 (min. 0-1-8), D=112/7-9-14 (min. 0-1-8), E=306/7-9-14 (min. 0-1-8)
Max Horz A=156(LC 11)
Max Uplift D=28(LC 11), E=122(LC 14)
Max Grav A=113(LC 28), D=130(LC 27), E=346(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-128/77, B-F=-113/43, C-F=-62/54, C-D=-97/47
BOT CHORD A-E=-62/74, D-E=-62/74
WEBS B-E=-261/170

- NOTES-**
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 3) Roof design snow load has been reduced to account for slope.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint D and 122 lb uplift at joint E.
 - 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.

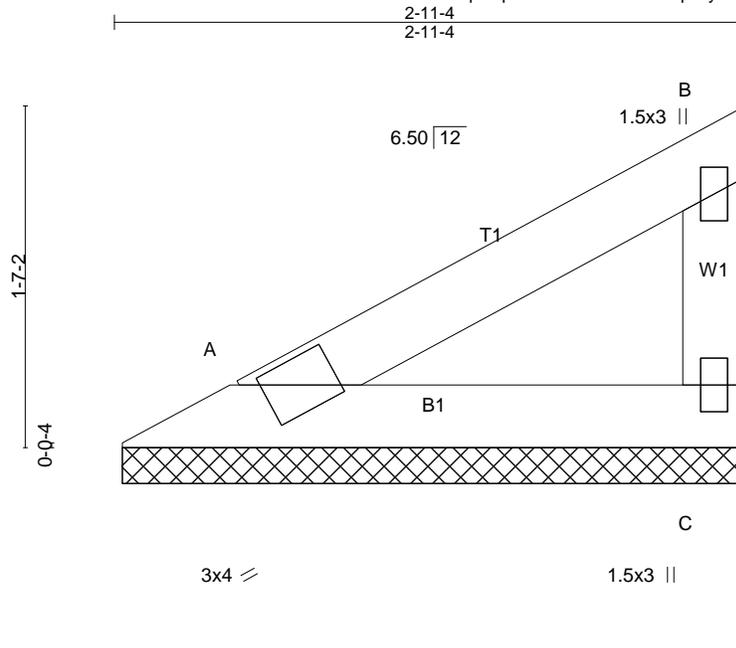


Job 72285969	Truss V8	Truss Type Valley	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton

Job Reference (optional)

8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:54:08 2022 Page 1
ID:q8b6pvs53xuf0EInrT79kLzqYHy-6Xl9UFYtInemqKi8lWLaWrCrrd1LLFa37HqMLWYqzoT



LOADING (psf) TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.05 WB 0.00 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 C n/a n/a	PLATES GRIP MT20 244/190 Weight: 10 lb FT = 20%
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LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-11-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=78/2-10-13 (min. 0-1-8), C=78/2-10-13 (min. 0-1-8)
Max Horz A=48(LC 11)
Max Uplift A=-11(LC 14), C=-25(LC 14)
Max Grav A=88(LC 2), C=88(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-42/30, B-C=-66/38
BOT CHORD A-C=-23/25

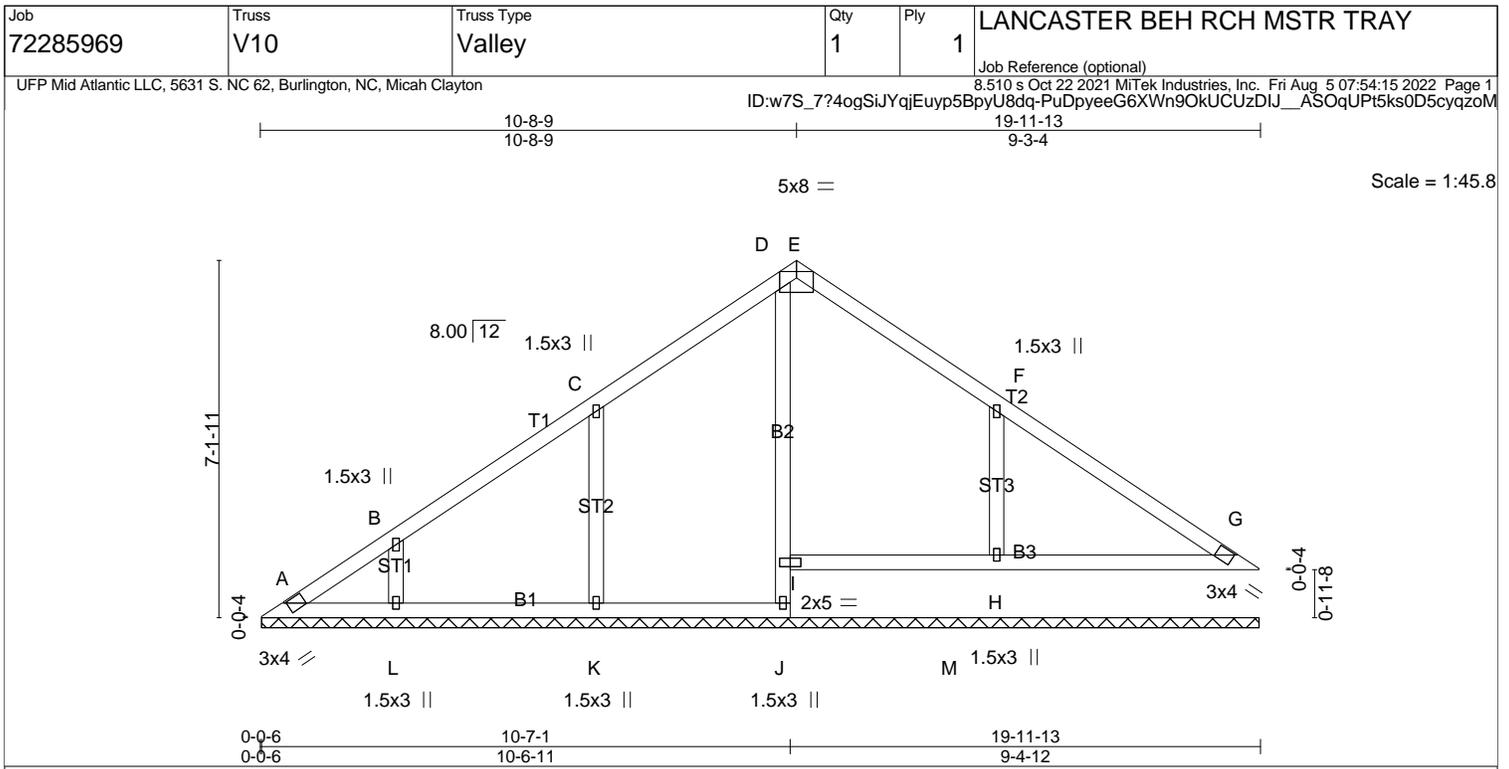
- NOTES-**
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 3) Roof design snow load has been reduced to account for slope.
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint A and 25 lb uplift at joint C.
 - 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.





LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	TC 0.27 BC 0.19 WB 0.09 Matrix-SH	in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.01 G n/a n/a	MT20 Weight: 85 lb	244/190 FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
 B2: 2x4 SP No.3
OTHERS 2x4 SP No.3

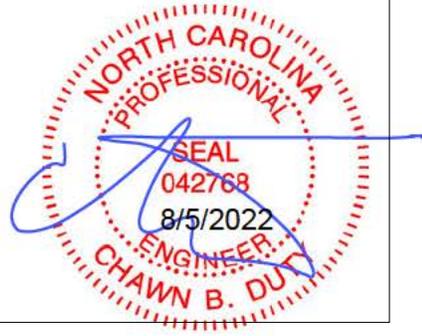
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
 10-0-0 oc bracing: I-J.

REACTIONS. (lb/size) A=51/19-11-1 (min. 0-2-8), G=141/19-11-1 (min. 0-2-8), J=32/19-11-1 (min. 0-2-8), L=200/19-11-1 (min. 0-2-8), K=291/19-11-1 (min. 0-2-8), L=248/19-11-1 (min. 0-2-8), H=383/19-11-1 (min. 0-2-8)
Max Horz A=173(LC 9)
Max Uplift A=67(LC 8), G=33(LC 13), I=14(LC 9), K=149(LC 12), L=118(LC 12), H=174(LC 13)
Max Grav A=117(LC 24), G=164(LC 28), J=104(LC 18), I=324(LC 23), K=417(LC 23), L=290(LC 23), H=505(LC 24)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-195/188, B-C=-165/157, C-D=-163/209, D-E=-133/130, E-F=-166/179, F-G=-93/75
BOT CHORD A-L=-29/44, K-L=-29/44, J-K=-29/44, I-J=0/0, D-I=-233/34, I-M=-34/51, H-M=-34/51, G-H=-34/51
WEBS C-K=-275/198, B-L=-229/158, F-H=-332/222

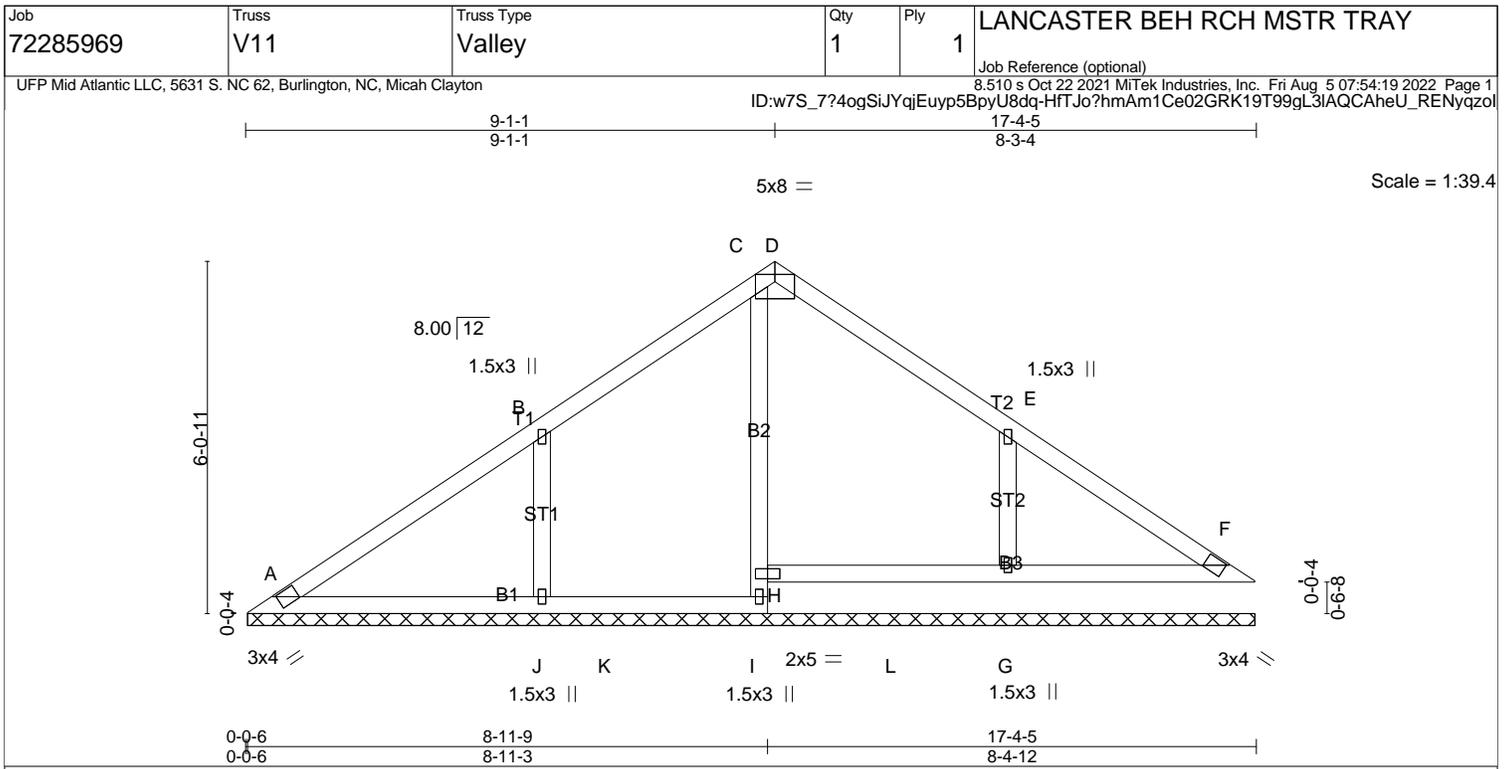
- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 8) Bearing at joint(s) I considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint A, 33 lb uplift at joint G, 14 lb uplift at joint I, 149 lb uplift at joint K, 118 lb uplift at joint L and 174 lb uplift at joint H.
 - 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) G, H.
 - 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.





LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	TC 0.26 BC 0.16 WB 0.07 Matrix-SH	in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 F n/a n/a	MT20 Weight: 71 lb	244/190 FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
 B2: 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: H-1.

REACTIONS. (lb/size) A=142/17-3-9 (min. 0-2-1), F=113/17-3-9 (min. 0-2-1), I=25/17-3-9 (min. 0-2-1), H=194/17-3-9 (min. 0-2-1), J=352/17-3-9 (min. 0-2-1), G=334/17-3-9 (min. 0-2-1)
Max Horz A=147(LC 9)
Max Uplift A=30(LC 13), F=9(LC 13), H=4(LC 9), J=176(LC 12), G=152(LC 13)
Max Grav A=174(LC 24), F=135(LC 28), I=96(LC 18), H=291(LC 23), J=458(LC 23), G=425(LC 24)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD A-B=-169/150, B-C=-138/159, C-D=-125/114, D-E=-129/130, E-F=-91/47
BOT CHORD A-J=-38/58, J-K=-38/58, I-K=-38/58, H-I=0/0, C-H=-205/27, H-L=-40/63, G-L=-40/63, F-G=-40/63
WEBS B-J=-311/215, E-G=-294/199

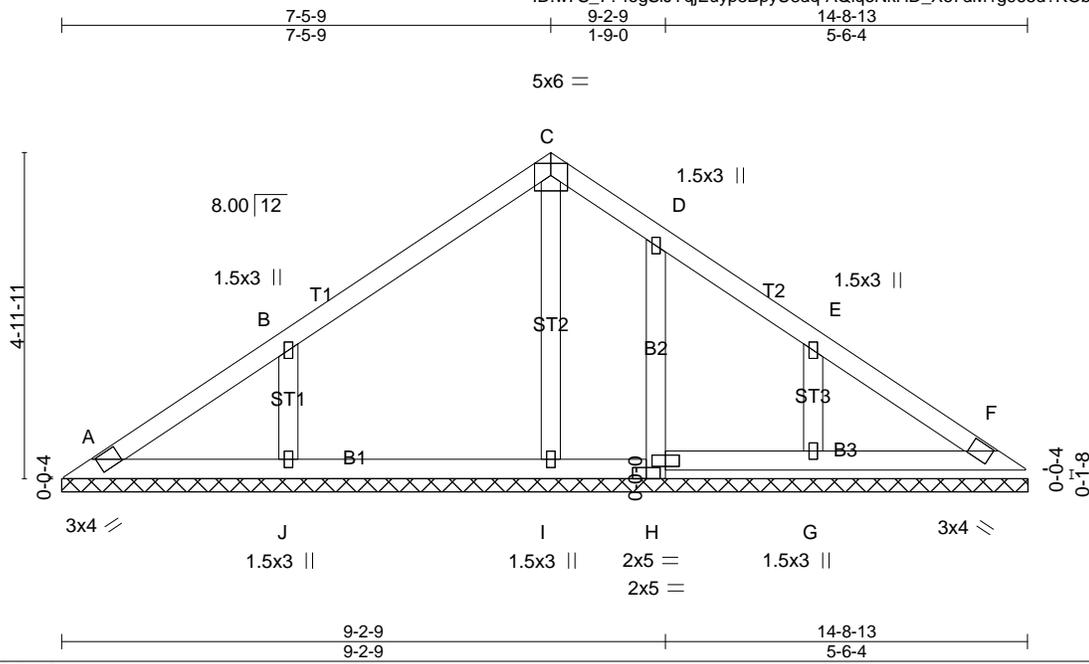
- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 8) Bearing at joint(s) H considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint A, 9 lb uplift at joint F, 4 lb uplift at joint H, 176 lb uplift at joint J and 152 lb uplift at joint G.
 - 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) F, G.
 - 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Job 72285969	Truss V12	Truss Type GABLE	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 ID:w7S_7?4ogSiJYqjEuyp5BpyU8dq-AQiqeNkHD_Xe7dM1g965d?KObg72L1MHZ6yeN8yqzoE
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Scale = 1:35.0

Plate Offsets (X,Y)-- [H:0-6-0,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.18	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.10	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.06	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-SH	Horz(CT) 0.00 F n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 63 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2 *Except*
 B2: 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=86/14-8-13 (min. 0-1-12), F=93/14-8-13 (min. 0-1-12), H=100/14-8-13 (min. 0-1-12), I=180/14-8-13 (min. 0-1-12), J=294/14-8-13 (min. 0-1-12), G=222/14-8-13 (min. 0-1-12)
 Max Horz A=120(LC 9)
 Max Uplift A=6(LC 8), H=66(LC 13), J=147(LC 12), G=102(LC 13)
 Max Grav A=107(LC 24), F=106(LC 2), H=134(LC 24), I=210(LC 23), J=357(LC 23), G=266(LC 24)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-101/89, B-C=-114/101, C-D=-109/114, D-E=-80/66, E-F=-90/50
 BOT CHORD A-J=-40/81, I-J=-40/81, H-I=-41/81, G-H=-39/80, F-G=-39/80, D-H=-116/82
 WEBS C-I=-146/17, B-J=-273/188, E-G=-195/131

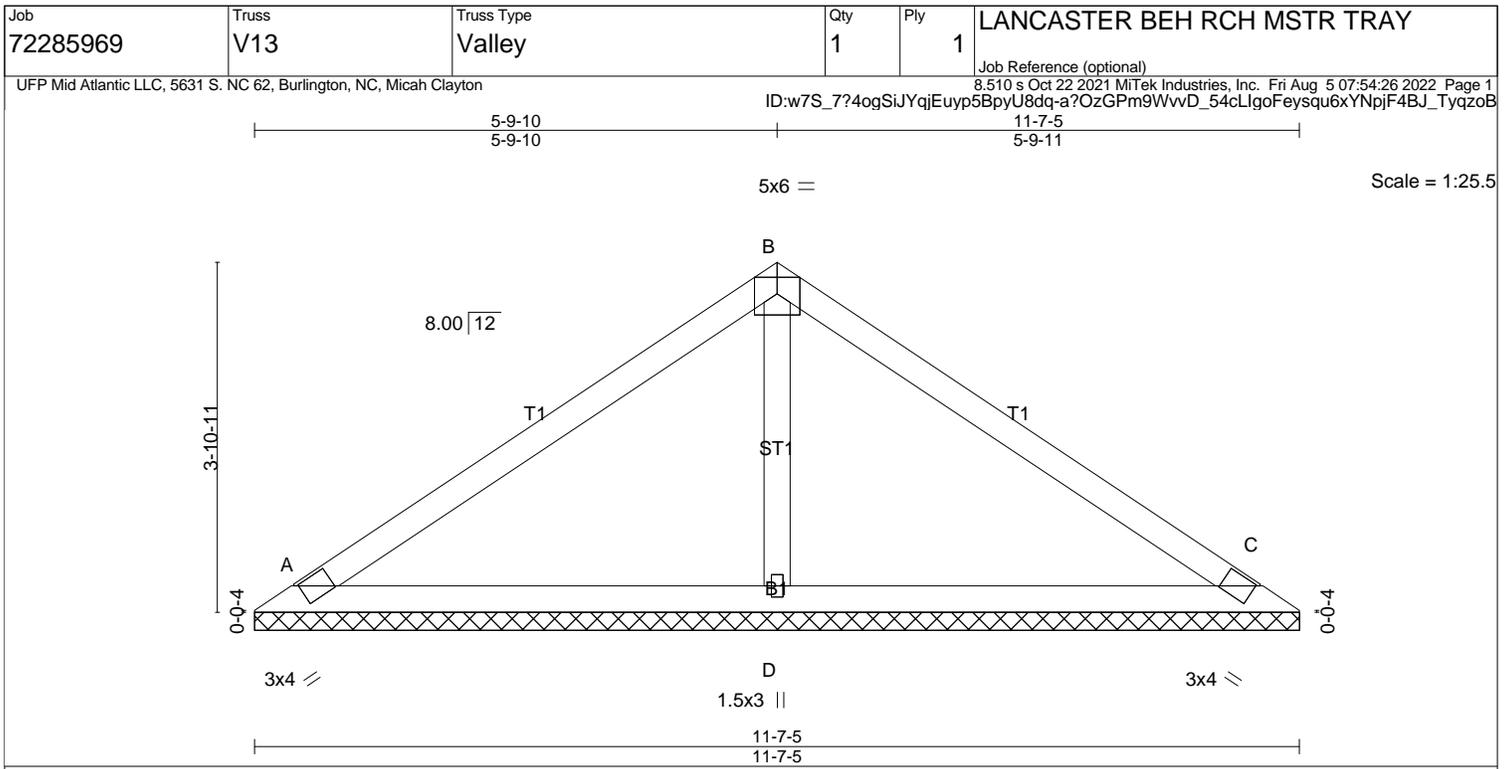
- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) H considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 6 lb uplift at joint A, 66 lb uplift at joint H, 147 lb uplift at joint J and 102 lb uplift at joint G.
 - Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) F, G.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.





LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.37	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.28	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.08	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-SH	Horz(CT) 0.00 C n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 41 lb	FT = 20%

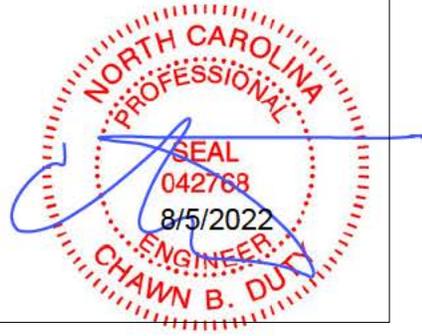
LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.3	

REACTIONS. (lb/size) A=181/11-7-5 (min. 0-1-8), C=181/11-7-5 (min. 0-1-8), D=397/11-7-5 (min. 0-1-8)
 Max Horz A=92(LC 8)
 Max Uplift A=-39(LC 12), C=-52(LC 13), D=-29(LC 12)
 Max Grav A=206(LC 2), C=206(LC 2), D=445(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-152/83, B-C=-144/69
 BOT CHORD A-D=-17/64, C-D=-17/64
 WEBS B-D=-264/89

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint A, 52 lb uplift at joint C and 29 lb uplift at joint D.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

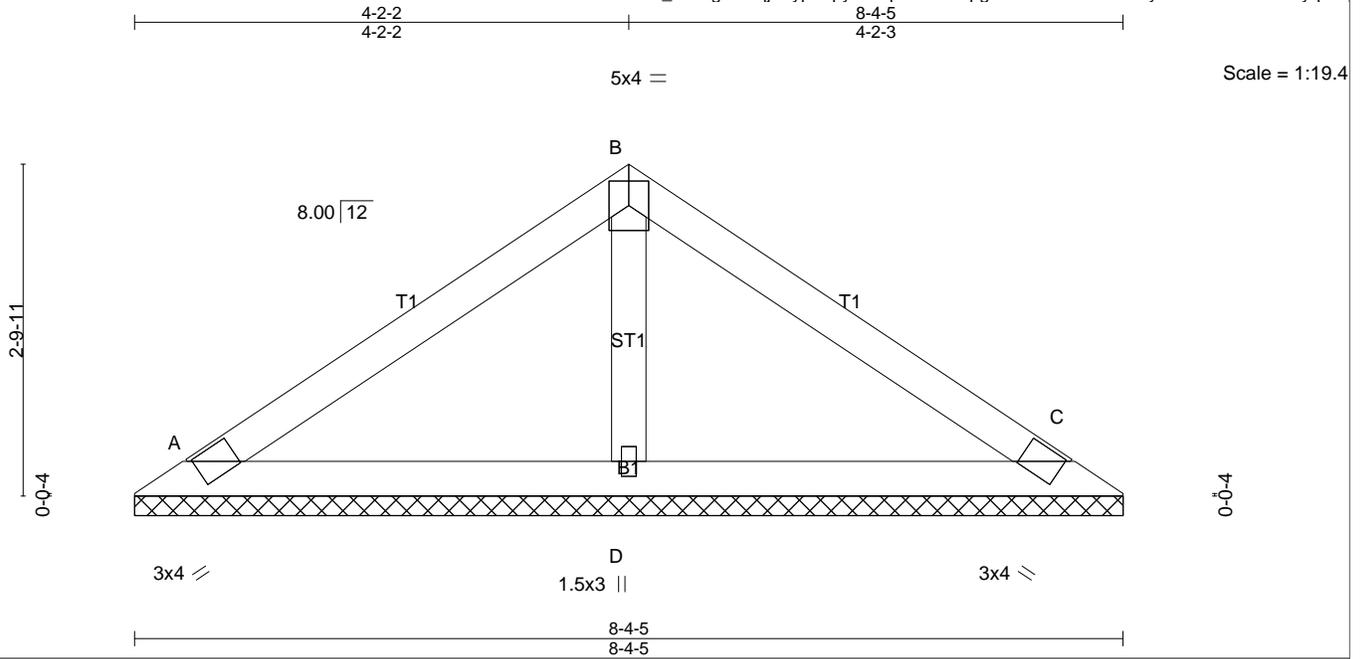
LOAD CASE(S) Standard



Job 72285969	Truss V14	Truss Type Valley	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 Job Reference (optional)
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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.17	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.14	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.04	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-SH	Horz(CT) 0.00 C n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014			Weight: 29 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.
OTHERS 2x4 SP No.3	

REACTIONS. (lb/size) A=126/8-4-5 (min. 0-1-8), C=126/8-4-5 (min. 0-1-8), D=276/8-4-5 (min. 0-1-8)
 Max Horz A=64(LC 8)
 Max Uplift A=-27(LC 12), C=-36(LC 13), D=-20(LC 12)
 Max Grav A=143(LC 2), C=143(LC 2), D=310(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-106/58, B-C=-100/50
 BOT CHORD A-D=-12/44, C-D=-12/44
 WEBS B-D=-184/66

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint A, 36 lb uplift at joint C and 20 lb uplift at joint D.
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



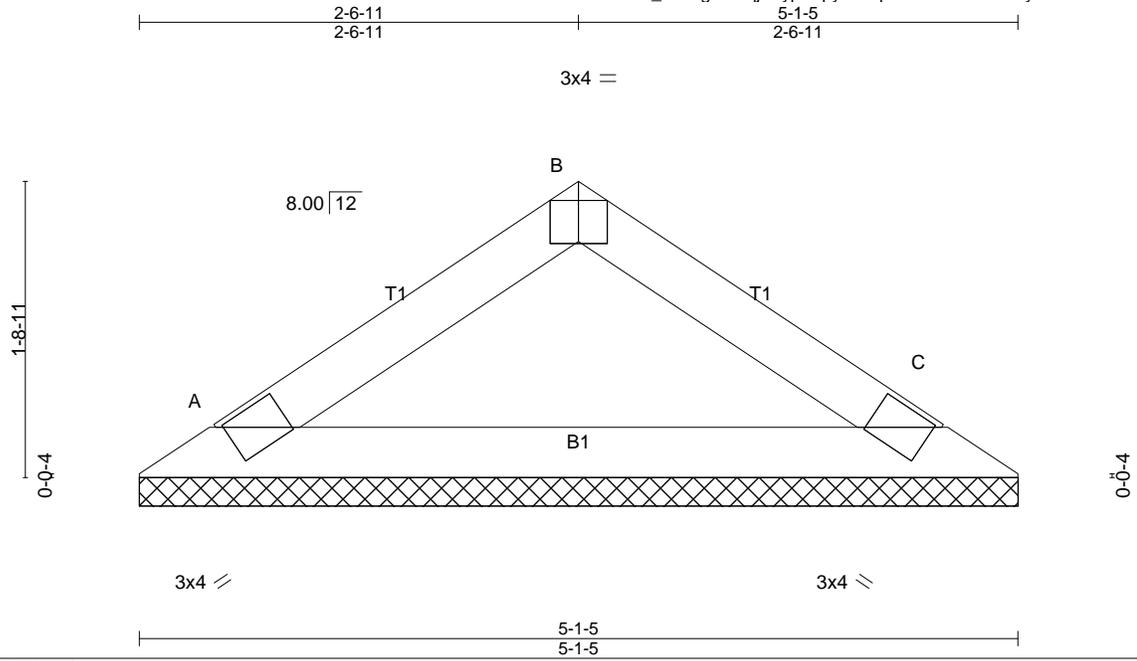
This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.



Job 72285969	Truss V15	Truss Type Valley	Qty 1	Ply 1	LANCASTER BEH RCH MSTR TRAY
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton
 Job Reference (optional) 8.510 s Oct 22 2021 MiTek Industries, Inc. Fri Aug 5 07:54:33 2022 Page 1

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Scale = 1:13.3

Plate Offsets (X,Y)-- [B:0-2-0,Edge]

LOADING (psf) TCLL (roof) 20.0 Snow (Ps/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.09 BC 0.13 WB 0.00 Matrix-SH	DEFL. Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 C n/a n/a	PLATES MT20 GRIP 244/190 Weight: 15 lb FT = 20%
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LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 5-2-1 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=149/5-1-5 (min. 0-1-8), C=149/5-1-5 (min. 0-1-8)
 Max Horz A=-36(LC 8)
 Max Uplift A=-21(LC 12), C=-21(LC 13)
 Max Grav A=168(LC 2), C=168(LC 2)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-162/63, B-C=-162/63
 BOT CHORD A-C=-13/119

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Ps=15.4 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint A and 21 lb uplift at joint C.
 - 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



This truss is to be fabricated per ANSI/TPI quality requirements. Plates shall be of size and type shown and centered at joints unless otherwise noted. This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFP company. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, delivery, erection and bracing available from SBCA and Truss Plate Institute.

