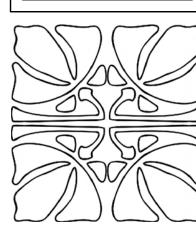
COMPTON RESIDENCE

CIDER HOUSE STUDIO, INC. 424 E. MAIN ST. CLAYTON, NC 27520 919, 624, 4776

THOMAS & ANDREA COMPTON

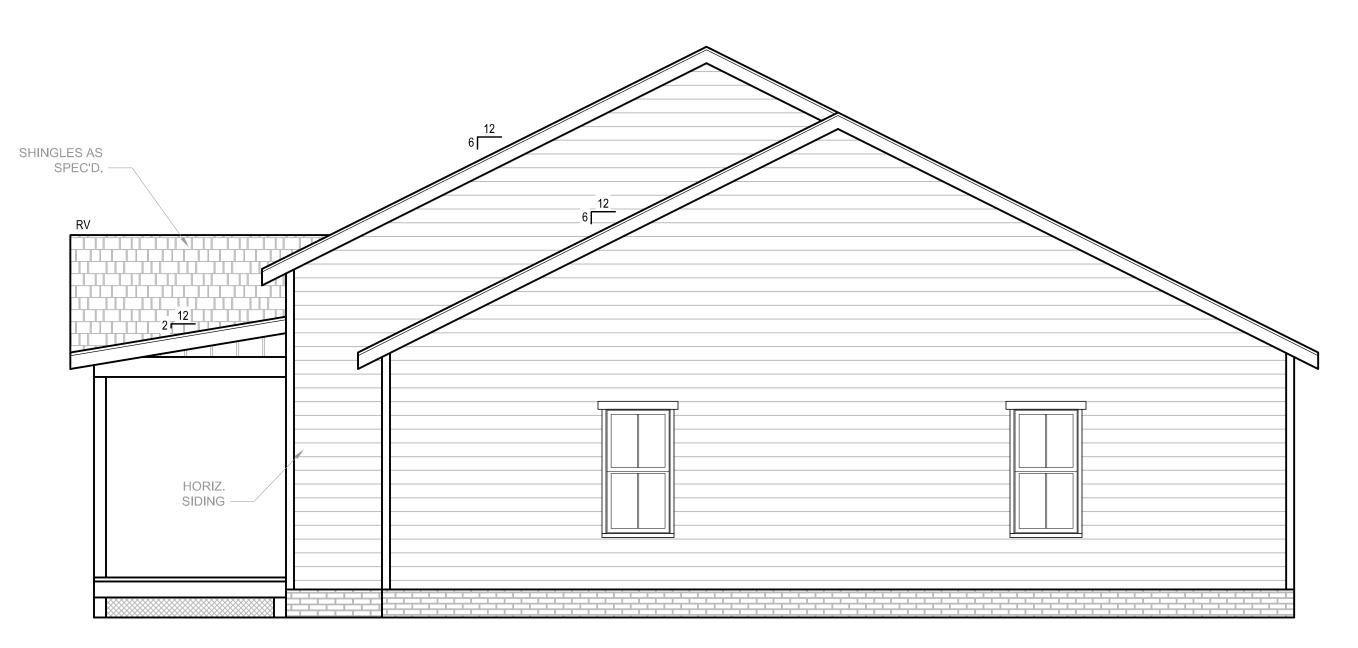




FRONT ELEVATION

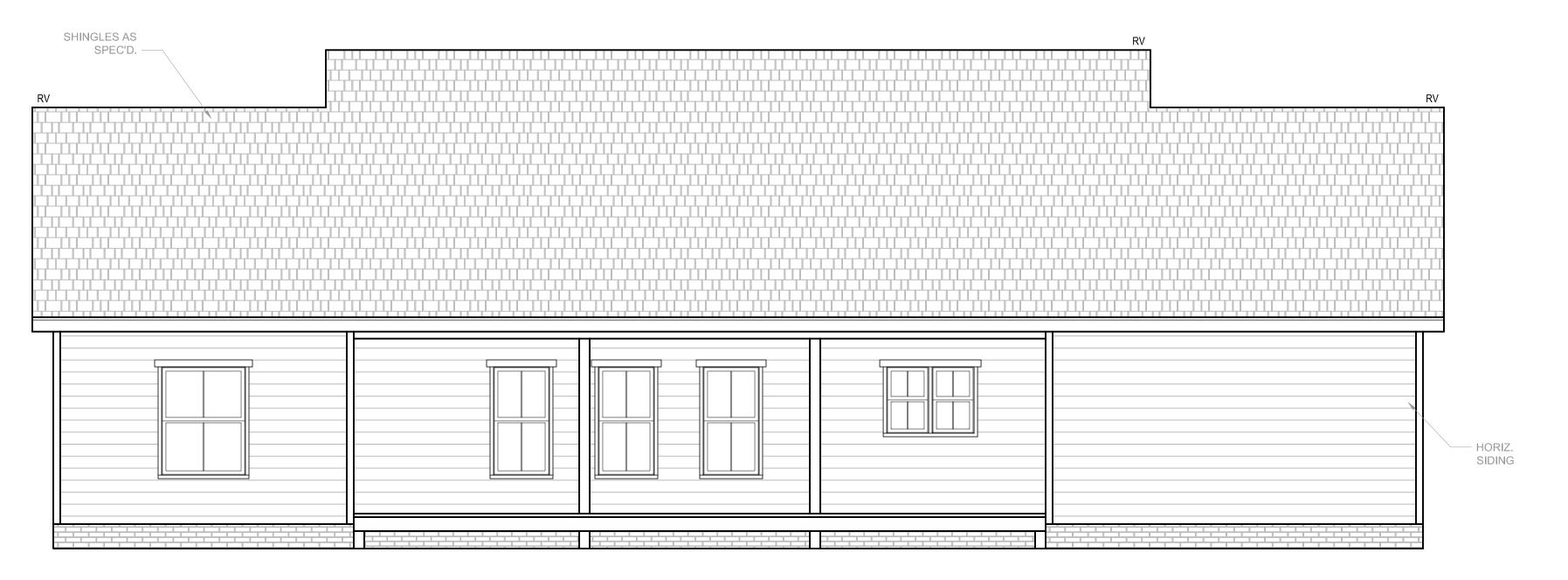
1/4" = 1'-0"

GRADE



1/4" = 1'-0"

RIGHT ELEVATION



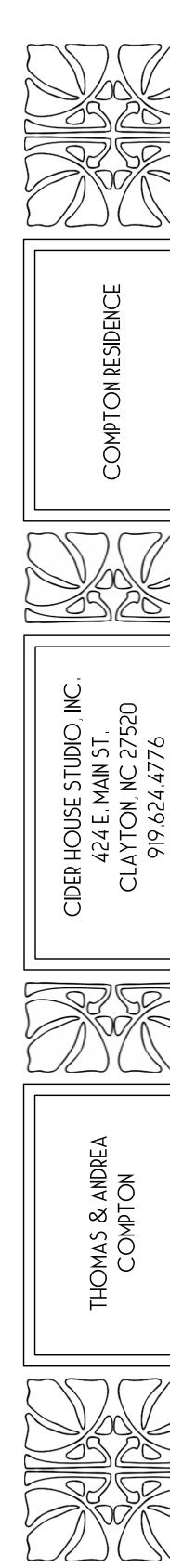
REAR ELEVATION

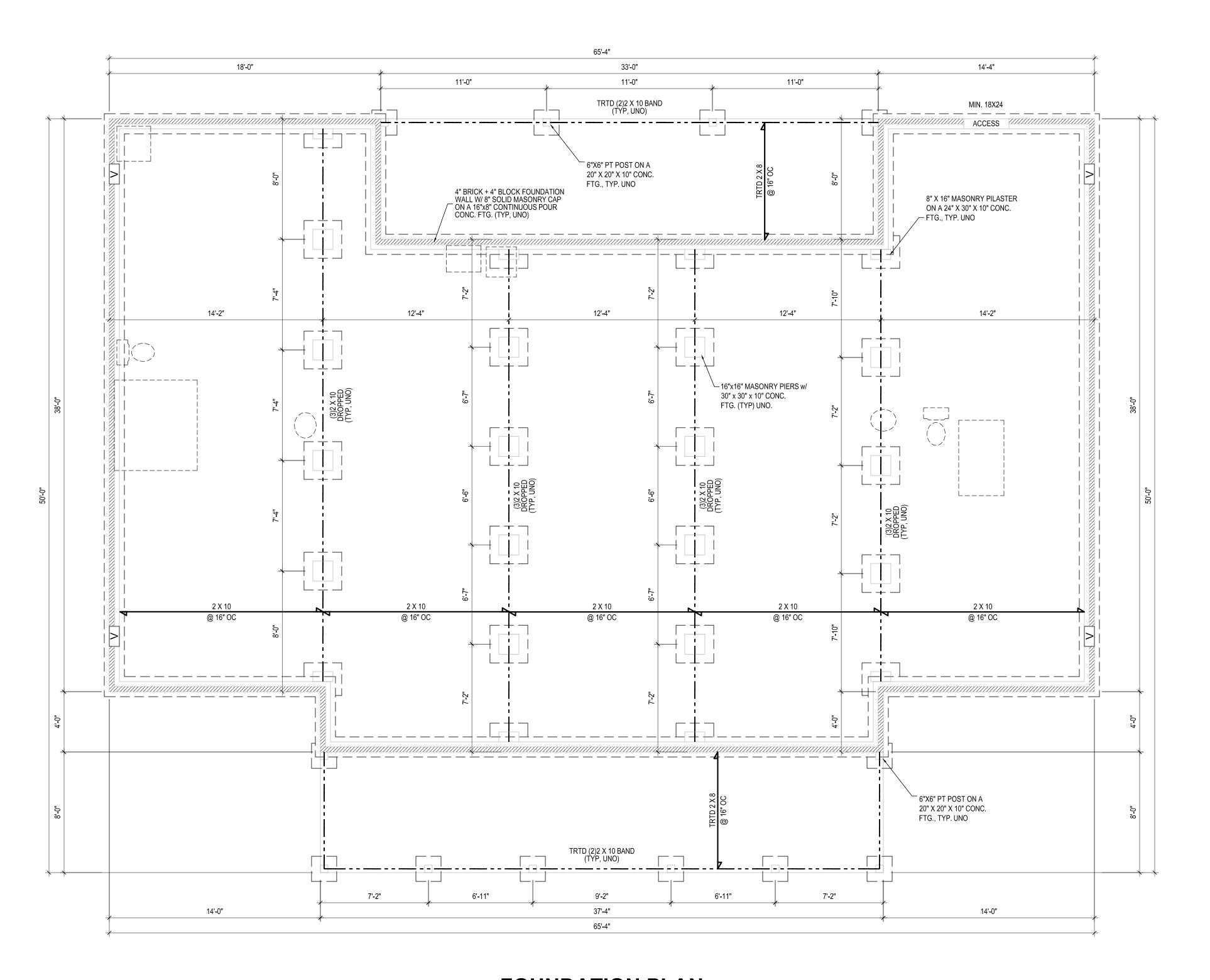
1/4" = 1'-0"



LEFT ELEVATION

1/4" = 1'-0"

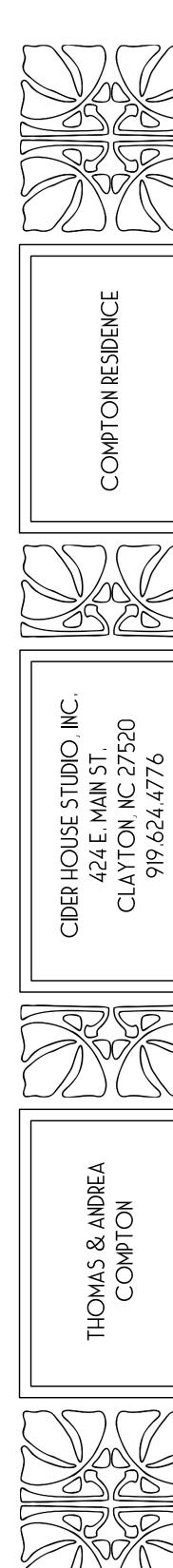


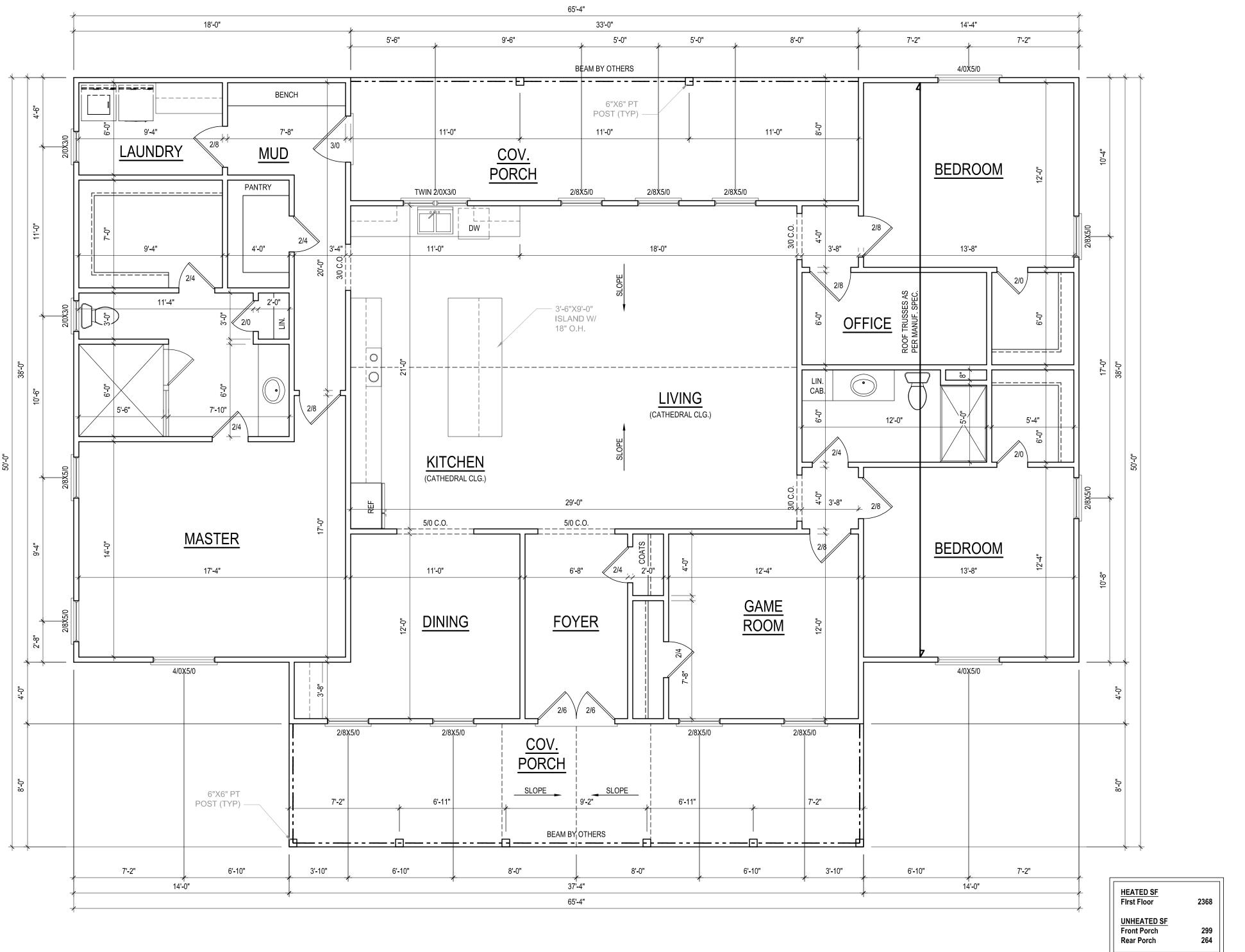




1/4" = 1'-0"

*ALL LUMBER TO BE #2 SYP, UNO





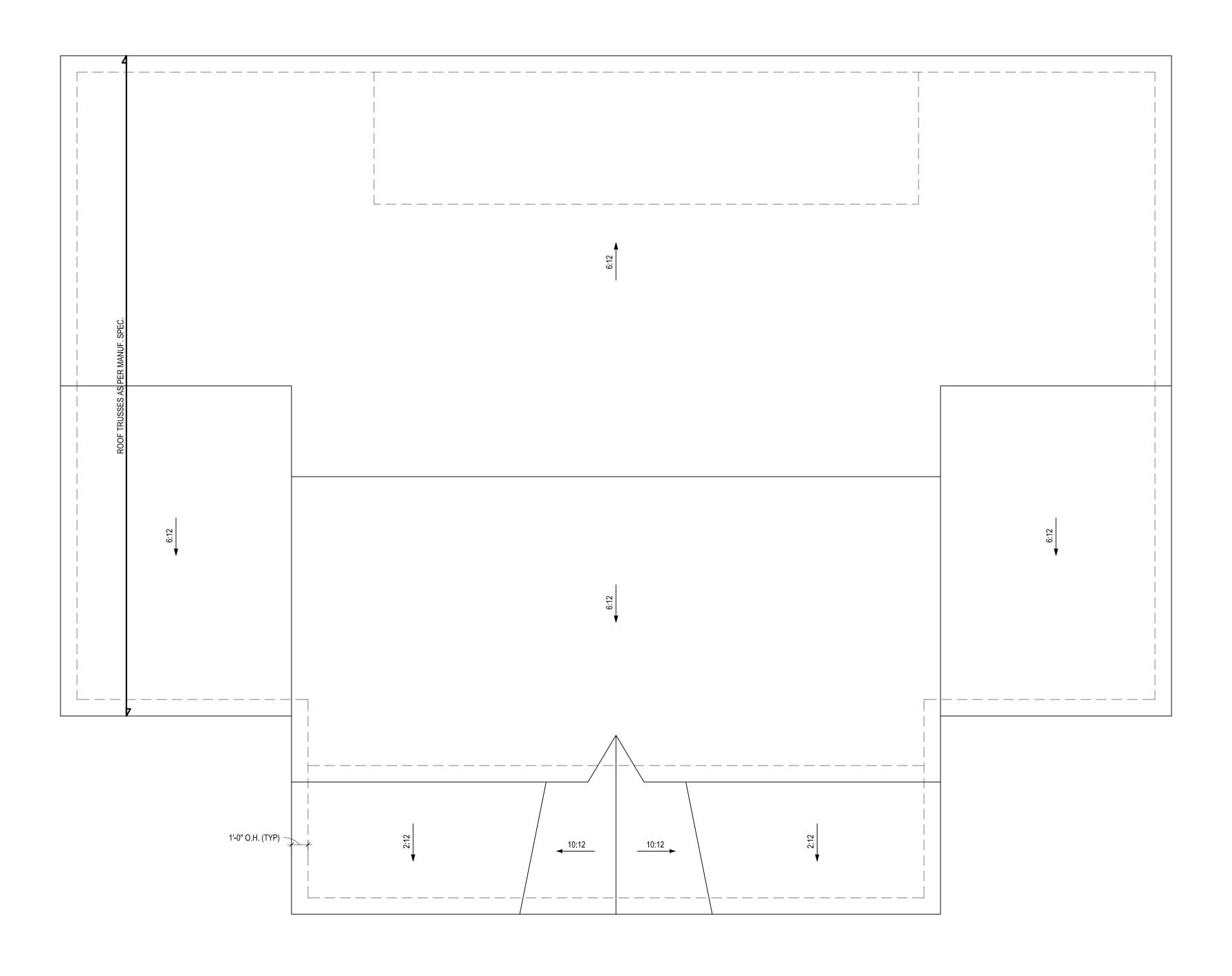
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FIRST FLOOR PLAN

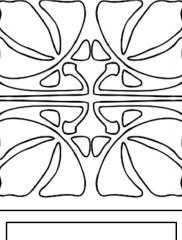
1/4" = 1'-0" CEILING HGT. = 8'-0"



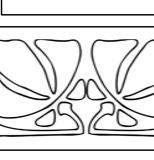


1/4" = 1'-0"

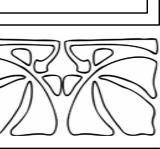
*ALL LUMBER TO BE #2 SYP, UNO BUILDER MAY USE ROOF TRUSSES. TRUSS DESIGN, LAYOUT, AND ENGINEERING TO BE PROVIDED BY TRUSS MANUFACTURER



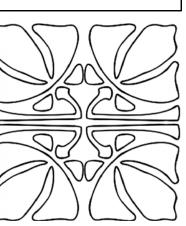
COMPTON RESIDENCE



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STRUCTURAL NOTES

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

DESIGN LOADS;

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION		
	(101)	(101)	LL	TL	
ALL FLOORS	40	10	L/360	L/240	
ATTIC (w/ walk up stairs)	30	10	L/360	L/240	
ATTIC (pull down access)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC		SEISMIC ZON	IES A, B & C		

- 3) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- 4) CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE, (U.N.O.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4°.0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.

- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PS), BASED ON 2x10) UNO.
 ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
 ALL LYL LUMBER TO BE 1.75° WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2260 PSI, E = 1.9M PSI (U.N.O.)
 ALL LSL LUMBER TO BE 3.5° WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 235° PSI, E = 1.6M PSI (U.N.O.)
 ALL PSI LUMBER TO BE 3.5° WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-112" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION, BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (12"2" x 4" LONG), LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.8: 1/2"/9 ANCHOR BOLTS SPACED AT 6-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION, ANCHOR BOLTS SHALL BE SPACED AT 3"0" O.C. FOR BASEMENTS, ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASORNY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- 11) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.
- WALL AND ROOF CLADDING VALUES:
 WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.
 ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:
 39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12
 8.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12
 18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12
 **MEAN ROOF HEIGHT 30'-0" OR LESS
- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- 14) REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 15) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 18) PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- 19) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 20) MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 21) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION.
 TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

CLIMATE ZONES	FENESTRATION U-FACTOR	SKYLIGHT b U-FACTOR	GLAZED FENESTRATION SHGC ^{b,k}	CEILING [™] R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE ⁱ	FLOOR R-VALUE	BASEMENT ^{C,Q} WALL R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE ^C WALL R-VALUE
3	0.35	0.55	0.30	38 or 30 cont	15 or 13 + 2.5 h	5/13 or 5/10 cont	19	<u>5/13</u> f	0	5/13
4	0,35	0,55	0.30	38 or 30 cont j	15 or 13 + <u>2.5</u> h	5/13 or 5/10 cont	19	10/15	10	10/15
5	0.35	0.55	NR	38 or 30 cont j	n 19, or 13 + 5 or 15 + 3	13/17 <u>or</u> 13/12.5 cont	30 ^g	10/15	10	10/19

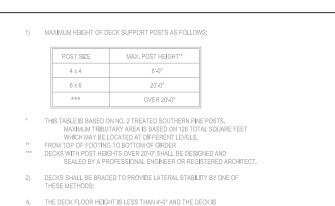
- OR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF; THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE. ISED AND INSTALLED IN A NOMINAL 2 × 6 FRAMING CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED.
- \cdots SQ. FT. OF CRAWL SPACE / 150 = \cdots SQ. FT. OF REQ'D VENTILATION WITHOUT CROSS VENTILATION \cdots SQ. FT. OF VENTILATION REQ'D / 0.45 SQ.FT. PER VENT = VENTS REQ'D1
- -- SQ, FT. OF CRAWL SPACE / 1500 = SQ, FT. OF REQ'D VENTILATION WITH CROSS VENTILATION -- SQ, FT. OF VENTILATION REQ'D / 0.45 SQ, FT. PER VENT = VENTS REQ'D2
- VENT LOCATIONS MAY VARY FROM THOSE SHOWN ON PLAN, HOWEVER VENTS SHALL BE PLACED TO PROVIDE ADEQUATE VENTILATION AT ALL POINTS AND TO PREVENT DEAD AIR POCKETS.



-- SQ. FT. OF ATTIC / 300 = -- SQ. FT. INLETS/OUTLETS REQUIRED



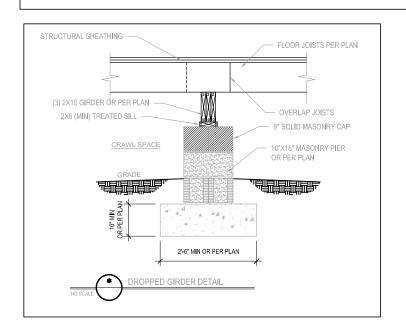


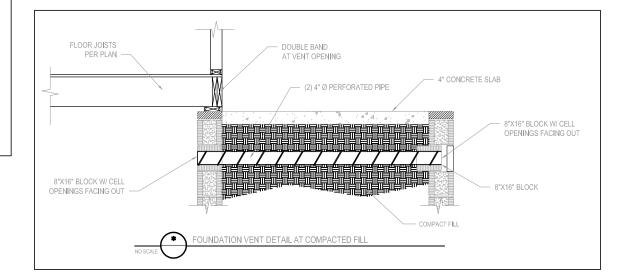


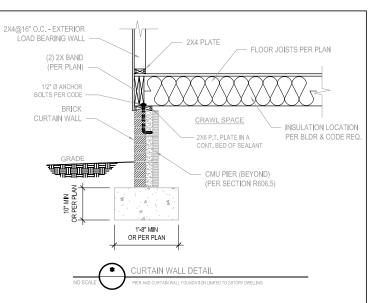
THE DECK FLOOR HEIGHT IS LESS THAN 4"0" AND THE DECK IS
ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4)
ABOVE: LATERAL BRACING IS NOT REQUIRED.
4 x 4 WOOD KINEE BRACES MAY BE PROVIDED ON EACH COLUMN IN
BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST
AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE
TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN
45" AND 60" FROM THE HORIZONTAL, KNEE BRACES SHALL BE BOLTED
TO THE POST AND GIRDER WITH ONE 5/8" OH OT DIPPED GALVANIZED
BOLT AT EACH END OF THE BRACE.
FOR FREESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL
BRACING, LATERAL STABLITY MAY BE PROVIDED BY EMBEDDING THE
POSTS IN ACCORDANCE WITH THE FOLLOWING:

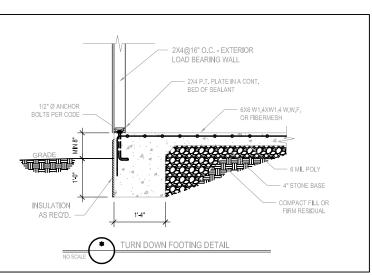
ST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

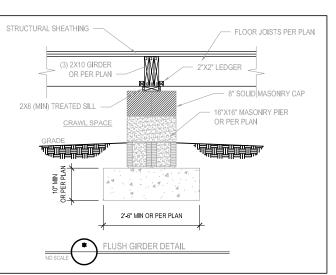
D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO (2) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2 x 6 S HOLD BE ATTACHED TO THE POSTS WITH ONE SIGN HOT DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER. E. FOR EMBEDMENT OF PILES IN COASTAL REGIONS. SEE CHAPTER 46.

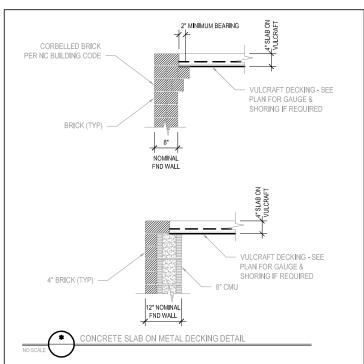


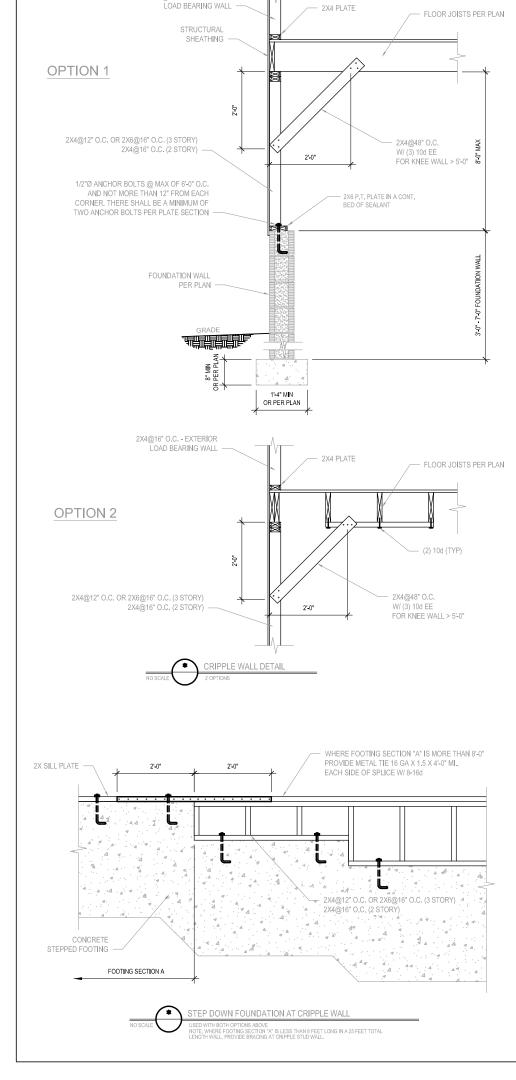




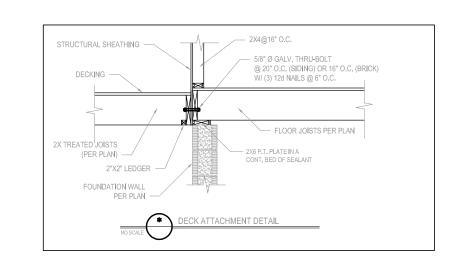








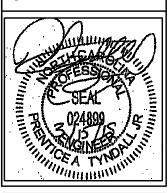
2X4@16" O.C. - EXTERIOR



neans, methods, techniques, sequences, procedures or safety precaution. Any deviations or discrepancies on plans are to be brought to the immediat pians are to be prought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. * Please review these documents carefully. Tyndall Engineering & Design, P.A. will

nterpret that all dimensions, recor etc. presented in these documents were

emed acceptable once construction





RESIDENCE

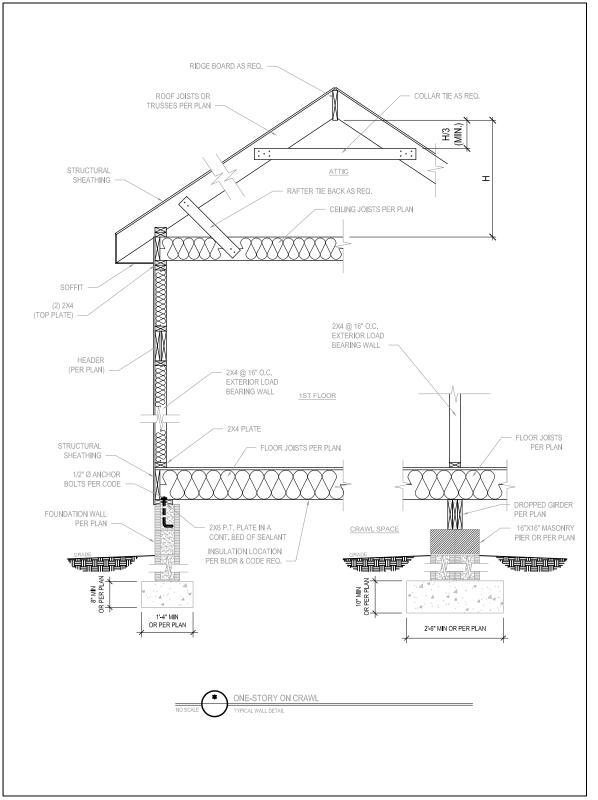
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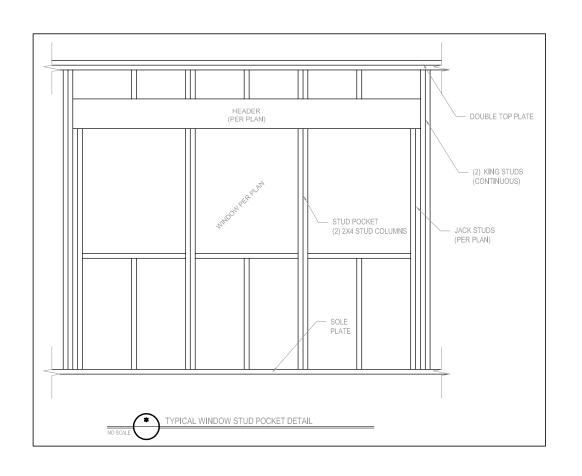
Project #: 03/15/19 Drawn/Design By: PTII DWG. Checked By: PTII

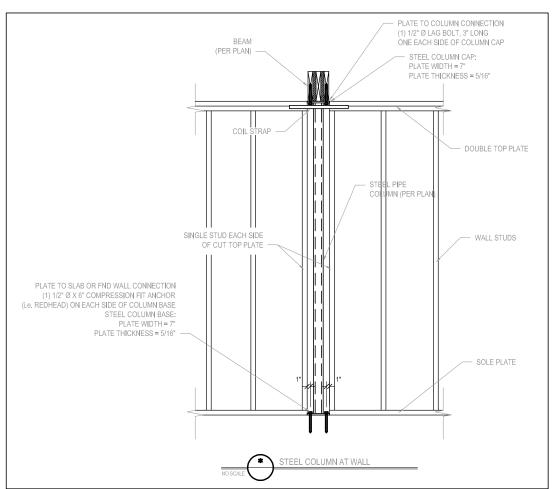
NOT TO SCALE REVISIONS Date: Remarks

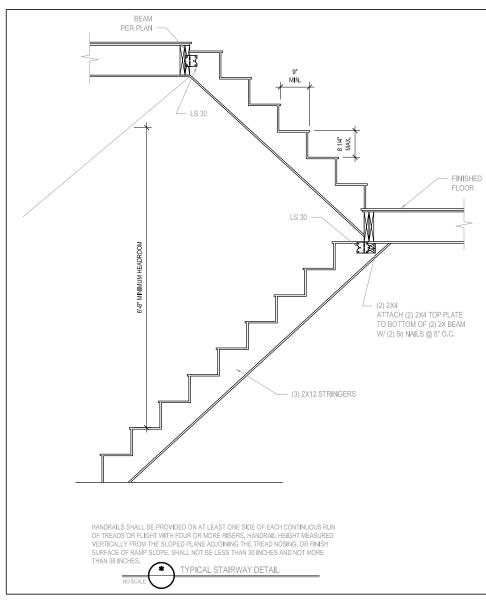
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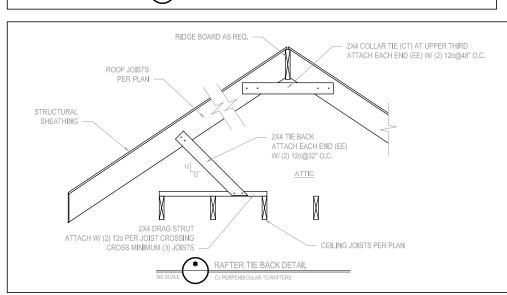
1 of 3

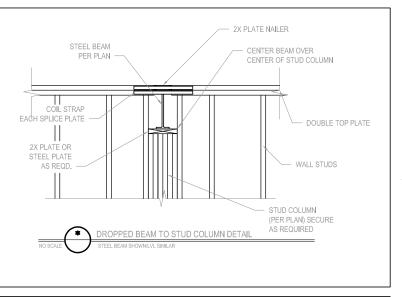


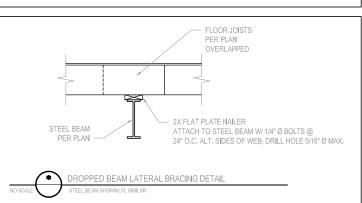


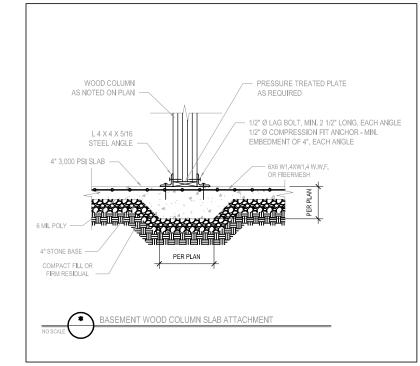


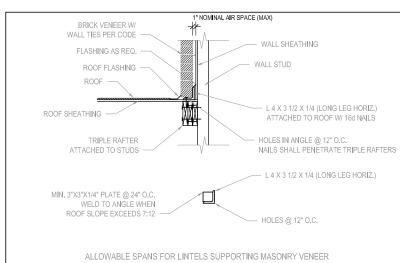










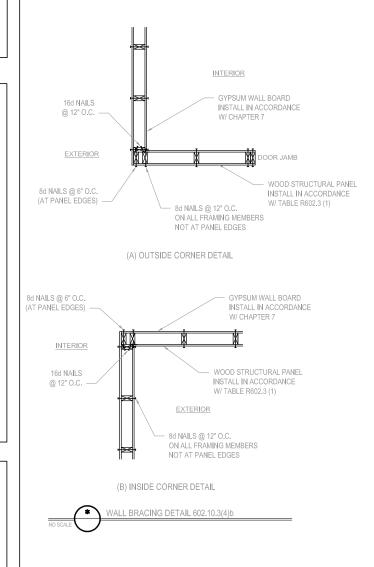


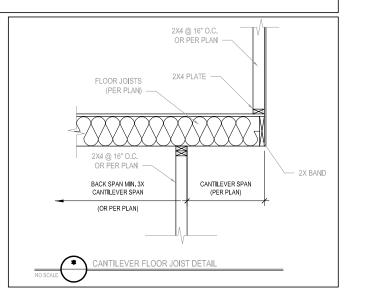
SIZE OF ANGLE (1,3)	NO STORY ABOVE (5)	1 STORY ABOVE (5)	2 STORIES ABOVE (5)	# OF ½" (OR EQUIV.) REINFORCING BARS IN REINFORCED LINTEL (2,4,5)
L3x3x1/4	6'-0"	4'-6"	3'-0"	1
L4 x 3 x 1/4	8'-0"	6'-0"	4'-6"	1
L 5 x 3½ x 5/16	10'-0"	8'-0"	6'-0"	2
L 6 x 3 ½ x 5/16	14'-0"	9'-6"	7'-0"	2
2L 5 x 3½ x 5/16	20'-0"	12'-0"	9'-6"	4

- 1. LONG LEG OF THE ANGLE SHALL BE PLACED IN A VERTICAL POSITION.
 2. DEPTH OF REINFORCED LINTELS SHALL NOT BE LESS THAN 8" AND ALL CELLS OF HOLLOW MASONRY LINTELS SHALL BE GROUTED. REINFORCING BARS SHALL EXTEND NOT LESS THAN 8" INTO THE SUPPORT.
 3. STEEL MEMBERS INDICATED ARE ADEQUATE TYPICAL EXAMPLES; OTHER STEEL MEMBERS MEETING STRUCTURAL DESIGN REQUIREMENTS SHALL BE PERMITTED TO BE USED.
 4. EITHER STEEL ANGLE OR REINFORCED LINTEL SHALL SPAN OPENING.
 5. SPANS OVER 4"-Q" SHALL BE SHORED UP UNTIL CURED.

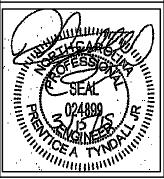
71	ANO OVER	17-0 0	11/1/1	ALL DE GRONED OF GRANE GONED.	
		(*	1	MASONRY VENEER SUPPORT FIG 703.8	8.3.1
	NO SCALE	$\overline{}$	Ţ		

SIMPSON STRONG-TIE	USP STRUCTURAL CONNECTORS			
PRODUCT NUMBER	PRODUCT NUMBER			
A35	MPA1			
ABE	PAE			
CBSQ	CBSQ			
CCQ	KCCQ			
CMSTC16	CMSTC16			
CS	RS			
H1	RT15			
H2.5A	RT7A			
H10	RT16			
HDQ8-SDS3	UPHD8			
HDU2-SDS2,5	PHD2			
HDU5-SDS2.5	PHD5			
HETA	HTA			
HGAM10KTA	HGAM			
HHDQ14-SDS2.5	UPHD14			
HTS	HTW			
НТТ	HTT			
HUS	HUS			
LTA1	LPTA			
LTHJA26	HJC26			
LTP4	MP4F			
LUS	JUS			
MAS	FA3			
MSTAM	MSTAM			
PC	PCM			
PHD-SDS3	PHD			
SSP	RSPT6			
STC	TR1			





ngineers seal does not include construction Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.
 Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A.
 Failure to do so will void Tyndall Engineering & Design, P.A. liability.
 Please review these documents carefully.
 Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed accentable once construction



TYNDALL ENGINEERING & DESIGN, P.A.



COMPTON RESIDENCE

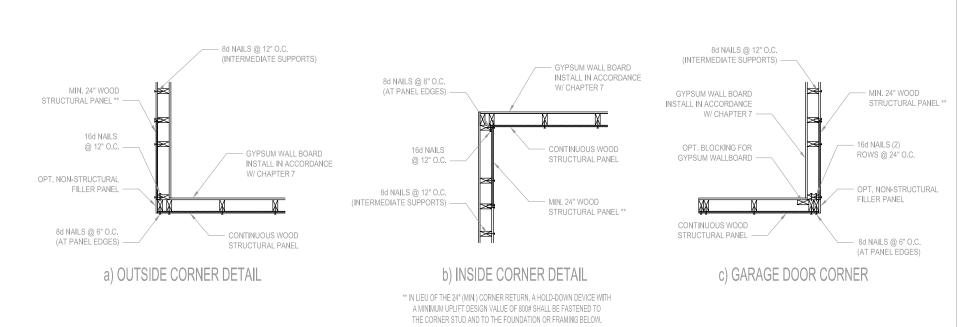
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Project #: 03/15/19 Drawn/Design By: PTII DWG. Checked By: PTII

NOT TO SCALE REVISIONS Date: Remarks

Sheet Number

2 of 3



B1: TYPICAL EXTERIOR CORNER FRAMING FOR CONTINUOUS SHEATHING NO SCALE

STRUCTURAL SHEATHING NOTES

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
 WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC
 BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNC
- 2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS
- 3) 3/8" WOOD STRUCTURAL PANEL)WSP) SECURE W/6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- AND 12" O.C. AT INTERMEDIATE SUPPORTS

 5. EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.103 (UNO)

 6. ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM HICKNESS OF 38"S. SHEATHING SHALL BE SECURED WITH MINIMUM 64 COMMON NAILS SPACED AT 6" O.C. AT FANILE EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.

 7. MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:

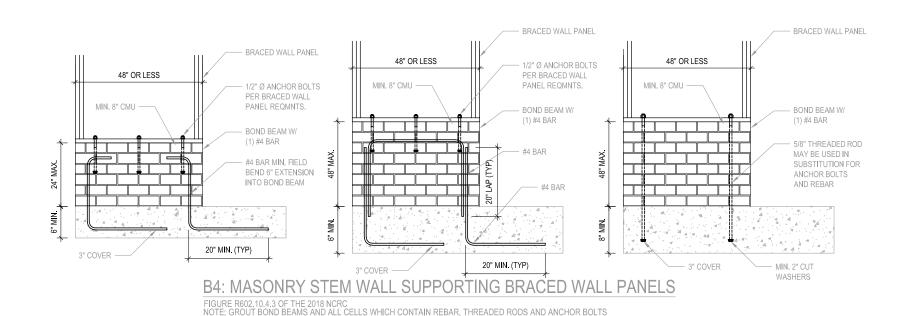
 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT

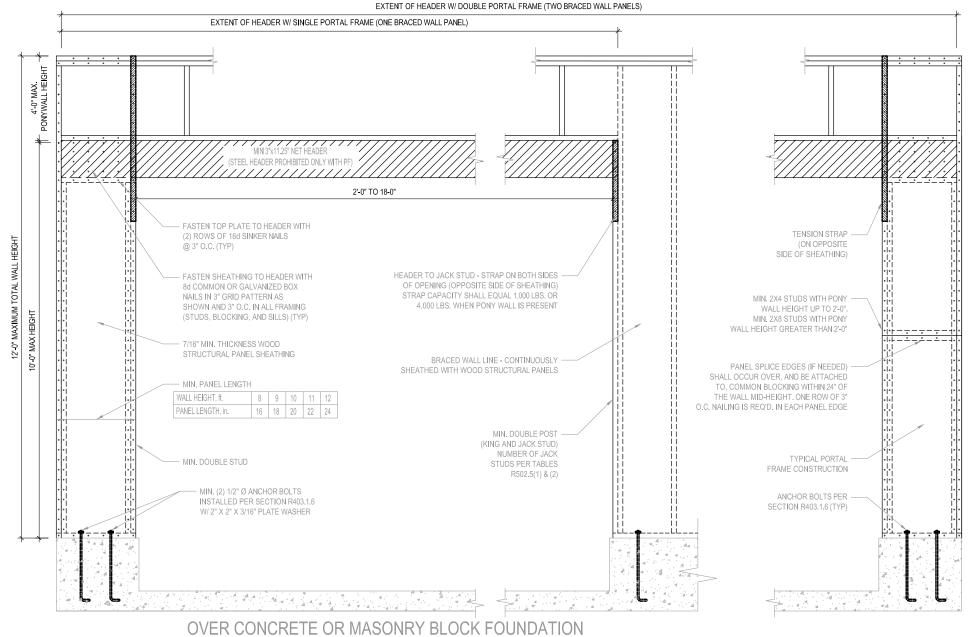
 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT

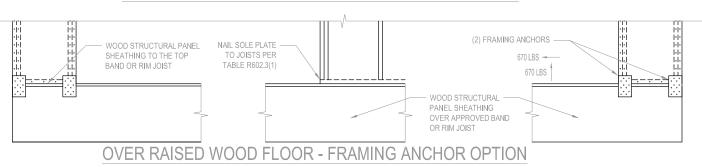
 49" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- 4 SHEATH INTERIOR AND EXTERIOR
- 8. FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602-10.3 (4). IN LIEU OF A CORNER RETURN, EITHER A MINIMUM 49" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNIER AND TO THE FOUNDATION OR FRAMING BELOW
- 5 MINIMUM 800# HOLD-DOWN DEVICE

		REQUIRED BRAC	ED WALL PANEL CONNECTIONS		
			REQUIRED CONNECTION		
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS	
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.	
GB	GYPSUM BOARD	1/2"	5d COOLER NAIL** @ 7" O.C.	5d COOLER NAIL** @ 7" O.C.	
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.	

OR EQUIVALENT PER TABLE R702.3.5 **B3: BRACE WALL PANEL CONNECTIONS NO SCALE





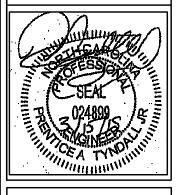


(WHEN PORTAL SHEATHING DOES NOT LAP OVER BAND OR RIM JOIST) ATTACH SHEATHING TO BAND NAIL SOLE PLATE - WOOD STRUCTURAL PANEL OR RIM JOIST WITH 8d COMMON TO JOISTS PER NAILS 3" O.C. TOP AND BOTTOM TABLE R602.3(1) BAND OR RIM JOIST - WOOD STRUCTURAL -OR RIM JOIST

OVER RAISED WOOD FLOOR - OVERLAP OPTION (WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)

B2: METHOD CS-PF: CONTINUOUSLY SHEATHED PORTAL FRAME

means, methods, techniques, sequences, procedures or safety precaution. Any deviations or discrepancies on * Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. * Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions recommendations. nterpret that all dimensions, recom etc. presented in these documents were





RESIDENCE

ATHING TAILS

 $\frac{Date.}{03/15/19}$ Drawn/Design By PTII DWG. Checked By

PTII NOT TO SCALE

REVISIONS

Sheet Number