

Trenco RE: J1121-6492 818 Soundside Rd Lot 14 Liberty Meadow Edenton, NC 27932 Site Information: Customer: Precision Custom Homes and Renovations Project Name: J1121-6492 Lot/Block: 14 Model: Liberty 2.0 Address: 3067 Slocomb Road Subdivision: Liberty Meadow State: NC City: Linden General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4 Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf This package includes 23 individual, dated Truss Design Drawings and 0 Additional Drawings.

							_
No	o. Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	150256509	A1	2/16/2022	21	150256529	V4	2/16/2022
2	150256510	A1GE	2/16/2022	22	150256530	V5	2/16/2022
3	150256511	A2	2/16/2022	23	150256531	V6	2/16/2022
4	150256512	A3	2/16/2022				
5	150256513	A3GE	2/16/2022				
6	150256514	B1GE	2/16/2022				
7	150256515	B2	2/16/2022				
8	150256516	C1	2/16/2022				
9	150256517	C1-GR	2/16/2022				
10) 150256518	C1GE	2/16/2022				
11	150256519	G1	2/16/2022				
12	150256520	G1GE	2/16/2022				
13	150256521	J1	2/16/2022				
14	150256522	J2	2/16/2022				
15	5 150256523	J2GE	2/16/2022				
16	6 150256524	J3	2/16/2022				
17	/ 150256525	J3GE	2/16/2022				
18		V1	2/16/2022				
19		V2	2/16/2022				
20		V3	2/16/2022				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

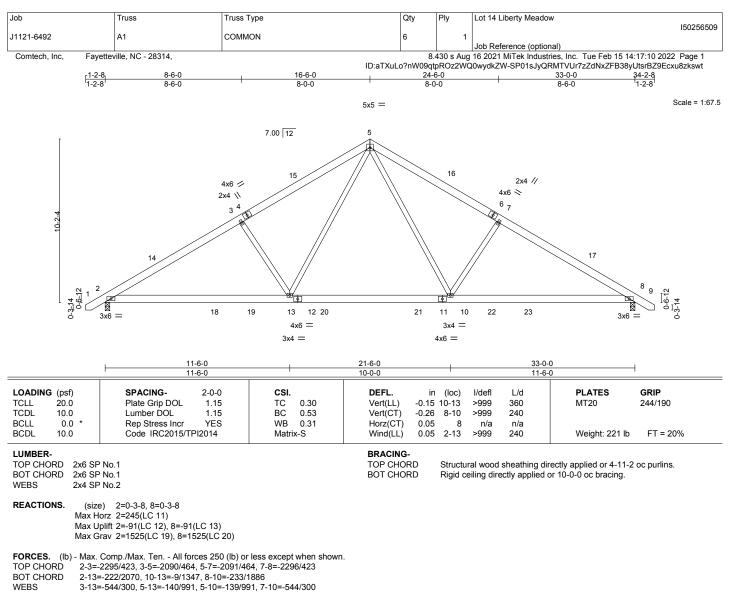
Truss Design Engineer's Name: Strzyzewski, Marvin

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Strzyzewski, Marvin



NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-6 to 3-4-7, Interior(1) 3-4-7 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 34-0-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

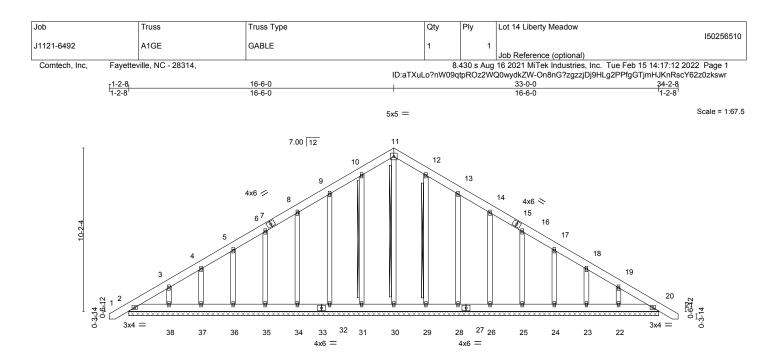
will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 2 and 91 lb uplift at joint 8.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





						33-0-0						1
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.04	Vert(LL)	-0.00	2 0	n/r	120	MT20	244/190
ICDL	10.0	Lumber DOL	1.15	BC	0.02	Vert(CT)	0.00	20	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.01	20	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S						Weight: 283 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

BRACING-TOP CHORD BOT CHORD

WFBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing 2x4 SPF No.2 - 11-30, 10-31, 12-29 T-Brace: Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 33-0-0.

Max Horz 2=306(LC 11) (lb) -Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22 Max Grav All reactions 250 lb or less at joint(s) 2, 20, 30, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-277/226, 10-11=-242/277, 11-12=-242/277

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTTeKe connectors. This does not have a seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

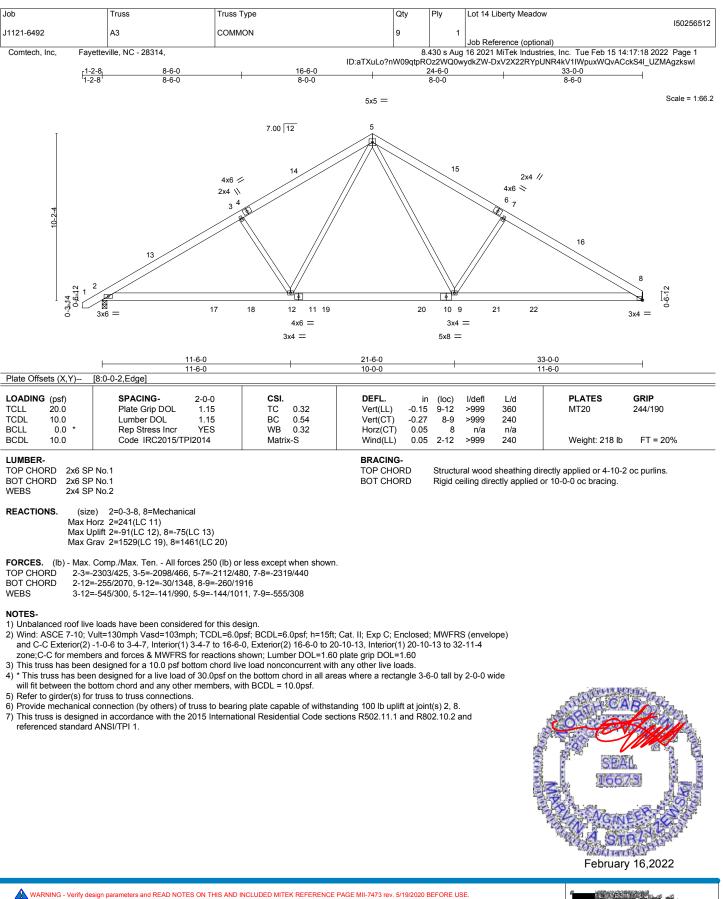


Job	Truss	Truss Type	Q	y PI	lv	Lot 14 Liberty Meadow	v]
J1121-6492	A2	COMMON	4	, '	יי 1		-	150256511
	tteville, NC - 28314,		7	8 430		Job Reference (option	al) ies, Inc. Tue Feb 15 14	17:16 2022 Page 1
		16-6-0	ID:aTXuLo?nW09q		/Q0wydk		mb7vtULpWR_HNWAG	GYgSXA4F6nzkswn
[<mark>1-2-8</mark> 1-2-8	<u>8-6-0</u> 8-6-0	8-0-0		8-0-0			8-6-0	34-2-8 1-2-8
			5x5 =					Scale = 1:67.5
T		7.00 12	5					
				_				
		15		\searrow	16	2x4 //		
		4x6 == 2x4 \\			\searrow	4x6 📎		
4		3 4	/ \\			⁶ 7		
10-2-4	/.		\			19		
							17	
	14						17	
N . 2					\sum			89_0
2 0-3-14 1-12 0-8-12 1 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1	Ø	the second se		[¢				11 σ 0-3-12 0-3-14
۳ ک ۲ 3	×6 = 18	19 13 12 20 4x6 =			1 10 3x4 =	22 23	⊠ 3x6 4x12	
		3x4 =			6 =			
	11-6-0		21-6-0			32-0-0	22.0	0
	11-6-0		10-0-0			10-6-0	33-0- 1-0-0	1
	8:0-7-6,0-1-1], [8:0-0-2,1-1-1]							
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15		DEFL. Vert(LL)	in (-0.15 10		l/defl L/d >999 360	PLATES MT20	GRIP 244/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES		Vert(CT) Horz(CT)	-0.26 8 0.05	8-10 8	>999 240 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.05		>999 240	Weight: 223 lb	FT = 20%
UMBER-			BRACING-					
TOP CHORD 2x6 SP I BOT CHORD 2x6 SP I	No.1		TOP CHOF BOT CHOF			al wood sneathing dir iling directly applied c	ectly applied or 4-11-2 r 10-0-0 oc bracing.	oc puriins.
WEBS 2x4 SP I WEDGE	No.2							
Right: 2x6 SP No.1								
) 2=0-3-8, 8=0-3-8							
Max Up	orz 2=245(LC 11) olift 2=-91(LC 12), 8=-91(LC 13)							
Max Gra	av 2=1525(LC 19), 8=1525(LC	20)						
	Comp./Max. Ten All forces 25 295/423, 3-5=-2090/464, 5-7=-	0 (lb) or less except when shown 2091/464 7-8=-2296/423	l.					
BOT CHORD 2-13=-	222/2070, 10-13=-9/1347, 8-10)=-233/1886						
	-544/300, 5-13=-140/991, 5-10=	-139/991, 7-10=-544/300						
NOTES- 1) Unbalanced roof live	loads have been considered for	this design.						
		DL=6.0psf; BCDL=6.0psf; h=15ft to 16-6-0, Exterior(2) 16-6-0 to 2						
cantilever right expos	ed ;C-C for members and force	s & MWFRS for reactions shown	; Lumber DOL=1.	60 plate g				
4) * This truss has been	designed for a live load of 30.0	hord live load nonconcurrent wit) tall by	2-0-0 wide	THE COL	1 Carl
	ottom chord and any other mem connection (by others) of truss to	bers, with BCDL = 10.0pst. o bearing plate capable of withsta	anding 100 lb uplifi	at joint(s	s) 2, 8.		REAL	SPE Post
6) This truss is designed referenced standard A		nternational Residential Code se	ctions R502.11.1 a	nd R802.	.10.2 ar	nd	10	mad
							SEA	
							2 166	
							The sea	S S
						4	A MGIN	E CAR
							A ST	AL SA
							A DELTER.	10.0000
							February	/ 16.2022

February 16,2022

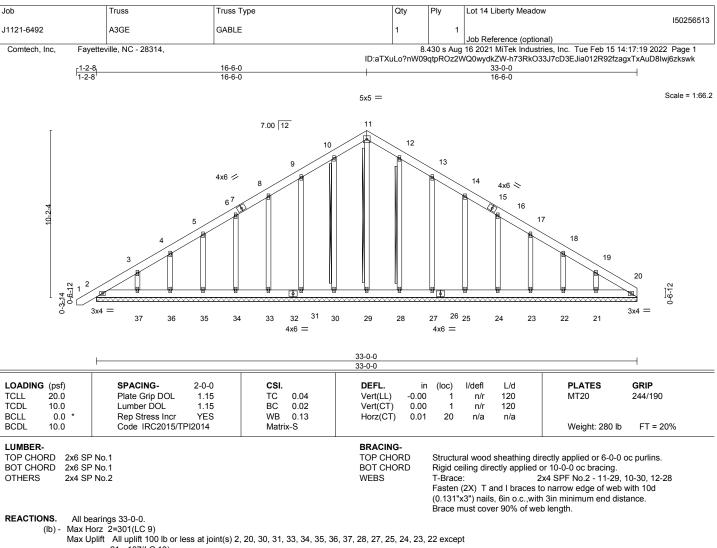
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss explants, see fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 ev. 5/19/2020 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, crection and bracing of fruses and russ systems, see ANSITPHI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





21=-107(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 20, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22 21

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-280/222, 10-11=-236/265, 11-12=-236/265

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated. 5) Gable requires continuous bottom chord bearing.

Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22 except (jt=lb) 21=107.

10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

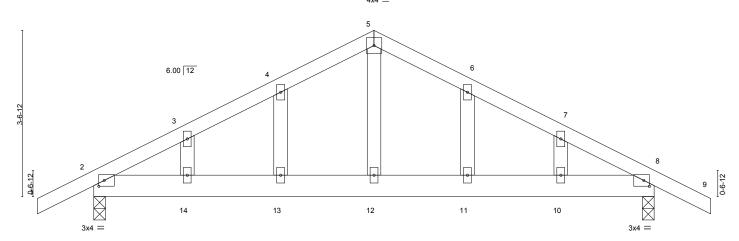
11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



🛕 WARNING - Verify design par rameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTTeKe connectors. This does not have a seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







F	<u> 6-0-0</u> 6-0-0					2-0-0		
Plate Offsets (X,Y)	[2:0-1-7,0-1-8], [8:0-1-7,0-1-8]					5-0-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.14 BC 0.19 WB 0.09 Matrix-S		in (loc) -0.02 10-11 -0.03 10-11 0.01 8 0.03 10-11	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 65 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x6 S WEBS 2x4 S	SP No.1 SP No.1 SP No.2 SP No.2	1	BRACING- TOP CHORI BOT CHORI				rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins.
Max Max Max	ze) 2=0-3-0, 8=0-3-0 Horz 2=-73(LC 17) Uplift 2=-143(LC 9), 8=-143(LC 8) Grav 2=550(LC 1), 8=550(LC 1) <. Comp./Max. Ten All forces 250 (lb) or	· less except when shown	1.					
BOT CHORD 2-14 8-1	=-617/666, 3-4=-570/687, 4-5=-561/740, 3 4=-494/491, 13-14=-494/491, 12-13=-494 0=-494/491 2=-513/328					COLUMN I	SE 166	73
	2313/320						Real and the	50
2) Wind: ASCE 7-10; gable end zone an Lumber DOL=1.60	ve loads have been considered for this de Vult=130mph Vasd=103mph; TCDL=6.0 d C-C Exterior(2) zone; porch left and rigi plate grip DOL=1.60	osf; BCDL=6.0psf; h=15ft nt exposed;C-C for memb	bers and forces & M	IWFRS for rea	ctions sho	own;	A ST	R/N Carlo
Gable End Details 4) All plates are 2x4 I	r wind loads in the plane of the truss only as applicable, or consult qualified building MT20 unless otherwise indicated.			ace), see Stan	dard Indu	stry		
7) * This truss has be	n designed for a 10.0 psf bottom chord liven designed for a live load of 30.0psf on				y 2-0-0 wi	de		
	bottom chord and any other members. al connection (by others) of truss to bearing	ng plate capable of withst	anding 100 lb uplift	at joint(s) exce	ept (jt=lb)			

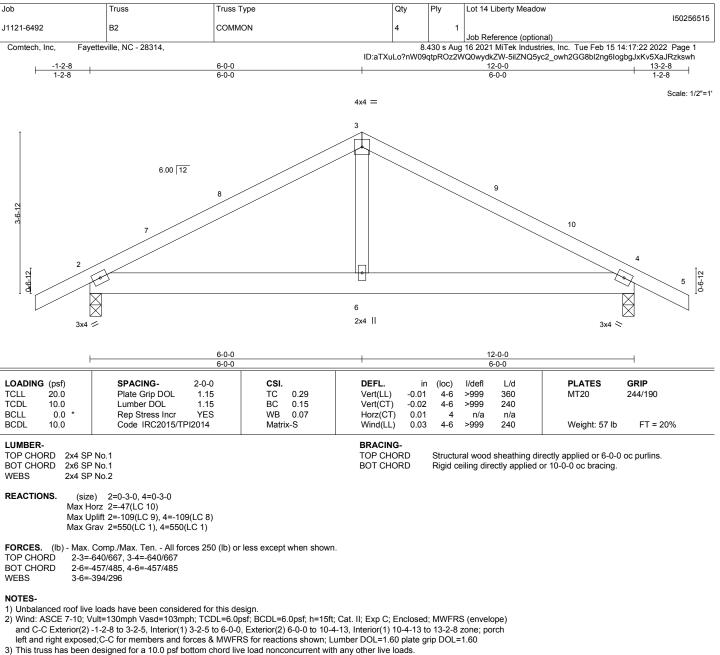
2=143, 8=143. 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and

 Inis truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.1 referenced standard ANSI/TPI 1.

February 16,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling of individual truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

TRENCO 818 Soundside Road Edenton, NC 27932



4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

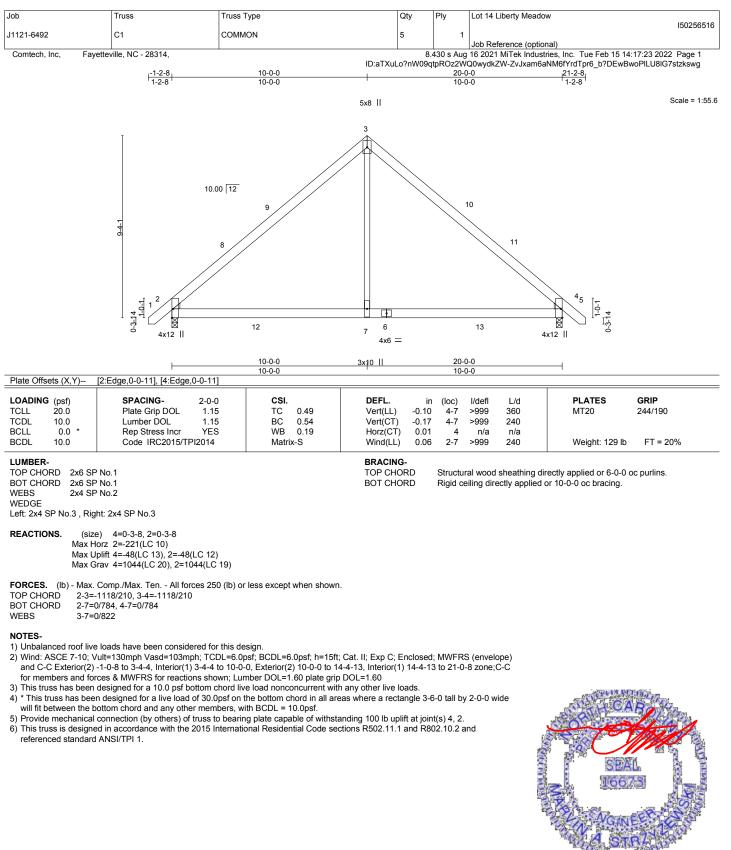
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=109, 4=109.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulpase with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSIVTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

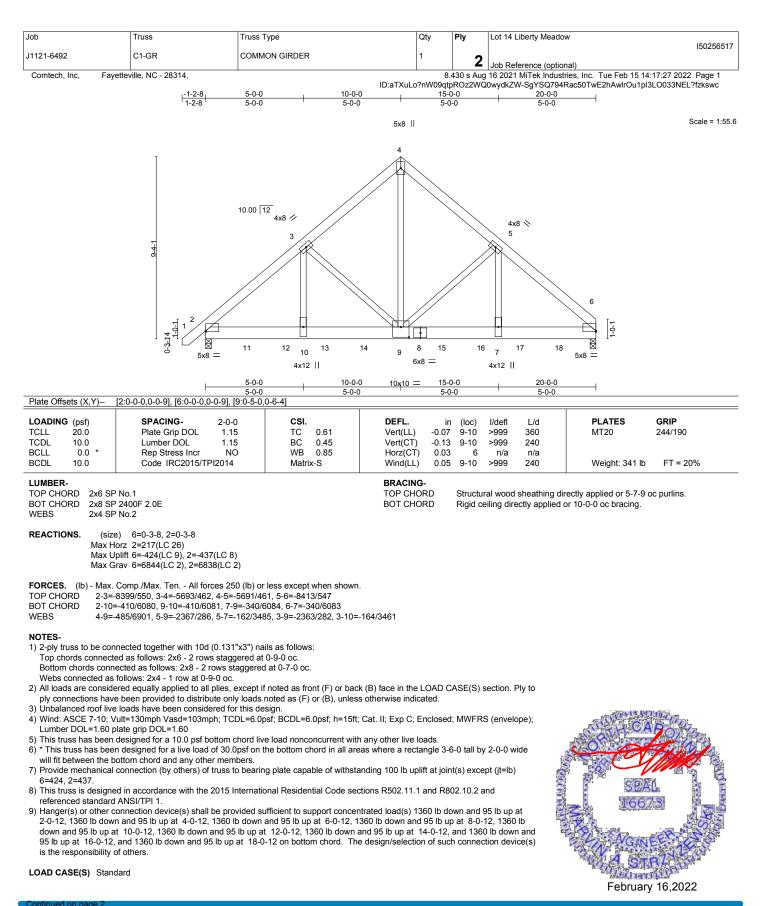




February 16,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outgains with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oblapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 B18 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 14 Liberty Meadow
					150256517
J1121-6492	C1-GR	COMMON GIRDER	1	2	Job Reference (optional)
				_	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Tue Feb 15 14:17:28 2022 Page 2

8.430 s Aug 16 2021 MiTek Industries, Inc. Tue Feb 15 14:17:28 2022 Page 2 ID:aTXuLo?nW09qtpROz2WQ0wydkZW-ws6qdTAiCukyecVQcPi9I2w3nCel4rFDI1_uX5zkswb

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

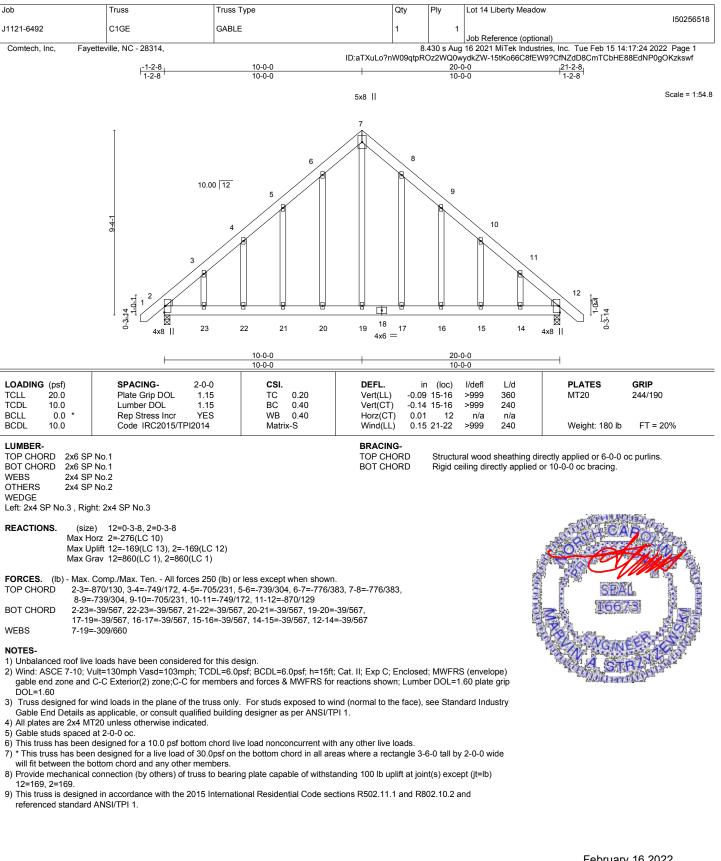
Uniform Loads (plf) Vert: 1-4=-60, 4-6=-60, 2-6=-20

Concentrated Loads (lb)

Vert: 9=-1290(F) 11=-1290(F) 12=-1290(F) 13=-1290(F) 14=-1290(F) 15=-1290(F) 16=-1290(F) 17=-1290(F) 18=-1290(F)

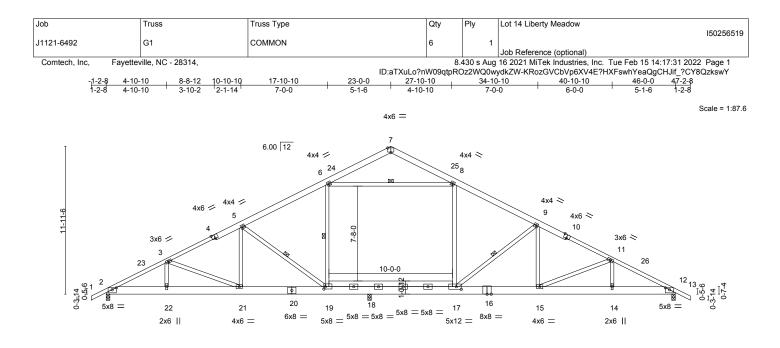
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidat truss eve and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This AND INCLODED with EnterPERCENCE PAGE MIT-1473 BV: 30 19/2/2/ BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is based building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 February 16,2022





Í	4-10-10	10-10-10	17-10-10	21-1-10	28-1-5	34-10-10	40-10-10	46-0-0	1
	4-10-10	6-0-0	7-0-0	3-3-0	6-11-11	6-9-5	6-0-0	5-1-6	1
Plate Offsets (X,Y)	[2:0-4-0.0-	1-15]. [4:0-3-0.Edae	1. [7:0-3-0.Edae]. [10	:0-3-0.Edael.	[12:0-4-0.0-1-15]. [17	:0-4-12.0-2-81. [19:0-1-	8.0-2-41		

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING - 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.26 BC 0.44 WB 0.42 Matrix-S	Vert(LL) -0.24 Vert(CT) -0.40 Horz(CT) 0.03	(loc) I/defi L/d 15-17 >999 360 15-17 >739 240 12 n/a n/a 15-17 >999 240	PLATES GRIP MT20 244/190 Weight: 376 lb FT = 20%
1-4,10- BOT CHORD 2x8 SF	No.1 *Except* 13: 2x4 SP No.1 2400F 2.0E *Except* 2x6 SP No.1 No.2		BRACING- TOP CHORD BOT CHORD WEBS	Rigid ceiling directly applied 8-5-9 oc bracing: 18-19 7-9-4 oc bracing: 17-18.	rectly applied or 3-11-0 oc purlins. or 10-0-0 oc bracing, Except: 9-17, 6-19, 5-19, 6-8
REACTIONS. (size Max H Max U					
TOP CHORD 2-3=- 8-9=- BOT CHORD 2-22=	Comp./Max. Ten All forces 250 (lb) of 2175/456, 3-5=-1759/445, 5-6=-1250/4 1235/382, 9-11=-2082/468, 11-12=-236 -304/1880, 21-22=-304/1880, 19-21=-1 7=-245/1812, 14-15=-325/2045, 12-14=-	13, 6-7=-378/186, 7-8=-35 7/451 94/1515, 18-19=-52/1043,	0/181,		

WEBS 9-17=-249/1812, 14-15=-329/2049, 12-14=-329/2049 1-15=-1058/260, 9-15=-26/593, 5-19=-875/234, 5-21=-23/446, 3-21=-393/120, 11-15=-281/104, 6-8=-846/319

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-7 to 3-2-5, Interior(1) 3-2-5 to 23-0-0, Exterior(2) 23-0-0 to 27-4-13, Interior(1) 27-4-13 to 47-2-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

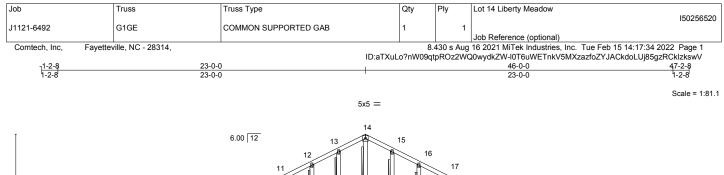
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 12=168, 18=120.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





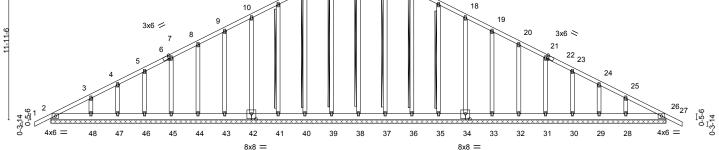


Plate Offsets (X,Y)	[6:0-1-9,Edge], [22:0-1-9,Ed	ge], [34:0-4-0,0)-4-8], [42	:0-4-0,0-4-8]	46-0-0						1
LOADING (psf) ICLL 20.0 ICDL 10.0 BCLL 0.0 *	Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.07 0.03 0.18	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.01	(loc) 27 27 26	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20	GRIP 244/190
3CDL 10.0	Code IRC2015/TPI20	014	Matrix	-S						Weight: 376 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP					BRACING- TOP CHOR		Ot		a ha a dhia ar dia	ectly applied or 6-0-0 c	

BOT CHORD

T-Brace:

WFBS

Rigid ceiling directly applied or 10-0-0 oc bracing.

Brace must cover 90% of web length.

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No.2

- REACTIONS. All bearings 46-0-0. (Ib) - Max Horz 2=245(LC
 - Max Horz 2=245(LC 16)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 26
 - Max Grav All reactions 250 lb or less at joint(s) 2, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 26
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-322/96, 11-12=-106/275, 12-13=-127/337, 13-14=-146/387, 14-15=-146/387, 15-16=-127/337, 16-17=-106/275 BOT CHORD 2-48=-82/271, 47-48=-82/271, 46-47=-82/271, 45-46=-82/271, 44-45=-82/271, 43-44=-82/271, 42-43=-82/271, 46-47=-82/271, 40-41=-82/271, 38-39=-82/271, 12-43=-82/271, 36-37=-82/271, 35-36=-82/271, 34-35=-82/271, 35-36=-82/271, 35-35=-82/271, 35-36=-82/271, 35-35=82/271, 35-35=82
 - 33-34=-82/271, 32-33=-82/271, 31-32=-82/271, 30-31=-82/271, 29-30=-82/271, 28-29=-82/271, 26-28=-82/271

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 26.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

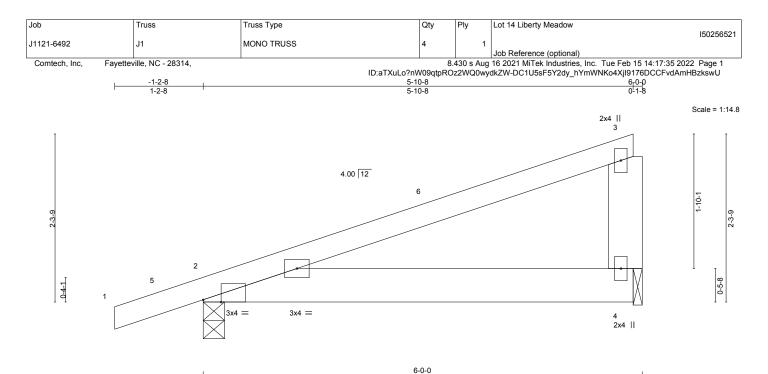
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oblapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



2x4 SPF No.2 - 14-38, 13-39, 12-40, 11-41

, 15-37, 16-36, 17-35





OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15	TC	0.41	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	-0.03	2-4	>999	240		
CLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
CDL 10.0	Code IRC2015/TP	12014	Matrix	κ-P	Wind(LL)	0.03	2-4	>999	240	Weight: 29 lb	FT = 20%

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No.1

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=83(LC 8) Max Uplift 2=-132(LC 8), 4=-90(LC 8) Max Grav 2=316(LC 1), 4=215(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

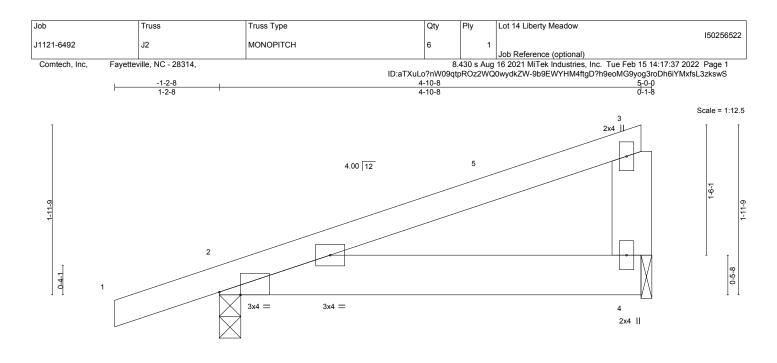
NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This term designed designed designed for the state of t
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=132.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling trusses and truss systems, see **AUSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in ((loc)	l/defl	L/d	PLATES	GRIP
ICLL 20.0	Plate Grip DOL 1.15	TC 0.26	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.08	Vert(CT)	-0.01	2-4	>999	240		
3CLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00		n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.01	2-4	>999	240	Weight: 24 lb	FT = 20%

BOT CHORD

5-0-0

Structural wood sheathing directly applied or 5-0-0 oc pur except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=72(LC 8) Max Uplift 2=-119(LC 8), 4=-72(LC 8) Max Grav 2=277(LC 1), 4=174(LC 1)

2x6 SP No 1

BOT CHORD 2x6 SP No.1

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

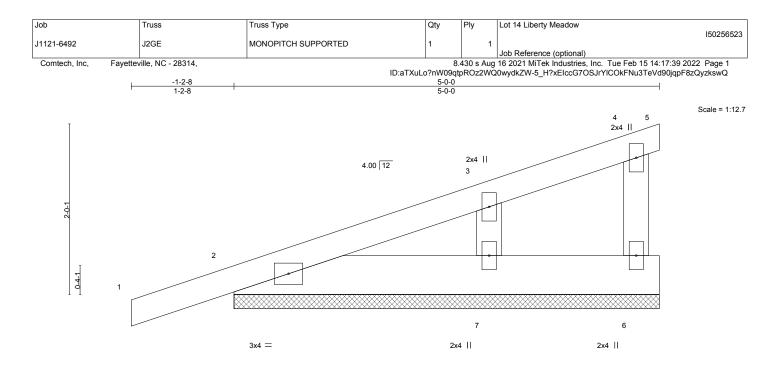
WFBS

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=119.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling trusses and truss systems, see **AUSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES PI2014	CSI. TC BC WB Matrix	0.07 0.02 0.03 -P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 -0.00	(loc) 1 1 5	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 24 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP	No.1		1		BRACING- TOP CHOF BOT CHOF	RD	except	end verti	cals.	rectly applied or 5-0-0 or 10-0-0 oc bracing.	oc purlins,

REACTIONS. All bearings 5-0-0.

(lb) - Max Horz 2=104(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 5, 6, 2, 7

Max Grav All reactions 250 lb or less at joint(s) 5, 6, 2, 7

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

Gable End Details as applicable, of consult qualified building designer as per 3)
 Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6, 2, 7.
- 8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



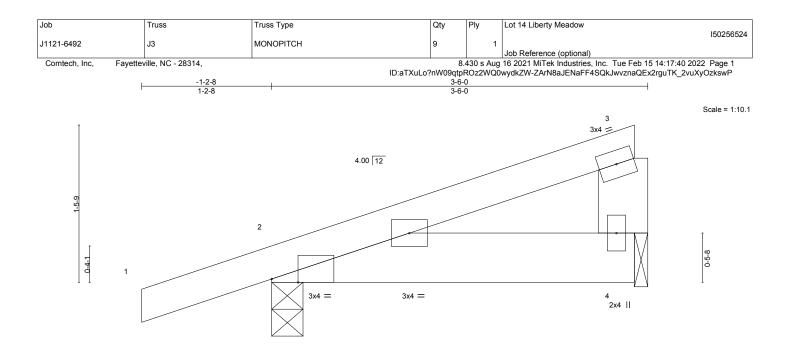


Plate Of	fsets (X,Y)	[2:0-2-15,Edge]		3-6-0	1
LOADIN TCLL TCDL BCLL	IG (psf) 20.0 10.0 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.09 BC 0.03 WB 0.01	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 2-4 >999 360 Vert(CT) -0.00 2-4 >999 240 Horz(CT) 0.00 n/a n/a	PLATES GRIP MT20 244/190
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.00 2 **** 240	Weight: 17 lb FT = 20%

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

BRACING-TOP CHORD

3-6-0

 TOP CHORD
 Structural wood sheathing directly applied or 3-6-0 oc purlins.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=56(LC 8) Max Uplift 2=-69(LC 8), 4=-14(LC 12) Max Grav 2=224(LC 1), 4=107(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

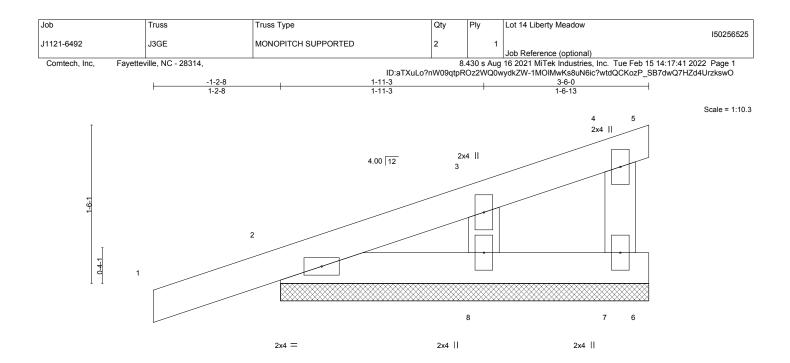
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

 This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling trusses and truss systems, see **AUSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.07 BC 0.02 WB 0.02 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 4 4	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 15 lb	GRIP 244/190 FT = 20%
LUMBER-	BRACING-	חי	Structu	ral wood	sheathing di	rectly applied or 3-6-0) oc purlins		

BOT CHORD 2x4 SP No.1 2x4 SP No.2 WEBS

TOP CHORD

ng directly applied or 3-6-0 oc purlins except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 7=3-6-0, 2=3-6-0, 8=3-6-0 Max Horz 2=79(LC 8)

Max Uplift 7=-26(LC 8), 2=-91(LC 8), 8=-38(LC 12)

Max Grav 7=52(LC 1), 2=164(LC 1), 8=125(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

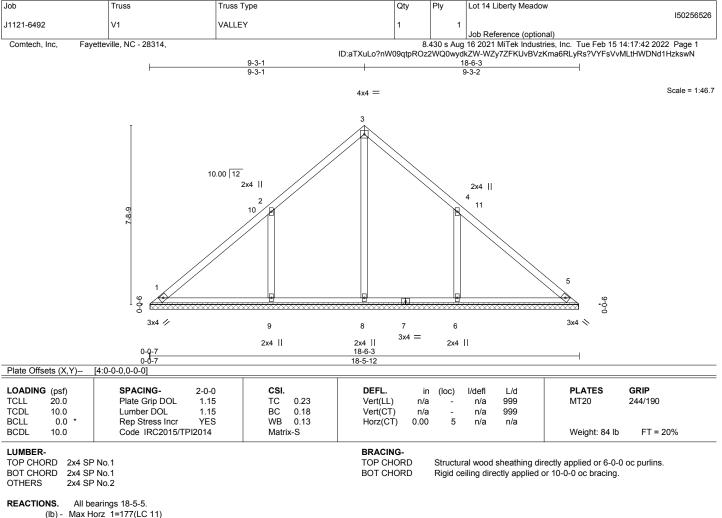
4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2, 8.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This AND INCLODED with EnterPERCENCE PAGE MIT-1473 BV: 30 19/2/2/ BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is based building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-172(LC 12), 6=-172(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=411(LC 22), 9=560(LC 19), 6=560(LC 20)

WEBS 2-9=-428/293, 4-6=-428/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 9-3-1, Exterior(2) 9-3-1 to 13-7-14, Interior(1) 13-7-14 to 18-1-6 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=172. 6=172.

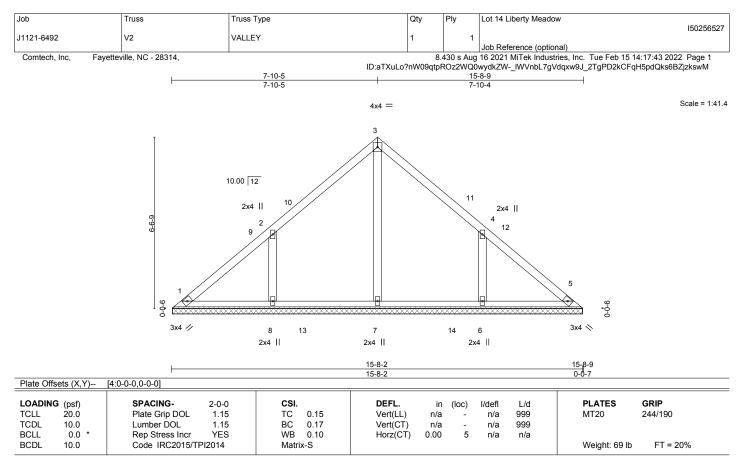
7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This AND INCLODED with EnterPERCENCE PAGE MIT-1473 BV: 30 19/2/2/ BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is based building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 15-7-11.

(lb) - Max Horz 1=-149(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-143(LC 12), 6=-142(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=411(LC 19), 8=429(LC 19), 6=429(LC 20)

WEBS 2-8=-356/254, 4-6=-356/254

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-10-5, Exterior(2) 7-10-5 to 12-3-1, Interior(1) 12-3-1 to 15-3-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=143, 6=142.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



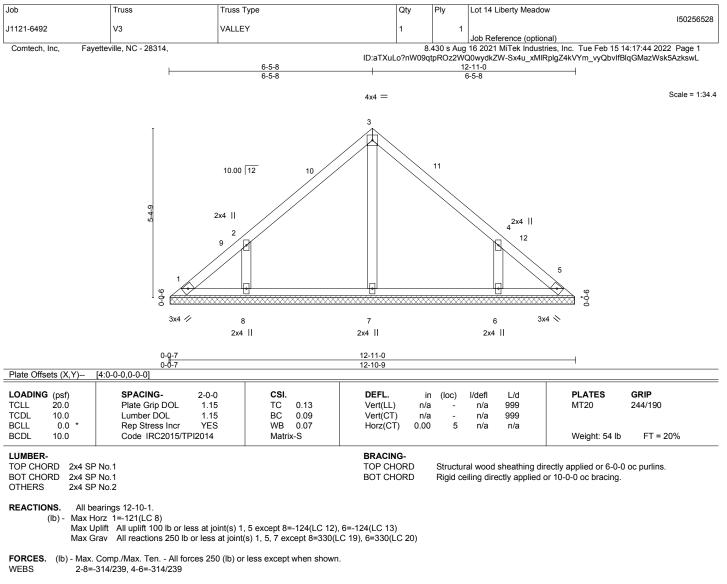
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulpase with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSIVTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-5-8, Exterior(2) 6-5-8 to 10-10-5, Interior(1) 10-10-5 to 12-6-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=124, 6=124.

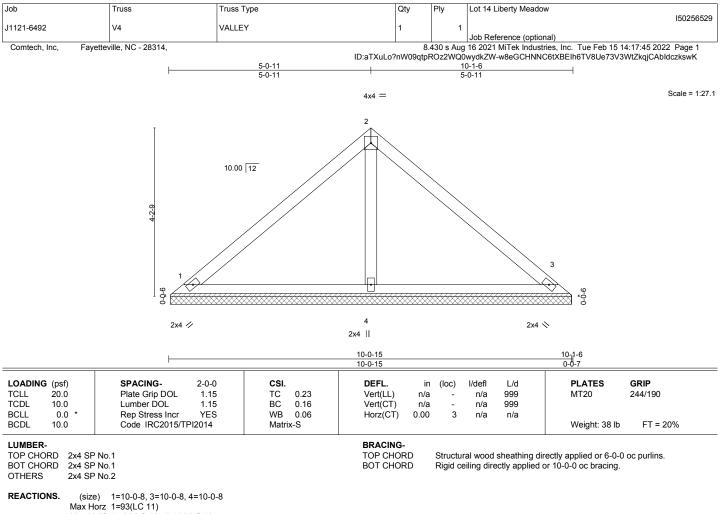
6) Non Standard bearing condition. Review required.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulcapes with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=199(LC 1), 3=199(LC 1), 4=347(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

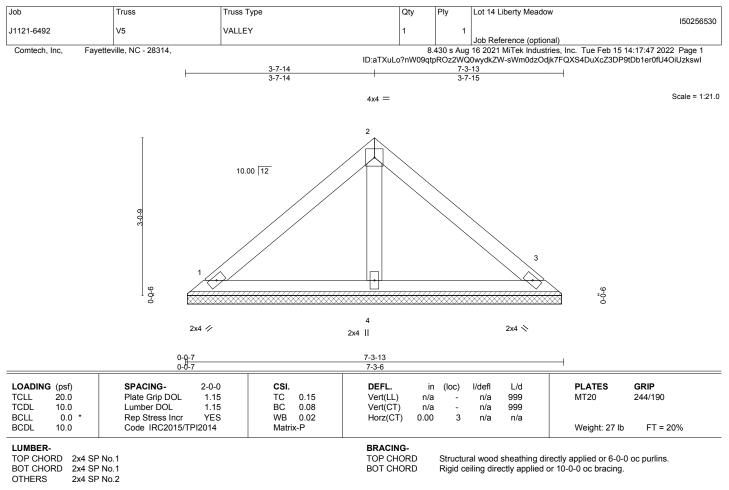
Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; b=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTTeKe connectors. This does not have a seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





REACTIONS. (size) 1=7-2-14, 3=7-2-14, 4=7-2-14 Max Horz 1=-65(LC 8) Max Uplift 1=-23(LC 13), 3=-29(LC 13)

Max Grav 1=151(LC 1), 3=151(LC 1), 4=220(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

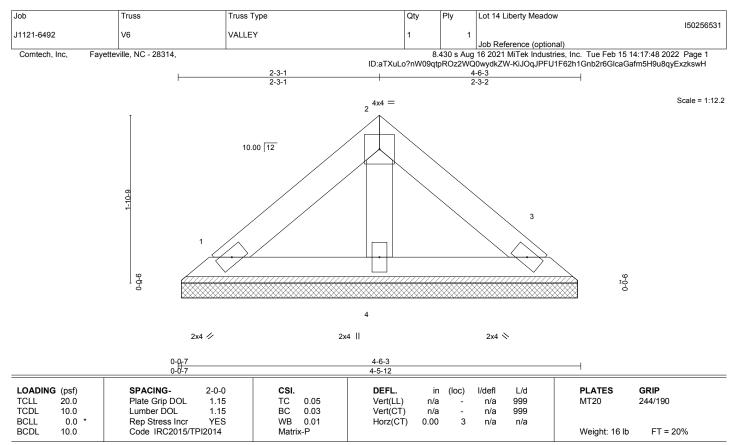
6) Non Standard bearing condition. Review required.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oblapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=4-5-5, 3=4-5-5, 4=4-5-5 Max Horz 1=-37(LC 8)

Max Uplift 1=-13(LC 13), 3=-16(LC 13)

Max Grav 1=86(LC 1), 3=86(LC 1), 4=125(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; b=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

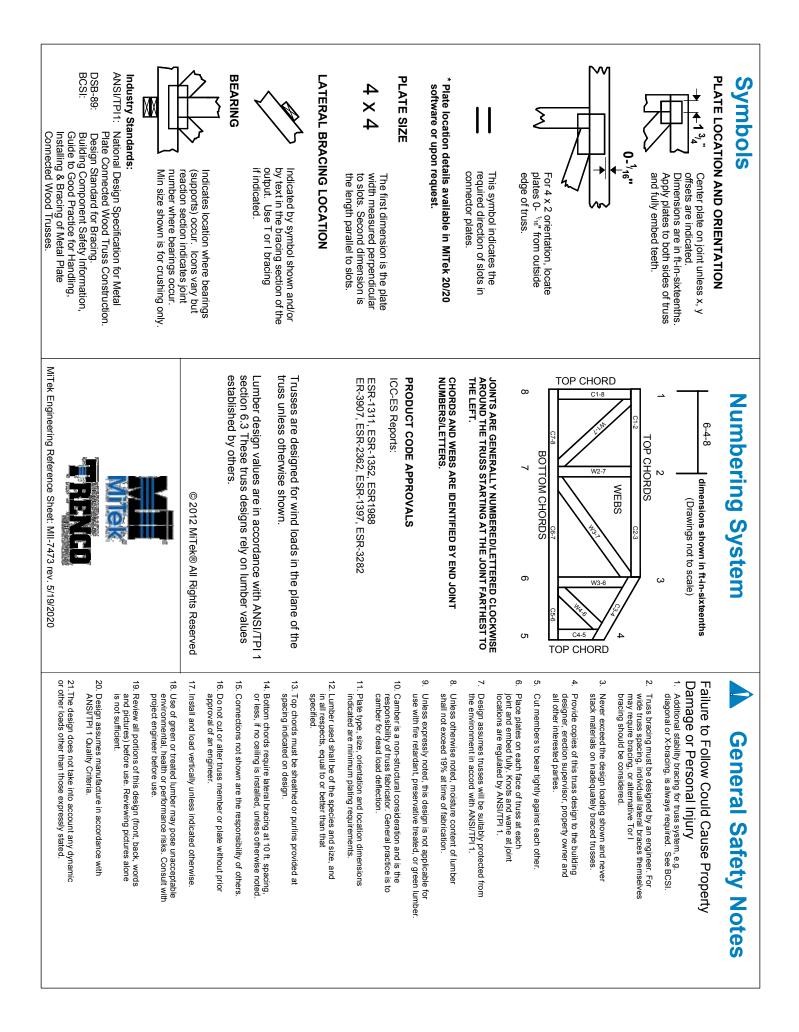


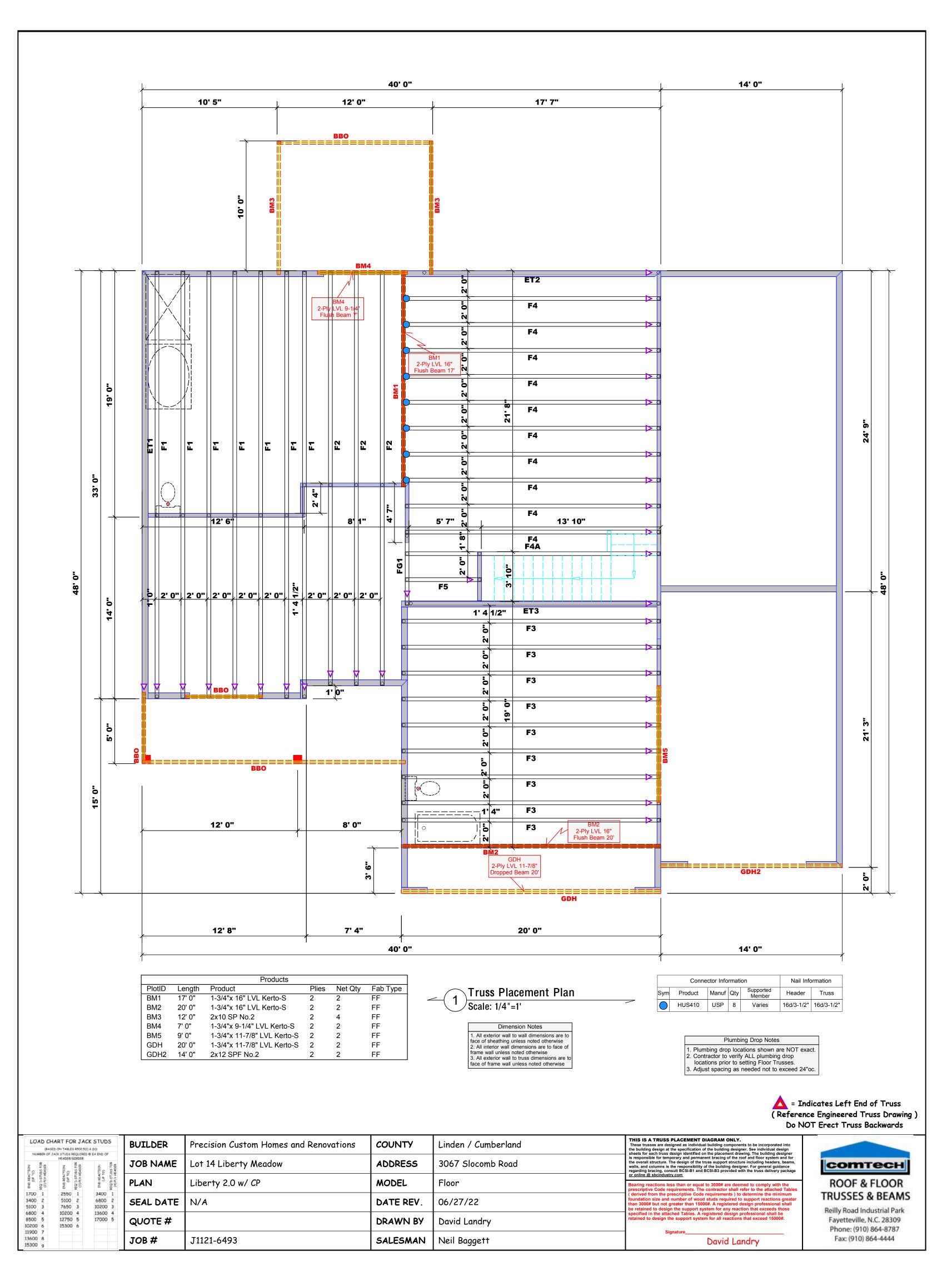
Structural wood sheathing directly applied or 4-6-3 oc purlins.

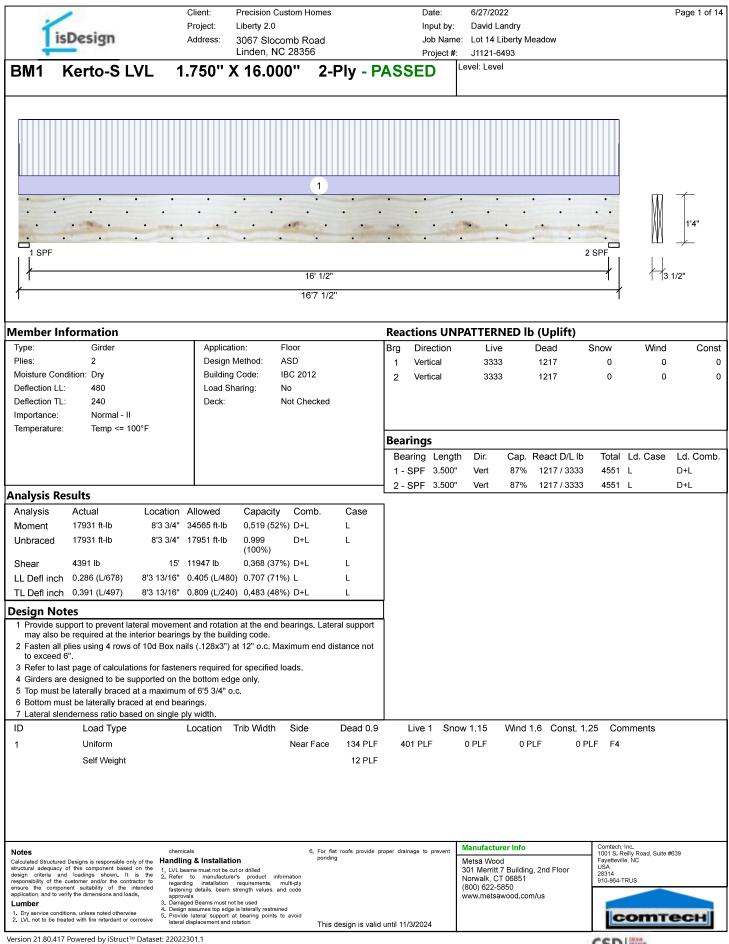
Rigid ceiling directly applied or 10-0-0 oc bracing.

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTTeKe connectors. This does not have a seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601









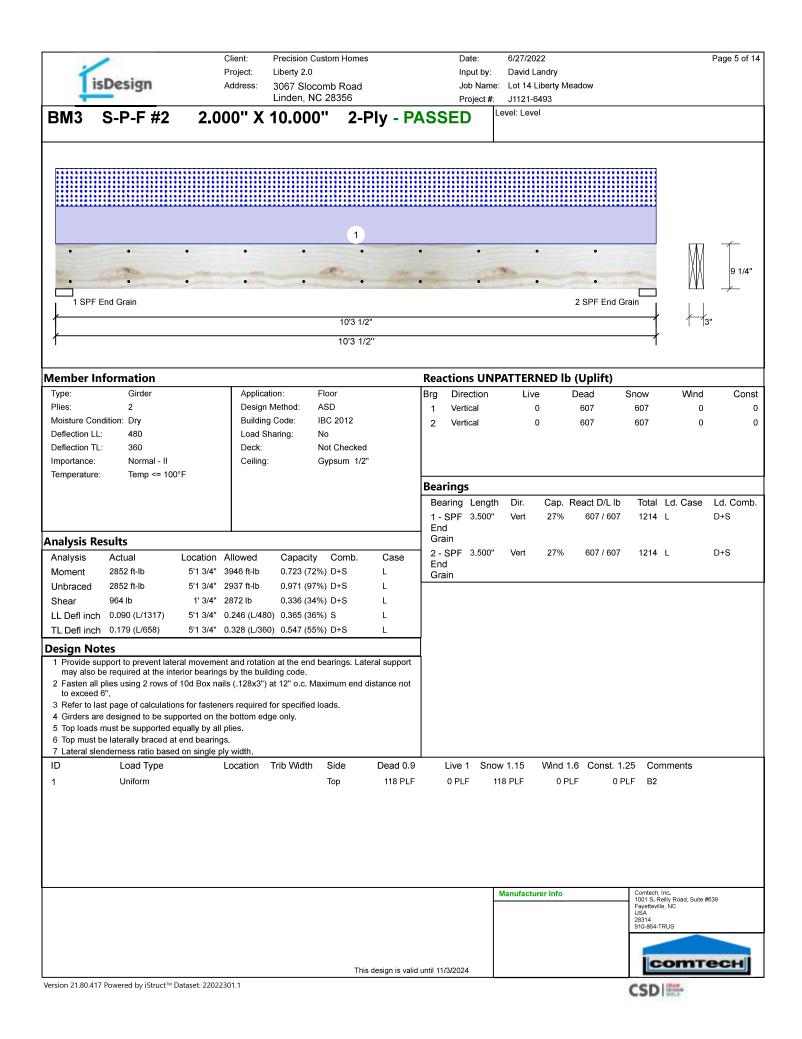
isDe	sign	Client: Project: Address:	Precision Custom Liberty 2.0 3067 Slocomb F Linden, NC 283	Road		Date: Input by: Job Name Project #:	6/27/2022 David Landry : Lot 14 Liberty Meadow J1121-6493		Page 2 of 14
BM1 Kei	rto-S LVL	1.750" >	(16.000''		PASSE		Level: Level		
	••••	•••	•••	•••	•	•	• • •		\mathbb{M}
		•••	•••	•••	•	•	• • •	\cdot \cdot \cdot	1'4"
1 SPF									
				16' 1/2"					3 1/2"
			1	6'7 1/2"				Ι	
Multi-Ply Analy									
Fasten all plies u Capacity	10 ising 4 rows of 10 81.7 %		.128x3") at 12"	o.c Maxim	um end dis	tance no	ot to exceed 6".		
Load Yield Limit per Foot	267.5 327.4								
Yield Limit per Faster Yield Mode	ier 81.9 lb IV								
Edge Distance	1 1/2"								
Min. End Distance Load Combination	3" D+L								
Duration Factor	1.00								
Notes		hemicals Indling & Installatio	20	 For flat roofs pro ponding 	vide proper drainage	to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Fayetteville, NC	Suite #639
Calculated Structured Design structural adequacy of this design criteria and loadi responsibility of the custome	component based on the 1 L ngs shown. It is the 2 P r and/or the contractor to r	VL beams must not be cu Refer to manufacture egarding installation	ut or drilled r's product information requirements, multi-ply	-			Metsä Wood 301 Merritt 7 Building, 2nd Flo Norwalk, CT 06851 (800) 622 5850		
ensure the component su application, and to verify the d Lumber 1. Dry service conditions, un	itability of the intended fi imensions and loads 3. [3. [ess noted otherwise 5. [astening details, beam : ipprovals Damaged Beams must no Design assumes top edge	strength values, and code t be used				(800) 622-5850 www.metsawood.com/us		тесн
2. LVL not to be treated with	infe retardant or corrosive	ateral displacement and r	otation	This design is	valid until 11/3/2	024		CON	neen

Version 21.80.417 Powered by iStruct™ Dataset: 22022301.1

-	Design	Client: Project: Address:	Liberty 2.0 3067 Sloco Linden, NC	28356			Project #:	J1121-64	ndry berty Meadow			Page 3 of 14
BM2 k	(erto-S LVL	1.750"	X 16.00	0" 2-	Ply - P/	ASSE	D	evel: Level.				
· ·	· · · ·	2	· ·	. 1.		••••	······	3				1
1 SPF				20'	15-			20 m		2 SPF	∦ _↓ ↓↓	1'4"
<u></u>				20'							7	172
/lember Inf	ormation					Reacti	ons UNF	PATTERN	IED lb (Uplift	;)		
Type: Plies: Moisture Cond Deflection LL: Deflection TL: Importance:	Girder 2	Buildin	Method: // g Code: I haring: I	Floor ASD BC 2012 No Not Checked		Brg D 1 V	irection ertical ertical	Live 400 400	Dead 2409 2409	Snow 135 135	0	Cons
Temperature:	Temp <= 100°F					1 - SP	gs g Length F 3.500" F 3.500"	Dir. Vert Vert	Cap. React D/l 54% 2409 / 4 54% 2409 / 4	401 28	al Ld. Case 11 L 11 L	Ld. Comb D+0.75(L+ D+0.75(L+
Analysis Re						2-01	0.000	Volt	21007	101 20		D . 0.10(L .
Analysis Moment Unbraced Shear LL Defl inch TL Defl inch	13439 ft-lb 13439 ft-lb 2461 lb 18 0.059 (L/3960) 10	cation Allowed 10' 34565 ft-lb 10' 13492 ft-lb 3'4 1/2'' 11947 lb 0' 1/16'' 0.489 (L/48 0' 1/16'' 0.978 (L/24)		D+L 6) D+L 6) 0.75(L+S)	Case L L L S) L							
may also be 2 Fasten all p to exceed 6 3 Refer to las 4 Girders are 5 Top loads m 6 Top must be 7 Bottom mus	port to prevent lateral r required at the interior lies using 3 rows of 100	bearings by the build box nails (.128x3") or fasteners required ted on the bottom ed lly by all plies. haximum of 8'9 7/16' end bearings.	lding code. at 12" o.c. Ma for specified I ge only. ' o.c.	aximum end di								
ID 1	Load Type Uniform	Location	Trib Width	Side Top	Dead 0.9 200 PLF		e 1 Snov PLF	w 1.15 0 PLF	Wind 1.6 Cons 0 PLF		Comments Wall Above, C10	26
2	Tie-In	0-0-0 to 20-0-0	1-0-0	Far Face	15 PSF	40 F		0 PSF	0 PSF		Floor Load	~~
3	Tie-In Self Weight	0-0-0 to 20-0-0	0-6-0	Near Face	27 PSF 12 PLF	0 F	PSF 2	27 PSF	0 PSF	0 PSF 、	J3	
structural adequacy or design criteria and responsibility of the c ensure the component	Designs is responsible only of the f this component based on the loadings shown. It is the Jacomer and/or the contractor to statuatily of the intervent y the dimensions and loads.	 LVL beams must not be Refer to manufacture regarding installation 	cut or drilled rer's product info requirements, r strength values, an not be used	pondin rmation nulti-ply d code	it roofs provide pr g	oper drainage	to prevent	Manufacture Metsä Wood 301 Merritt 7 Norwalk, CT (800) 622-58 www.metsaw	Building, 2nd Floor 06851 50	1001 Fayel USA 2831-	ech, Inc. S. Reilly Road, Suite / teville, NC 4 64-TRUS	#639

	1		Precision Custom Homes	Date:	6/27/2022	Page 4 of 14
1	isDesign		Liberty 2.0 3067 Slocomb Road	Input by	r: David Landry me: Lot 14 Liberty Meadow	
+	150 congin	/1001003.	Linden, NC 28356	Project		
BM2	Kerto-S LV	′L 1.750")	(16.000" 2-P	y - PASSED	Level: Level	
_[• • •	• • •	• • • •	• • • •	• • • •	
.						· · C
Ŀ—	• • •		• • • •		• • • •	
1 SPF						2 SPF /
†			20'			1 3 1/2"
/			20'			
Multi-Ply	y Analysis					
Fasten all	l plies using 3 rows	s of 10d Box nails (128x3") at 12" o.c Ma	ximum end distance	not to exceed 6".	
Capacity		11.2 %				
Load Yield Limit p	er Foot	27.5 PLF 245.6 PLF				
Yield Limit p		81.9 lb.				
Yield Mode		IV t t tou				
Edge Distan Min. End Dis		1 1/2" 3"				
Load Combi		D+L				
Duration Fac	ctor	1.00				
Nata -		chemicals	6 East 4-1	ofe provide proper discission to success	Manufacturer Info	Comtech, Inc.
Notes Calculated Struc	ctured Designs is responsible only o		on ponding	ofs provide proper drainage to prevent	Metsä Wood	 1001 S. Reilly Road, Suite #639 Fayetteville, NC
design criteria	uacy of this component based on and loadings shown. It is f the customer and/or the contracto	the 2 Refer to manufacture	's product information		301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the co	omponent suitability of the inter to verify the dimensions and loads.		requirements, multi-ply trength values, and code		(800) 622-5850 www.metsawood.com/us	1.5 004 1100
Lumber	conditions, unless noted otherwise	 Damaged Beams must no 4. Design assumes top edge 	is laterally restrained			
2. LVL not to b	be treated with fire retardant or corro	5. Provide lateral support a lateral displacement and r		ign is valid until 11/3/2024		соттесн
	417 Doworod by iStructTM D					

Version 21.80.417 Powered by iStruct™ Dataset: 22022301.1



1	isDesign	Client: Project: Address:	Precision Custom Homes Liberty 2.0 3067 Slocomb Road Linden, NC 28356	Date: Input by: Job Nam Project #	e: Lot 14 Liberty Meadow	Page 6 of 14
BM3	S-P-F #2	2.000" X	10.000" 2-Ply	- PASSED	Level: Level	
					• •	
	_					↓ ↓ ↓ ↓ 9 1/4"
	• PF End Grain	• •	• •	• •	• • — 2 SPF End	
			10'3 1/2"			/ 3"
∤			10'3 1/2"			
Maria: Di	y Analysis					
Fasten al Capacity Load Yield Limit p Yield Mode Edge Distan Min. End Di: Load Combi Duration Fa	eer Foot ber Fastener ice stance ination	rs of 10d Box nails 0.0 % 0.0 PLF 157.4 PLF 78.7 lb. IV 1 1/2" 3" 1.00	(.128x3") at 12" o.c Maxi	imum end distance n	ot to exceed 6".	
					Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetterville, NC
						Fayetteville, NC USA 28314 910-864-TRUS
			This design	n is valid until 11/3/2024		соттесн

1	Design	Client: Project:	Precision Custom Hom Liberty 2.0		In	ate: put by:	6/27/202: David La	ndry				Page 7 of 14
IS	Design	Address:	3067 Slocomb Road Linden, NC 28356	1		ob Name: roject #:	Lot 14 Lil J1121-64	perty Meadow 93				
BM4	Kerto-S LVI	_ 1.750'	' X 9.250''	2-Ply -	PASSI	ED L	evel: Level					
				3								
	2											
			1									
•	•	•	•	•	•		•				MM	
	C. C. Mar		-1000	the	-	-Uri	-				ŇŇ	9 1/
•		•	•			100	•				ĽŸĽ	<u> </u>
1 SPF	End Grain				2 S	PF End G	Brain					/
			6'				1				1 1	3 1/2"
1			6'7"				.1					
/ember In	formation				Reaction	ns UNP	ATTERN	ED lb (Uplif	t)			
Туре:	Girder	Applica	tion: Floor		-	ection	Live	Dead	-, Sno	ow	Wind	Const
Plies:	2 dilion: Day	-	Method: ASD		1 Vert		2149	1903		0	0	0
Moisture Con Deflection LL:	-	Building Load Si			2 Vert	ical	2149	1903		0	0	0
Deflection TL:		Deck:	Not Checke	ed								
Importance: Temperature:	Normal - II Temp <= 100°F											
remperature.	Temp <= 100 1				Bearing	S						
					Bearing	-	Dir.	Cap. React D			. Case	Ld. Comb.
					1 - SPF End	3.500"	Vert	39% 1903 / 2	2149	4053 L		D+L
nalysis Re	sults				Grain	0.500") (a at	200/ 4000 //	140	4050		Dul
Analysis		cation Allowed	Capacity Comb.	Case	2 - SPF End	3.500	Vert	39% 1903 / 2	(149	4053 L		D+L
Moment Unbraced		'3 1/2" 12542 ft-lb '3 1/2" 9934 ft-lb	0.460 (46%) D+L 0.581 (58%) D+L	L	Grain							
Shear		1' 3/4" 6907 lb	0.398 (40%) D+L	L								
LL Defl inch		'3 1/2" 0.153 (L/480		L								
		"3 1/2" 0.204 (L/360	0) 0.514 (51%) D+L	L	ļ							
Design Not	tes pport to prevent lateral n	novement and rotatic	n at the end bearings I	ateral support								
may also b	e required at the interior plies using 2 rows of 10d	bearings by the buil	ding code.									
3 Refer to las	st page of calculations fo											
	e designed to be support nust be supported equal		ge only.									
	e laterally braced at end st be laterally braced at	-										
	nderness ratio based on	-										
ID	Load Type	Location	Trib Width Side	Dead 0.9	Live			Wind 1.6 Con		Comm	ents	
1	Uniform		Тор	102 PLF 349 PLF	304 PLI 349 PLI		0 PLF 0 PLF	0 PLF 0 PLF		F2 A1		
2 3	Uniform Uniform		Тор Тор	349 PLF 120 PLF	349 PL 0 PL		0 PLF	0 PLF 0 PLF	0 PLF 0 PLF	A1 Wall Ab	ove	
-	Self Weight		19P	7 PLF	5 T E				с. LI			
	5											
Notes	Designs is responsible only of the	chemicals Handling & Installati		or flat roofs provide pr ending	oper drainage to	prevent	Manufacture Metsä Wood	er Info	10	omtech, Inc. 001 S. Reilly F ayetteville, NC	Road, Suite #6	39
structural adequacy design criteria and	of this component based on the loadings shown. It is the	1. LVL beams must not be o 2. Refer to manufacture	ut or drilled er's product information			3		Building, 2nd Floo 06851	r U 24	SA 3314		
ensure the compor	customer and/or the contractor to nent suitability of the intended rify the dimensions and loads	regarding installation	requirements, multi-ply strength values, and code			((800) 622-58 www.metsaw	50	9	10-864-TRUS		
		Damaged Beams must n	ot be used			'						
_umber 1. Dry service condit	ions, unless noted otherwise ated with fire retardant or corrosive	 Design assumes top edg Provide lateral support 	e is laterally restrained							lan	т	a constall

	1	Client: Project:	Precision Custom Ho Liberty 2.0	mes	Date: Input by:	6/27/2022 David Landry	Page 8 of 14
	isDesign	Address:	3067 Slocomb Roa Linden, NC 28356		Job Nam	e: Lot 14 Liberty Meadow	
BM	4 Kerto-S L	VL 1.750'	' X 9.250"		Project #	:: J1121-6493 Level: Level	
				_ · · ,			
	•	•	•	•	•	• -	
						• <11/2"	9 1/4
•	•	•	•	•	•	•	
	I SPF End Grain				2 SPF End	t Grain ∧	
	ſ		6'				1/2 "
1			6'7"			1	
Multi-	Ply Analysis						
	all plies using 2 rows		(.128x3") at 12" o.	.c Maximu	m end distance n	ot to exceed 6".	
Capacity Load		0.0 % 0.0 PLF					
	nit per Foot nit per Fastener	163.7 PLF 81.9 lb.					
Yield Mo	de	IV					
Edge Dis Min End	stance I Distance	1 1/2" 3"					
	mbination	5					
Duration	Factor	1.00					
Notes		chemicals		For flat roofs provid	le proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
structural a design cr	Structured Designs is responsible only o adequacy of this component based on riteria and loadings shown. It is ity of the customer and/or the contractor	the 1. LVL beams must not be the 2. Refer to manufacture	cut or drilled er's product information	ponding		Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	Fayetteville, NC USA 28314 910-864-TRUS
ensure th	e component suitability of the inter , and to verify the dimensions and loads.		requirements, multi-ply strength values, and code ot be used			(800) 622-5850 www.metsawood.com/us	
1. Dry ser	rvice conditions, unless noted otherwise t to be treated with fire retardant or corro	 Design assumes top edg Provide lateral support 	e is laterally restrained at bearing points to avoid	This design is v	alid until 11/3/2024		соттесн
Version 2	1 80 417 Powered by iStruct™ D	atacot: 22022301.1					CCD LIBAW

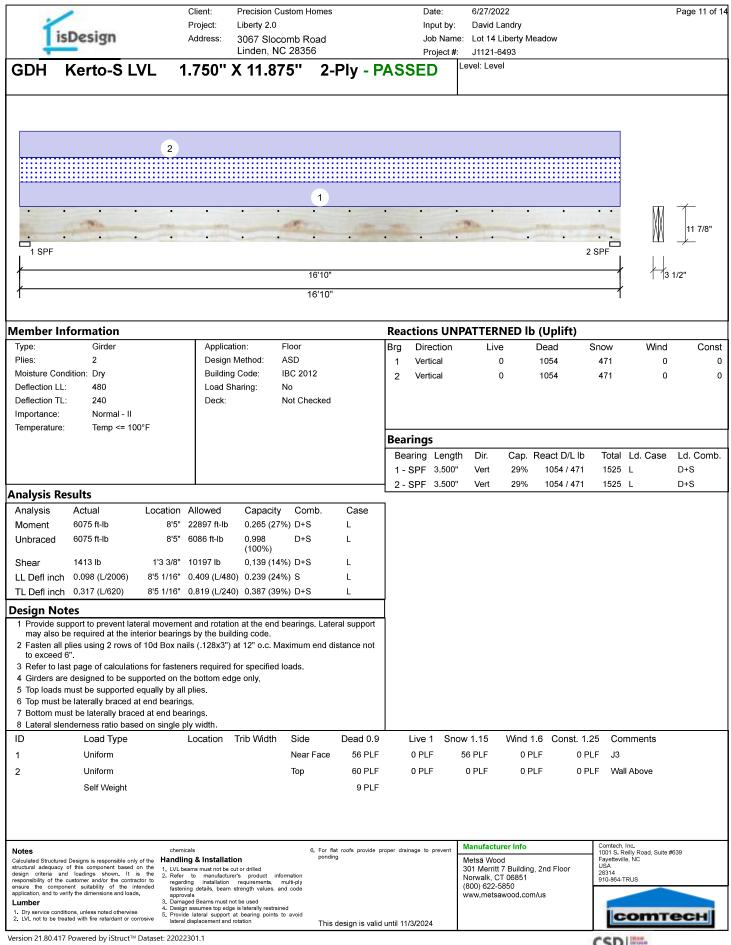
l by iS

	-	Client:	Precision Custom H	lomes	Date:	6/27/2022				Page 9 of 14
Tis	Design	Project: Address:	Liberty 2.0 3067 Slocomb R	oad	Input Job N		ndry berty Meadow			
÷			Linden, NC 2835	6	Projec		93			
BM5	Kerto-S LVI	L 1.750'	' X 11.875'	" 2-Ply	- PASSE	D ^{Level: Level}				
					1					
	3		2							_/_
•		•	•	• •	•	- The P			M	
	Crittin .		100	IN The	and and	- Win			IĂIĂ	11 7/8
	• • •	Test - 1 - 11-	•	• •	•				<u>v v</u> v	
	End Grain		8'		28	SPF End Grain				1/01
			8'8 11/16''			I	-			3 1/2"
I			8811/10				I			
/lember In	formation				Reactions l	JNPATTERN	ED lb (Uplift)			
Type: Plies:	Girder 2	Applica	tion: Floor Method: ASD		Brg Directio	on Live 1772	Dead 2203	Snow	Wind	Const
Moisture Con			g Code: IBC 20'	12	1 Vertical 2 Vertical	1772	2203	1044 1033	0 0	0
Deflection LL: Deflection TL:		Load S Deck:	haring: No Not Che	akad						
Importance:	240 Normal - II	Deck.	NOT CHE	ескей						
Temperature:	Temp <= 100°F				Beeringe					
					Bearings Bearing Le	ngth Dir.	Cap. React D/L	b Total Ld	Case	Ld. Comb.
					1 - SPF 4.6	-	32% 2203 / 211		. 0000	D+0.75(L+8
Analysis Re	sults				End Grain					
Analysis	Actual Lo	cation Allowed	Capacity Cor		2 - SPF 4.0 End)63" Vert	36% 2179 / 208	9 4268 L		D+0.75(L+8
Moment Unbraced		l'4 5/8" 19911 ft-lb l'4 5/8" 11006 ft-lb	0.376 (38%) D+L 0.738 (74%) D+0		Grain					
Shear		'4 1/2" 8867 lb	0.309 (31%) D+L							
LL Defl inch	0.059 (L/1643) 4'4	11/16" 0.203 (L/48	0) 0.292 (29%) 0.75	i(L+S) L						
		11/16" 0.406 (L/24	0) 0.298 (30%) D+0	.75(L+S) L]					
Design Not	pport to prevent lateral n	novement and rotation	on at the end bearing	s. Lateral support	4					
may also b	e required at the interior blies using 2 rows of 100	bearings by the buil	ding code.							
to exceed 6	5".	, ,								
	st page of calculations for designed to be support		•							
	nust be supported equal e laterally braced at end									
7 Bottom mu	st be laterally braced at inderness ratio based on	end bearings.								
ID	Load Type	0 1 <i>j</i>	Trib Width Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6 Const.	1.25 Comm	ents	
1	Part. Uniform	0-0-0 to 8-8-11	Тор	135 PLF	404 PLF	0 PLF	0 PLF (PLF F3		
2	Uniform		Тор	120 PLF	0 PLF	0 PLF) PLF Wall Ab	ove	
3	Uniform		Тор	238 PLF 9 PLF	0 PLF	238 PLF	0 PLF (PLF C1		
	Self Weight			9 PLF						
						Manufacture	r Info	Comtech, Inc.		
Notes Calculated Structured	Designs is responsible only of the of this component based on the		ion	 For flat roofs provide p ponding 	roper drainage to preve	Metsä Wood		1001 S. Reilly F Fayetteville, NC	Road, Suite #	339
design criteria and responsibility of the	I loadings shown. It is the customer and/or the contractor to	 Refer to manufactur regarding installation 	er's product information requirements, multi-ply			Norwalk, CT		28314 910-864-TRUS		
ensure the compor application, and to ver Lumber	ent suitability of the intended ify the dimensions and loads.	fastening details, beam approvals 3. Damaged Beams must r	strength values, and code ot be used			(800) 622-58 www.metsaw				
1. Dry service condit	ions, unless noted otherwise ted with fire retardant or corrosive	 Design assumes top edg Provide lateral support lateral displacement and 	at bearing points to avoid	This design is valid	until 11/3/2024			CO	т	есн
				200.9.1 10 Valla						

	isDesign		Client: Project: Address:	Precision Custom Liberty 2.0 3067 Slocomb F Linden, NC 283	Road		Date: Input by: Job Name: Project #:	6/27/2022 David Landry Lot 14 Liberty Meadow J1121-6493	Page 10 of 14
BM5	Kerto-S	LVL	1.750'	' X 11.875		y - PA		evel: Level	
				•		•			<u> </u>
	•	•	•	·	·	•	·		11 7/8"
	•	•	•	•	•	•	•		
	SPF End Grain			8'			2 SPF E	End Grain	3 1/2"
≁'				8'8 11/16"				ł	
	y Analysis Il plies using 2 ro	ows of 10d	Box nails ((.128x3") at 12"	o.c Maximi	um end di	istance no	t to exceed 6".	
Load Yield Limit p	per Foot	0.0 PLF 163.7 PL	.F						
	per Fastener	81.9 lb. IV							
Edge Distar Min. End Di	nce	1 1/2" 3"							
Load Comb	ination	3 1.00							
Notes	utural David		nicals ling & Installati	on	 For flat roofs prov ponding 	ide proper draina	ge to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adeo design criteria responsibility o ensure the c	actured Designs is responsible of quacy of this component base a and loadings shown. It of the customer and/or the cor- component suitability of the to vorify the dimensioner and lo	id on the 1 LVL is the 2 Refe intractor to rega intended faste	beams must not be o er to manufacture rding installation ening details, beam		. •		:	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	USA 28314 910-864-TRUS
Lumber 1. Dry service	d to verify the dimensions and lo conditions, unless noted otherw be treated with fire retardant or	ads appr 3 Dam 4 Desi vise 5 Prov	rovals naged Beams must n ion assumes top edo	ot be used e is laterally restrained at bearing points to avoid	This design is	valid until 11/3.		www.metsawood.com/us	соттесн
Varcian 21.90) 417 Powered by iStruct	TM Dataset: 2202	22201.1		-				CCD LIBAR

Version 21.80.417 Powered by iStruct™ Dataset: 22022301

CSD

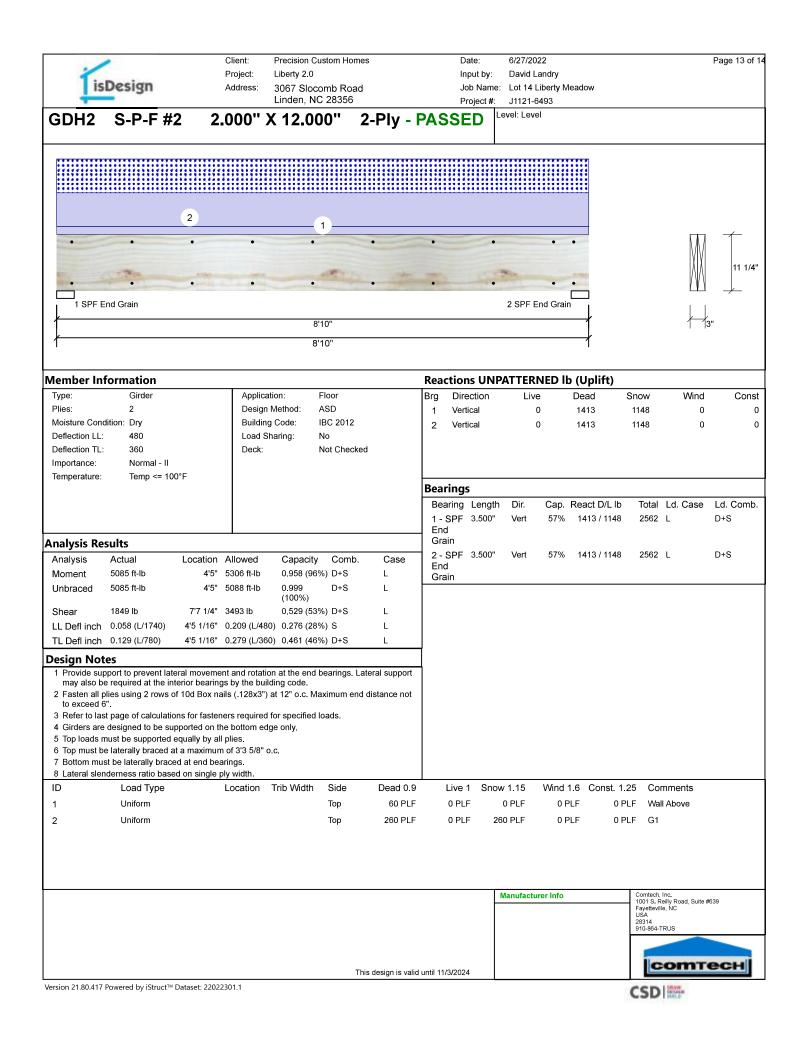


CSD

1	isDesign		Client: Project: Address:	Precision Custom Liberty 2.0 3067 Slocomb F Linden, NC 283	Road	Jol	out by:	6/27/2022 David Landry Lot 14 Liberty Meadow J1121-6493	Page 12 of 14
GDH	Kerto-S	LVL	1.750"	X 11.875"		- PASSEE	·	evel: Level	
•	• •	•	• •	• •	• •	• •	•	• • •	
1 SPF	•••	•	•••	• •	• •	•••	•	• • •	2 SPF
					16'10"				3 1/2"
/					16'10"				
Multi-Ply	Analysis								
Fasten all	-	rows of 1 29.7		(.128x3") at 12"	o.c Maxim	um end distar	nce no	t to exceed 6".	
Capacity Load Yield Limit pe	r Foot	29.7 56.0 188.3	PLF						
Yield Limit pe Yield Mode		94.1 I IV							
Edge Distanc		1 1/2' 3"	n						
Min. End Dist Load Combin Duration Fact	ation	3" D+S 1.15							
Notes			chemicals	41 - 1-	 For flat roofs pro ponding 	vide proper drainage to j		Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
structural adequa design criteria responsibility of t	ured Designs is responsibl acy of this component ba and loadings shown, the customer and/or the nponent suitability of th	It is the 2.	LVL beams must not be Refer to manufactu regarding installation				:	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	Fayetteville, NC USA 28314 910-864-TRUS
application, and to Lumber 1. Dry service co	overify the dimensions and onditions, unless noted other treated with fire retardant	Iloads. 3. arwise 5	approvals Damaged Beams must Design assumes top ed	not be used ge is laterally restrained t at bearing points to avoid	This desire '	volid uptil 11/2/2021	ľ	www.metsawood.com/us	соттесн
L 21.00.4	117 Poworod by iStru				This design is	valid until 11/3/2024			

Version 21.80.417 Powered by iStruct™ Dataset: 22022301.1

CSD



isDe	esign	Client: Project: Address:	Precision Custom Home Liberty 2.0 3067 Slocomb Road Linden, NC 28356		Date: Input by: Job Name Project #:	6/27/2022 David Landry : Lot 14 Liberty Meadow J1121-6493	Page 14 of 14
GDH2	S-P-F #2	2.000" 2	X 12.000" 2	2-Ply - PA	SSED	Level: Level	
					I		
•	• •	•	•	• •	•	•••	
1 SPF End	• •	•	•	• •	•	• •	\pm \square \perp
	Grain		8'10"				3"
∤			8'10"				
Multi-Ply Ana		10d Box nails	(.128x3") at 12" o.c	Maximum end	distance no	nt to exceed 6"	
Capacity Load	0.0						
Yield Limit per Foot	157	7.4 PLF					
Yield Limit per Faste Yield Mode	ener 78. IV	7 lb.					
Edge Distance	1 1	/2"					
Min. End Distance Load Combination	3"						
Duration Factor	1.0	0					
					Г	Manufacturer Info	Comtech, Inc.
							Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
			Th	is design is valid until 1	1/3/2024		соттесн



RE: J1121-6493

Lot 14 Liberty Meadow

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Precision Custom Homes and RenovationsProject Name: J1121-6493Lot/Block: 14Model: Liberty 2.0Address: 3067 Slocomb RoadSubdivision: Liberty MeadowCity: LindenState: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

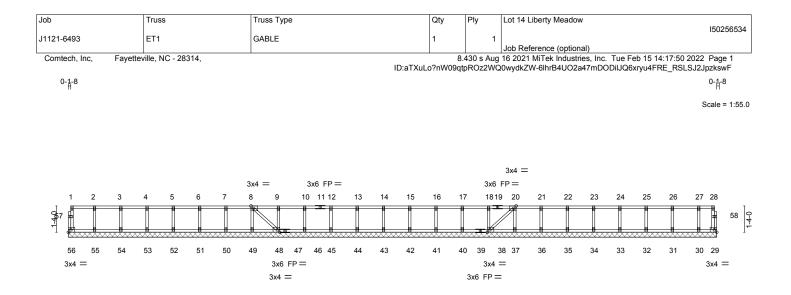
This package includes 10 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	150256534	ET1	2/16/2022
2	150256535	ET2	2/16/2022
3	150256536	ET3	2/16/2022
4	150256537	F1	2/16/2022
5	150256538	F2	2/16/2022
6	150256539	F3	2/16/2022
7	150256540	F4	2/16/2022
8	150256541	F4A	2/16/2022
9	150256542	F5	2/16/2022
10	150256543	FG1	2/16/2022

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844 IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters

designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the design for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





<u>| 14-0 | 2-8-0 | 4-0-0 | 5-4-0 | 6-8-0 | 8-0-0 | 9-4-0 | 10-8-0 | 12-0-0 | 13-4-0 | 14-8-0 | 16-0-0 | 17-4-0 | 18-8-0 | 20-0-0 | 21-4-0 | 22-8-0 | 24-0-0 | 25-4-0 | 26-8-0 | 28-0-0 | 29-4-0 | 30-8-0 | 32-0-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | 14-0 | </u>

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. i	n (loc) l/defl L/d	PLATES	GRIP	
TCLL 40.0	Plate Grip DOL 1.00	TC 0.06	Vert(LL) n/	a - n/a 999	MT20	244/190	
TCDL 10.0	Lumber DOL 1.00	BC 0.01	Vert(CT) n/	a - n/a 999			
BCLL 0.0	Rep Stress Incr YES	WB 0.03	Horz(CT) -0.0) 38 n/a n/a			
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 147 lb	FT = 20%F, 11%E	
LUMBER-			BRACING-				
TOP CHORD 2x4	SP No.1(flat)		TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,				
BOT CHORD 2x4	SP No.1(flat)			except end verticals.			
WEBS 2x4	SP No.3(flat)	BOT CHORD	CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.				
OTHERS 2x4	SP No.3(flat)				•		

REACTIONS. All bearings 32-11-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 56, 29, 55, 54, 53, 52, 51, 50, 49, 48, 46, 45, 44, 43, 42, 41, 40, 38, 37, 36, 35, 34, 33, 32, 31, 30

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



32-11-0



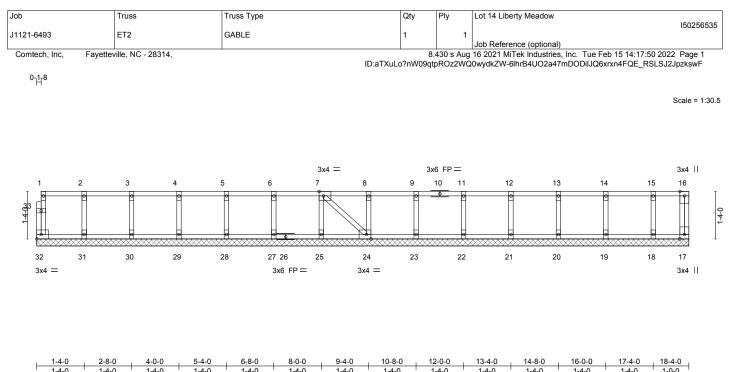


Plate Offsets (X,Y)	[7:0-1-8,Edge], [24:0-1	-8,Edge]	1-4-0 1			1-4-1	0 1	-4-0 1-4	-0 1-4-0 1	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	1.00 NO	CSI. TC BC WB	0.07 0.01 0.03	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	- I - I	lefl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015	/TPI2014	Matri	x-S					Weight: 84 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 3	BRACING- TOP CHOR		tructural v		directly applied or 6-0-	0 oc purlins,				

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)OTHERS2x4 SP No.3(flat)

REACTIONS. All bearings 18-4-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 32, 17, 31, 30, 29, 28, 27, 25, 24, 23, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

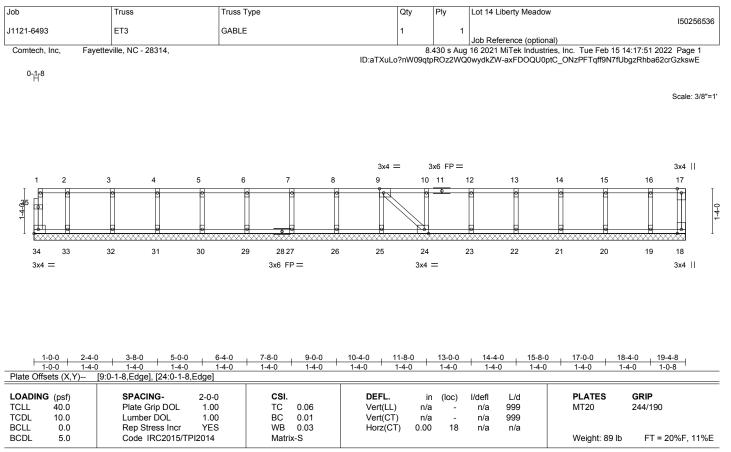
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.







LUMBER	

OTHERS

TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)

BRACING-TOP CHORD Structu except BOT CHORD Rigid (

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 19-4-8.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 34, 18, 33, 32, 31, 30, 29, 27, 26, 25, 24, 23, 22, 21, 20, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

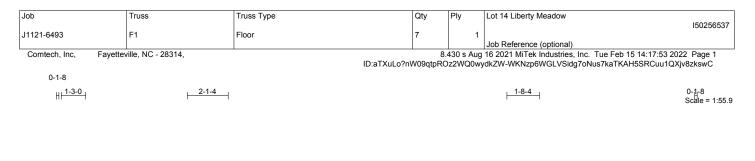
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

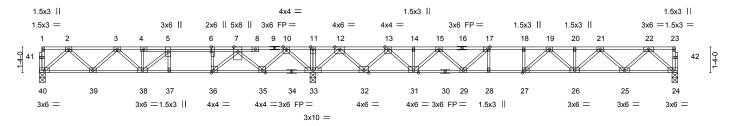
Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.









 	<u> </u>		32-11-0 18-9-12						
Plate Offsets (X,	Y) [6:0-3-0,0-0-0], [17:0-1-8,Edge], [27:0-1	-8,Edge], [36:0-1-8,Edge]							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.63 BC 0.82 WB 0.63 Matrix-S	Vert(LL) -0.2	27 26-27 >820 360	PLATES MT20 Weight: 180 lb	GRIP 244/190 FT = 20%F, 11%E			
BOT CHORD 2	TOP CHORD2x4 SP No.1(flat)TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.BOT CHORD2x4 SP No.1(flat)BOT CHORDBOT CHORDRigid ceiling directly applied or 6-0-0 oc bracing.WEBS2x4 SP No.3(flat)BOT CHORDRigid ceiling directly applied or 6-0-0 oc bracing.								
	REACTIONS. (size) 40=0-3-8, 24=0-3-8, 33=0-3-8 Max Grav 40=662(LC 3), 24=882(LC 4), 33=2190(LC 1)								
TOP CHORD	Max. Comp./Max. Ten All forces 250 (lb) of 2-3=-1110/36, 3-4=-1659/212, 4-5=-1659/21 7-10=-392/1404, 10-11=0/2571, 11-2=0/257 14-15=-1631/0, 15-17=-2564/0, 17-18=-2931 20-21=-2592/0, 21-22=-1586/0	9, 5-6=-1602/621, 6-7=-160 71, 12-13=-40/475, 13-14=-	-1631/0,						
BOT CHORD	39-40=0/704, 38-39=-106/1489, 37-38=-621/ 33-35=-1709/0, 32-33=-1291/0, 31-32=-200/ 27-28=0/2931, 26-27=0/2856, 25-26=0/2193	935, 29-31=0/2220, 28-29=	,						
WEBS 2-40=-935/0, 2-39=-59/564, 3-39=-528/98, 4-38=-325/0, 5-38=0/648, 22-24=-1266/0, 22-25=0/881, 21-25=-845/0, 21-26=0/542, 19-26=-358/0, 19-27=-253/331, 12-33=-1704/0, 12-32=0/1314, 10-33=-1399/0, 10-35=0/993, 7-35=-1033/0, 7-36=0/1214, 6-36=-690/0, 13-32=-1288/0, 13-31=0/991, 15-31=-835/0, 15-29=0/579, 17-29=-718/0									
NOTES-	oor live leade have been considered for this d								

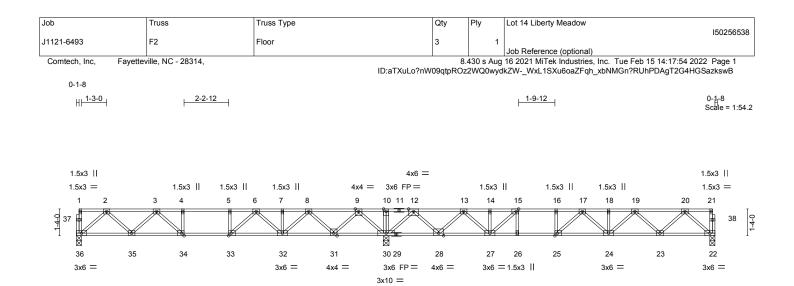
1) Unbalanced floor live loads have been considered for this design.

- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.







L	15-5-12		31-11-0					
	15-5-12 X) [45:0 4 9 5daal (25:0 4 9 5daal (22:0	4 0 54-01 124-0 4 0 54-0	1			16-5-4		1
Plate Offsets (X,	Y) [15:0-1-8,Edge], [25:0-1-8,Edge], [33:0-	1-8,Eagej, [34:0-1-8,Eage					1	
LOADING (psf) TCLL 40.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00	CSI. TC 0.85 BC 0.91	DEFL. Vert(LL) Vert(CT)	in (loc) -0.18 24-25 -0.25 24-25	l/defl >999 >776	L/d 480 360	PLATES MT20	GRIP 244/190
BCLL 0.0 BCDL 5.0	Rep Stress Incr YES	WB 0.54 Matrix-S	Horz(CT)	0.04 22	n/a	n/a	Weight: 167 lb	FT = 20%F, 11%E
	2x4 SP No.1(flat) 2x4 SP No.1(flat)			BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.				
	2x4 SP No.3(flat)		BOT CHOR				or 6-0-0 oc bracing.	
	(size) 36=0-3-0, 30=0-3-8, 22=0-3-0 Max Grav 36=719(LC 3), 30=2079(LC 1), 22= - Max. Comp./Max. Ten All forces 250 (lb) or 2-3=-1229/0, 3-4=-1941/0, 4-5=-1941/0, 5-6= 8-9=-241/659, 9-10=0/2218, 10-12=0/2218, 1 14-15=-1712/332, 15-16=-2291/0, 16-17=-22 19-20=-1395/0	ess except when shown. 1941/0, 6-7=-1408/202, 7 2-13=-523/817, 13-14=-1	7-8=-1408/202, 712/332,					
BOT CHORD	35-36=0/769, 34-35=0/1668, 33-34=0/1941, 30-31=-1214/0, 28-30=-1129/0, 27-28=-547/ 24-25=0/2385, 23-24=0/1918, 22-23=0/852							
WEBS								
,	loor live loads have been considered for this de	esign.						

2) All plates are 3x4 MT20 unless otherwise indicated.

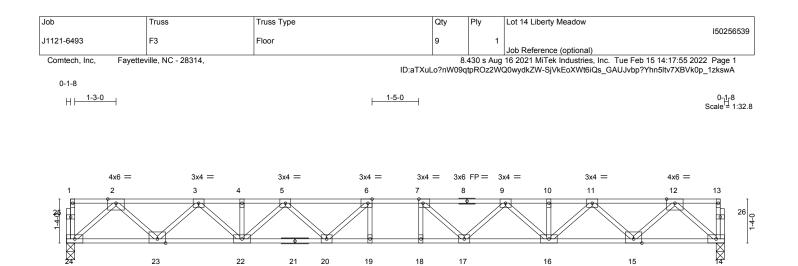
3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.







3x4 =

3x6 =

4x6 =

3x6 =

Plate Offsets (X,Y)	[6:0-1-8,Edge], [7:0-1-8,Edge]		19-11-0 19-11-0				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.95 WB 0.55 Matrix-S	Vert(LL) -0.33	n (loc) I/defl L/d 3 18-19 >724 480 5 18-19 >526 360 3 14 n/a n/a	PLATES MT20 M18AHS Weight: 106 lb	GRIP 244/190 186/179 FT = 20%F, 11%E	
BOT CHORD 2x4 SF	2 No.1(flat) 2 No.1(flat) 2 No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 5-9-6 oc purlins, except end verticals. Rigid ceiling directly applied or 2-2-0 oc bracing.				
	e) 24=0-3-0, 14=0-3-0 rav 24=1075(LC 1), 14=1075(LC 1) Comp./Max. Ten All forces 250 (lb) or	less except when shown.					

 TOP CHORD
 2-3=-2005/0, 3-4=-3408/0, 4-5=-3408/0, 5-6=-4160/0, 6-7=-4391/0, 7-9=-4160/0, 9-10=-3408/0, 10-11=-3408/0, 11-12=-2005/0

 BOT CHORD
 23-24=0/1172, 22-23=0/2810, 20-22=0/3918, 19-20=0/4391, 18-19=0/4391, 17-18=0/4391, 16-17=0/3918, 15-16=0/2810, 14-15=0/1172

 WEBS
 2-24=-1557/0, 2-23=0/1160, 3-23=-1119/0, 3-22=0/814, 5-22=-693/0, 5-20=0/469, 6-20=-575/87, 12-14=-1557/0, 12-15=0/1160, 11-15=-1119/0, 11-16=0/814, 9-16=-693/0, 9-17=0/469, 7-17=-575/87

NOTES-

3x6 =

4x6 =

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 1.5x3 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

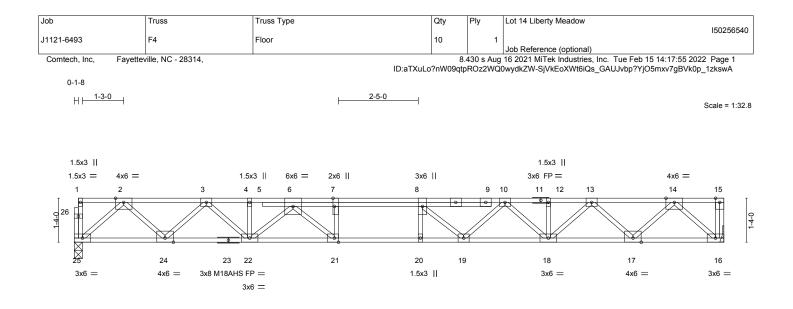
3x6 = 3x10 M18AHS FP =

3x4 =

Strongbacks to be attached to walls at their outer ends or restrained by other means.







			19-8-0					
Plate Offsets (X, Y) [7:0-3-0,Edge], [21:0-1-8,Edge]		19-8-0					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.39 BC 0.88 WB 0.54 Matrix-S	DEFL. in Vert(LL) -0.29 Vert(CT) -0.40 Horz(CT) 0.08	20 >806 20 >587	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 112 lb	GRIP 244/190 186/179 FT = 20%F, 11%E	
BOT CHORD 23 WEBS 23 REACTIONS.	TOP CHORD2x4 SP No.1(flat)TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.WEBS2x4 SP No.3(flat)BOT CHORDRigid ceiling directly applied or 10-0-0 oc bracing.							
TOP CHORD BOT CHORD WEBS	Max. Comp./Max. Ten All forces 250 (lb) or 2-3=-1976/0, 3-4=-3346/0, 4-6=-3350/0, 6-7= 10-12=-3354/0, 12-13=-3354/0, 13-14=-1976 24-25=0/1156, 22-24=0/2765, 21-22=0/3951 17-18=0/2764, 16-17=0/1157 2-25=-1537/0, 2-24=0/1140, 3-24=-1097/0, 3 7-21=-557/0, 14-16=-1541/0, 14-17=0/1139, 10-19=0/581, 8-19=-621/0	-4437/0, 7-8=-4437/0, 8- /0 , 20-21=0/4437, 19-20=0/ -22=0/790, 6-22=-803/0,	10=-4140/0, 4437, 18-19=0/3823, 6-21=0/970,					

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x4 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Refer to girder(s) for truss to truss connections.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.





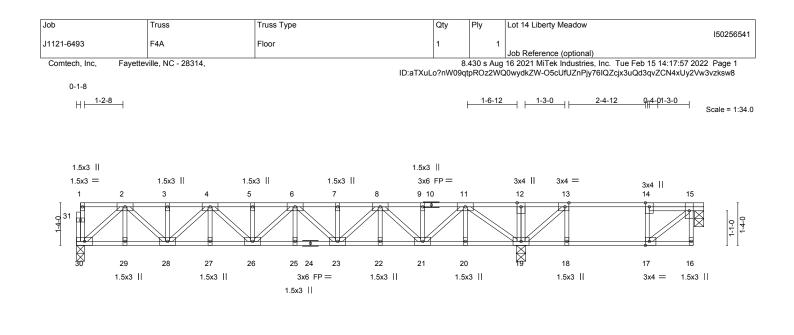


Plate Offsets (X,Y)	[13:0-1-8,Edge], [17:0-1-8,Edge]	13-11-4 13-11-4				6-7-10 17-10-0 1-2-6 1-2-6	19-4-0 19-8 ₁ 0 1-6-0 0-4-0
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.40 BC 0.38 WB 0.37 Matrix-S	DEFL. in Vert(LL) -0.08 Vert(CT) -0.11 Horz(CT) 0.03	25 >999 25 >999	480 360	PLATES MT20 Weight: 115	GRIP 244/190 Ib FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF REACTIONS. (siz Max C	BRACING- TOP CHORD BOT CHORD	except end ve Rigid ceiling d	rticals.	rectly applied or 6-0- or 10-0-0 oc bracing 8.			

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-1332/0, 3-4=-1332/0, 4-5=-1971/0, 5-6=-1971/0, 6-7=-1964/0, 7-8=-1964/0, 8-9=-1310/0, 9-11=-1310/0, 11-12=0/359, 12-13=0/359 BOT CHORD 29-30=0/776, 28-29=0/776, 27-28=0/1734, 26-27=0/1734, 25-26=0/2050, 23-25=0/2050, $22\text{-}23\text{=}0/1721,\,21\text{-}22\text{=}0/1721,\,20\text{-}21\text{=}0/755,\,19\text{-}20\text{=}0/755$

15-17=-104/317, 2-30=-1022/0, 2-28=0/751, 4-28=-542/0, 4-26=0/320, 8-23=0/344, 8-21=-568/0, 11-21=0/769, 11-19=-1228/0, 13-19=-573/0 WEBS

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x6 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

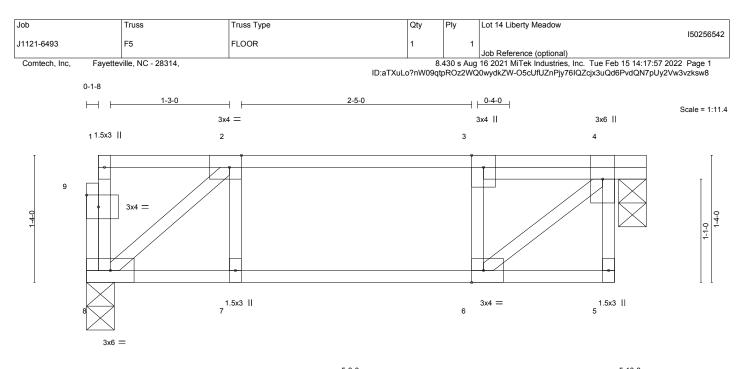
5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

6) CAUTION, Do not erect truss backwards.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTTeKe connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		5-6-6	-		5-10-	
Plate Offsets (X,Y)	[2:0-1-8,Edge], [6:0-1-8,Edge], [9:0-1-8	,0-1-8]	8		0-4-0	0
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.11 WB 0.18 Matrix-S	DEFL. ir Vert(LL) -0.01 Vert(CT) -0.01 Horz(CT) -0.01	6 >999 480 6 >999 360	PLATES MT20 Weight: 32 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 5-10-8 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.		
REACTIONS. (size Max G	e) 8=0-3-8, 4=0-3-8 irav 8=288(LC 1), 4=294(LC 1)					

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-294/0, 3-4=-296/0

BOT CHORD 7-8=0/294, 6-7=0/294 WEBS 2-8=-382/0, 4-6=0/383

NOTES-

1) Unbalanced floor live loads have been considered for this design.
2) Plates checked for a plus or minus 1 degree rotation about its center.
3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

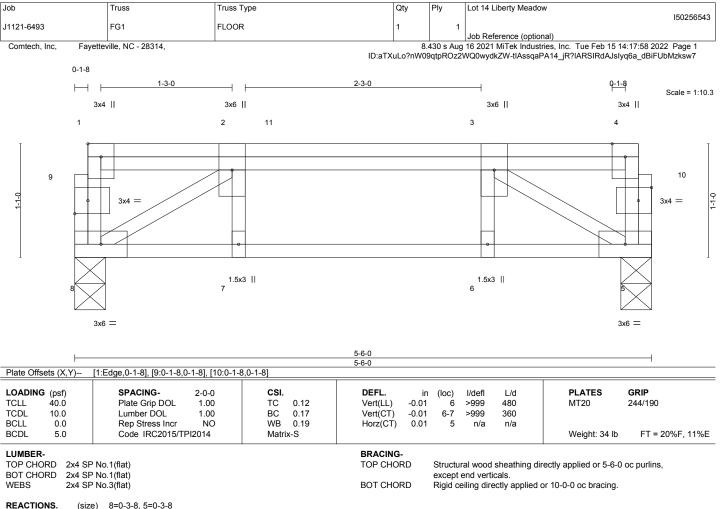
Strongbacks to be attached to walls at their outer ends or restrained by other means.

4) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

5) CAUTION, Do not erect truss backwards.







Max Grav 8=455(LC 1), 5=476(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-683/0

 BOT CHORD
 7-8=0/683, 6-7=0/683, 5-6=0/683

 WEBS
 2-8=-802/0, 3-5=-801/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb) Vert: 3=-172 11=-194





