

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 24818-24818A KB Home 238.2338.B

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I43618186 thru I43618199

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



November 13,2020

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the design for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



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TRENCO A MITek Affiliate 818 Soundside Road

Edenton, NC 27932



	12-0-6	18-2-0	23-10-11		36-0-8		
	12-0-6	6-1-10	5-8-11	1	12-1-13	1	
Plate Offsets (X,Y) [11:0-4-0),0-4-8], [14:0-4-0,0-4-8]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.63 BC 0.80 WB 0.33	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.25 2-14 -0.43 10-11 0.07 10	l/defl L/d >999 240 >994 180 n/a n/a	PLATES MT20	GRIP 244/190
BCDI 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 250 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3		BF TC BC JC	RACING- DP CHORD DT CHORD DINTS	Structural wood Rigid ceiling diru 1 Brace at Jt(s):	sheathing directly ap ectly applied or 10-0-0 : 15	pplied or 3-6-8 oc purlins 0 oc bracing.	
REACTIONS. (size) 2=0- Max Horz 2=12 Max Uplift 2=-5 Max Grav 2=14	3-8, 10=Mechanical 29(LC 20) 3(LC 16), 10=-37(LC 17) 495(LC 2), 10=1432(LC 2)						
FORCES. (lb) - Max. Comp./// TOP CHORD 2-4=-2502/93, 8-10=-2459/11 BOT CHORD 2-14=-110/215 WEBS 4-14=-551/234	lax. Ten All forces 250 (lb) or less exc 4-5=-2262/109, 5-6=-350/78, 6-7=-351/ 12 52, 11-14=0/1684, 10-11=-7/2103 4, 5-14=-37/787, 7-11=-32/763, 8-11=-52	ept when shown. 82, 7-8=-2210/124, 34/236, 5-15=-1436/94.					
 WEBS 4-14=53/723- 7-15=-1436/94 NOTES- 1) Unbalanced roof live loads ha 2) Wind: ASCE 7-10; Vult=115m gable end zone and C-C Exte 35-11-12 zone; cantilever left reactions shown; Lumber DO 3) TCLL: ASCE 7-10; PT=20.0 p roof snow: Lumber DDL=1.1f 4) Unbalanced snow loads have 5) This truss has been designed non-concurrent with other live 6) This truss has been designed non-concurrent with other live 6) This truss has been designed will fit between the bottom ch 8) Refer to girder(s) for truss to 9) Provide mechanical connection 10) One H2.5A Simpson Strong connection is for uplift only a 11) ATTIC SPACE SHOWN IS J 	ave been considered for this design. app Vasd=91mph; TCDL=6.0psf; BCDL= prior(2) -0-10-8 to 2-8-12, Interior(1) 2-8- and right exposed ; end vertical left and L=1.60 plate grip DOL=1.60 sf (roof live load: Lumber DOL=1.15 Pla 5 Plate DOL=1.15); Category II; Exp B; F been considered for this design. If or greater of min roof live load of 12.0 a loads. If or a 10.0 psf bottom chord live load no ad for a live load of 20.0psf on the bottor ord and any other members, with BCDL truss connections. on (by others) of truss to bearing plate ca- I-Tie connectors recommended to conne and does not consider lateral forces. DESIGNED AS UNINHABITABLE.	=6.0psf; h=30ft; Cat. II; Ex 12 to 18-2-0, Exterior(2) ' right exposed;C-C for me te DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10 psf or 1.00 times flat roof nconcurrent with any othen n chord in all areas where = 10.0psf. apable of withstanding 10 ct truss to bearing walls of	 kp B; Enclosed; I 18-2-0 to 21-6-4, embers and force load of 11.6 psf er live loads. e a rectangle 3-6 00 lb uplift at joint due to UPLIFT at 	MWFRS (envelop Interior(1) 21-6- es & MWFRS for); Pf=11.6 psf (fla on overhangs -0 tall by 2-0-0 w t(s) 10. t(s) 2. This	be) 4 to fat ride	SEAL 45844	



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-9-2, Exterior(2) 2-9-2 to 18-2-0, Corner(3) 18-2-0 to 21-9-10, Exterior(2) 21-9-10 to 37-2-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

7) All plates are 1.5x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Gable studs spaced at 2-0-0 oc.

- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



TRENGINEERING BY REACTOR A MiTek Atfiliate 818 Soundside Road

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	L		36-0)-8						
	I		36-0)-8					I	
Plate Offsets (X,Y) [2:0-3-8,E	dge]								
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 11.6/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.07 BC 0.06 WB 0.12 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 0.00 0.01	(loc) 1 1 23	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 239 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEDO 2x4 SP No.2 or 2x4 SPF No.2			E 7	BRACING- FOP CHORD	Structura except e Rigid cei	al wood nd verti ling dire	sheathin cals.	g directly app	olied or 6-0-0 oc purlins	i,
OTHERS	2x4 SP No.3		N	WEBS	1 Row at	midpt		12-33, 11-	-34, 13-32	

Left: 2x4 SP No.3

- REACTIONS. All bearings 36-0-8. (Ib) - Max Horz 2=128(LC 16)
 - Max Uplift All uplift 100 lb or less at joint(s) 2, 34, 35, 36, 38, 39, 40, 41, 42, 32, 31, 29, 28, 27, 26, 25, 24
 - Max Grav All reactions 250 lb or less at joint(s) 23, 2, 33, 34, 35, 36, 38, 39, 40, 41, 42, 32, 31, 29, 28, 27, 26, 25, 24

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 11-12=-96/254

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-8-12, Exterior(2) 2-8-12 to 18-2-0, Corner(3) 18-2-0 to 21-9-4, Exterior(2) 21-9-4 to 35-10-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 1.5x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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	6-11-5	5-1-11	1-7-11	1	6-11	1-5	1	
Plate Offsets (X,Y) [2:0-1-8,0	0-5-3], [2:0-0-0,0-0-12], [4:0-2-0,0-0-0], [6:Edge,0-0-12], [6:0-1-8,0)-5-3]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.37 BC 0.42 WB 0.52	DEFL. Vert(LL) - Vert(CT) - Horz(CT)	in (loc) -0.06 2-14 -0.13 2-14 0.01 10	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 122 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 2 BOT CHORD 2x4 SP No.2 or 2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3 , Right: 2x4 SF	2x4 SPF No.2 2x4 SPF No.2 P No.3	BF TC BC	RACING- OP CHORD Str DT CHORD Rig	ructural woo gid ceiling di	d sheathing rectly applie	directly applied or 6-0-0 oc	ed or 6-0-0 oc purlins bracing.	
REACTIONS. All bearings 8- (lb) - Max Horz 2=-73 Max Uplift All u Max Grav All r 10=8	10-8 except (jt=length) 2=0-3-8, 12=0-3 3(LC 21) Jplift 100 lb or less at joint(s) 2, 10, 6 ex eactions 250 lb or less at joint(s) 6, 9, 8 64(LC 2)	-8. cept 11=-140(LC 7) , 12 except 2=549(LC 2),						
FORCES. (lb) - Max. Comp./M TOP CHORD 2-3=-673/59, 3 BOT CHORD 2-14=-66/548 WEBS 4-10=-644/56,	ax. Ten All forces 250 (lb) or less exc -4=-519/76, 4-5=0/268 5-10=-315/136, 4-14=-39/453, 3-14=-29	ept when shown. 93/132						
 NOTES- 1) Unbalanced roof live loads ha 2) Wind: ASCE 7-10; Vult=115m gable end zone and C-C Exte 21-6-8 zone; cantilever left an reactions shown; Lumber DOI 3) Truss designed for wind loads Gable End Details as applicat 4) TCLL: ASCE 7-10; Pr=20.0 ps roof snow: Lumber DOL=1.15 5) Unbalanced snow loads have 6) This truss has been designed non-concurrent with other live 7) All plates are 1.5x4 MT20 unle 8) Gable studs spaced at 2-00 0; 9) This truss has been designed 10) * This truss has been designed 10) * This truss has been designed 	ve been considered for this design. ph Vasd=91mph; TCDL=6.0psf; BCDL= trior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 d right exposed ; end vertical left and ri- L=1.60 plate grip DOL=1.60 is in the plane of the truss only. For stud- ble, or consult qualified building designes f (roof live load: Lumber DOL=1.15 Pla Plate DOL=1.15); Category II; Exp B; F been considered for this design. for greater of min roof live load of 12.0 loads. ess otherwise indicated. bc. for a 10.0 psf bottom chord live load not lead for a live load of 20.0psf on the bottom hord and any other members.	6.0psf; h=30ft; Cat. II; Ex to 10-4-0, Exterior(2) 10- ght exposed;C-C for mem s exposed to wind (norma r as per ANSI/TPI 1. te DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10 psf or 1.00 times flat roof nconcurrent with any othe om chord in all areas whe	p B; Enclosed; MW 4-0 to 13-4-0, Interin bers and forces & M al to the face), see S sf (ground snow); P load of 11.6 psf on o er live loads. re a rectangle 3-6-0	YFRS (envelo or(1) 13-4-0 WWFRS for Standard Ind Pf=11.6 psf (f overhangs 0 tall by 2-0-0	ope) to lustry lat) wide	A CONTRACTOR OF A CONTRACTOR O	SEAL 45844	60 111111111111111111111111111111111111

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1



Plate Offsets (X,Y) [6:0-5-4,0)-1-8], [7:0-4-8,0-6-0], [8:0-5-0,0-1-8]	1	1					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNO	CSI. TC 0.34 BC 0.45 WB 0.65	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.09 6-7 -0.18 6-7 0.05 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 442 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x8 SP DSS WEBS 2x4 SP No.3 *Ex 3-7: 2x4 SP No.3	ccept* 2 or 2x4 SPF No.2	В Ті В	RACING- OP CHORD OT CHORD	Structural wood Rigid ceiling di	d sheathir rectly app	ig directly ap lied or 10-0-0	plied or 6-0-0 oc purlin) oc bracing.	S.
REACTIONS. (size) 1=(0- Max Horz 1=67 Max Uplift 1=-2! Max Grav 1=73	-3-8 + TBE4 Simpson Strong-Tie) (req. (LC 41) 52(LC 12), 5=-299(LC 13) 81(LC 2), 5=8448(LC 2)	0-3-14), 5=(0-3-8 + TBE	4 Simpson Strong PLY-TO- FACE M	-Tie) (req. 0-4 PLY CONNEC OUNT HANG	-7) CTION RE ER (SPEC	EQUIRES TH	HAT AN APPROVED	ED FOR
FORCES. (lb) - Max. Comp./M TOP CHORD 1-2=-13256/45 BOT CHORD 1-8=-421/1175 WEBS 3-7=-240/7891	ES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. LOADS REPORTED IN NOTES. FACE MOUNT HANGER SHALL BE 2HORD 1-2=-13256/453, 2-3=-9354/347, 3-4=-9354/347, 4-5=-13533/466 ATTACHED WITH A MINIMUM OF 0.148"x 3" NAILS PER HANGER 2HORD 1-8=-421/11759, 7-8=-421/11759, 6-7=-365/12009, 5-6=-365/12009 Solution of the second seco							- BE ER
 NOTES- n/a s-ply truss to be connected to Top chords connected as folls Bottom chords connected as folls Bottom chords connected as follows: 2 All loads are considered equa ply connections have been pr Unbalanced roof live loads ha Wind: ASCE 7-10; Vult=115m gable end zone; cantilever lef TCLL: ASCE 7-10; Pr=20.0 pr roof snow: Lumber DOL=1.15 Unbalanced snow loads have This truss has been designed * This truss has been designed 	gether with 10d (0.131"x3") nails as follows: 2x6 - 2 rows staggered at 0-9-0 oc. follows: 2x8 - 4 rows staggered at 0-4-0 2x4 - 1 row at 0-9-0 oc. Illy applied to all plies, except if noted as ovided to distribute only loads noted as ive been considered for this design. TCDL=6.0psf; BCDL= t and right exposed ; end vertical left an sf (roof live load: Lumber DOL=1.15 Pla been considered for this design. for a 10.0 psf bottom chord live load no d for a live load of 20.0psf on the bottor ord and any other members. onnectors recommended to connect true and does not consider lateral forces. IS26 (14-10d Girder, 4-10d Truss) or equ t truss(es) to back face of bottom chord	ows: oc. s front (F) or back (B) fac (F) or (B), unless otherw =6.0psf; h=30ft; Cat. II; E d right exposed; Lumber te DOL=1.15); Pg=15.0 Partially Exp.; Ct=1.10 unconcurrent with any oth n chord in all areas wher ss to bearing walls due to uivalent spaced at 9-10- l.	e in the LOAD CA rise indicated. xp B; Enclosed; M DOL=1.60 plate g psf (ground snow); ner live loads. re a rectangle 3-6- o UPLIFT at jt(s) 1 8 oc max. starting	SE(S) section. WFRS (envelo rip DOL=1.60 ; Pf=11.6 psf (f 0 tall by 2-0-0 v and 5. This at 2-0-12 from	Ply to ppe) lat wide the	A Community of the second seco	SEAL 45844	2020
WARNING - Verify design parar Design valid for use only with MIT a truss system. Before use, the bu building design. Bracing indicatec is always required for stability and fabrication, storage, delivery, erec Safety Information available from	neters and READ NOTES ON THIS AND INCLUDEI ek® connectors. This design is based only upon pr iliding designer must verify the applicability of desi i is to prevent buckling of individual truss web and/ to prevent collapse with possible personal injury a tion and bracing of trusses and truss systems, see m Truss Plate Institute, 2670 Crain Highway, Suite	D MITEK REFERENCE PAGE M arameters shown, and is for an gn parameters and properly inc or chord members only. Additi und property damage. For gene ANS/TP/1 Quality Q 203 Waldorf, MD 20601	III-7473 rev. 5/19/2020 f individual building com sorporate this design int onal temporary and per eral guidance regarding Criteria, DSB-89 and B	BEFORE USE. ponent, not o the overall manent bracing the CSI Building Con	ponent		ENGINEERING BY ANITE 818 Soundside Road Edenton, NC 27932	CO k Affiliate

Job	Truss	Truss Type	Qty	Ply	KB Home 238.2338.B	
						43618193
24818-24818A	CG	COMMON GIRDER	1	2		
				J	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,			8.420 s O	ct 9 2020 MiTek Industries, Inc. Fri Nov 13 07:56:38 2020 F	Page 2
		ID:I	ovi??x6A9	z5 viqm k	AghrzC2Gy-PtvZxc5uNQQJOzrAOMJz4PhYIJGjhg7frURyN6	δyJfXd

NOTES-

12) Fill all nail holes where hanger is in contact with lumber.

Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1412 lb down and 57 lb up at 8-0-12, 1412 lb down and 57 lb up at 10-0-12, and 1412 lb down and 57 lb up at 14-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-43, 3-5=-43, 1-5=-20 Concentrated Loads (lb)

Vert: 7=-1110(B) 11=-1110(B) 12=-1110(B) 13=-1110(B) 14=-1110(B) 15=-1110(B) 16=-1110(B) 17=-1110(B) 18=-1110(B) 19=-1165(B)

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0- <u>1-8</u> 0-1-8	7-5-12 7-4-4	<u>11-3-12</u> 3-10-0		12-8-0 1-4-4				18-8-0 6-0-0		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.39 BC 0.48 WB 0.22 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.08 -0.17 0.00	(loc) 2-10 6-8 6	l/defl >999 >764 n/a	L/d 240 180 n/a		PLATES MT20 Weight: 79 lb	GRIP 197/144 FT = 20%
LUMBER-		BRACIN	IG-							

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD WEBS 2x4 SP No.3

REACTIONS. 10=0-3-8, 6=0-3-8, 2=0-3-0 (size) Max Horz 2=44(LC 16) Max Uplift 10=-30(LC 12), 6=-58(LC 13), 2=-44(LC 12) Max Grav 10=959(LC 2), 6=427(LC 35), 2=263(LC 34)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 3-4=-28/445, 4-5=-274/35, 5-6=-586/93

BOT CHORD 6-8-47/525

WEBS 3-10=-402/129, 4-10=-714/58, 4-8=-5/386, 5-8=-378/125

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 9-4-0, Exterior(2) 9-4-0 to 12-4-0, Interior(1) 12-4-0 to 19-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 10, 6, and 2. This connection is for uplift only and does not consider lateral forces.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

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	7-4-0	9-4-0		18-8-	0		
	7-4-0	2-0-0		9-4-0)		1
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.60 BC 0.48	DEFL. in Vert(LL) -0.02 Vert(CT) -0.04	(loc) l/defl 6-8 >999 6-8 >999	L/d 240 180	PLATES MT20	GRIP 197/144
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.26 Matrix-S	Horz(CT) -0.01	6 n/a	n/a	Weight: 86 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 2 BOT CHORD 2x4 SP No.2 or 2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3	2x4 SPF No.2 2x4 SPF No.2	BI TC BC	RACING- DP CHORD Structur OT CHORD Rigid ce	al wood sheathir iling directly app	ng directly app lied or 5-10-1	blied or 7-9-1 oc purlir 0 oc bracing.	s.
REACTIONS. All bearings 11 (Ib) - Max Horz 10=4 Max Uplift All u Max Grav All r	-4-0 except (jt=length) 11=0-3-8. 4(LC 16) uplift 100 lb or less at joint(s) 11 except eactions 250 lb or less at joint(s) 8, 6 e	10=-297(LC 12), 6=-234(xcept 10=1290(LC 2), 11:	(LC 34) =336(LC 7)				
FORCES. (lb) - Max. Comp./M TOP CHORD 2-3=-466/756, BOT CHORD 2-11=-662/468 WEBS 4-10=-984/411	ax. Ten All forces 250 (lb) or less exc 3-4=-568/1246, 4-5=-575/1244, 5-6=-4 , 10-11=-662/468, 8-10=-973/472, 6-8= , 5-10=-508/167, 3-10=-524/181	cept when shown. 84/1049 973/472					
 NOTES- 1) Unbalanced roof live loads ha 2) Wind: ASCE 7-10; Vult=115m gable end zone and C-C Exter zone; cantilever left and right shown; Lumber DOL=1.60 pla 3) Truss designed for wind loads Gable End Details as applicat 4) TCLL: ASCE 7-10; Pr=20.0 ps roof snow: Lumber DOL=1.15 5) Unbalanced snow loads have 6) This truss has been designed non-concurrent with other live 7) All plates are 1.5x4 MT20 unlt 8) Gable Studs spaced at 2-0-0 of 9) This truss has been designed 10) * This truss has been designed 10) * This truss has been designed 10) * Dhis truss has been designed 11) One H2.5A Simpson Strong connection is for uplift only a 	ve been considered for this design. ph Vasd=91mph; TCDL=6.0psf; BCDL rior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-5 exposed ; end vertical left and right exp te grip DOL=1.60 is in the plane of the truss only. For stud- ble, or consult qualified building designed of (roof live load: Lumber DOL=1.15 Pla Plate DOL=1.15); Category II; Exp B; been considered for this design. for greater of min roof live load of 12.0 loads. ess otherwise indicated. DC. for a 10.0 psf bottom chord live load nu- ted for a live load of 20.0psf on the botth nord and any other members. -Tie connectors recommended to connu- ind does not consider lateral forces.	=6.0psf; h=30ft; Cat. II; E; 3 to 9-4-0, Exterior(2) 9-4- bosed;C-C for members a ds exposed to wind (norm er as per ANSI/TPI 1. ate DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10 psf or 1.00 times flat roof onconcurrent with any oth om chord in all areas whe ect truss to bearing walls	xp B; Enclosed; MWFRS 0 to 12-4-0, Interior(1) 12 nd forces & MWFRS for r al to the face), see Stand osf (ground snow); Pf=11. f load of 11.6 psf on overf her live loads. ere a rectangle 3-6-0 tall b due to UPLIFT at jt(s) 10. s due to UPLIFT at jt(s) 6	(envelope) -4-0 to 19-6-8 eactions ard Industry 6 psf (flat hangs by 2-0-0 wide This and 11. This		SEAL 45844	2020

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ENGINEERING BY TREENCO A MITek Atfiliate 818 Soundside Road Edenton, NC 27932



minim November 13,2020

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Edenton, NC 27932



- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-7-9 to 3-7-9, Interior(1) 3-7-9 to 4-2-8, Exterior(2) 4-2-8 to 7-2-8, Interior(1) 7-2-8 to 7-9-7 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



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2x4 💋

2x4 📚

Plate Offsets (X,Y) [2:0-2-0.E	dael	4-4-8 4-4-8				4-5- 0-0-	0 8
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.21 WB 0.00 Matrix-P	DEFL. Vert(LL) n Vert(CT) n Horz(CT) 0.0	in (loc) l/ /a - /a - 00 3	/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 12 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.3		BR TO	ACING- P CHORD Struct	tural wood sh	eathing directly app	lied or 4-5-0 oc purlir	IS.

 TOP CHORD
 Structural wood sheathing directly applied or 4-5-0 oc purlins

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=4-4-0, 3=4-4-0 Max Horz 1=10(LC 20) Max Uplift 1=-3(LC 16), 3=-3(LC 17) Max Grav 1=126(LC 2), 3=126(LC 2)

BOT CHORD 2x4 SP No.3

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



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