

Trenco 818 Soundside Rd Edenton, NC 27932

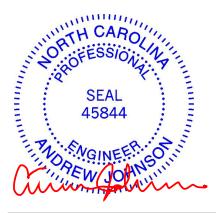
Re: 240_3174_B KB Home 240.3174.B

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I45649700 thru I45649717

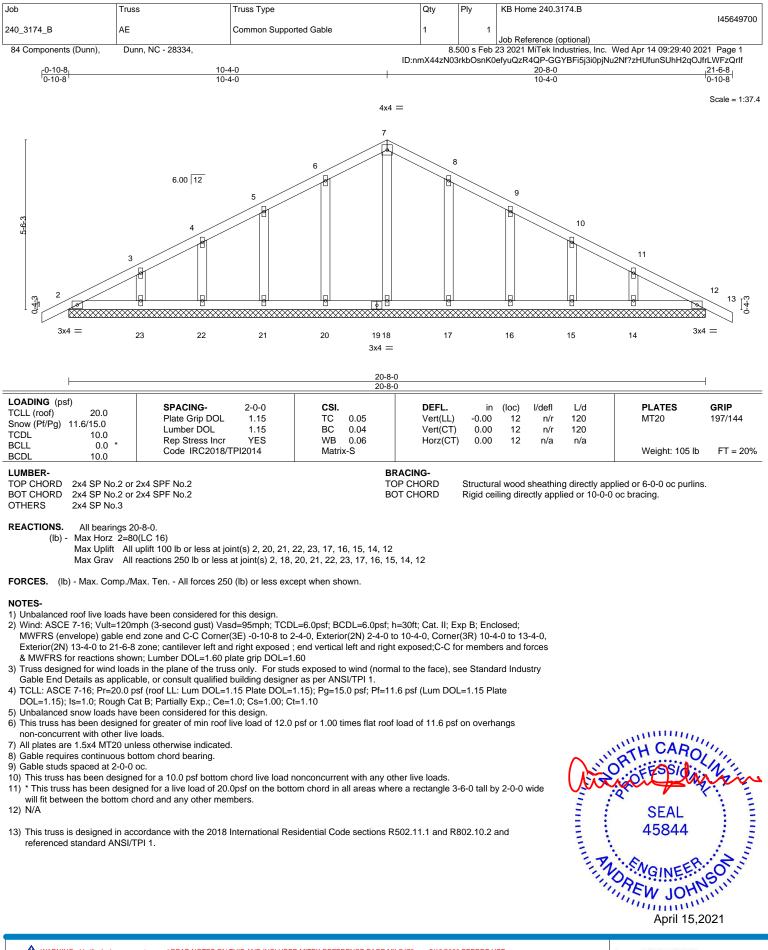
My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844

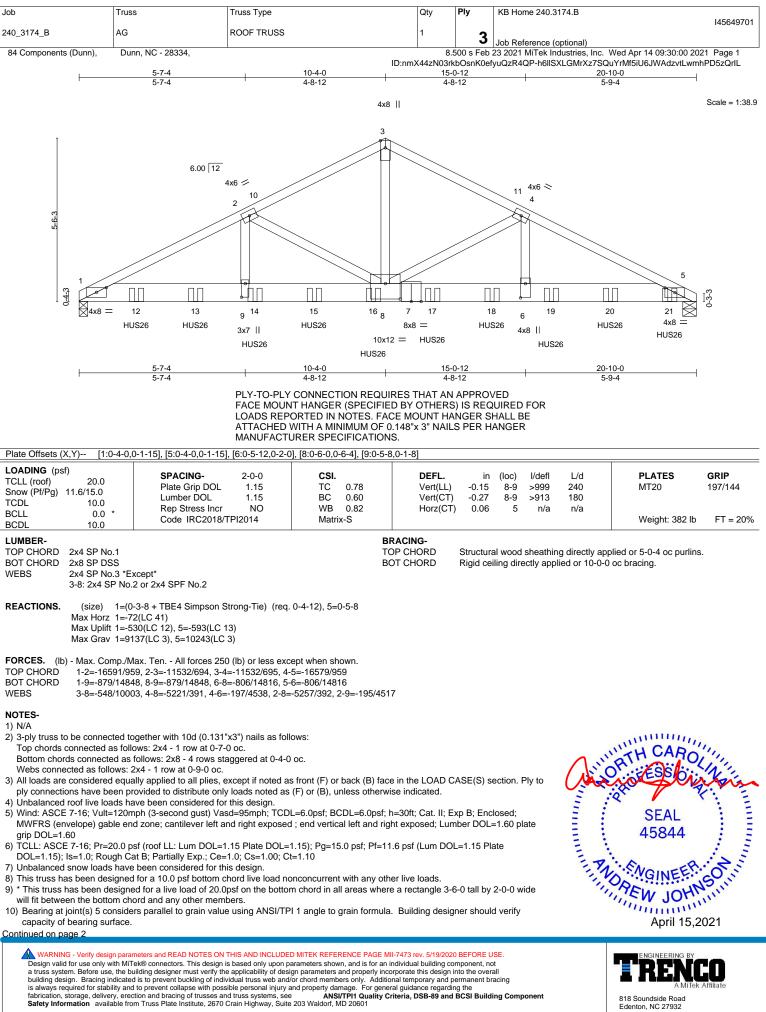


April 15,2021

Johnson, Andrew **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



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Job	Truss	Truss Type	Qty	Ply	KB Home 240.3174.B
					I45649701
240_3174_B	AG	ROOF TRUSS	1	2	
				3	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	500 s Feb	23 2021 MiTek Industries, Inc. Wed Apr 14 09:30:00 2021 Page 2
		ID:nm	X44zN03rl	kbOsnK0ef	yuQzR4QP-h6llSXLGMrXz7SQuYrMf5iU6JWAdzvtLwmhPD5zQrlL

NOTES-

11) TBE4 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.

12) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

14) Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 1-11-4 from the left end to 19-11-4 to connect truss(es) to back face of bottom chord.

15) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

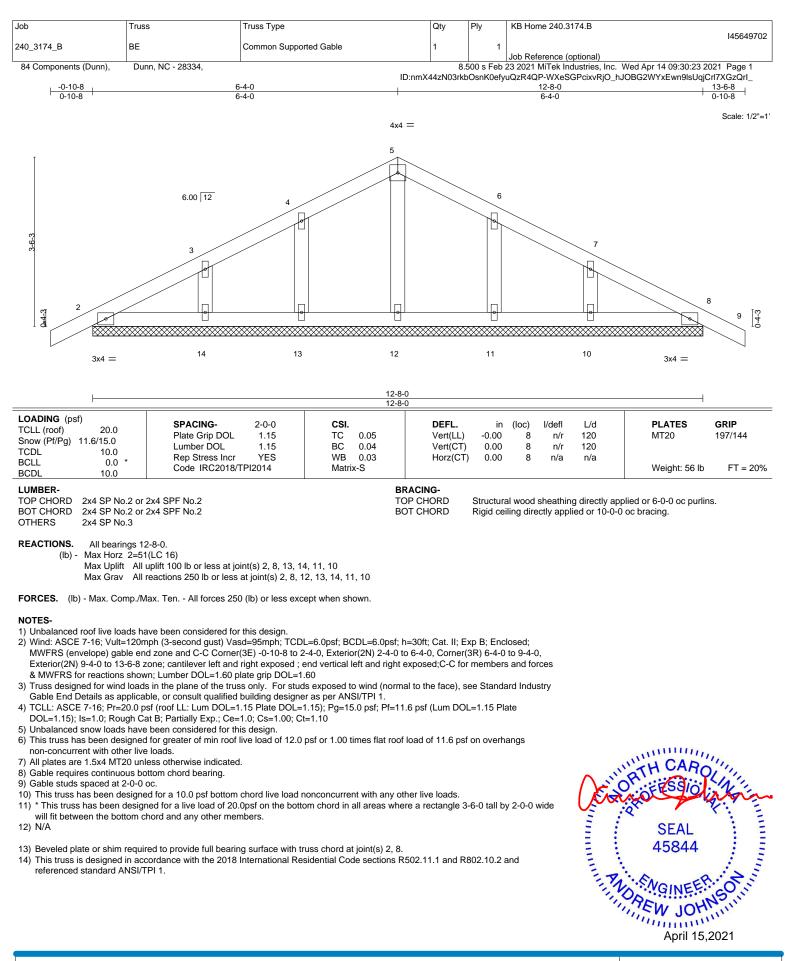
Uniform Loads (plf) Vert: 1-3=-43, 3-5=-43, 1-5=-20

Concentrated Loads (lb)

Vert: 12=-1278(B) 13=-1278(B) 14=-1278(B) 15=-1278(B) 16=-1278(B) 17=-1278(B) 18=-1278(B) 19=-1278(B) 20=-1278(B) 21=-1279(B)

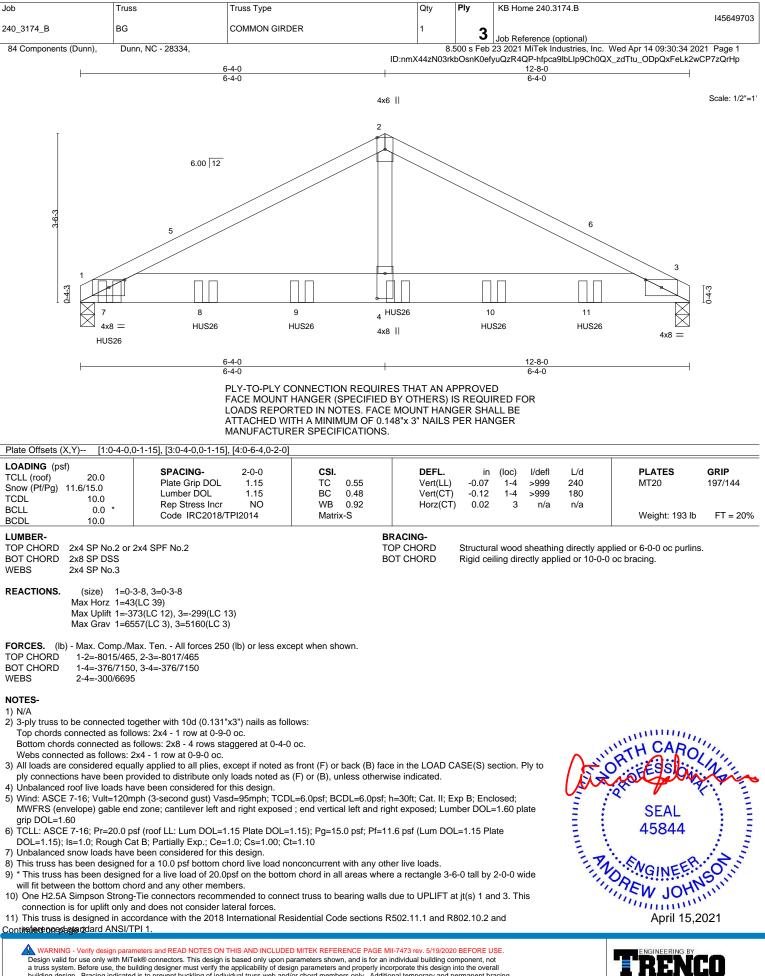
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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Job	Truss	Truss Type	Qty	Ply	KB Home 240.3174.B
					145649703
240_3174_B	BG	COMMON GIRDER	1	2	
				<u>່</u> ວ	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	500 s Feb	23 2021 MiTek Industries, Inc. Wed Apr 14 09:30:34 2021 Page 2
		ID:nm)	X44zN03rk	bOsnK0ef	/uQzR4QP-hfpca9lbLlp9Ch0QX_zdTtu_ODpQxFeLk2wCP7zQrHp

NOTES-

12) Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 0-7-4 from the left end to 10-7-4 to connect truss(es) to back face of bottom chord.

13) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

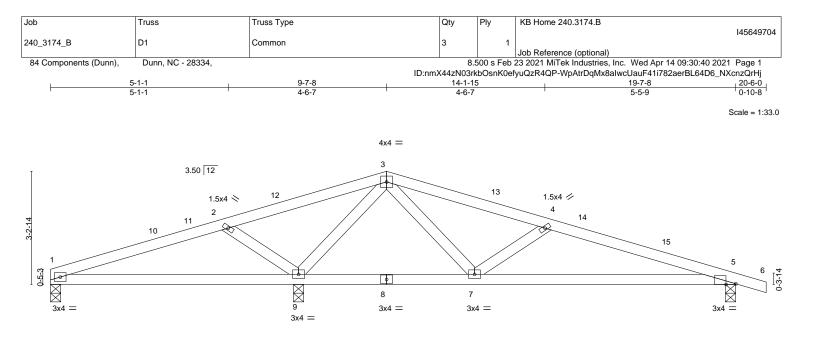
Uniform Loads (plf) Vert: 1-2=-43, 2-3=-43, 1-3=-20

Concentrated Loads (lb)

Vert: 4=-1278(B) 7=-1284(B) 8=-1278(B) 9=-1278(B) 10=-1278(B) 11=-1278(B)

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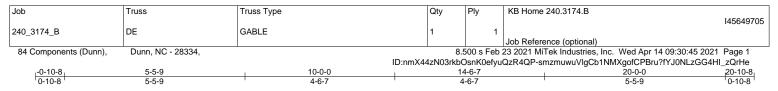


 	7-1-4	12-1-12				19-7-8		
Plate Offsets (X,Y) [5:0-3-5,	7-1-4'	5-0-8				7-5-12		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 0.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.44 BC 0.52 WB 0.29 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.09 5-7 -0.19 5-7 0.01 5	l/defl >999 >785 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 80 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or BOT CHORD 2x4 SP No.2 or WEBS 2x4 SP No.3 REACTIONS. (size) 1=0- Max Horz 1=-4 Max Uplift 1=-2	2x4 SPF No.2 3-8, 9=0-3-8, 5=0-3-8	TC BC		Structural wood			lied or 6-0-0 oc purlin c bracing.	S.
TOP CHORD 1-2=-40/290, 2 BOT CHORD 1-9=-256/66, 5 WEBS 2-9=-424/158, NOTES- 1) Unbalanced roof live loads ha 2) Wind: ASCE 7-16; Vult=120m MWFRS (envelope) gable en Interior(1) 12-7-8 to 20-6-0 zc MWFRS for reactions shown; 3) TCLL: ASCE 7-16; Pr=20.0 p DOL=1.15); Is=1.0; Rough Cz 4) Unbalanced snow loads have	3-9=-842/156, 3-7=-18/489, 4-7=-410/1 ave been considered for this design. nph (3-second gust) Vasd=95mph; TCDI d zone and C-C Exterior(2E) 0-1-12 to 3 one; cantilever left and right exposed ; er Lumber DOL=1.60 plate grip DOL=1.6(sf (roof LL: Lum DOL=1.15 Plate DOL=' at B; Partially Exp.; Ce=1.0; Cs=1.00; Cr been considered for this design. I for greater of min roof live load of 12.0	56 L=6.0psf; BCDL=6.0psf; H I-1-12, Interior(1) 3-1-12 t nd vertical left and right ex 1.15); Pg=15.0 psf; Pf=11 i=1.10	o 9-7-8, Exterior(2 xposed;C-C for m .6 psf (Lum DOL=	2R) 9-7-8 to 12 embers and for =1.15 Plate			WH CARC	1110.
 6) This truss has been designed 7) * This truss has been designed will fit between the bottom che 8) One H2.5A Simpson Strong- connection is for uplift only ar 	I for a 10.0 psf bottom chord live load no ad for a live load of 20.0psf on the bottor ord and any other members. Fie connectors recommended to connec d does not consider lateral forces. ordance with the 2018 International Resi	n chord in all areas where t truss to bearing walls du	e a rectangle 3-6-(ue to UPLIFT at jt((s) 1, 9, and 5.		A	SEAL 45844	A Community

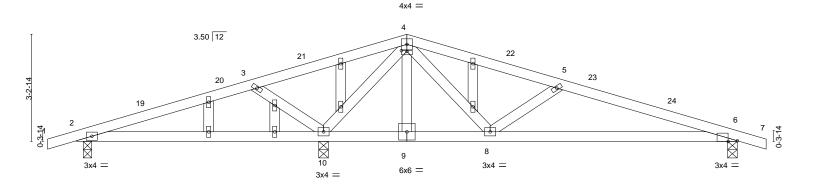


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Scale = 1:34.9

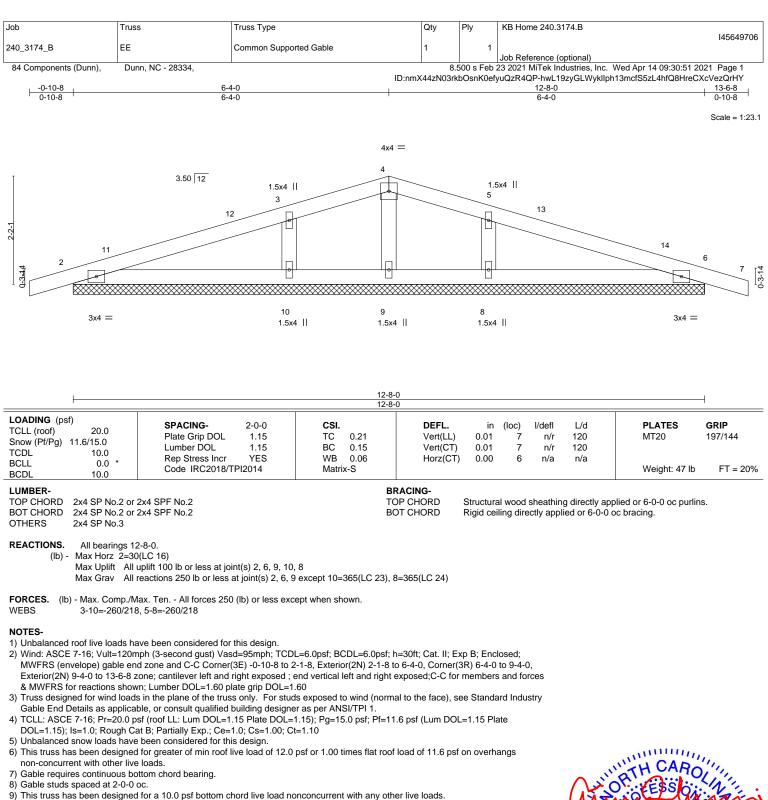


07278	7-5-12	12-6-4	L ,			20-0-0		
0 ₇ 2 <u>78</u> 0-2-8	7-3-4	5-0-8				7-5-12		
Plate Offsets (X,Y) [4:0-2-0,0	0-0-4], [6:0-3-5,Edge]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.46 BC 0.53 WB 0.29 Matrix-S	Vert(CT) -0	in (loc) 09 6-8 19 6-8 01 6		L/d 240 180 n/a	PLATES MT20 Weight: 93 lb	GRIP 197/144 FT = 20%
		DD	ACINC					
LUMBER- TOP CHORD 2x4 SP No.2 or 3 BOT CHORD 2x4 SP No.2 or 3 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3		TO				directly appli d or 6-0-0 oc	ed or 6-0-0 oc purlin bracing.	s.
Max Horz 2=45 Max Uplift 10=-	0-3-8, 6=0-3-8, 2=0-3-0 (LC 16) 57(LC 12), 6=-79(LC 13), 2=-55(LC 12) 027(LC 2), 6=475(LC 37), 2=254(LC 36	;)						
TOP CHORD 2-3=-46/279, 3 BOT CHORD 6-8=-96/722	ax. Ten All forces 250 (lb) or less exc 1-4=-67/543, 4-5=-452/61, 5-6=-784/140), 4-10=-836/156, 4-8=-19/487, 5-8=-41							
 2) Wind: ASCE 7-16; Vult=120m MWFRS (envelope) gable end Interior(1) 13-0-0 to 20-10-8 z & MWFRS for reactions show 3) Truss designed for wind loads Gable End Details as applical 4) TCLL: ASCE 7-16; Pr=20.0 pp DOL=1.15); Is=1.0; Rough Ca 5) Unbalanced snow loads have 6) This truss has been designed non-concurrent with other live 7) All plates are 1.5x4 MT20 unl 8) Gable studs spaced at 2-0-0 9) This truss has been designed 10) * This truss has been designed 10) the H2.5A Simpson Strong This connection is for uplift of 	ess otherwise indicated. bc. for a 10.0 psf bottom chord live load no red for a live load of 20.0psf on the botto hord and any other members. -Tie connectors recommended to conne only and does not consider lateral forces cordance with the 2018 International Res	2-1-8, Interior(1) 2-1-8 to end vertical left and right e 60 s exposed to wind (norma r as per ANSI/TPI 1. 1.15); Pg=15.0 psf; Pf=11. =1.10 psf or 1.00 times flat roof enconcurrent with any othe om chord in all areas when st truss to bearing walls d	10-0-0, Exterior(2R) 1 exposed;C-C for merr I to the face), see Sta 6 psf (Lum DOL=1.1 load of 11.6 psf on ov er live loads. re a rectangle 3-6-0 ta ue to UPLIFT at jt(s)	0-0-0 to 13- bers and for andard Indus 5 Plate verhangs all by 2-0-0 v 10, 6, and 2	rces stry	Annum Ballin	SEAL 45844	All Annun An

April 15,2021

ENGINEERING BY A MITEK Affilia 818 Soundside Road Edenton, NC 27932

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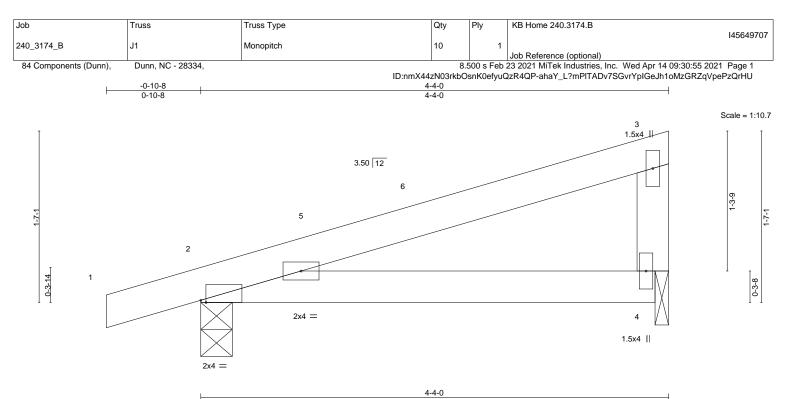


- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) N/A
- 12) N/A
- 13) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

SEAL 45844 April 15,2021

> ENGINEERING BY TREENCO AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

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			4-4-0		I	
Plate Offsets (X,Y) [2:0-0-9,E	Edge]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 PC/L 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.28 BC 0.20 WB 0.00	DEFL. in (lor Vert(LL) -0.02 2- Vert(CT) -0.03 2- Horz(CT) 0.00 1-	-4 >999 240	MT20	GRIP 197/144
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-P			Weight: 16 lb	FT = 20%
LUMBER-			ACING-			

 LOWBER-TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2
 Display a construction of the sector of

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=48(LC 13) Max Uplift 2=-52(LC 12), 4=-19(LC 16) Max Grav 2=237(LC 23), 4=158(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

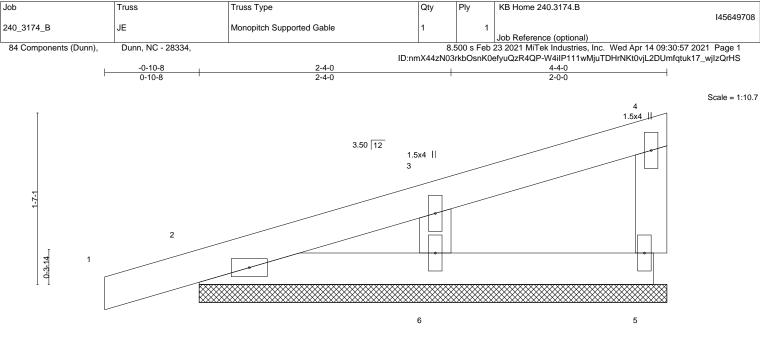
NOTES-

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-2-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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2x4 =

1.5x4 ||

1.5x4 ||

LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.06 BC 0.04 WB 0.06 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 1 1 5	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 17 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 2	2x4 SPF No.2		RACING- DP CHORD	Structura	ıl wood	sheathin	g directly ap	blied or 4-4-0 oc purlin	IS,

BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3

except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 5=4-4-0, 2=4-4-0, 6=4-4-0 (size) Max Horz 2=48(LC 13) Max Uplift 5=-7(LC 12), 2=-37(LC 12), 6=-26(LC 16) Max Grav 5=63(LC 23), 2=137(LC 23), 6=198(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 2-2-4, Exterior(2N) 2-2-4 to 4-2-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 9) will fit between the bottom chord and any other members.

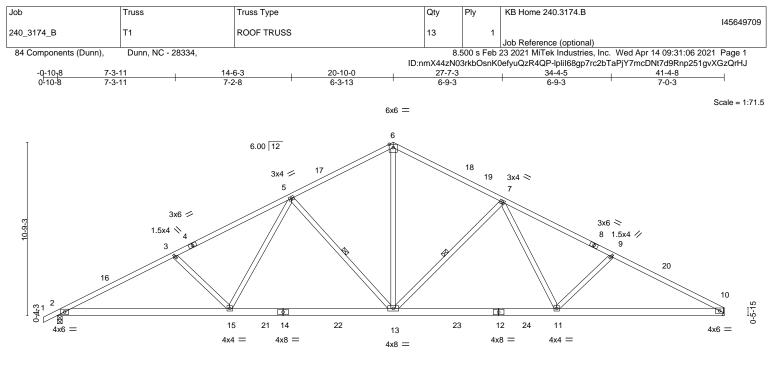
10) N/A

- 11) N/A
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





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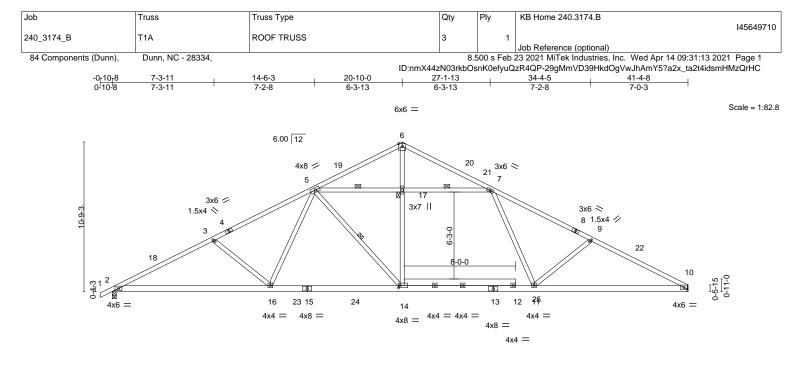


)-8-4	20-10-0)-11-12			41-4-8		
10)-8-4	10-1-12	<u> </u>	0-1-12		10-4-12			
LOADING (psf) TCLL (roof) 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.83	DEFL. Vert(LL)	in (loc) -0.25 13-15		L/d 40	PLATES MT20	GRIP 197/144	
Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.91 WB 0.65	Vert(CT) Horz(CT)	-0.44 13-15 0.12 10		80 n/a			
BCDL 10.0	Code IRC2018/TPI2014	Matrix-S					Weight: 246 lb	FT = 20%	
LUMBER- TOP CHORD 2x4 SP No.2 or 8-10: 2x4 SP No. BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3	2x4 SPF No.2 *Except* 0.1	T	BRACING- TOP CHORD BOT CHORD	Structural wood Rigid ceiling dire 2-2-0 oc bracing 1 Row at midpt	ectly applied 1: 2-15.		d. bracing, Except:		
Max Horz 2=10 Max Uplift 2=-1	-3-8, 10=Mechanical 61(LC 16) 10(LC 16), 10=-91(LC 17) 867(LC 3), 10=1816(LC 3)								
	/lax. Ten All forces 250 (lb) or less e 3, 3-5=-3176/181, 5-6=-2171/215, 6-7 99		8,						
WEBS 6-13=-63/157	12, 13-15=-107/2398, 11-13=-17/2424 7, 7-13=-801/204, 7-11=-8/724, 9-11= , 3-15=-419/201	,							
NOTES-									
	ave been considered for this design.								
 Wind: ASCE 7-16; Vult=120n MWFRS (envelope) gable en 24-11-10, Interior(1) 24-11-10 	nph (3-second gust) Vasd=95mph; TC nd zone and C-C Exterior(2E) -0-10-8 0 to 41-3-12 zone; cantilever left and	to 3-3-2, Interior(1) 3-3-2 to ight exposed ; end vertical	o 20-10-0, Exterior l left and right exp	(2R) 20-10-0 to					
	RS for reactions shown; Lumber DOL osf (roof LL: Lum DOL=1.15 Plate DO			=1.15 Plate					
DOL=1.15); Is=1.0; Rough C	at B; Partially Exp.; Ce=1.0; Cs=1.00;		• •			111	LA CARO	11,	
	e been considered for this design. d for greater of min roof live load of 12 e loads.	.0 psf or 1.00 times flat roo	of load of 11.6 psf	on overhangs	1	<u>A. I. OF</u>	STEPS ON	N	
7) * This truss has been designed	d for a 10.0 psf bottom chord live load ed for a live load of 20.0psf on the bot ord and any other members, with BC	tom chord in all areas whe		-0 tall by 2-0-0 w	ide		SEAL	A Martin	
8) Refer to girder(s) for truss to		DE = 10.0p31.					SEAL	=	
10) One H2.5A Simpson Strong	on (by others) of truss to bearing plate g-Tie connectors recommended to cor and does not consider lateral forces.					Annun ANO	45844		
	cordance with the 2018 International I	Residential Code sections	R502.11.1 and R8	02.10.2 and		TNO	NGINEER	O'III	
12) ATTIC SPACE SHOWN IS	DESIGNED AS UNINHABITABLE.					in the	SEW JOHN	11111	
							April 15-2	021	



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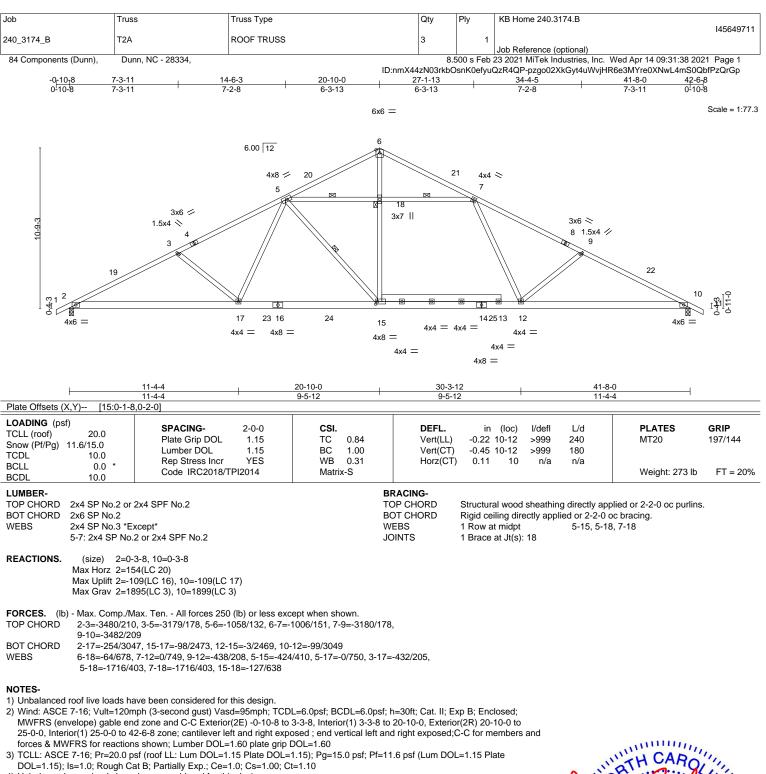
	11-4-4	20-10-0	30-3-12		41-4-8		
Plate Offsets (X,Y) [14:0-	11-4-4	9-5-12	9-5-12		11-0-12		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.84 BC 0.97 WB 0.31 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.20 2-16 -0.42 2-16 0.11 10	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 271 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 8-10: 2x4 SP BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3	No.1	T B W	OT CHORD		sheathing directly appli ctly applied or 2-2-0 oc 5-14, 5-17, 17	bracing.	
Max Horz 2= Max Uplift 2=	=0-3-8, 10=Mechanical =161(LC 16) =-110(LC 16), 10=-91(LC 17) =1887(LC 3), 10=1840(LC 3)						
TOP CHORD 2-3=-3461/2 9-10=-3393 9000000000000000000000000000000000000	/Max. Ten All forces 250 (lb) or less e 211, 3-5=-3161/174, 5-6=-1054/133, 6-7 /208 3030, 14-16=-105/2456, 11-14=-11/244 /644, 7-11=0/708, 9-11=-404/210, 5-14 205, 5-17=-1703/403, 7-17=-1703/403, i	′=-1002/152, 7-9=-3128/183 7, 10-11=-114/2968 =-430/404, 5-16=-0/750,	3,				
 Wind: ASCE 7-16; Vult=12 MWFRS (envelope) gable 24-11-10, Interior(1) 24-11 members and forces & MW TCLL: ASCE 7-16; Pr=20.0 DOL=1.15); Is=1.0; Rough Unbalanced snow loads hat This truss has been design non-concurrent with other I This truss has been design non-concurrent with other I This truss has been design will fit between the bottom Refer to girder(s) for truss Provide mechanical conne One H2.5A Simpson Stro connection is for uplift on This truss is designed in a referenced standard ANS 	and for a 10.0 psf bottom chord live load gned for a live load of 20.0psf on the bo chord and any other members, with BC to truss connections. ction (by others) of truss to bearing plat ing-Tie connectors recommended to co y and does not consider lateral forces. accordance with the 2018 International	to 3-3-2, Interior(1) 3-3-2 to right exposed ; end vertical =1.60 plate grip DOL=1.60 L=1.15); Pg=15.0 psf; Pf=1 Ct=1.10 2.0 psf or 1.00 times flat roo nonconcurrent with any oth tom chord in all areas when DL = 10.0psf. e capable of withstanding 1 neect truss to bearing walls	20-10-0, Exterior left and right expo 1.6 psf (Lum DOL= f load of 11.6 psf c ner live loads. re a rectangle 3-6- 00 lb uplift at joint(due to UPLIFT at	(2R) 20-10-0 to sed;C-C for =1.15 Plate on overhangs 0 tall by 2-0-0 wi (s) 10. (s) 2. This	de	SEAL 45844	Summer Com

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4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

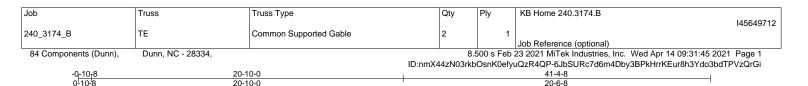
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

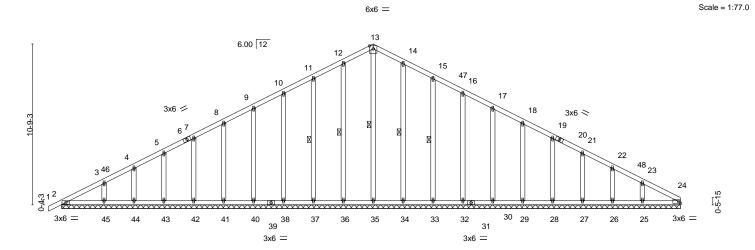
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1

10) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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<u> </u>		<u>41-4-8</u> 41-4-8							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 Poll 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.08 BC 0.06 WB 0.13	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 0.00 0.01	(loc) 1 1 24	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20	GRIP 197/144
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-S	- (-)					Weight: 287 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No 2 or 2	2x4 SPF No 2		ACING- P CHORD	Structura	lwood	sheathir	a directly an	blied or 6-0-0 oc purlins	

BOT CHORD

WEBS

Rigid ceiling directly applied or 10-0-0 oc bracing.

13-35, 12-36, 11-37, 14-34, 15-33

1 Row at midpt

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 OTHERS
 2x4 SP No.3

REACTIONS. All bearings 41-4-8.

(lb) - Max Horz 2=160(LC 20)

Max Uplift All uplift 100 lb or less at joint(s) 2, 36, 37, 38, 40, 41, 42, 43, 44, 45, 34, 33, 32, 30, 29, 28, 27, 26, 25

Max Grav All reactions 250 lb or less at joint(s) 2, 35, 36, 37, 38, 40, 41, 42, 43, 44, 45, 34, 33, 32, 30, 29, 28, 27, 26, 25, 24

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 12-13=-125/277, 13-14=-125/277

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-3-2, Exterior(2N) 3-3-2 to 20-10-0, Corner(3R) 20-10-0 to 24-10-0, Exterior(2N) 24-10-0 to 41-4-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 1.5x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

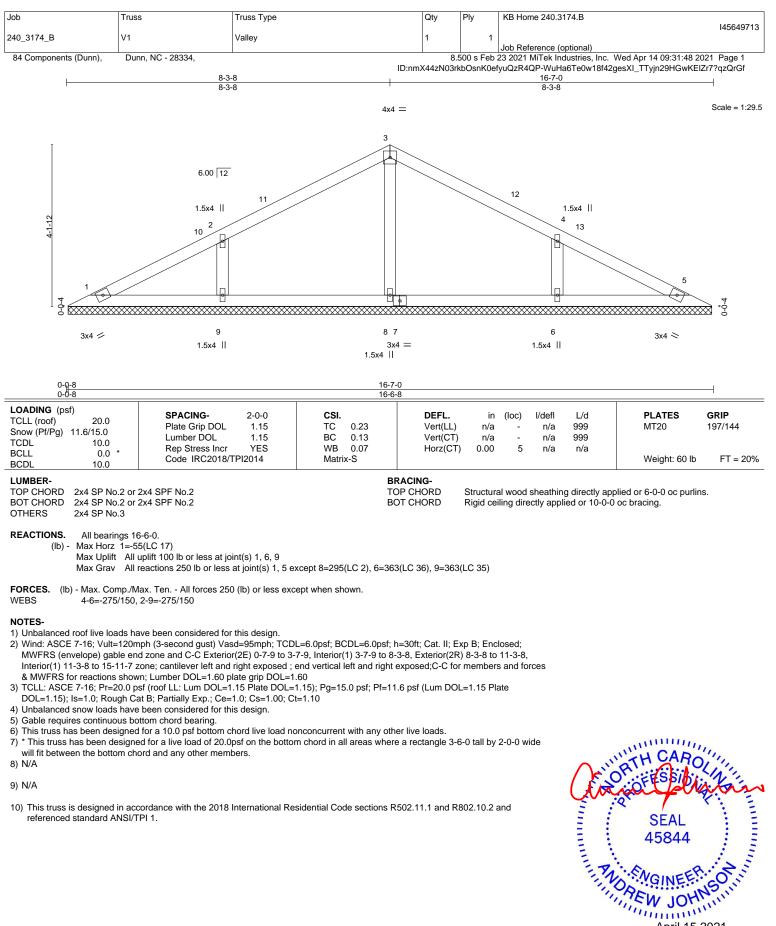
12)N/A

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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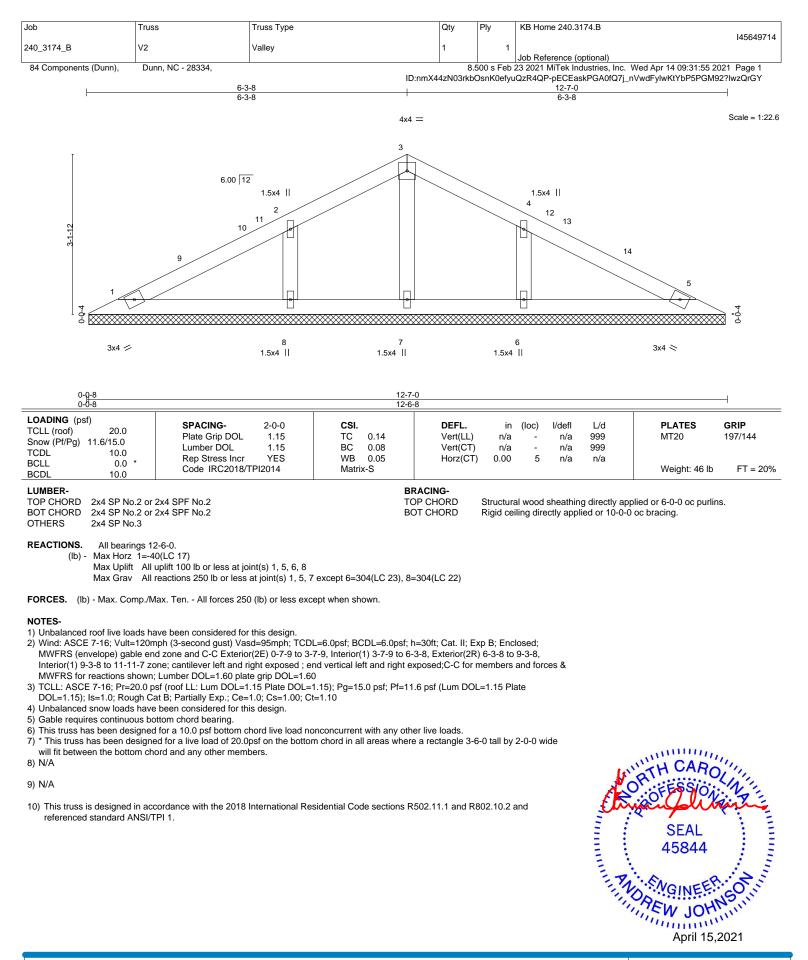
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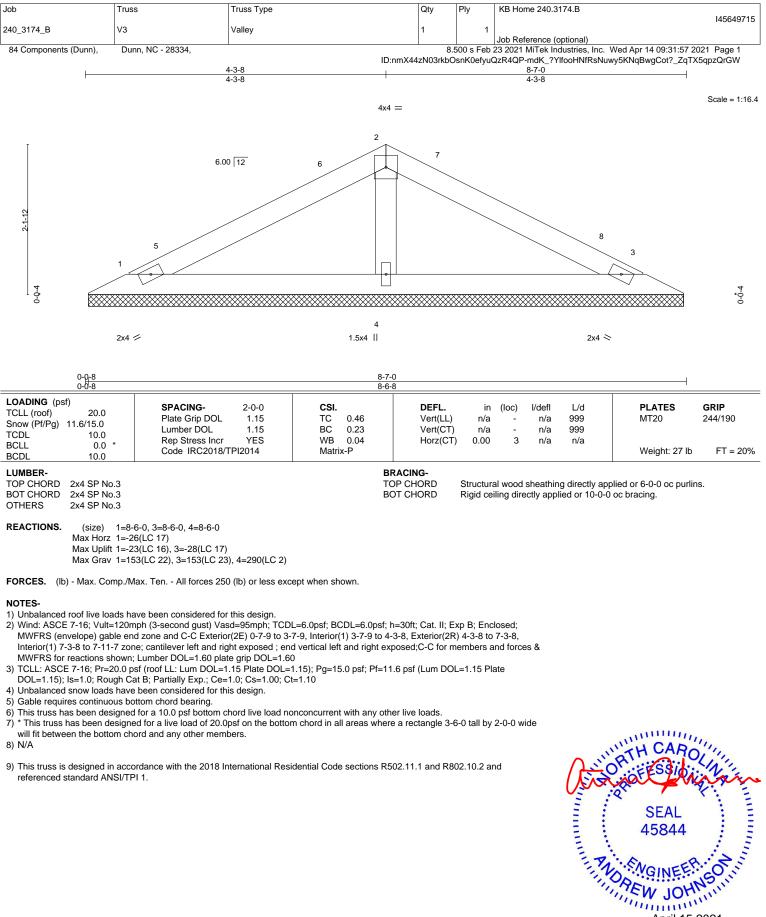


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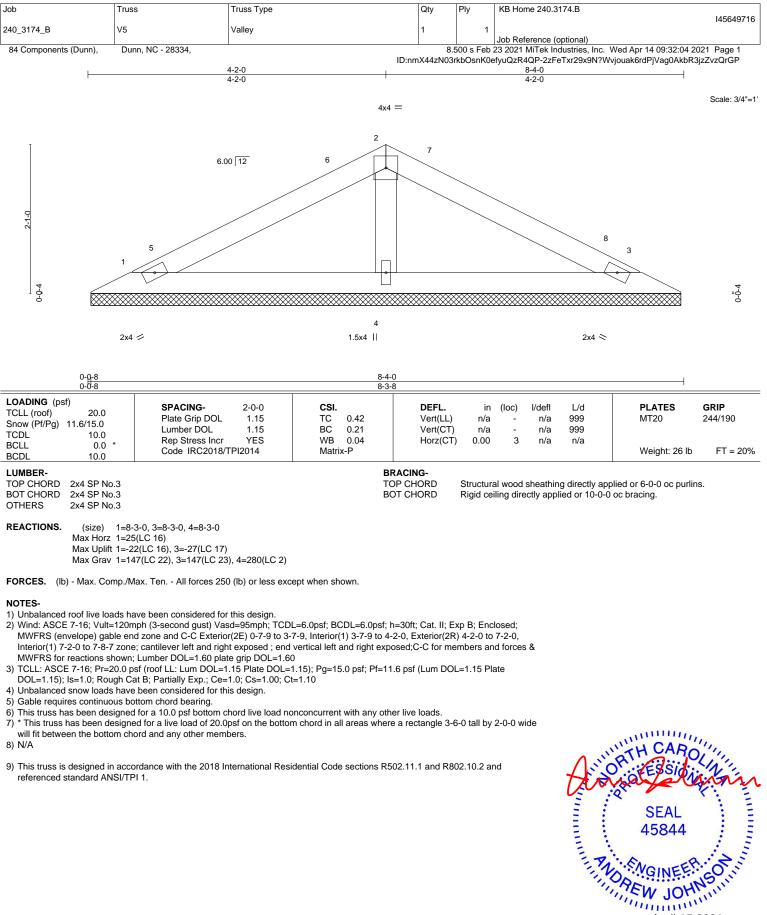


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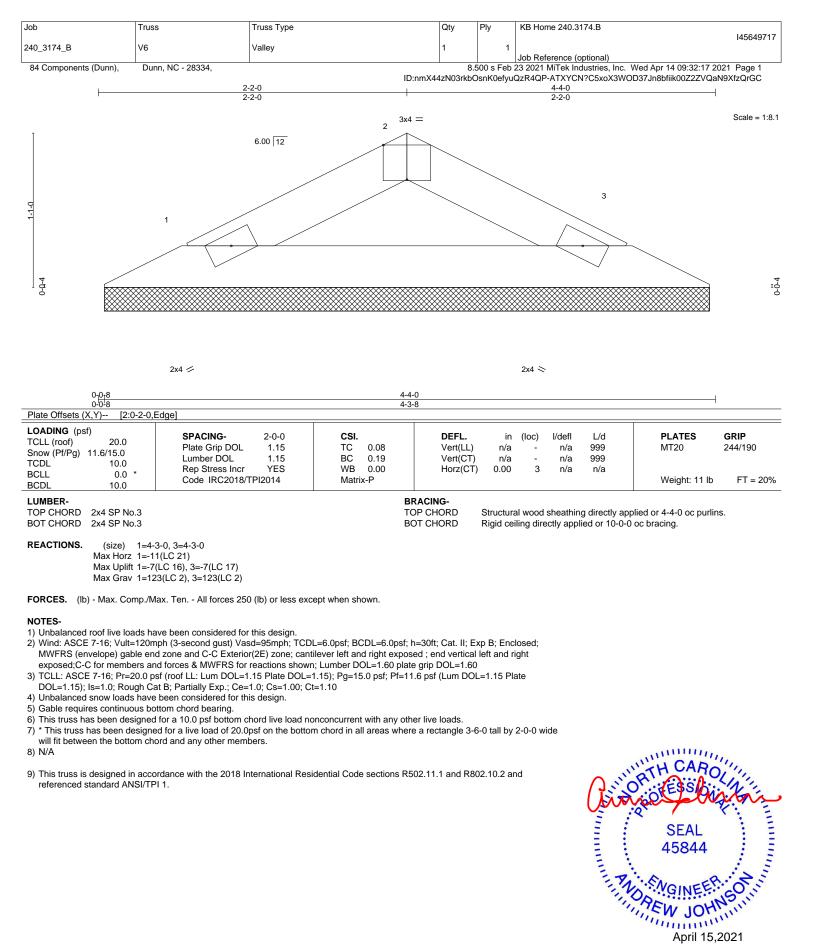
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