

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 2489104 Lamco Custom - Jackson

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E15026092 thru E15026109

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



October 28,2020

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



0-	0-0-8 32-8-0	1
0-	0-d-8 32-7-8	
Plate Offsets (X,Y)	[5:0-1-15,0-0-0], [6:0-1-12,0-2-4], [6:0-0-0,0-1-12], [11:0-2-0,Edge], [16:0-1-12,0-2-4], [16:0-0-0,0-1-12], [17:0-1-15,0-0-0], [20:0-0,0-1-12], [17:0-1-15,0-0-0], [20:0-0,0-1-12], [10:0-1,0-1,0-1,0-1,0-1,0-1,0-1,0-1,0-1,0-1,	5], [20:0-3-1
	Edael	

LOADING (psf)	0.0 SPACING-	- 2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
Snow (Pf/Pg) 15.4/2	0.0 Plate Grip	DOL 1.15	TC	0.13	Vert(LL)	-0.00	2-36	>999 >000	360 240	MT20	244/190
TCDL 1	0.0 Rep Stress	s Incr YES	WB	0.05	Horz(CT)	0.01	2-30	>333 n/a	240 n/a		
BCLL BCDL 1	0.0 Code IRC:	2015/TPI2014	Matrix-	S	Wind(LL)	0.01	2-36	>999	240	Weight: 202 lb	FT = 20%
LUMBER-				BR	ACING-						
TOP CHORD 2x4 S	P No.2			то	P CHORD	Structura	al wood	sheathin	g directly a	pplied or 6-0-0 oc purlins	
BOT CHORD 2x4 S	P No.2			BO	T CHORD	Rigid cei	ling dire	ectly appl	ied or 10-0	-0 oc bracing.	
OTHERS 2x4 S	P No.3					-	-			2	

WEDGE

Right: 2x4 SP No.2

REACTIONS. All bearings 32-8-0

- (lb) Max Horz 2=80(LC 13)
  - Max Uplift All uplift 100 lb or less at joint(s) 30, 31, 33, 34, 35, 36, 27, 26, 24, 23, 22, 21
  - Max Grav All reactions 250 lb or less at joint(s) 2, 2, 29, 30, 31, 33, 34, 35, 28, 27, 26, 24, 23, 22, 20 except 36=272(LC 34), 21=259(LC 35)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

# NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 16-6-0, Corner(3) 16-6-0 to 19-6-0, Exterior(2) 19-6-0 to 32-8-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 30, 31, 33, 34, 35, 36, 27, 26, 24, 23, 22, 21.



ENGINEERING BY REENCO A MiTek Affiliate 818 Soundside Road

Edenton, NC 27932

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A MITER 818 Soundside Road



0-Q-8 0-0-8	<u>32-11-8</u> 32-11-0						33-0-0	
Plate Offsets (X,Y) [5:0-1-15	,0-0-0], [6:0-1-12,0-2-4], [6:0-0-0,0-1-12	?], [11:0-2-0,Edge], [16:0-1-	12,0-2-4], [16:0-0	0-0,0-1-12], [17	:0-1-15,0-	0-0]		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.13 BC 0.09 WB 0.16 Matrix-S	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.00 20-22 -0.01 20-22 0.01 20 0.00 2-37	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 203 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2		BR. TOI	ACING- P CHORD S	Structural wood	sheathing	directly app	blied or 6-0-0 oc purlins	í.

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purli BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 32-11-0.

2x4 SP No 3

BOT CHORD 2x4 SP No 2

(lb) - Max Horz 2=-80(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 31, 32, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22 Max Grav All reactions 250 lb or less at joint(s) 2, 2, 30, 31, 32, 34, 35, 36, 29, 28, 27, 25, 24, 23, 20 except 37=272(LC 34), 22=272(LC 35)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

OTHERS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 16-6-0, Corner(3) 16-6-0 to 19-6-0, Exterior(2) 19-6-0 to 33-10-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 31, 32, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22.



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LOADING (psf TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	) 20.0 15.4/20.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.90 BC 1.00 WB 0.31 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.34 13-14 -0.60 13-14 0.09 10 0.06 14	l/defl L/ >999 360 >648 240 n/a n/ >999 240	d PLATES 00 MT20 0 a 0 Weight: 174 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3		BI TC BC	Racing- DP Chord DT Chord	Structural wood Rigid ceiling dire 2-2-0 oc bracing	sheathing dire ectly applied of 13-14.	ectly applied. r 10-0-0 oc bracing, Except:	

WEDGE Right: 2x4 SP No.2

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-2436/121, 3-5=-2320/150, 5-6=-1904/187, 6-7=-1888/195, 7-9=-2242/178, 9-10=-2344/147

 BOT CHORD
 2-16=-58/2092, 14-16=-24/1811, 13-14=0/1263, 11-13=-22/1786, 10-11=-80/1996

 WEBS
 6-13=-26/725, 7-13=-482/137, 7-11=-29/274, 6-14=-26/751, 5-14=-505/137, 5-16=-7/333

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-0 to 2-3-3, Interior(1) 2-3-3 to 16-6-0, Exterior(2) 16-6-0 to 19-9-3, Interior(1) 19-9-3 to 32-7-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Refer to girder(s) for truss to truss connections.

SEAL 16673 October 28,2020

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REACTIONS. (size) 10=Mechanical, 2=0-3-8 Max Horz 2=81(LC 13) Max Grav 10=1297(LC 2), 2=1368(LC 2)



0-0 <u>-8 5-4-2</u> 0-0-8 5-3-10	11-9-6 6-5-4	16-6-0 4-8-10	<u>21-2-10</u> 4-8-10	27-7-14 6-5-4	<u>32-11-8 33-0</u> -0 5-3-10 0-0-8
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.90 BC 1.00 WB 0.31 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) l/defl L/d 0.34 14-15 >999 360 0.61 14-15 >643 240 0.09 10 n/a n/a 0.06 15 >999 240	PLATES         GRIP           MT20         244/190           Weight: 175 lb         FT = 20%
LUMBER-		F	RACING.		

BOT CHORD

Structural wood sheathing directly applied.

1-4-12 oc bracing: 14-15.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

# REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=-80(LC 14)

Max Grav 2=1370(LC 2), 10=1370(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2458/120, 3-5=-2342/149, 5-6=-1924/185, 6-7=-1924/185, 7-9=-2342/149, 9-10=-2458/120
- 2-17=-45/2112, 15-17=-5/1829, 14-15=0/1281, 12-14=-13/1829, 10-12=-49/2112 BOT CHORD
- 6-14=-25/752, 7-14=-505/136, 7-12=-8/335, 6-15=-25/752, 5-15=-505/136, 5-17=-8/335 WEBS

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 16-6-0, Exterior(2) 16-6-0 to 19-6-0, Interior(1) 19-6-0 to 33-10-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members, with BCDL = 10.0psf.



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Job	Truss	Truss Type	Qty	Ply	Lamco Custom - Jackson	-
					I	E15026100
2489104	C3G	Common Girder	1	2		
				3	Job Reference (optional)	
Builders FirstSource (Alberm	arle), Albemarle, NC - 28	3001,		240 s Ma	9 2020 MiTek Industries, Inc. Tue Oct 27 15:44:50 2020	Page 2
		ID:YXQEN	ID:YXQEN8tS3WJK82CD2Om5agyQqML-H5JoJuYKvLyauz1ur6kMN2qRh_?vuYp9kfBXfOyP/			

#### LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 8=-1212(B) 6=-1212(B) 10=-1212(B) 11=-1212(B) 12=-1212(B) 13=-1212(B) 14=-1212(B) 15=-1212(B) 16=-1128(B) 17=-1128(B)

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	L		6-0-0				_	
	1		5-11-8				1	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.19 BC 0.13 WB 0.07	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc -0.01 2- -0.02 2- 0.00	c) l/defl 6 >999 6 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/1F12014	Mault-F	VVIIIU(LL)	0.02 2-	0 >999	240	Weight. 24 lb	FT = 2070
LUMBER-		BF	ACING-					

BOT CHORD

# LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.2
OTHERS	2x4 SP No 3

REACTIONS. All bearings 5-11-8.

Max Horz 2=52(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 5, 2, 6 Max Grav All reactions 250 lb or less at joint(s) 5, 2, 2 except 6=317(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 5-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 8) will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2, 6.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only design parameters and READ NOTES ON TIPS ON MILE OPEN MILE REFERENCE PAGE mil-14/3 (4) and ( fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS//TPI1 Qu** Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.62 BC 0.54 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.06 -0.13 0.00 0.10	(loc) 2-4 2-4 2-4	l/defl >999 >532 n/a >682	L/d 360 240 n/a 240	PLATES MT20 Weight: 22 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-		BF	ACING-						

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=52(LC 12) Max Uplift 2=-56(LC 12), 4=-49(LC 12)

Max Grav 2=294(LC 2), 4=224(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

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#### NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-10-4 zone; cantilever left and right exposed ; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. ARXING - Verify design parameters and KEAD NOTES ON THIS AND INCLODED WITEK REFERENCE PAGE MIT-14's rev. or 19/20/20 DEFORE 052. Design valif for use only with MiTeKe connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** 
 Satisfies
 Ansi/TPI Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.10 BC 0.14 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.00 -0.00 0.00 0.00	(loc) 2 2-4 2	l/defl >999 >999 n/a ****	L/d 360 240 n/a 240	PLATES MT20 Weight: 8 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-		BR	ACING-						

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=23(LC 12) Max Uplift 2=-24(LC 12), 4=-1(LC 16)

Max Grav 2=152(LC 2), 4=56(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

818 Soundside Road

Edenton, NC 27932

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Matrix-S

BRACING-

TOP CHORD

BOT CHORD

TOP CHORD
BOT CHORD
OTHERS

LUMBER-

BCDL

BOT CHORD 2x4 SP No.2 2x4 SP No.3

REACTIONS. All bearings 16-6-12.

10.0

2x4 SP No.2

Max Horz 1=-88(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 9, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 8 except 9=375(LC 25), 6=375(LC 26)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2015/TPI2014

WEBS 2-9=-281/125, 4-6=-280/125

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 8-3-12, Exterior(2) 8-3-12 to 11-3-12, Interior(1) 11-3-12 to 16-1-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 6.



Weight: 67 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

FT = 20%

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only design parameters and READ NOTES ON TIPS ON MILE OPEN MILE REFERENCE PAGE mil-14/3 (4) and ( fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qu** Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





(lb) - Max Horz 1=-71(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1, 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=265(LC 2), 8=307(LC 25), 6=307(LC 26)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 6-9-12, Exterior(2) 6-9-12 to 9-9-12, Interior(1) 9-9-12
- to 13-1-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8, 6.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



![](_page_17_Figure_0.jpeg)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 5-3-12, Exterior(2) 5-3-12 to 8-3-12, Interior(1) 8-3-12 to 10-1-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_17_Picture_12.jpeg)

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only design parameters and READ NOTES ON TIRS AND INCLODED MITER REFERENCE PAGE mit-143 a few, 3192/2020 BEFORE DSE. Design valid for use only with MITeK go connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** 
 Satisfies
 Ansi/TPI Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_17_Picture_14.jpeg)

![](_page_18_Figure_0.jpeg)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 3-9-12, Exterior(2) 3-9-12 to 6-9-12, Interior(1) 6-9-12 to 7-1-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_18_Picture_10.jpeg)

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![](_page_18_Picture_12.jpeg)

![](_page_19_Figure_0.jpeg)

0-0 <sub>7</sub> 6	4-7-8						
0-0-6	4-7-2	1					
Plate Offsets (X,Y) [2:0-2-0,Edge]							

LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.06 BC 0.16 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 13 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2		BR TO	ACING- P CHORD S	Structural wood	l sheathin	q directly ap	plied or 4-7-8 oc purlir	IS.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=4-6-12, 3=4-6-12 Max Horz 1=-21(LC 10) Max Grav 1=146(LC 2), 3=146(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

![](_page_19_Figure_14.jpeg)

👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED WITER REFERENCE PAGE MIL-14's rev. or 19/20/20 Der Vice USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_19_Picture_16.jpeg)

![](_page_20_Figure_0.jpeg)