

RICHARDSON RESIDENCE

PROJECT#
DRB2101-0274
DATE
03/03/2022
DRAWN/DESIGNED BY
DRB
CHECKED BY
MMB
SCALE
1/4" = 1'-0"

CLIENT NAME
PERSONAL RESIDENCE
PROJECT NAME
drbhomedesign.com

PERSONAL RESIDENCE

DRB DESIGN
drbdesign@drbhomedesign.com 919.631.5979
250 Shipwash Dr Suite 105 Garner, NC 27529

CLIENT NAME
Stephen and Leslie Richardson
760 Owens Way
Opelika, AL 36804
stephenr0784@gmail.com
919-820-0597

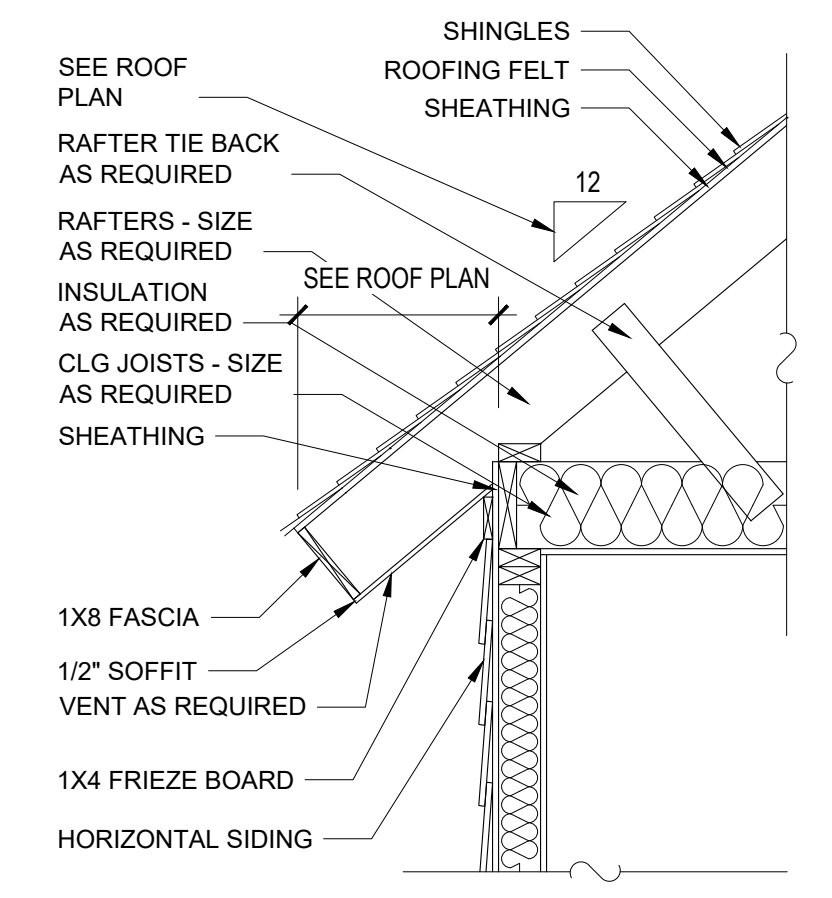
SHEET NAME
ELEVATIONS
SHEET #

A1
of 6

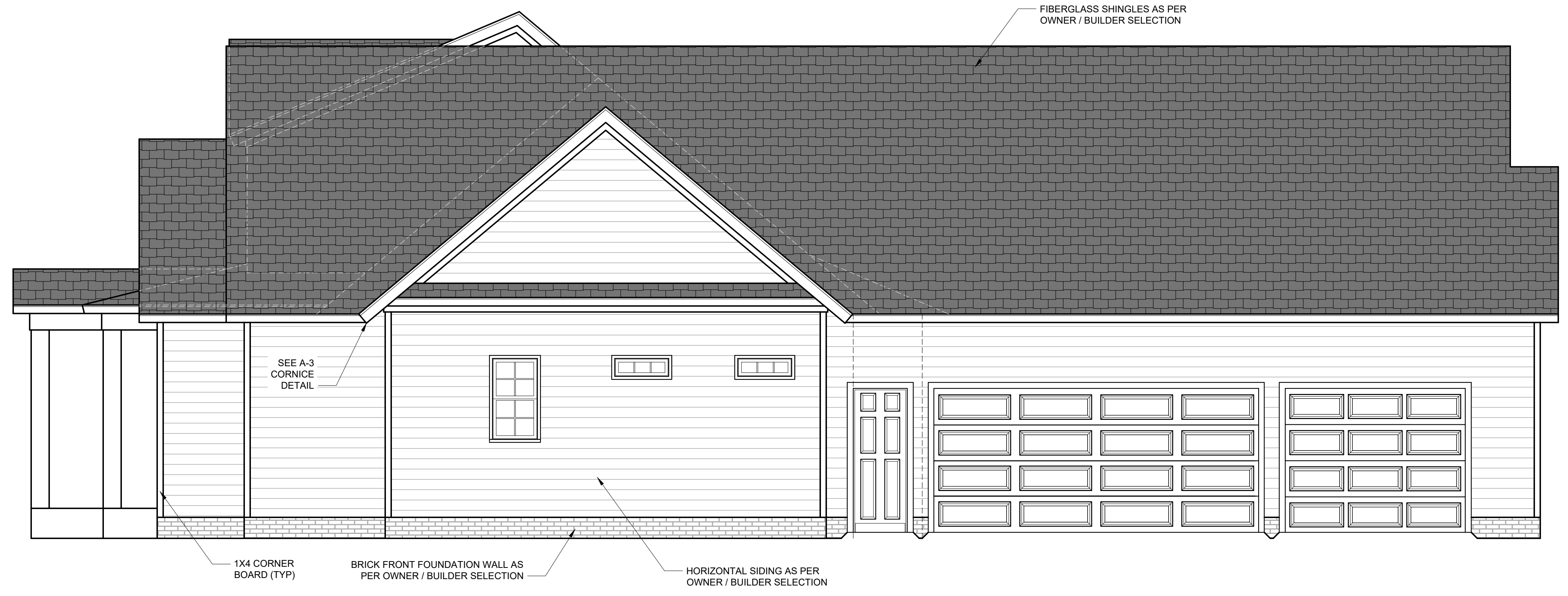


FRONT ELEVATION
1/4" = 1'-0"

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- Should these plans require structural calculations for permitting the contractor shall be required to obtain the services of a structural engineer after notifying DRB DESIGN that such services are required.
- Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN.
- Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.
- Communication is imperfect and every contingency cannot be anticipated.
- Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs.
- A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all responsibilities for all consequences.
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- Written dimensions on these plans always have precedence over scaled dimensions.
- It is the contractor's responsibility to verify and be responsible for all dimensions and square footage prior to construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square footage errors once construction has begun.
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A-3 CORNICE DETAIL
NTS



RIGHT ELEVATION
1/4" = 1'-0"

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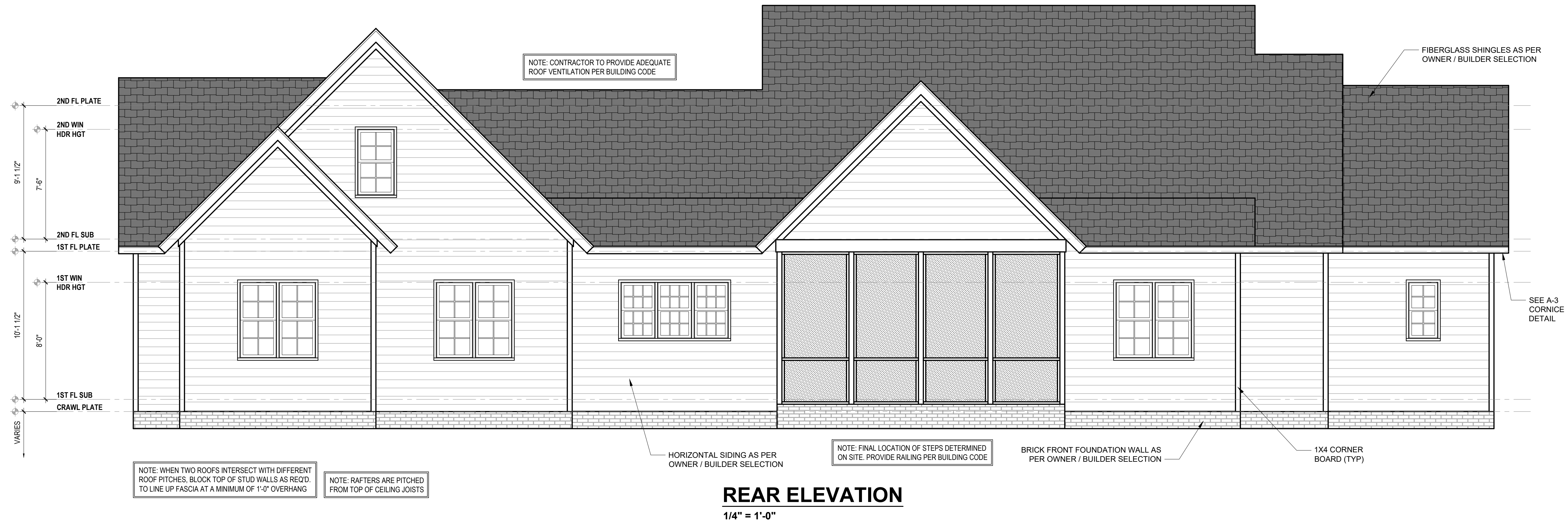
CLIENT NAME
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PROJECT NAME
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PERSONAL RESIDENCE

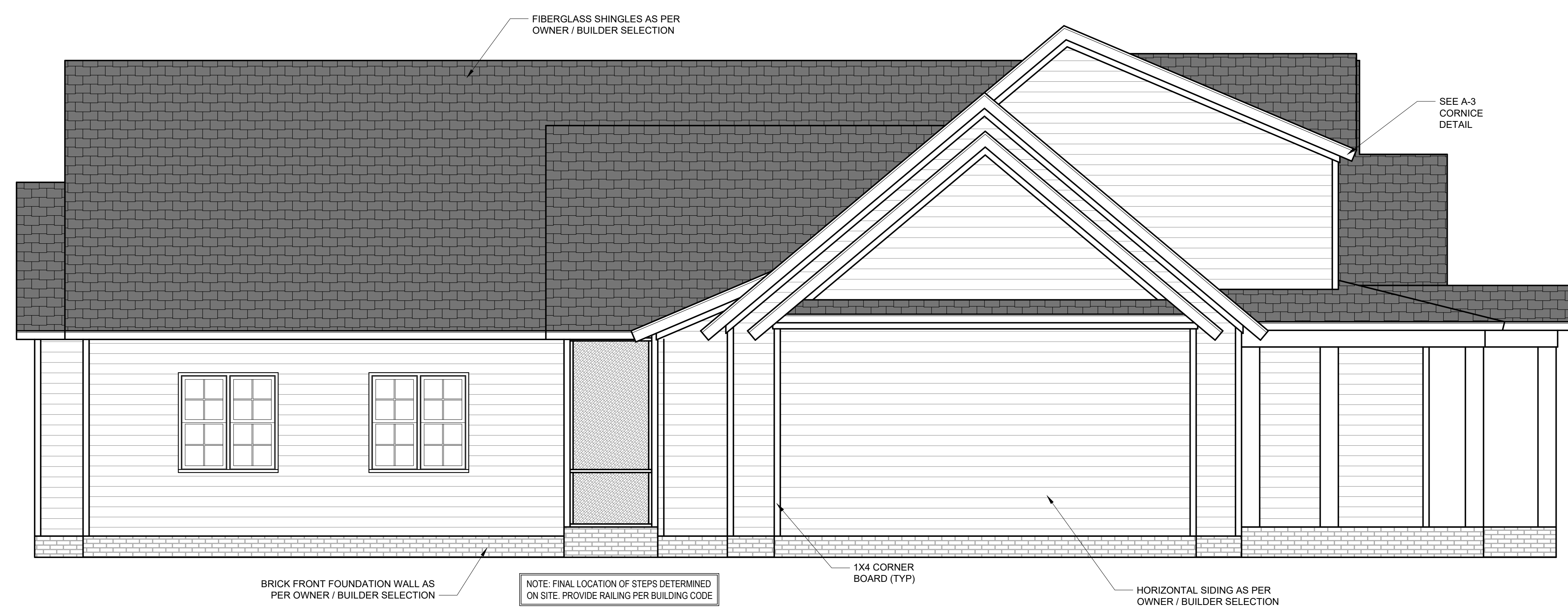
DRB DESIGN
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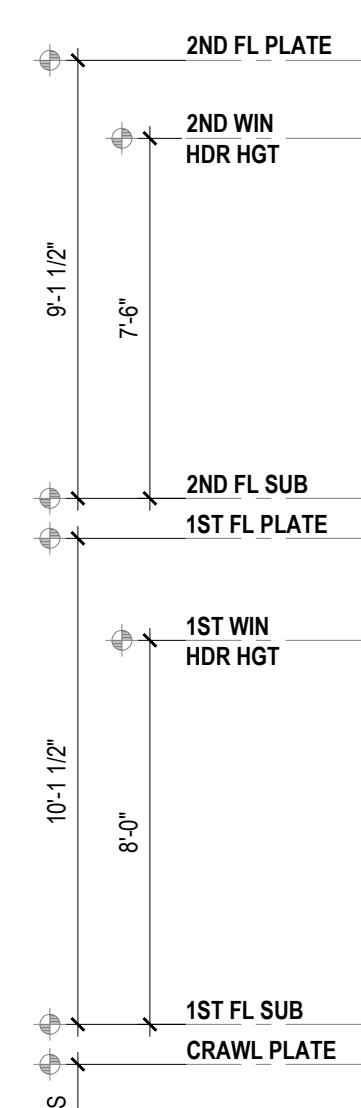
SHEET NAME
ELEVATIONS
SHEET #
A2
of 6



REAR ELEVATION
1/4" = 1'-0"



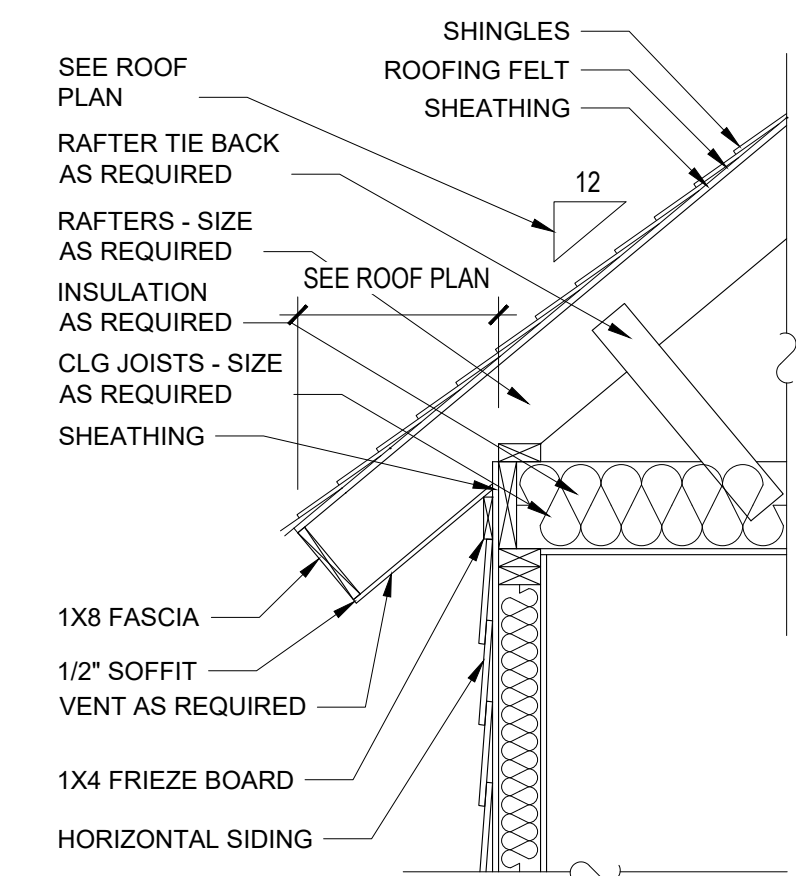
LEFT ELEVATION
1/4" = 1'-0"



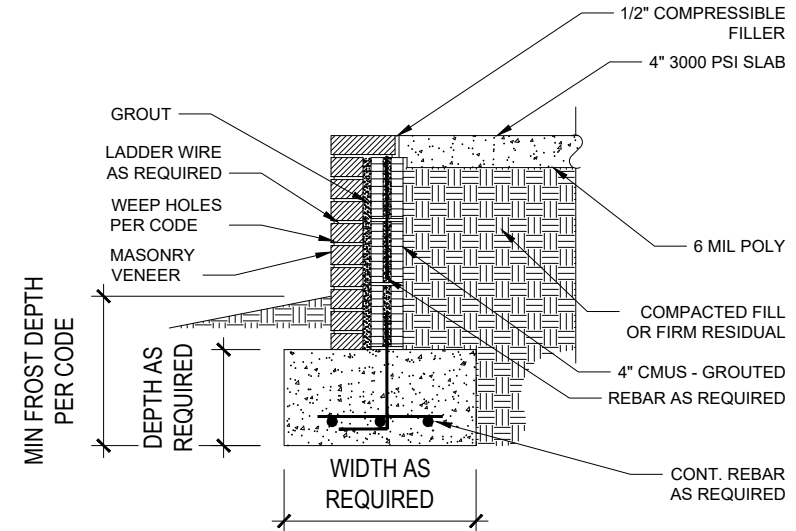
NOTE: WHEN TWO ROOFS INTERSECT WITH DIFFERENT ROOF PITCHES, BLOCK TOP OF STUD WALLS AS REQ'D. TO LINE UP FASCIA AT A MINIMUM OF 1'-0" OVERHANG

NOTE: RAFTERS ARE PITCHED FROM TOP OF CEILING JOISTS

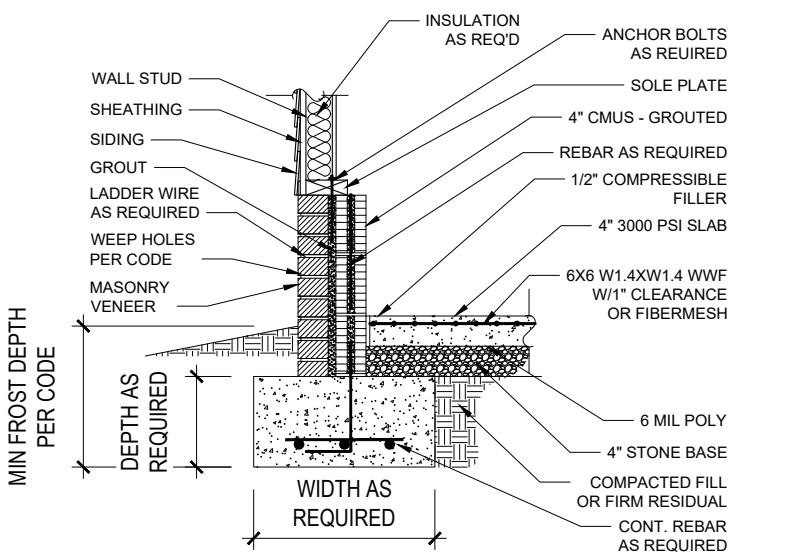
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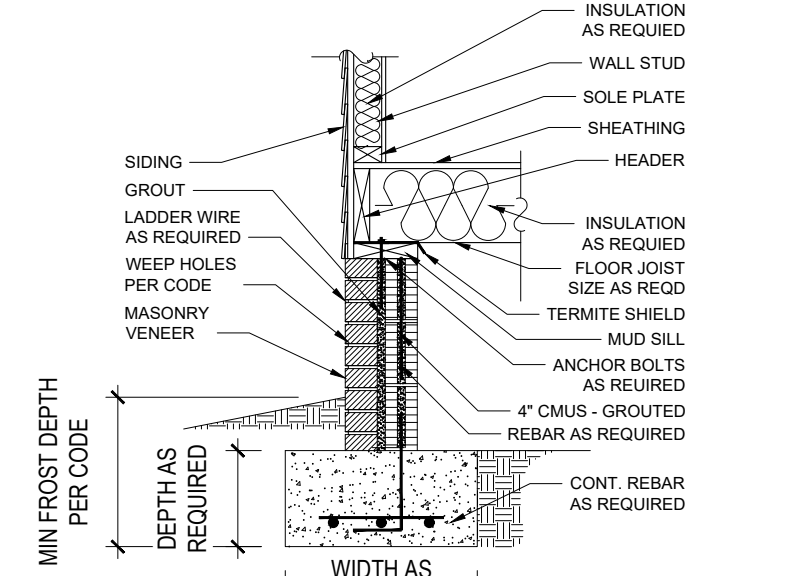
A-3 CORNICE DETAIL
NTS



A-22 FOUNDATION WALL DETAIL
NTS

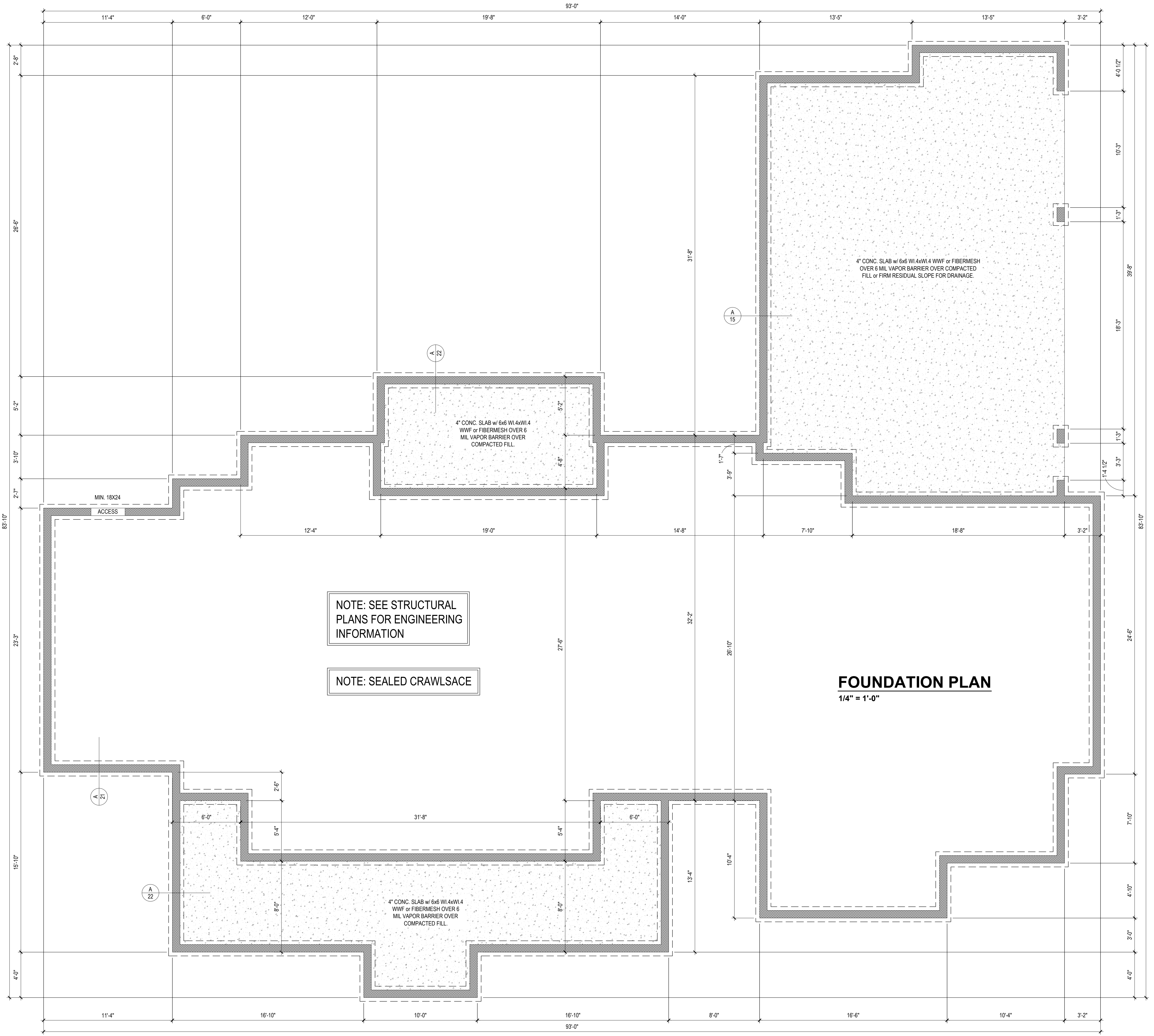


A-15 FOUNDATION WALL DETAIL
NTS



A-21 FOUNDATION WALL DETAIL
NTS

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NOTE: SEE STRUCTURAL PLANS FOR ENGINEERING INFORMATION

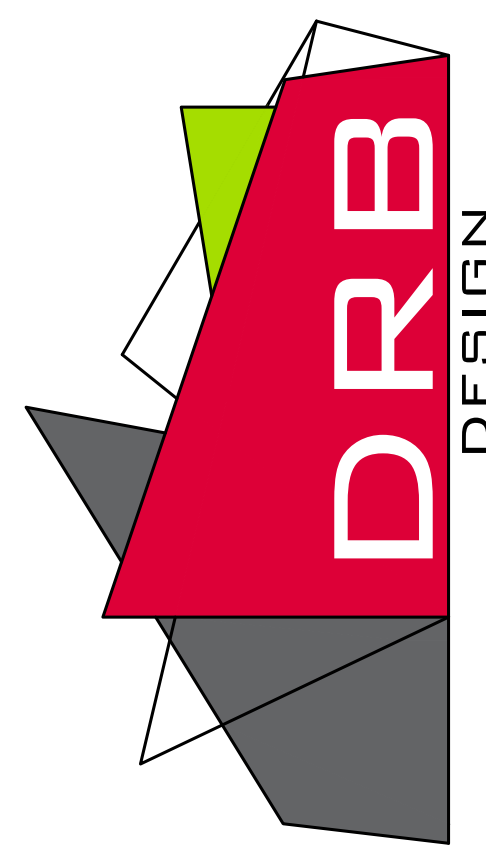
NOTE: SEALED CRAWLSACE

FOUNDATION PLAN
1/4" = 1'-0"

PROJECT#
DRB2101-0274
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WEBSITE
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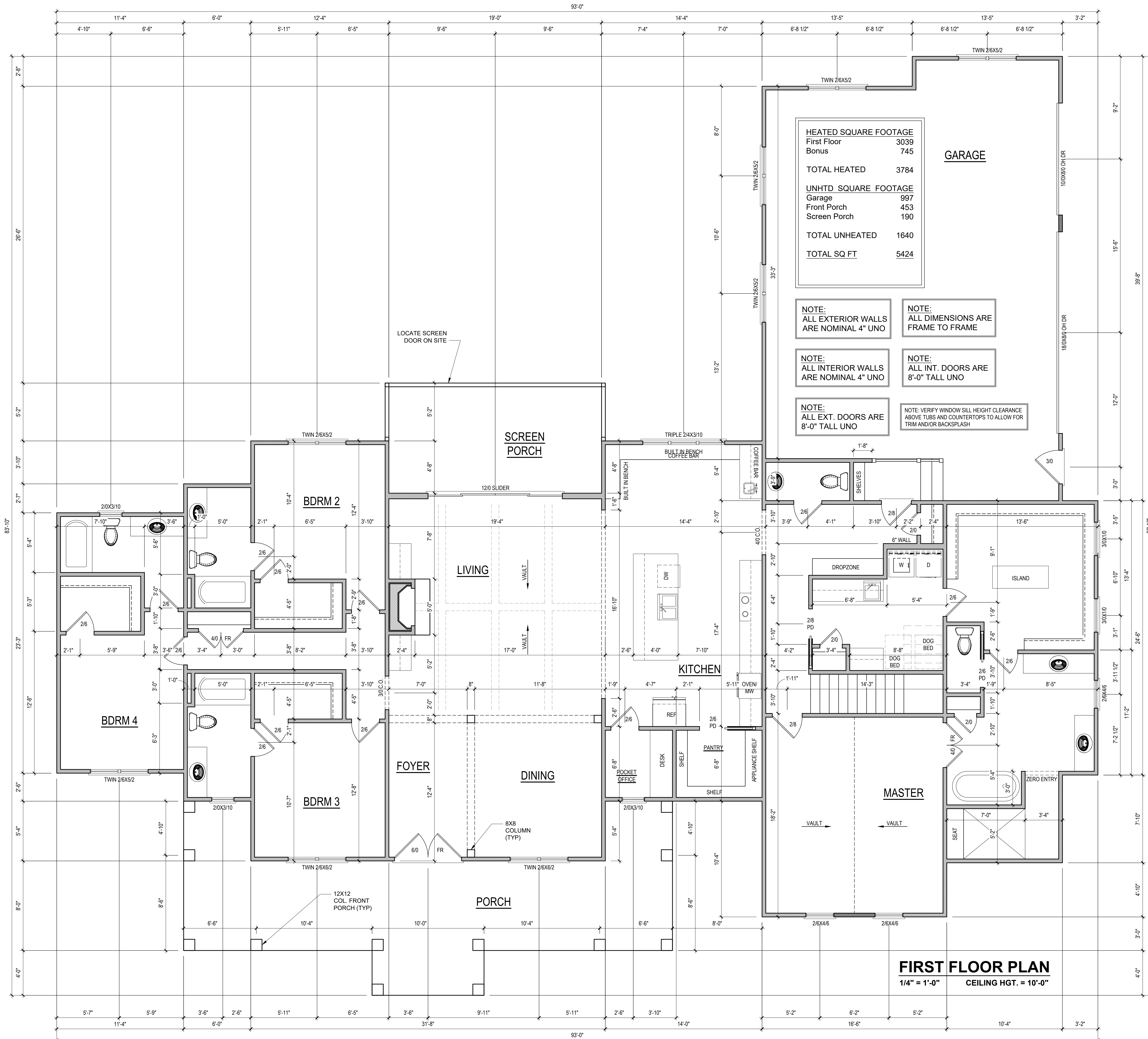
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SHEET NAME
FOUNDATION
SHEET#
A3
of 6



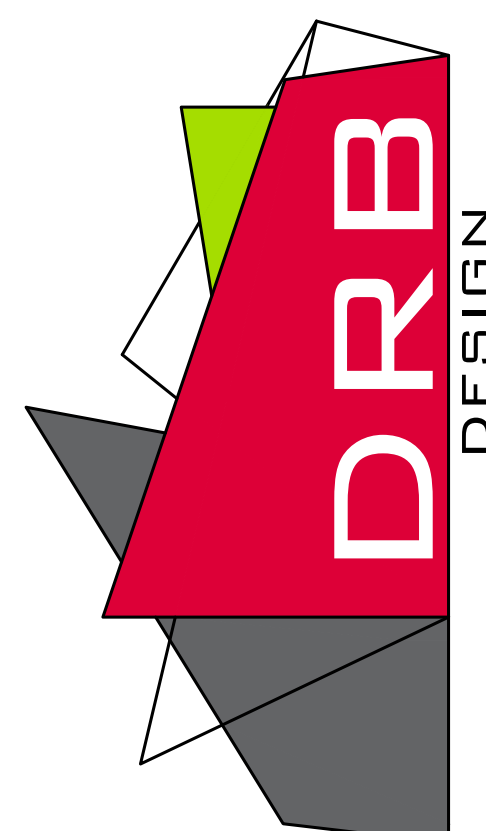
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FIRST FLOOR PLAN
1/4" = 1'-0" CEILING HGT. = 10'-0"

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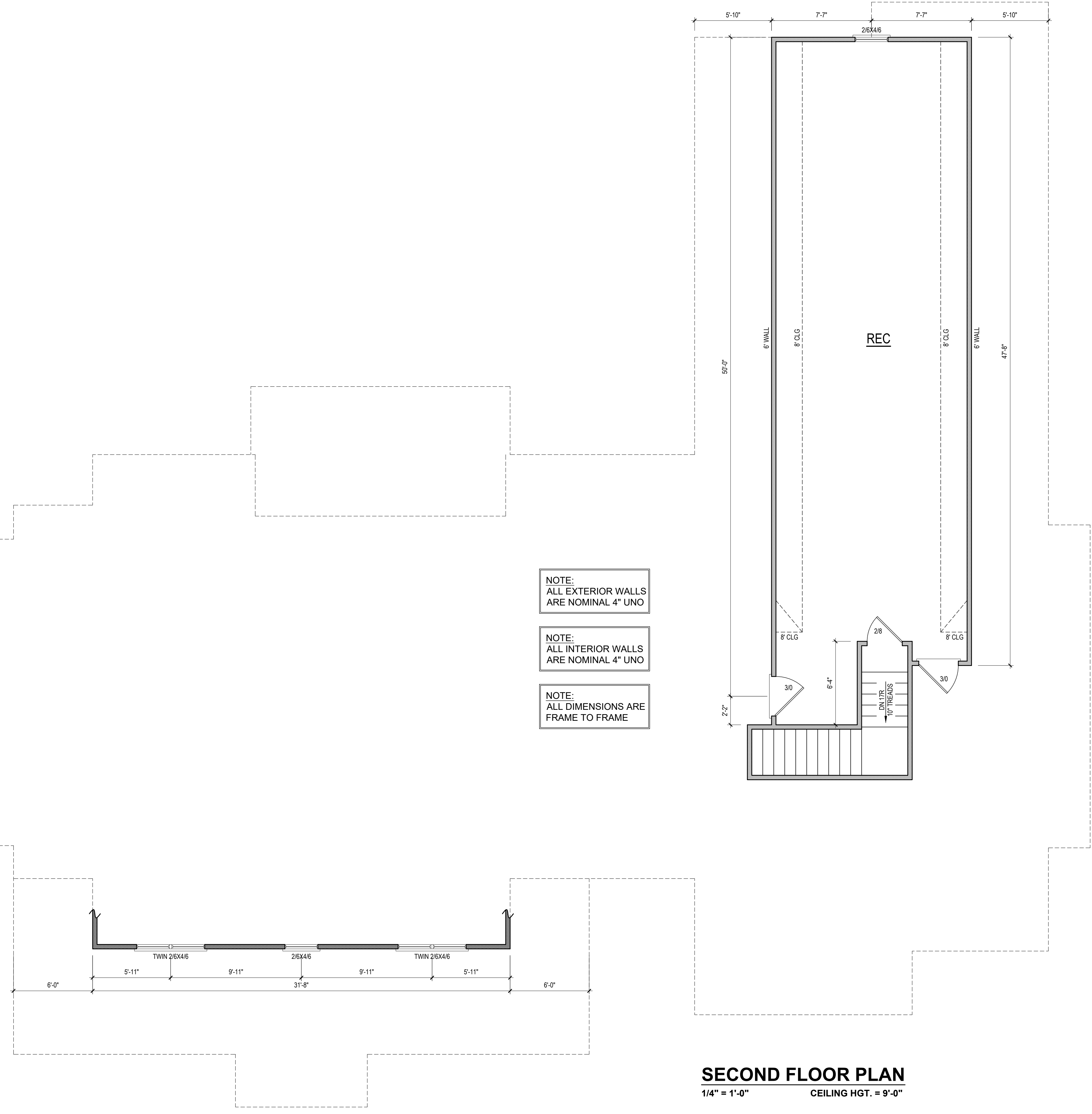


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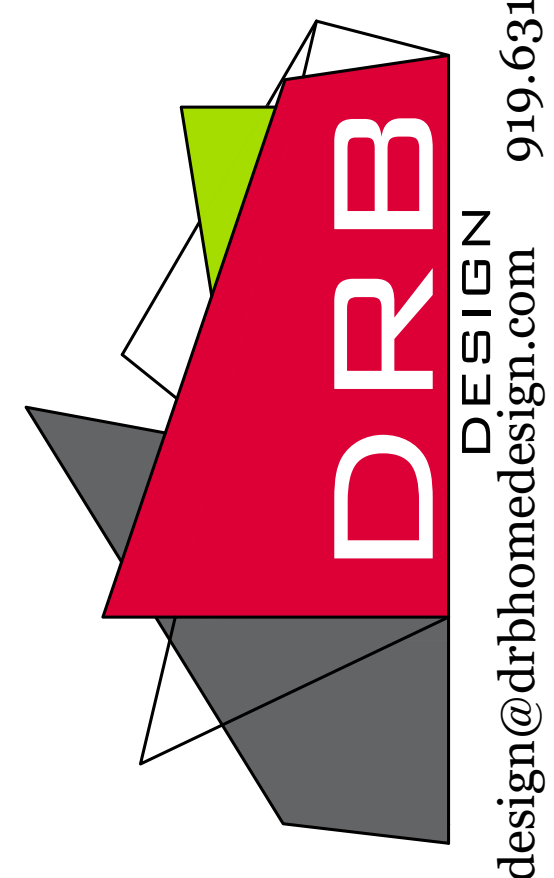
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SHEET NAME
1ST_FLOOR
SHEET#
A4
of 6

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SECOND FLOOR PLAN
1/4" = 1'-0" CEILING HGT. = 9'-0"



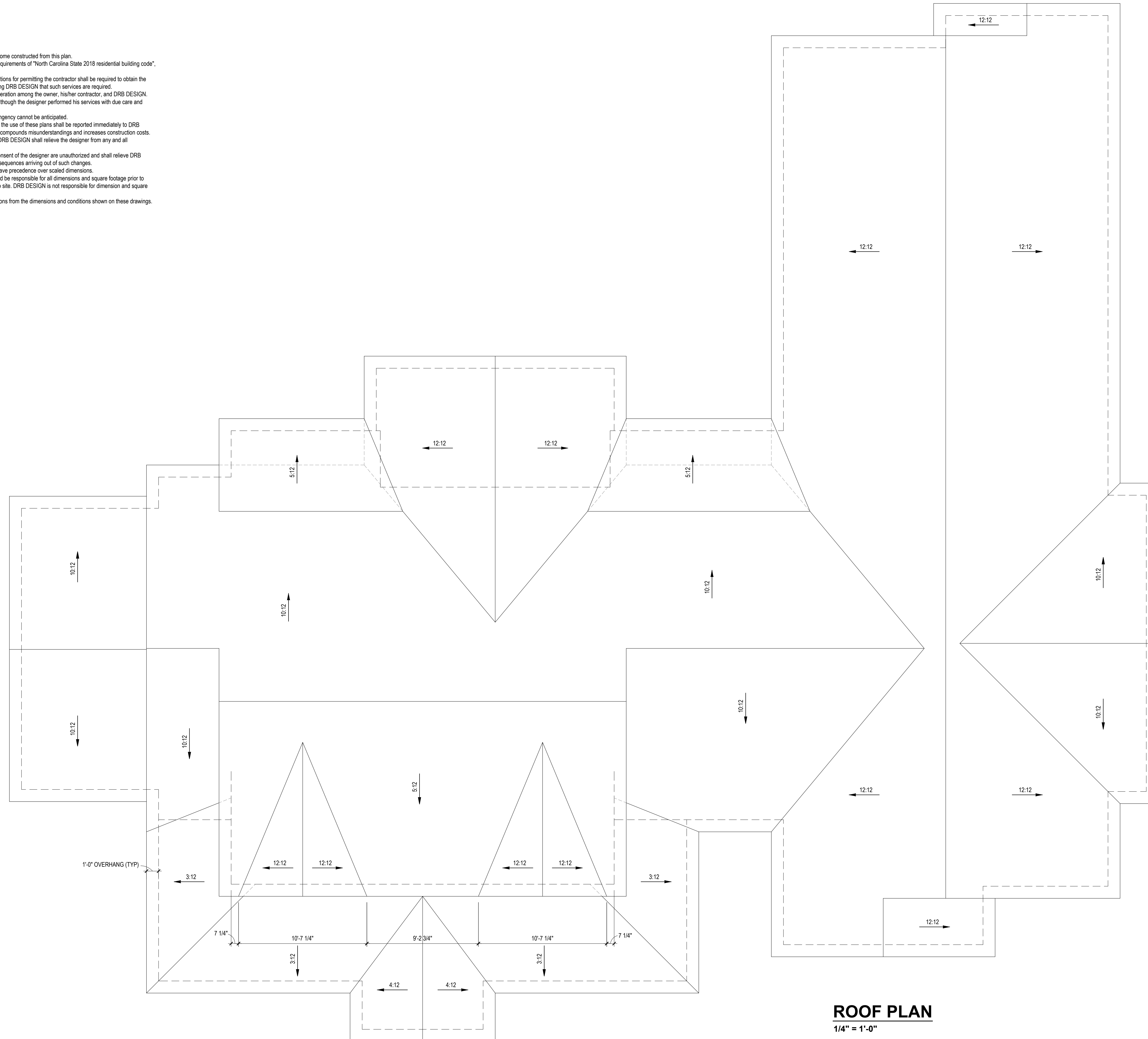
PROJECT#
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DATE
03/03/2022
DRAWN/DESIGNED BY
DRB
CHECKED BY
MMB
SCALE
1/4" = 1'-0"

PROJECT NAME
PERSONAL RESIDENCE
WEBSITE
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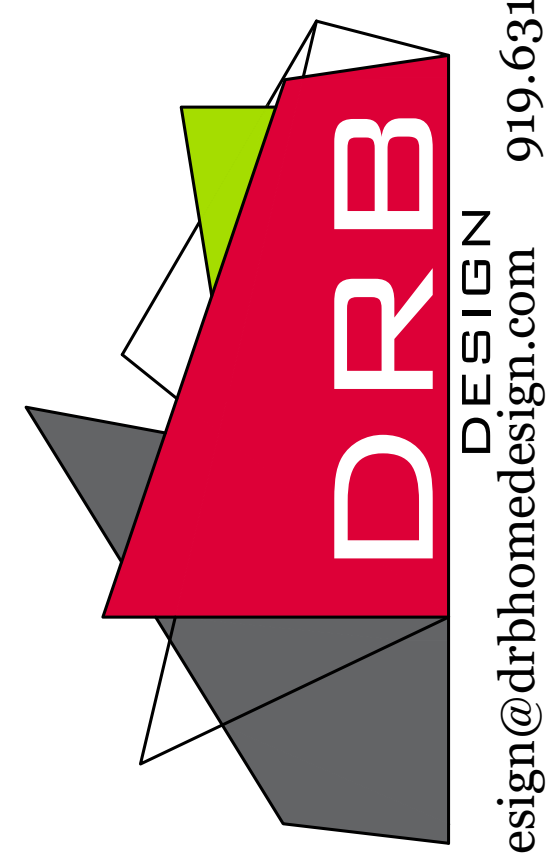


ROOF PLAN
1/4" = 1'-0"

PROJECT #
DRB2101-0274
DATE
03/03/2022
DRAWN/DESIGNED BY
DRB
CHECKED BY
MMB
SCALE
1/4" = 1'-0"

WEBSITE
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PROJECT NAME
PERSONAL RESIDENCE



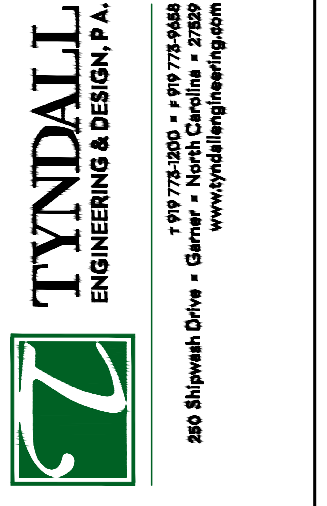
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SHEET NAME
ROOF
SHEET #
A6
of 6

Engineers and designers do not include construction means, methods, techniques, sequences, procedures or safety precautions. Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



TYNDALL
ENGINEERING & DESIGN, P.A.
1101 72nd Ave., Suite 100
Beverly Hills, FL 33433
www.tyndallengineering.com

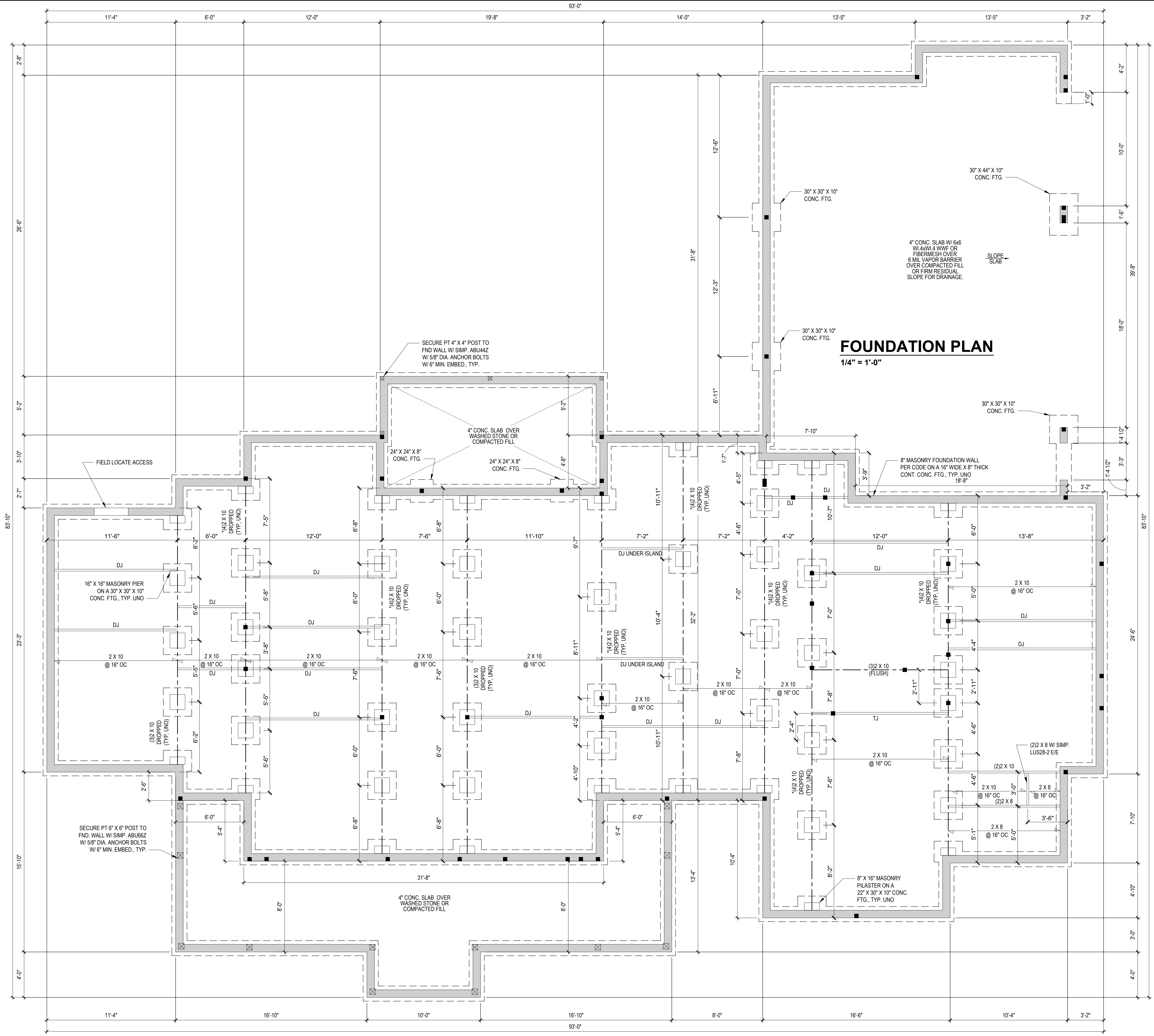
CLIENT: STEVEN RICHARDSON
PROJECT: RICHARDSON RESIDENCE

FOUNDATION PLAN 1ST FLOOR FRAMING

Project #: DRB2101-0274
Date: 04/08/22
Engineered by: HJS
DWG. Checked by: PAT
Scale: SEE PLAN

REVISIONS		
No.	Date	Remarks

Sheet Number
S1
1 of 7



*NOTE: SECURE 4-PLY W/ 1/2"Ø THRU-BOLTS @ 24" O.C. (OR EQUIV. STRUCTURAL SCREWS)

FILENAME: Z:\WPB\08_2021\082101-0274\STEPHEN_RICHARDSON\082101-0274_LEANS_SWDG_06_PRENCE_TYNDALL_LAST_PLOT_DATE:4/8/2022 8:17 AM

DESIGN LOADS	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no storage)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, P.A. IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- ALL LUMBER SHALL BE SYP #2 (UNO).
- ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND F_b = 2600 PSI (E = 1.8M PSI).
- (E LEVEL MICROLAM)
- ALL LVL LUMBER IS TO BE 1.5E (F_b = 2225 PSI)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 5'-8". MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-0". OTHERWISE, REFER TO TABLE R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (U.N.O.)
- REFER TO 2018 IRC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- F_y = 50 KSI MIN. (U.N.O.)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE f_c = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 12"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 2'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (U.N.O.)
- PROVIDE A MINIMUM OF 300# LIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.
- MAXIMUM MASONRY PER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES:

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
- WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 IRC.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- REFERENCE FIGURE R602.10.4.3 OF THE 2018 NRC.
- INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (U.N.O.)
- 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS.
- 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS.
- EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (U.N.O.)
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8" SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
 - 2' ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT
 - 3' ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT.
 - 4' FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- SHEATH INTERIOR & EXTERIOR
- FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN lieu of a CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- MINIMUM 800# HOLD-DOWN DEVICE

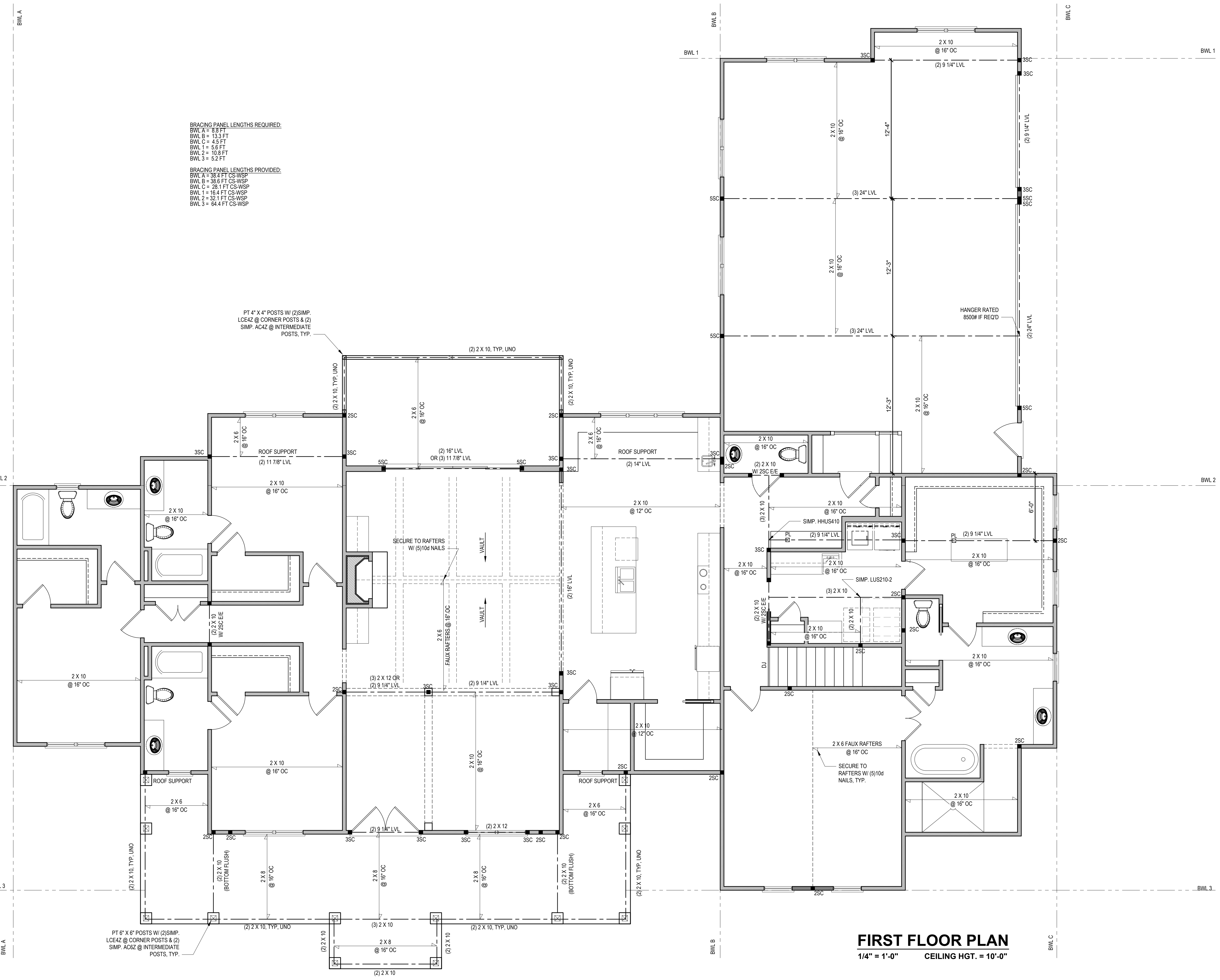
BRACING PANEL LENGTHS REQUIRED:

BWL A = 8.8 FT
 BWL B = 13.3 FT
 BWL C = 4.9 FT
 BWL 1 = 5.6 FT
 BWL 2 = 10.8 FT
 BWL 3 = 5.2 FT

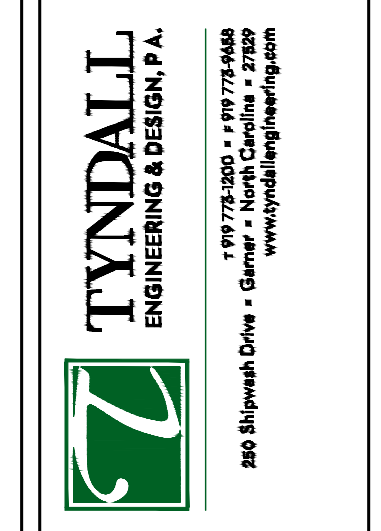
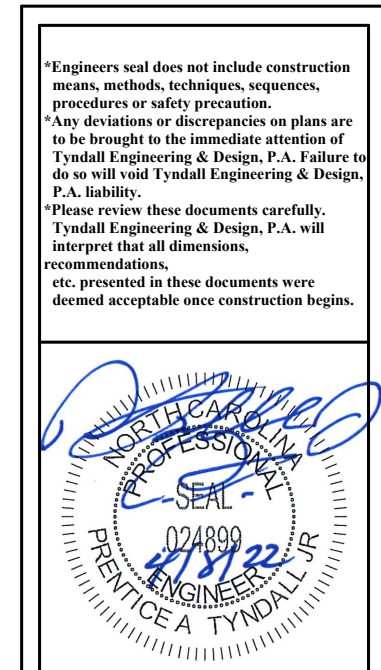
BRACING PANEL LENGTHS PROVIDED:

BWL A = 38.4 FT CS-WSP
 BWL B = 38.6 FT CS-WSP
 BWL C = 28.1 FT CS-WSP
 BWL 1 = 16.4 FT CS-WSP
 BWL 2 = 32.1 FT CS-WSP
 BWL 3 = 64.4 FT CS-WSP

PT 4" X 4" POSTS W/ (2) SIMP. LCE4Z @ CORNER POSTS & (2) SIMP. AC6Z @ INTERMEDIATE POSTS, TYP.



FIRST FLOOR PLAN
 1/4" = 1'-0" CEILING HGT. = 10'-0"



Client: **STEVEN RICHARDSON**
 Project: **RICHARDSON RESIDENCE**

**1ST FLOOR HEADER
 2ND FLOOR FRAMING**

Project #: **DRB2101-0274**
 Date: **04/08/22**
 Engineered by: **HJS**
 DWG. Checked by: **PAT**
 Scale: **SEE PLAN**

REVISIONS		
No.	Date	Remarks

Sheet Number
S2
 2 of 7

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DESIGN LOADS

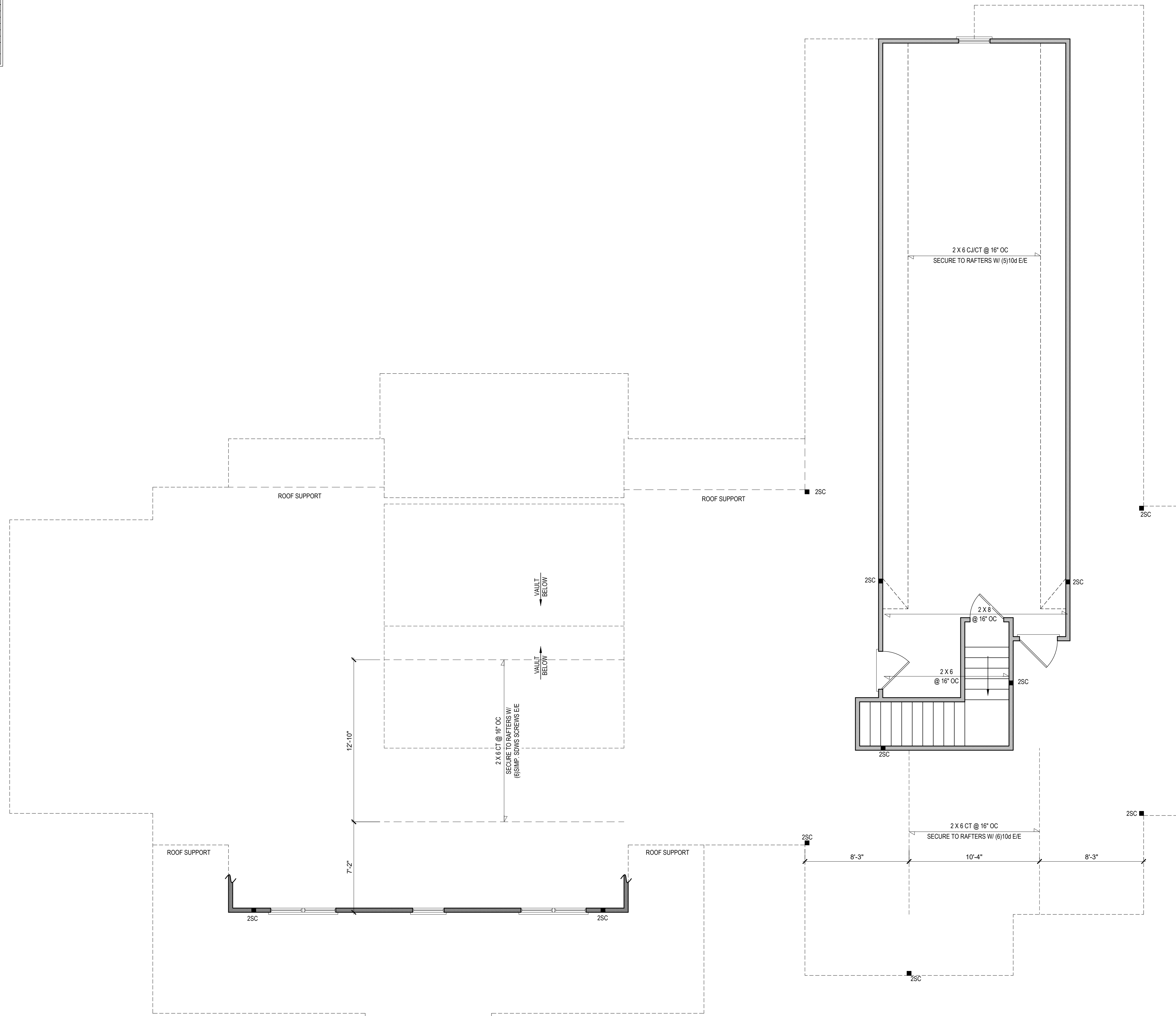
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TB
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, P.A. IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- ALL LUMBER SHALL BE SYP #2 (UNO)
ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND F_b = 2600 PSI, E = 1.9M PSI
(E LEVEL MICROLAM)
ALL LVL LUMBER IS TO BE 1.5SE (F_b = 2325 PSI)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10# NAILS @ 6" O.C. PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-6". MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-4". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
F_y = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, f_c = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 12"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 12" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
- PROVIDE A MINIMUM OF 30# PLFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.
- MAXIMUM MASONRY HEIGHT SHALL NOT EXCEED 10' UNLESS ITS LEAST HORIZONTAL DIMENSION.
- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
- WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NRC.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
 - REFERENCE FIGURE R602.10.4.3 OF THE 2018 NRC.
- INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE SB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
 - 12" GYPSUM BOARD (GB) MINIMUM LENGTH OF 6'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 56 COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
 - 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
 - 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT
 - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT.
 - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- SHEATH INTERIOR & EXTERIOR
- FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
 - MINIMUM 800# HOLD-DOWN DEVICE



SECOND FLOOR PLAN
1/4" = 1'-0"
CEILING HGT. = 9'-0"

*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precautions.
Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.
*Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.

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CLIENT: **STEVEN RICHARDSON**
PROJECT: **RICHARDSON RESIDENCE**

2ND FLOOR HEADER
2ND FLR. CLG. FRAMING

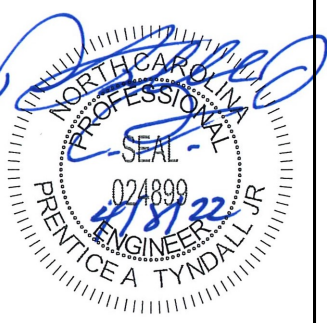
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Date:	04/08/22
Engineered by:	HJS
DWG. Checked By:	PAT
Scale:	SEE PLAN

No.	Date:	Remarks

Sheet Number
S3
3 of 7

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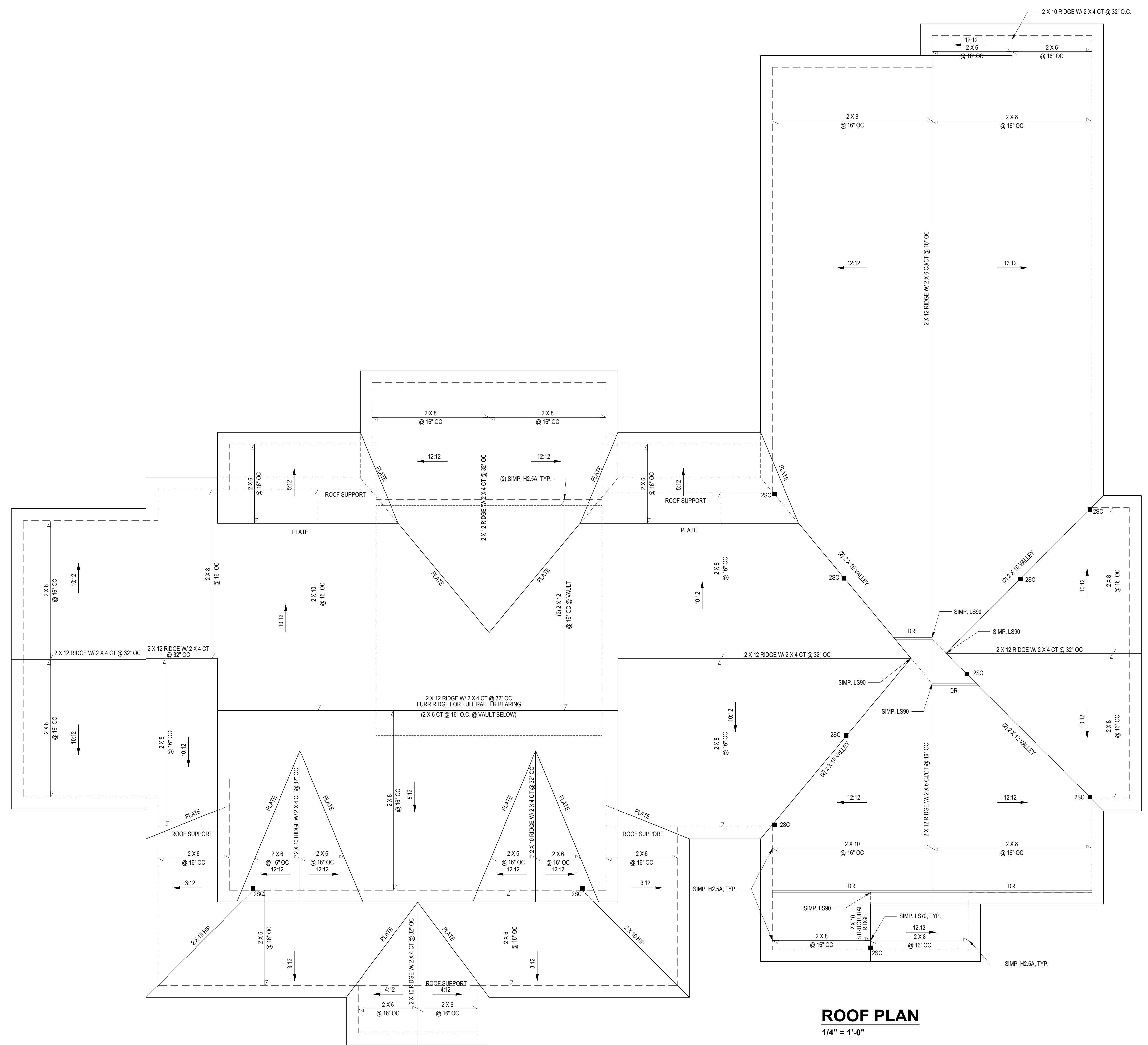
Client: **STEVEN RICHARDSON**
Project: **RICHARDSON RESIDENCE**

ROOF PLAN

Project #: DRB2101-0274
Date: 04/08/22
Engineered by: HJS
DWG. Checked by: PAT
Scale: SEE PLAN

REVISIONS		
No.	Date	Remarks

Sheet Number
S4
4 of 7



ROOF PLAN
1/4" = 1'-0"

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STRUCTURAL NOTES

1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF 'NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE', IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

Table with columns: DESIGN LOADS, LIVE LOAD (PSF), DEAD LOAD (PSF), DEFLECTION (LL, TL), and values for various load conditions like ALL FLOORS, ATTIC, EXTERNAL BALCONY, ROOF TRUSS, WIND LOAD, and SEISMIC.

- 3) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
4) CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE (U.N.C.)
5) MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING...

DEFINITIONS FOR COMMON ABBREVIATIONS

Table mapping abbreviations to full names: ALT = ALTERNATE, CANT = CANTILEVER, CJ = CEILING JOIST, CMU = CONCRETE MASONRY UNIT, COL = COLUMN, CONC = CONCRETE, CONT = CONTINUOUS, CT = COLLAR TIE, DBL = DOUBLE, DJM = DOUBLE JOIST, DR = DOUBLE RAFTER, EA = EACH, EE = EACH END, FJ = FLOOR JOIST, FND = FOUNDATION, FTG = FOOTING, GALV = GALVANIZED, HORIZ = HORIZONTAL, HT = HEIGHT, MANUF = MANUFACTURER, MAX = MAXIMUM, MIN = MINIMUM, NOM = NOMINAL, O.C. = ON CENTER, PL = POINT LOAD, REIN = REINFORCED, REQD = REQUIRED, RJ = ROOF JOIST, RS = ROOF SUPPORT, SC = STUD COLUMN, SCH = SCHEDULE, SPEC = SPECIFIED, THK = THICK, TJK = TRIPLE JOIST, TRTD = TREATED, TYP = TYPICAL, UNO = UNLESS NOTED OTHERWISE, W = WIDE FLANGE BEAM, WWF = WELDED WIRE FABRIC, XJ = EXTRA JOIST.

1) MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLOWS:

Table with columns: POST SIZE, MAX. POST HEIGHT**, and values: 4 x 4 (8'-0"), 6 x 6 (20'-0"), *** (OVER 20'-0").

* THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET WHICH MAY BE LOCATED AT DIFFERENT LEVELS. ** FROM TOP OF FOOTING TO BOTTOM OF GIRDER. *** DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.

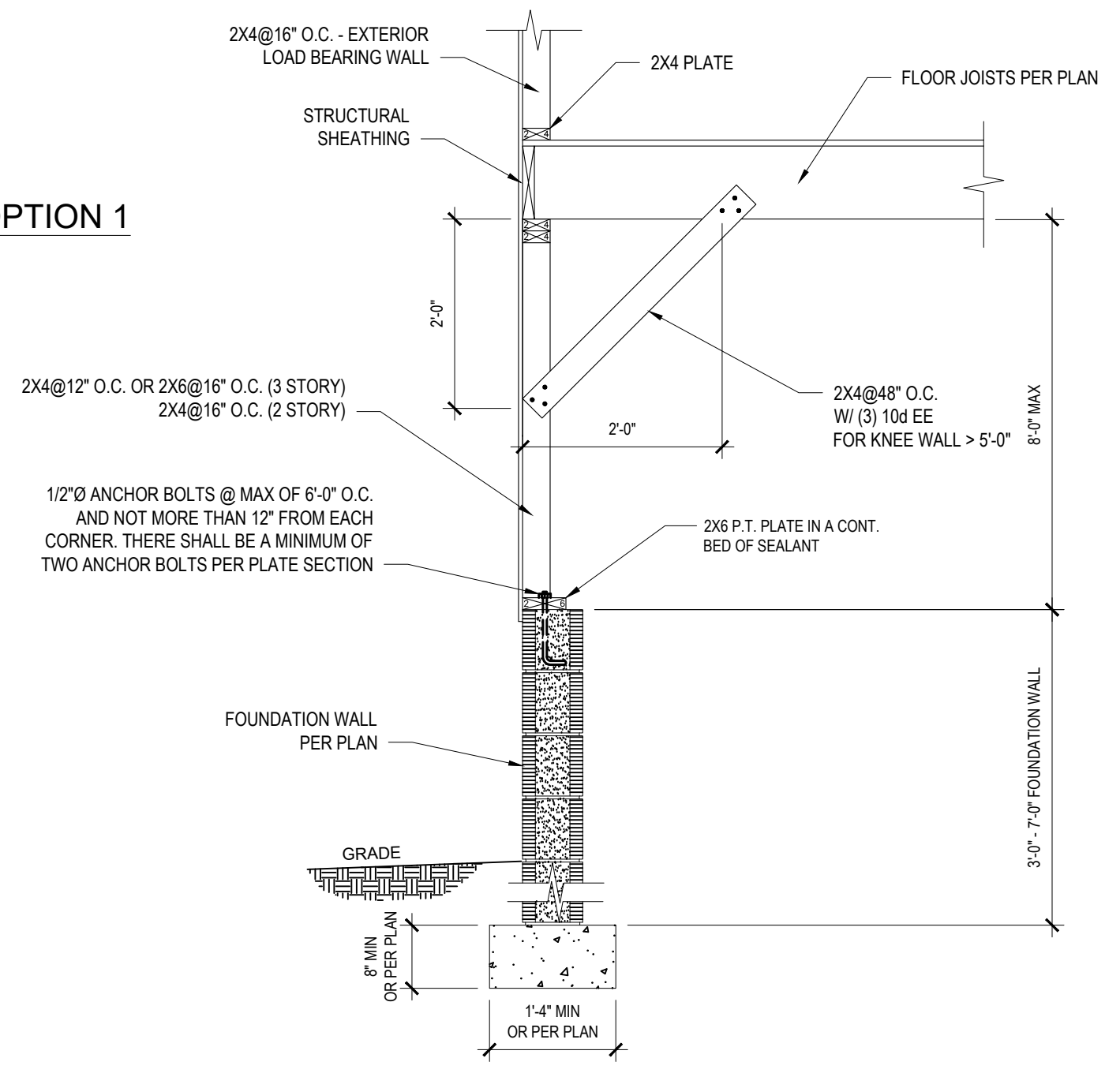
2) DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THESE METHODS:

- A. THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4) ABOVE. LATERAL BRACING IS NOT REQUIRED.
B. 4 x 4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE BOLTED TO THE POST AND GIRDER WITH ONE S/8" HOT DIPPED GALVANIZED BOLT AT EACH END OF THE BRACE.

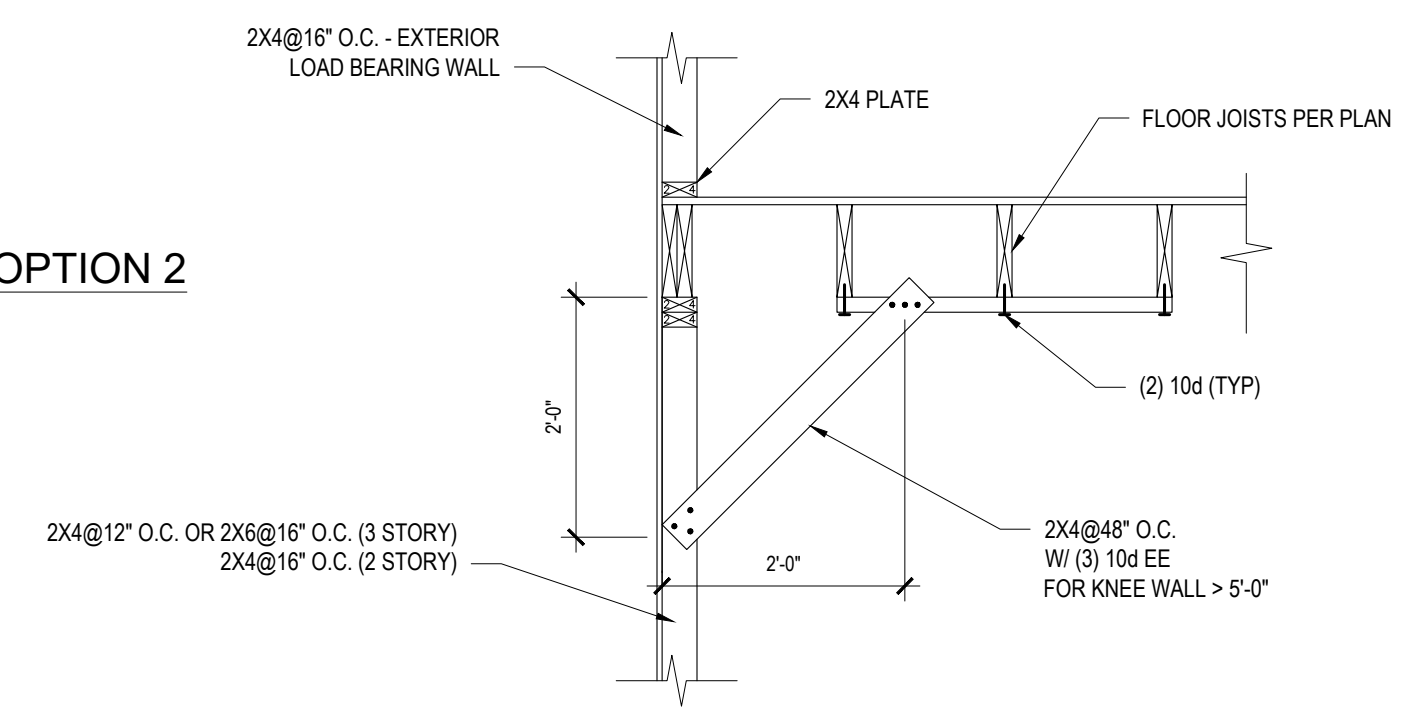
Table with columns: POST SIZE, MAX. TRIBUTARY AREA, MAX. POST HEIGHT, EMBEDMENT DEPTH, and CONCRETE DIAMETER. Values for 4 x 4 and 6 x 6 posts.

- D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO (2) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2 x 6 SHALL BE ATTACHED TO THE POSTS WITH ONE S/8" HOT DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER.
E. FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.

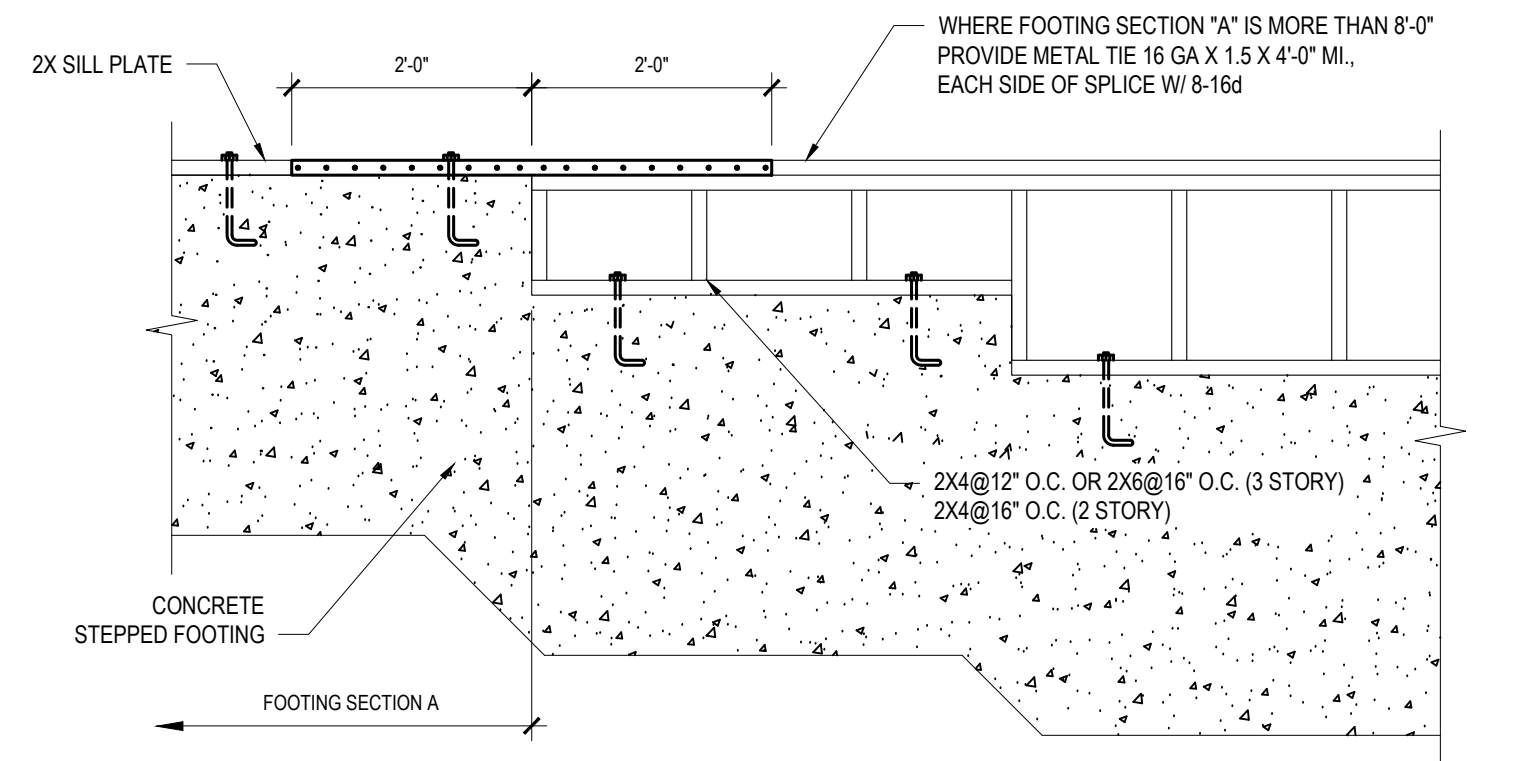
OPTION 1



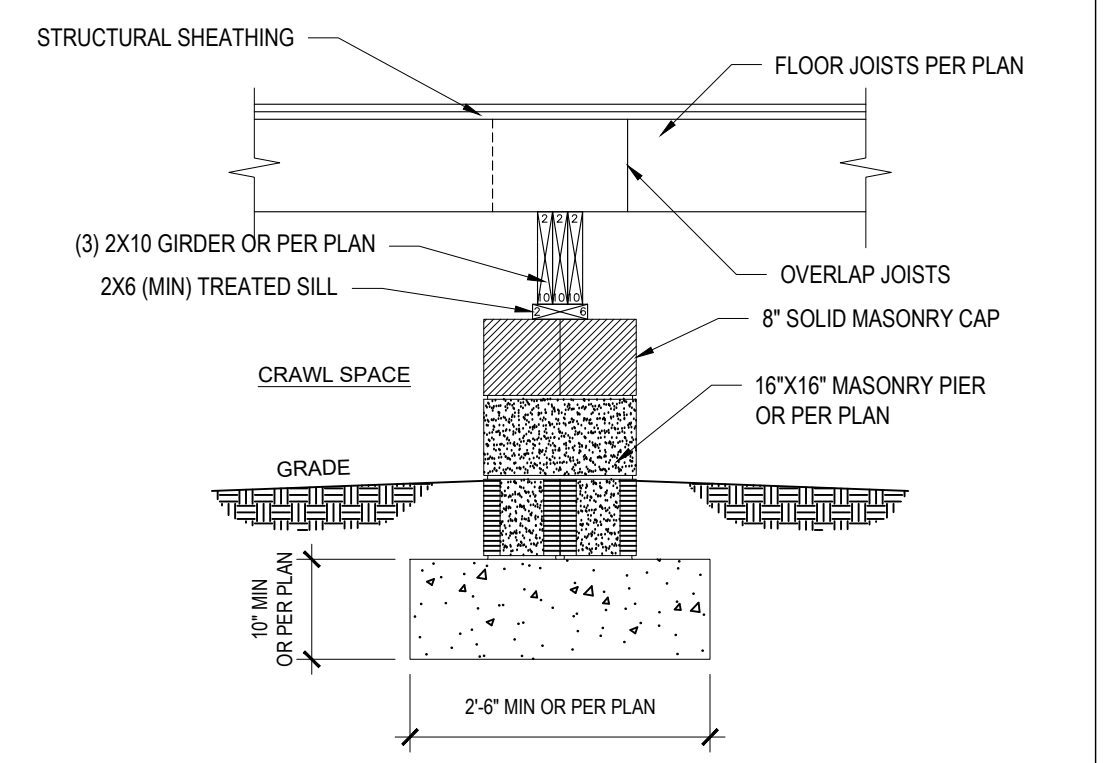
OPTION 2



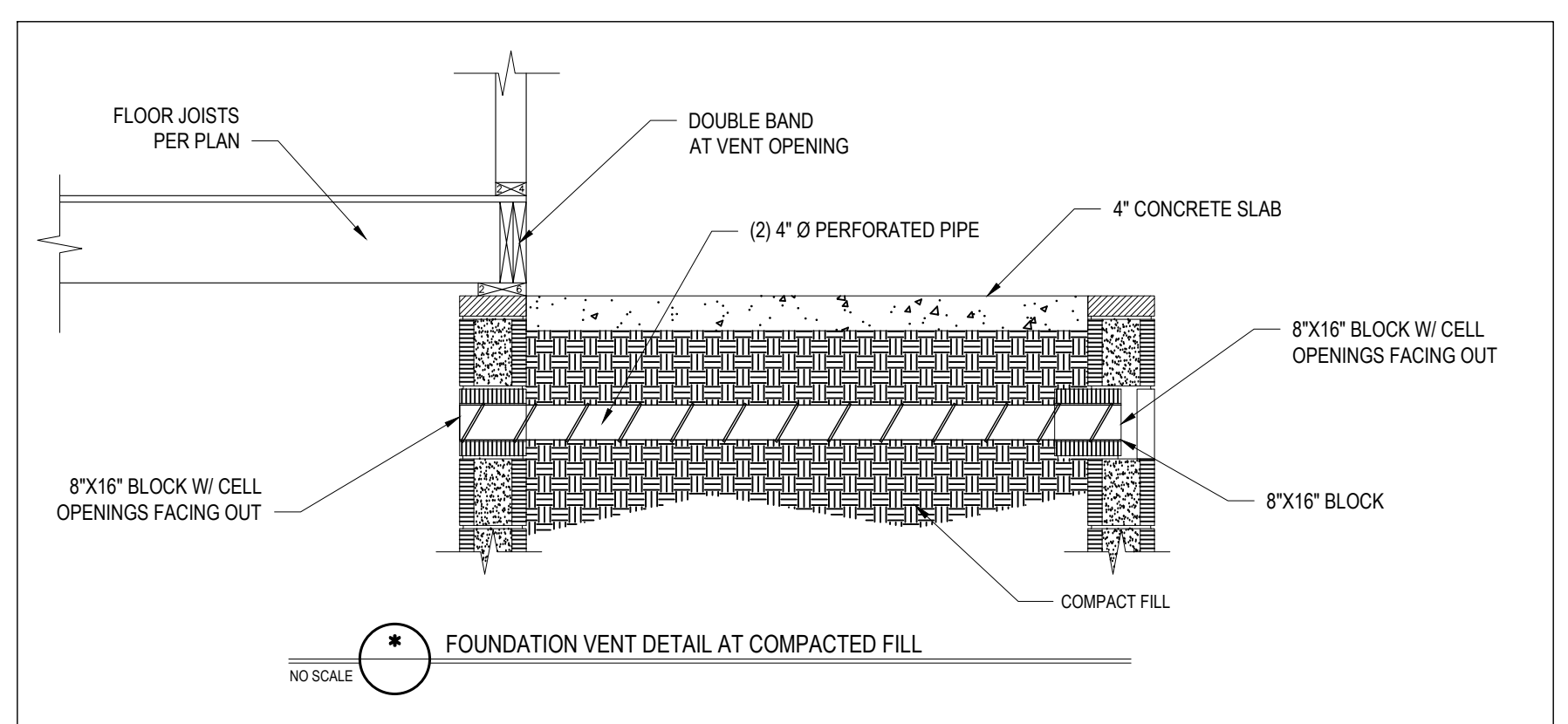
CRIPPLE WALL DETAIL (2 OPTIONS)



STEP DOWN FOUNDATION AT CRIPPLE WALL



DROPPED GIRDER DETAIL



FOUNDATION VENT DETAIL AT COMPACTED FILL

Table N1102.1 CLIMATE ZONES 3-5 with columns for Climate Zones, Fenestration U-factor, Skylight R-value, Glazed Fenestration SHGC, Ceiling R-value, Wood Framed Wall R-value, Mass Wall R-value, Floor R-value, Basement Wall R-value, Slab R-value and Depth, and Crawl Space R-value.

TABLE N1102.1 CLIMATE ZONES 3-5. R-VALUES ARE MINIMUM. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE LISTED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.

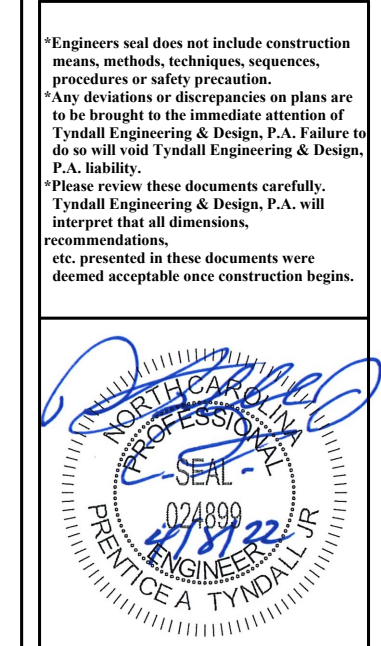
2847 SQ. FT. OF CRAWL SPACE / 150 = 19 SQ. FT. OF REQ'D VENTILATION WITHOUT CROSS VENTILATION. 19 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 22 VENTS REQ'D (BASED ON 8" X 16" VENTS)

2847 SQ. FT. OF CRAWL SPACE / 1500 = 1.9 SQ. FT. OF REQ'D VENTILATION WITH CROSS VENTILATION. 1.9 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 2.2 VENTS REQ'D (BASED ON 8" X 16" VENTS)

CRAWL SPACE VENTILATION CALCULATION. NO SCALE.

3857 SQ. FT. OF ATTIC / 300 = 13 SQ. FT. INLETS/OUTLETS REQUIRED. NO SCALE.

ATTIC VENTILATION CALCULATION. NO SCALE.



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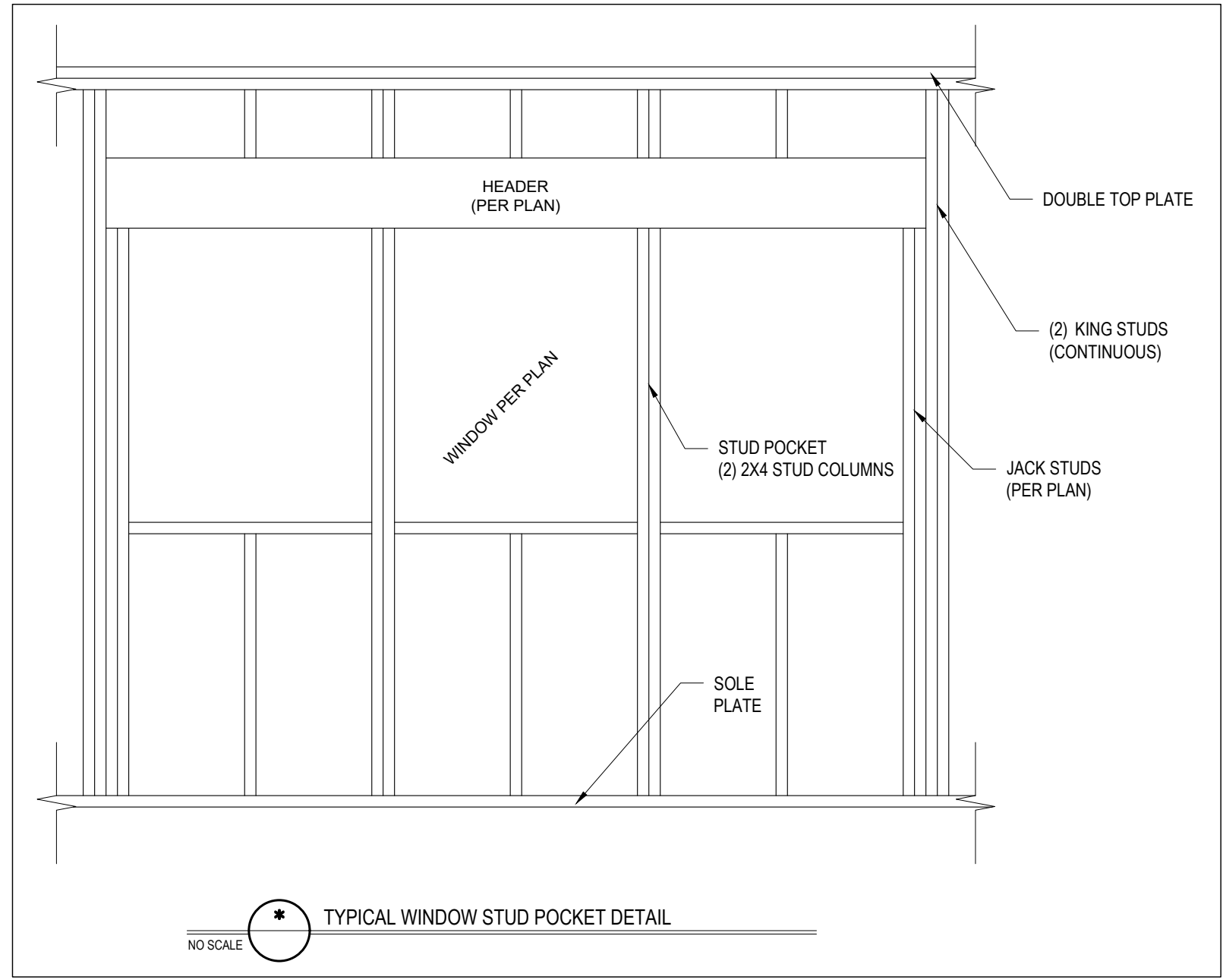
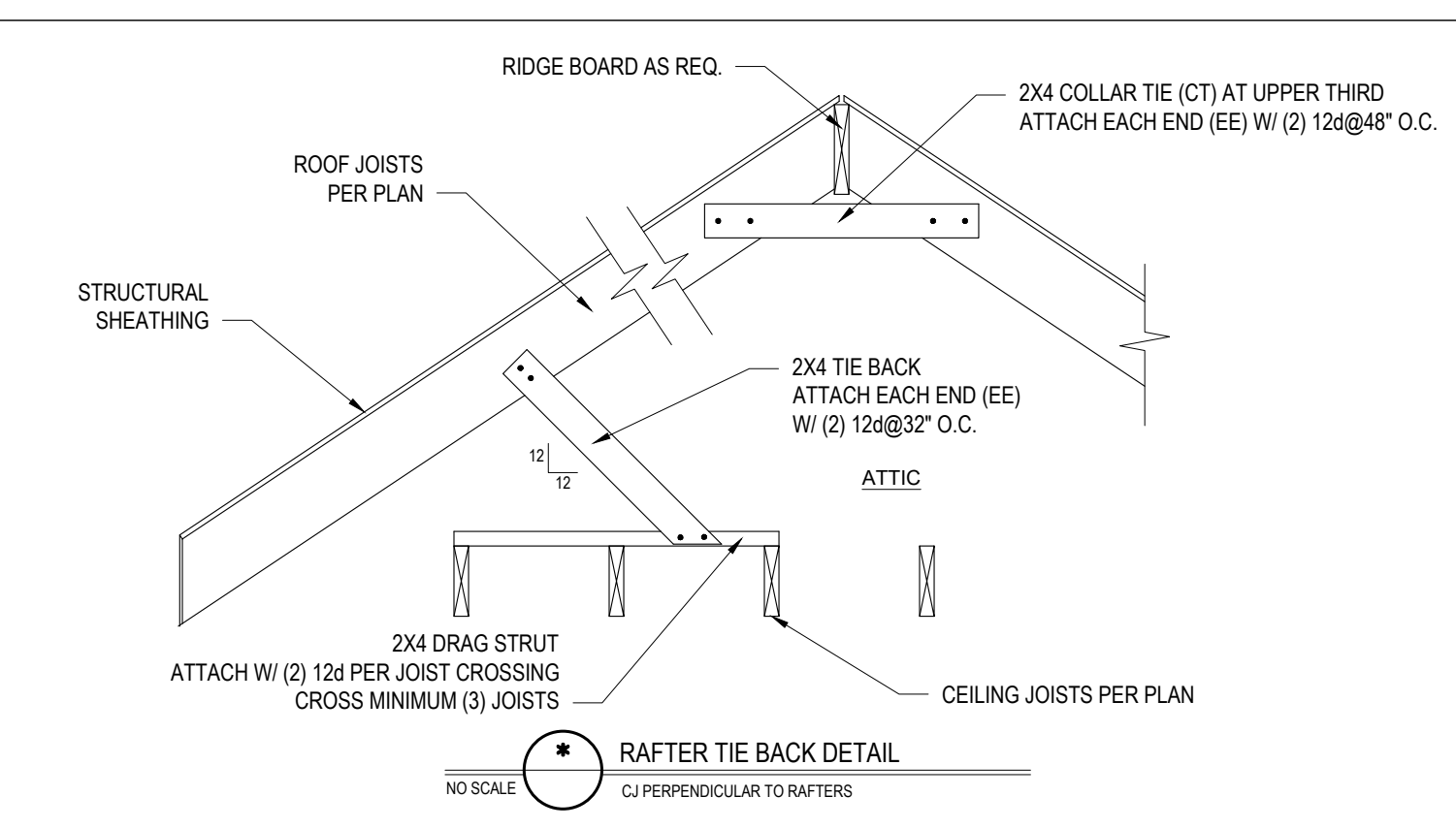
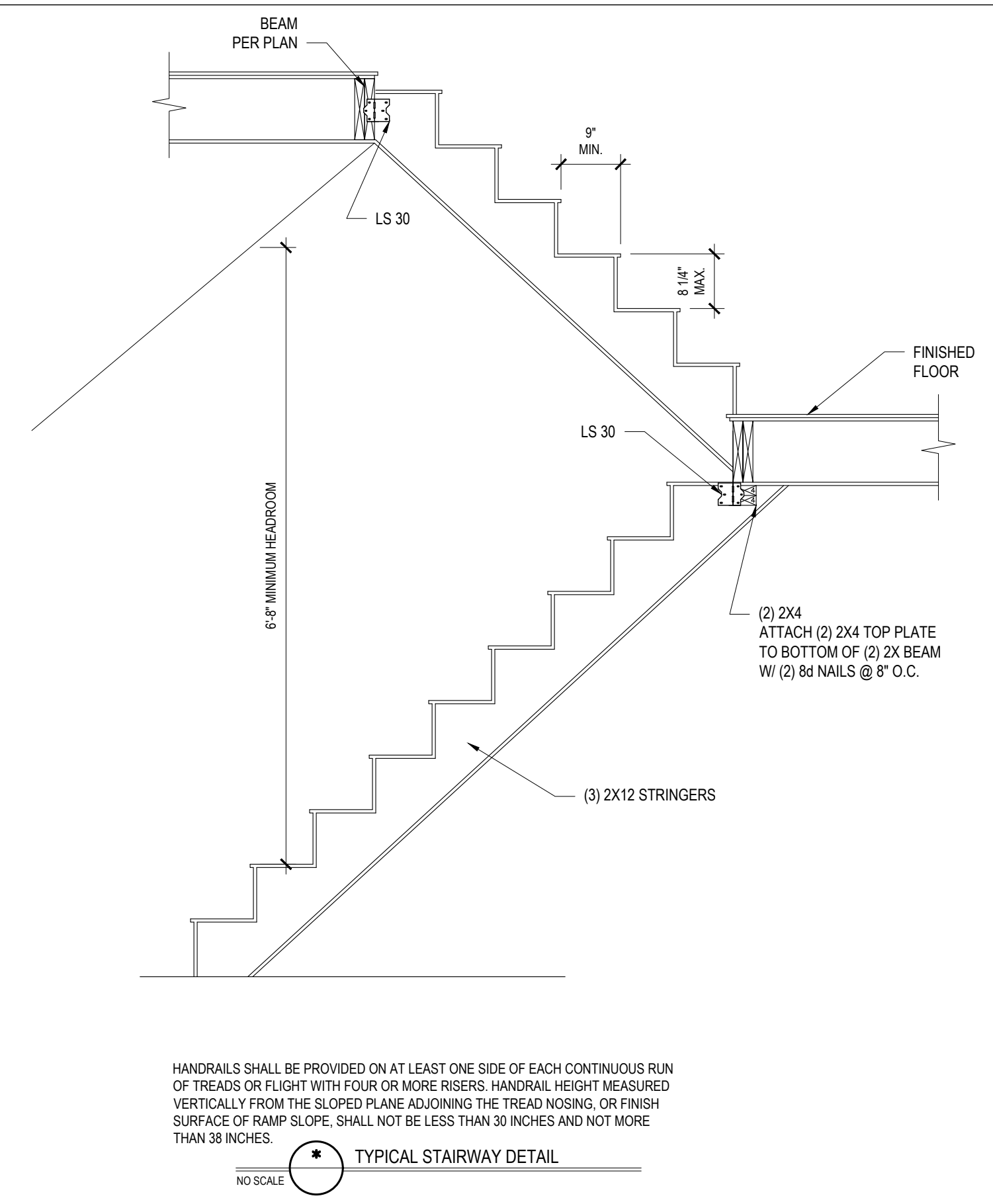
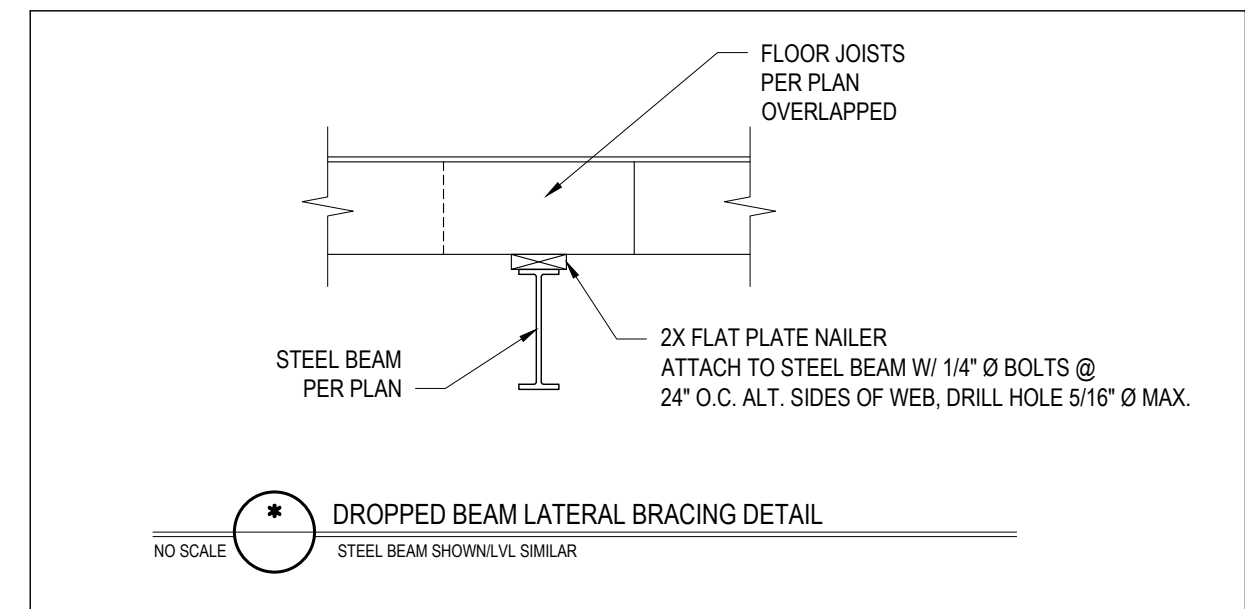
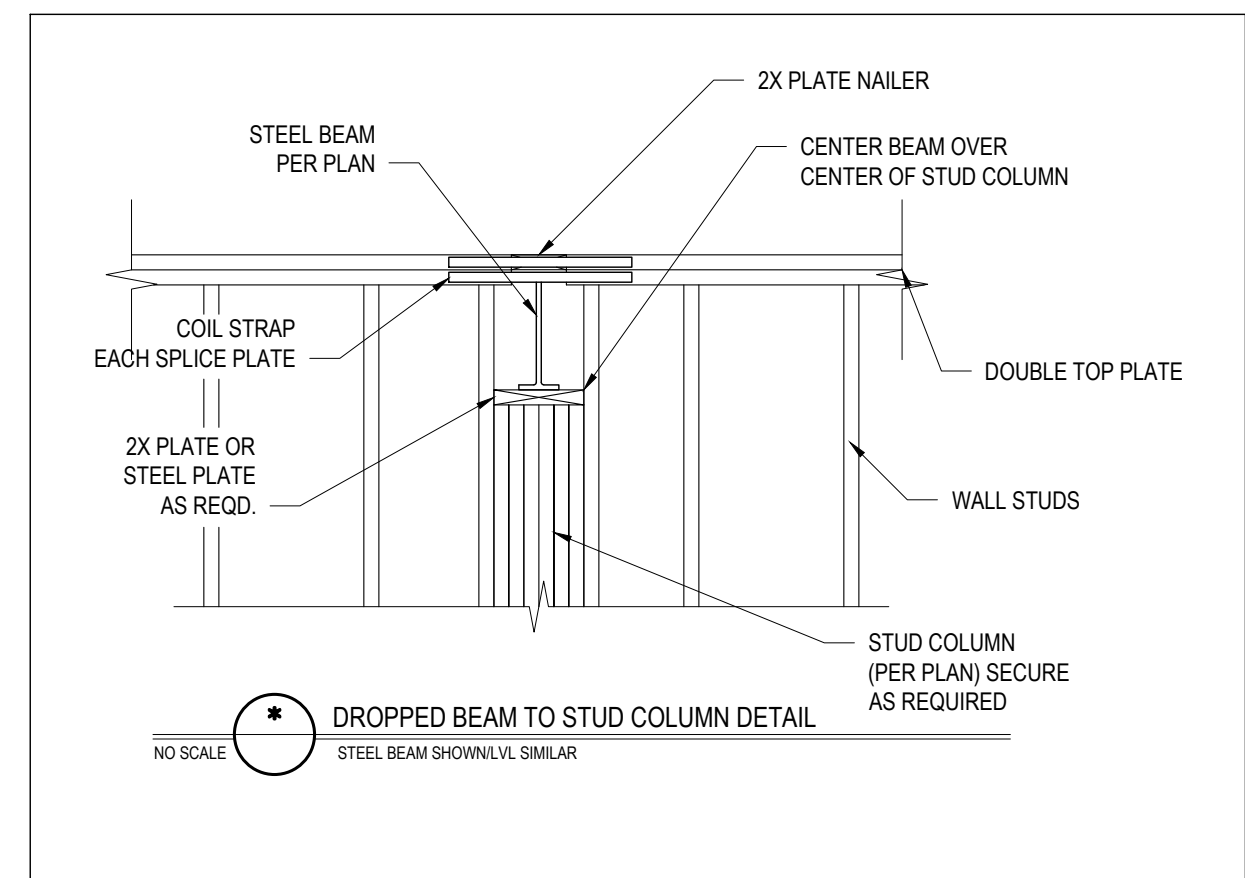
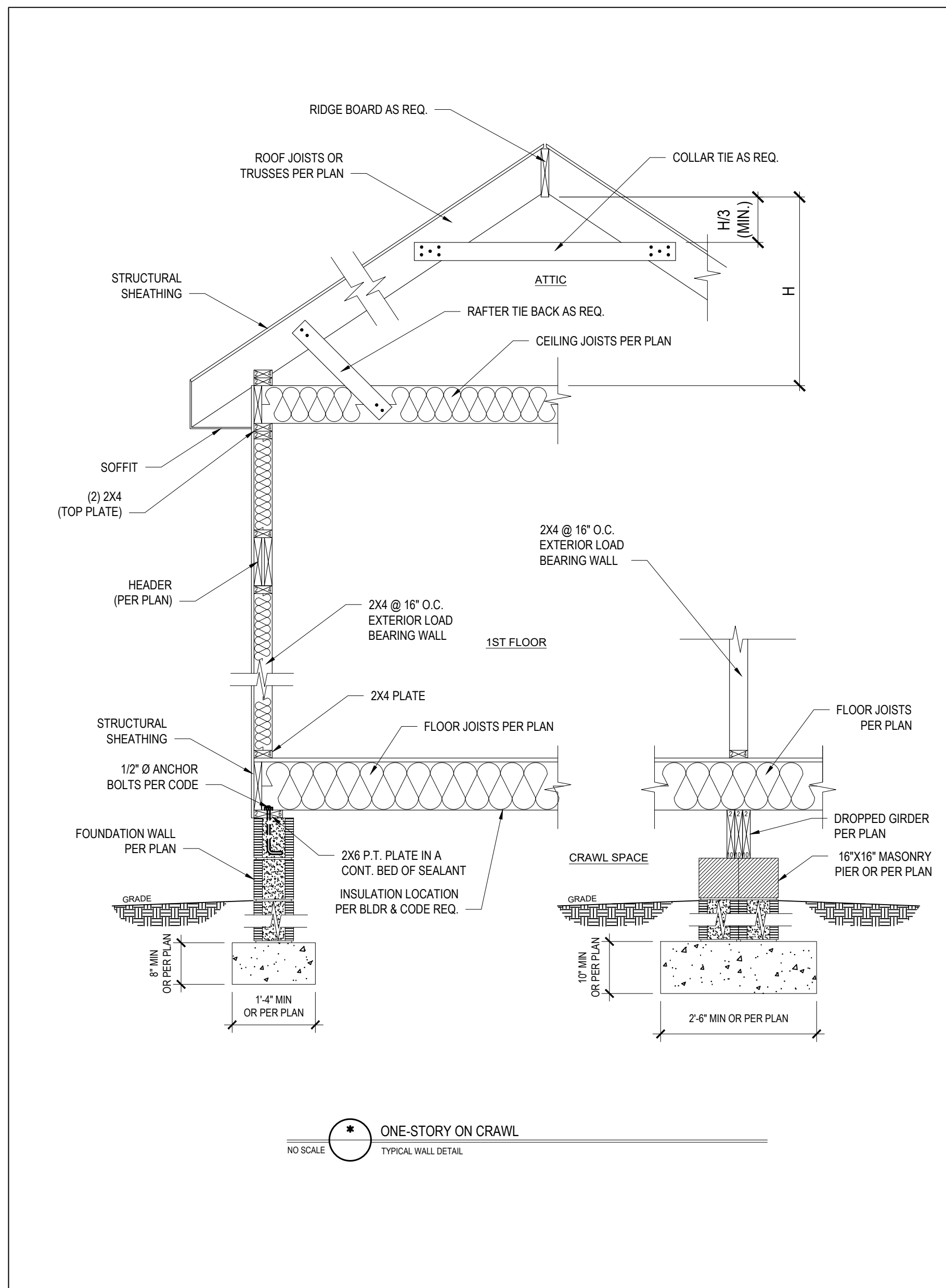
STANDARD DETAILS

Project #: DRB2101-0274 Date: 04/08/22 Engineer: HJS DWG. Checked by: PAT Scale: SEE PLAN

Table with columns: No., Date, Remarks. Contains revision entries.

Sheet Number D1 5 of 7

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Engineers and designers shall not be held responsible for construction methods, techniques, sequences, procedures or safety precautions. Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyn dall Engineering & Design, P.A. Failure to do so will void Tyn dall Engineering & Design, P.A. liability. Please review these documents carefully. Tyn dall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.

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Client: **STEVEN RICHARDSON**
Project: **RICHARDSON RESIDENCE**

**STANDARD
DETAILS**

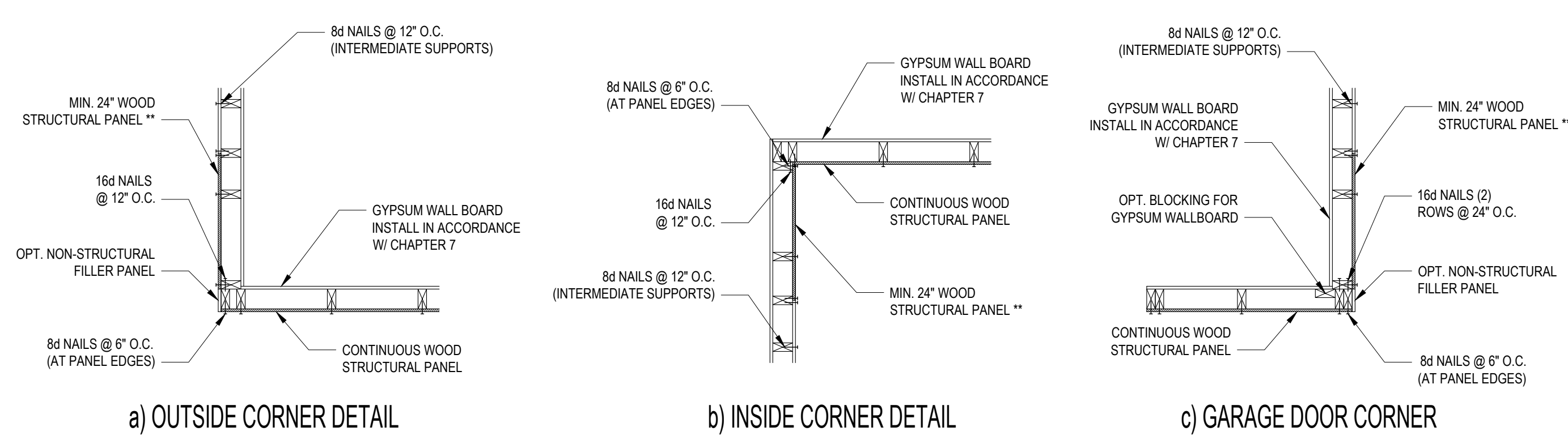
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Date: **04/08/22**
Engineered by: **HJS**
DWG. Checked by: **PAT**
Scale: **SEE PLAN**

REVISIONS		
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Sheet Number
D2
6 of 7

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No.	Date	Remarks



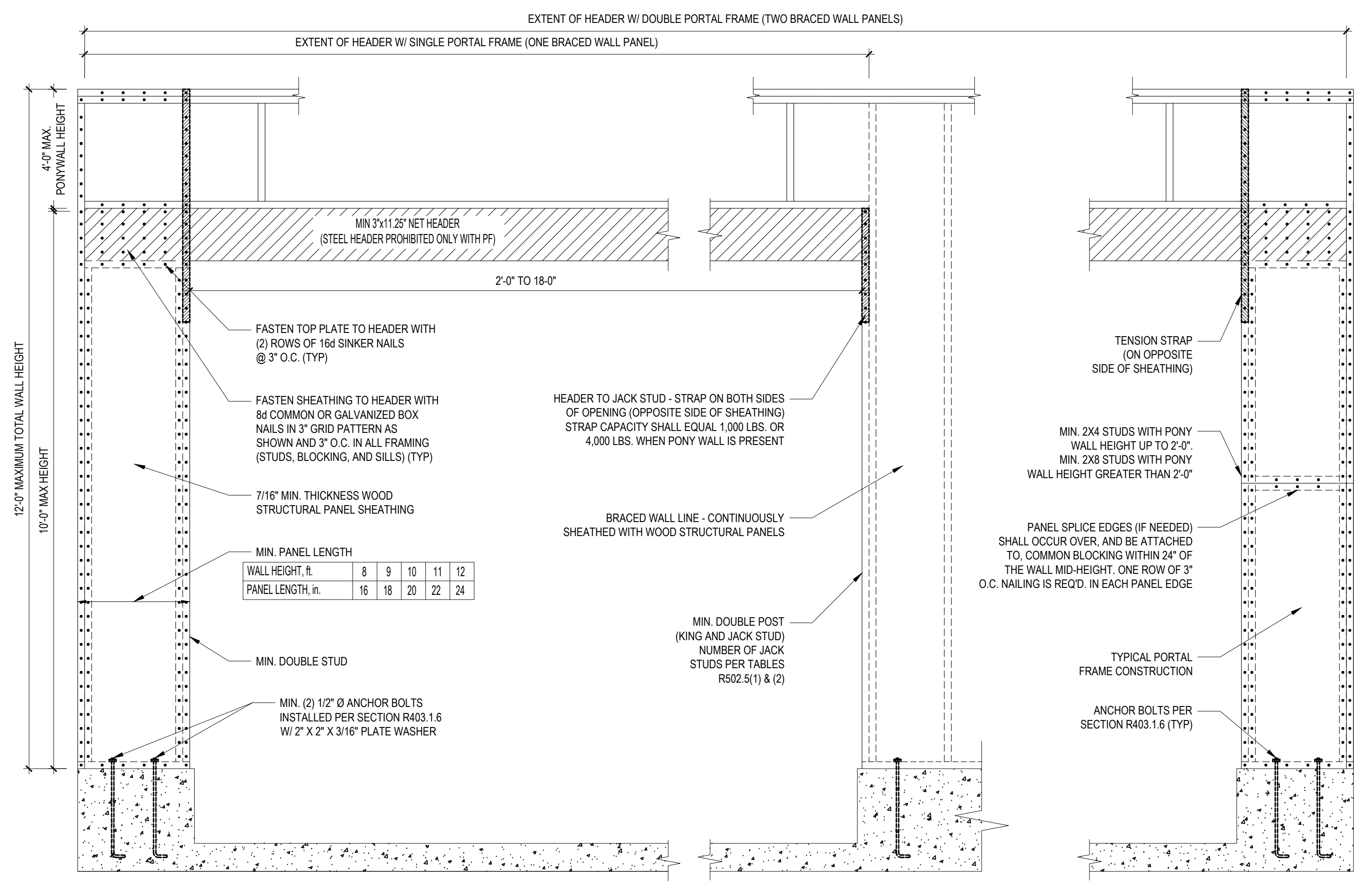
B1: TYPICAL EXTERIOR CORNER FRAMING FOR CONTINUOUS SHEATHING
NO SCALE

STRUCTURAL SHEATHING NOTES

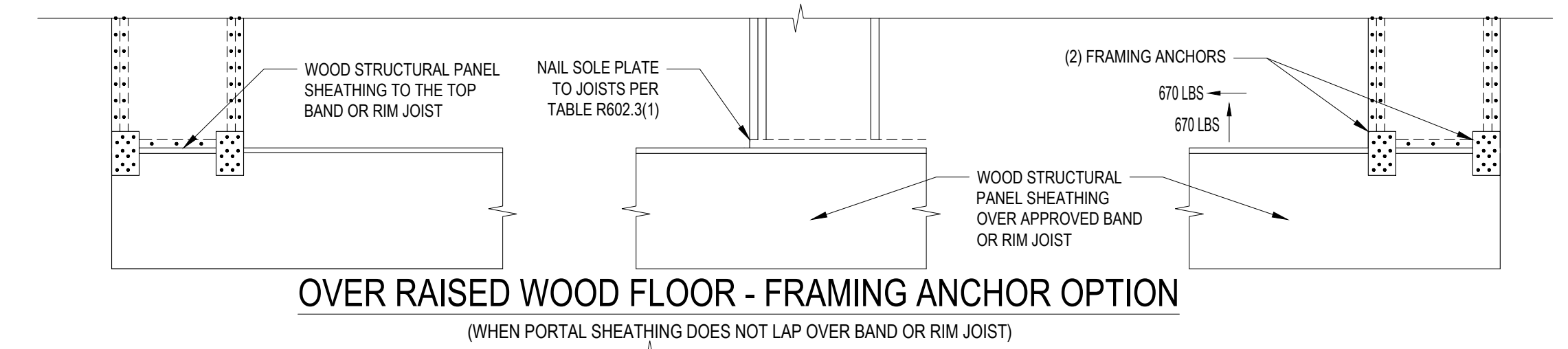
- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
- WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10.3 OF THE 2018 NCR.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCR.
- INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO).
- 12" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING).
- 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE W/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS.
- EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO).
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
 - 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT
 - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT
 - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- SHEATH INTERIOR AND EXTERIOR.
- FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3.14. IN LIEU OF A CORNER RETURN EITHER A MINIMUM 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- MINIMUM 800# HOLD-DOWN DEVICE.

REQUIRED BRACED WALL PANEL CONNECTIONS				
METHOD	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION	
			@ PANEL EDGES	@ INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAIL** @ 7" O.C.	5d COOLER NAIL** @ 7" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.

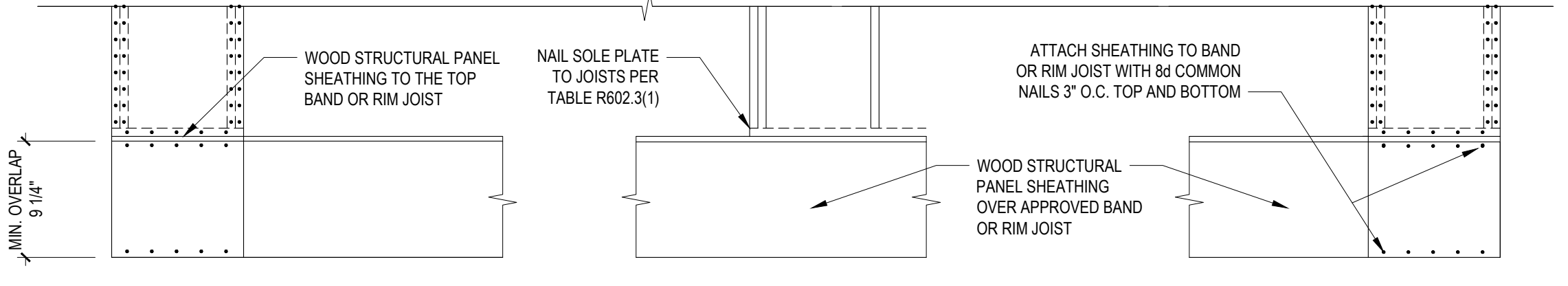
**OR EQUIVALENT PER TABLE R702.3.5
B3: BRACE WALL PANEL CONNECTIONS
NO SCALE



OVER CONCRETE OR MASONRY BLOCK FOUNDATION

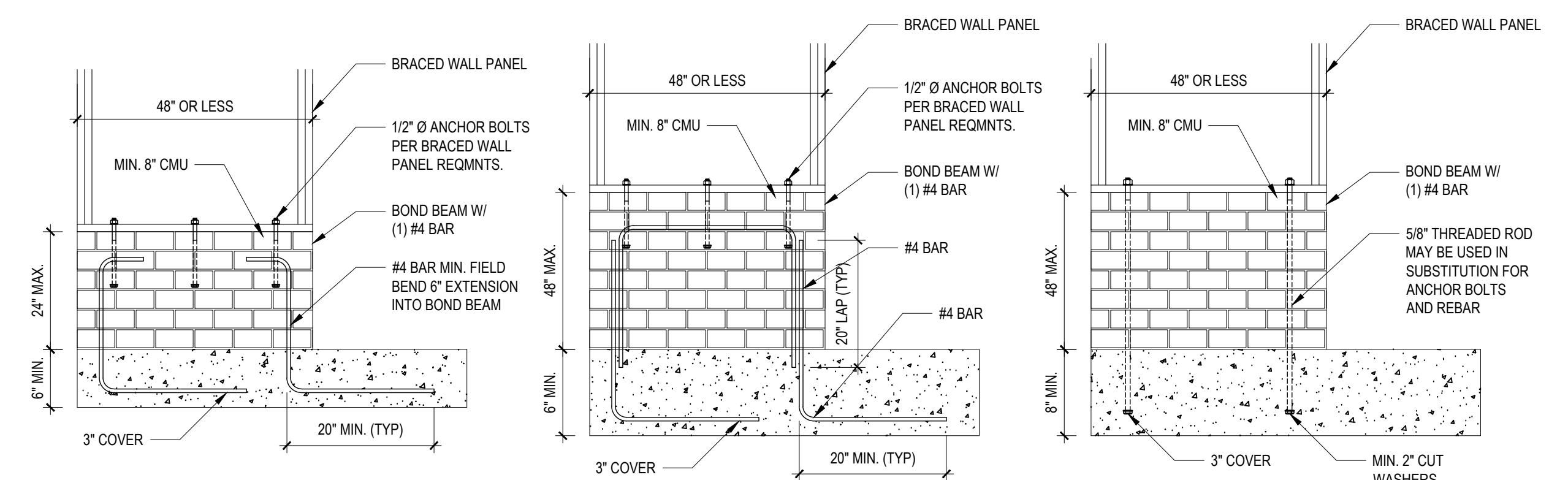


OVER RAISED WOOD FLOOR - FRAMING ANCHOR OPTION
(WHEN PORTAL SHEATHING DOES NOT LAP OVER BAND OR RIM JOIST)



OVER RAISED WOOD FLOOR - OVERLAP OPTION
(WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)

B2: METHOD CS-PF: CONTINUOUSLY SHEATHED PORTAL FRAME
FIGURE R602.10.1



B4: MASONRY STEM WALL SUPPORTING BRACED WALL PANELS
FIGURE R602.10.4.3 OF THE 2018 NCR
NOTE: GROUT BOND BEAMS AND ALL CELLS WHICH CONTAIN REBAR, THREADED RODS AND ANCHOR BOLTS

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