

RE: J0224-1259 Lot 16 Williams Farms Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Customer: Project Name: J0224-1259 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

21

22

23

163945408

163945409

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 23 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	
21	163945407	PB6	

3/4/2024 PB7 3/4/2024 PB8 3/4/2024

Date

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





⊢—	<u>4-3-8</u> <u>11-1-10</u> <u>17-</u> <u>4-3-8</u> <u>6-10-2</u> <u>6-</u>	11-13 24-11- 10-2 6-11-1	-8 <u>27-7-12 31-</u>	- <u>11-3 35-9-2 39-9-2</u>	<u>2 41-11-0 47-11-0</u> 2-1-14 6-0-0	
Plate Offsets (X,Y)	[2:0-3-6,Edge], [6:0-5-4,0-3-0], [13:0-3-	0,0-3-8], [19:0-5-0,0-3-0],	[39:0-0-9,0-1-10]	-5-0 -5-5-140-0	2-1-14 0-0-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.43 BC 0.56 WB 0.37 Matrix-S	DEFL. in Vert(LL) -0.20 Vert(CT) -0.33 Horz(CT) 0.10 Wind(LL) 0.12	n (loc) l/defl L/d 43-44 >999 360 43-44 >999 240 35 n/a n/a 46-47 >999 240	PLATES GRIP MT20 244/190 Weight: 440 lb FT = 20%	
LUMBER- TOP CHORD 2x6 S BOT CHORD 2x6 S WEBS 2x4 S OTHERS 2x4 S	SP No.1 SP No.1 SP No.2 SP No.2		BRACING- TOP CHORD BOT CHORD WEBS JOINTS	Structural wood sheathing of except 2-0-0 oc purlins (5-1-2 max Rigid ceiling directly appliec 8-4-14 oc bracing: 2-47,46- 1 Row at midpt 1 Brace at Jt(s): 31, 26, 21,	directly applied or 3-9-10 oc purlins,): 6-13, 7-39. I or 10-0-0 oc bracing, Except: 47. 3-46, 4-44, 6-43, 19-43 18, 12, 9	
REACTIONS. All (lb) - Max Max Max	bearings 6-3-8 except (jt=length) 2=0-3-8, Horz 2=345(LC 11) Uplift All uplift 100 lb or less at joint(s) e 38=-289(LC 20), 39=-746(LC 13) Grav All reactions 250 lb or less at joint 35=370(LC 22), 39=1782(LC 20), 4	40=0-3-8. xcept 2=-330(LC 12), 37= (s) 37, 38 except 2=1854(0=348(LC 2)	-147(LC 13), (LC 2),	. 21000 at 0(0), 0 () 20, 2 ()		
FORCES. (lb) - Ma TOP CHORD 2-3 8-1 20- 32 11- 19- 20- 20- 20-	x. Comp./Max. Ten All forces 250 (lb) oi =-4065/1009, 3-4=-3064/795, 4-6=-2391/ 0=-306/142, 10-13=-307/141, 13-15=-348 22=-326/0, 22-23=-350/0, 23-27=-373/0, 1 -33=-426/258, 33-34=-441/153, 34-35=-50 12=-1921/634, 12-14=-1877/617, 14-18=- 21=-2234/763, 21-24=-2242/765, 24-26=- 31=-2311/831, 31-39=-2283/897	less except when shown 727, 6-7=-1885/667, 7-8= //137, 15-17=-350/102, 17 27-28=-397/0, 28-30=-419 09/214, 7-9=-1914/634, 9- 1903/643, 18-19=-1914/6 2264/789, 26-29=-2281/8	-320/158, -20=-375/86, 9/0, 30-32=-480/0, -11=-1893/616, 49, 108,			
29-31=-2311/831, 31-39=-2283/827 BOT CHORD 2-47=-868/3708, 46-47=-875/3705, 44-46=-463/2557, 43-44=-295/1918, 41-43=-319/2144, 40-41=-319/2144, 39-40=-319/2144, 38-39=-188/422, 37-38=-188/422 WEBS 3-46=-1244/432, 4-46=-41/554, 4-44=-944/394, 6-44=-159/1047, 7-43=-34/668, 32-39=-643/379, 19-41=0/309, 19-43=-487/234, 19-20=-261/244						
 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) Provide adequate drainage to prevent water ponding. 5) All plates are 2x4 MT20 unless otherwise indicated. 6) Gable studs spaced at 1-4-0 oc. 7) This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Continued on page 2 						
WARNING - Veri Design valid for us a truss system. Be building design. B is always required	fy design parameters and READ NOTES ON THIS ANI e only with MITek® connectors. This design is based fore use, the building designer must verify the applic racing indicated is to prevent buckling of individual tri for stability and to prevent collapse with possible per	D INCLUDED MITEK REFERENC only upon parameters shown, an ability of design parameters and p uss web and/or chord members c sonal injury and property damage	E PAGE MII-7473 rev. 1/2/2023 nd is for an individual building c properly incorporate this design only. Additional temporary and a. For general guidance regard	BEFORE USE. component, not i nito the overall permanent bracing ling the	TRENGINEERING BY A MITEK Atfiliate	

Is a may required and the most to produce a compact many possible provide and interview and the product of a second and the product of the second and the product of the second and the se and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Lot 16 Williams Farms	
						63945387
J0224-1259	A1-GE	GABLE	1	1		
					Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,		8	3.430 s Jar	6 2022 MiTek Industries, Inc. Thu Feb 29 10:20:15 2024 F	Page 2
		ID:2GNsY	062BI49K	gBFP3SIm	nayOXVO-RfC?PsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4	zJC?f

NOTES-

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 330 lb uplift at joint 2, 147 lb uplift at joint 37, 289 lb uplift at joint 38 and 746 lb uplift at joint 39.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





F	5-7-10 10-11-13	20-11-4	24-11-3	28-9-4	34-9-4	40-11-0	
Plate Offsets (X,Y)	[3:0-5-4,0-2-12], [5:0-5-4,0-3-5], [9:Edge	e,0-1-11]	4-0-0	3-10-0	0-0-0	0-1-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.57 BC 0.52 WB 0.76 Matrix-S	DEFL. in Vert(LL) -0.16 Vert(CT) -0.27 Horz(CT) 0.02 Wind(LL) 0.08	(loc) l/defl 16-17 >999 16-17 >999 11 n/a 14 >999	L/d 360 240 n/a 240	PLATES MT20 M18AHS Weight: 338 lb	GRIP 244/190 186/179 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP 1-17: 2	P No.1 P No.1 P No.2 *Except* No.2 *Except* x6 SP No.1		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood except end verti Rigid ceiling dire 1 Row at midpt	sheathing dire cals, and 2-0-0 ctly applied or 3-1	ctly applied or 6-0-0 c) oc purlins (6-0-0 ma: 6-0-0 oc bracing. 14, 4-14, 2-17, 6-12	oc purlins, x.): 3-5.
REACTIONS. (size Max H Max U Max G	e) 11=0-3-8, 17=0-3-8, 9=0-3-0 orz 17=-262(LC 8) plift 11=-97(LC 13), 17=-28(LC 12), 9=- rav 11=2575(LC 2), 17=1473(LC 2), 9=	599(LC 25) 56(LC 12)					
FORCES. (lb) - Max. TOP CHORD 2-3=- 8-9=- BOT CHORD 16-17 WEBS 2-16= 8-11=	Comp./Max. Ten All forces 250 (lb) or 1313/382, 3-4=-960/369, 4-5=-959/369, 394/1234 7=-151/1000, 14-16=-88/1105, 12-14=0/ 68/348, 3-16=-4/463, 4-14=-306/196, 6 2434/668, 2-17=-1352/275, 6-12=-717	less except when shown. 5-6=-1058/361, 6-8=-726/ 491, 11-12=-931/376, 9-11 -14=-162/795, 8-12=-278/ 270	166, =-931/376 1680,				
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V and C-C Exterior(2) Exterior(2) 24-11-3 t reactions shown; Lu 3) Provide adequate dr 4) All plates are MT20 5) This truss has been 6) * This truss has been will fit between the b 7) Provide mechanical 17 and 599 lb uplift a 8) Graphical purlin repr	e loads have been considered for this de /ult=130mph Vasd=103mph; TCDL=6.0p 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 10-1 o 31-114, Interior(1) 31-1-14 to 41-8-7 : mber DOL=1.60 plate grip DOL=1.60 'ainage to prevent water ponding. plates unless otherwise indicated. designed for a 10.0 psf bottom chord liv n designed for a live load of 30.0psf on t oottom chord and any other members, wi connection (by others) of truss to bearin at joint 9. resentation does not depict the size or th	sign. sf; BCDL=6.0psf; h=15ft; C 1-13, Exterior(2) 10-11-13 t zone; porch right exposed; e load nonconcurrent with a he bottom chord in all area th BCDL = 10.0psf. g plate capable of withstan we orientation of the purlin a	Cat. II; Exp C; Enclosed to 17-2-7, Interior(1) 17 C-C for members and for any other live loads. s where a rectangle 3-6 ding 97 lb uplift at joint along the top and/or bot	; MWFRS (envelo -2-7 to 24-11-3, proces & MWFRS f 6-0 tall by 2-0-0 w 11, 28 lb uplift at tom chord.	pe) or de ioint	NUMPTH C	AROUNT



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A Mi Tek 818 Soundside Road Edenton, NC 27932



L	5-7-10	10-11-13	20-11-4	24-11-3 28	-9-4	34-9-4	40-11-0	
	5-7-10	5-4-2	9-11-7	4-0-0 3-	10-1 '	6-0-0	6-1-12	1
Plate Offsets (X,Y)-	[3:0-5-4,0-2-12	2], [5:0-4-0,0-2-13], [12	:0-3-0,0-4-4]					
LOADING (psf)	SPACIN	NG- 2-0-0	CSI.	DEFL. in (loc	c) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate G	rip DOL 1.15	TC 0.45	Vert(LL) -0.16 16-1	7 >999	360	MT20	244/190
TCDL 10.0	Lumber	DOL 1.15	BC 0.51	Vert(CT) -0.25 16-1	7 >999	240		
BCLL 0.0 *	Rep Str	ess Incr YES	WB 0.68	Horz(CT) 0.01 1	2 n/a	n/a		
BCDL 10.0	Code IF	RC2015/TPI2014	Matrix-S	Wind(LL) 0.04 14-1	6 >999	240	Weight: 338 lb	FT = 20%
							0	

LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sheathing	g directly applied or 6-0-0 oc purlins,
BOT CHORD	2x6 SP No.1		except end verticals, and	2-0-0 oc purlins (6-0-0 max.): 3-5.
WEBS	2x4 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly appli	ed or 6-0-0 oc bracing.
	1-17: 2x6 SP No.1	WEBS	1 Row at midpt	3-14, 4-14, 6-12, 2-17

REACTIONS. (size) 12=0-3-8, 17=0-3-8, 9=0-3-0 Max Horz 17=-262(LC 8) Max Uplift 12=-178(LC 8), 17=-21(LC 12), 9=-118(LC 25) Max Grav 12=2275(LC 2), 17=1187(LC 19), 9=274(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-967/282, 3-4=-440/222, 4-5=-439/223, 5-6=-531/216, 6-8=-216/641, 8-9=-132/439

BOT CHORD 16-17=-148/809, 14-16=-84/843, 12-14=-588/419, 11-12=-313/113, 9-11=-313/113

WEBS 3-16=-7/544, 3-14=-541/110, 4-14=-464/250, 6-14=-294/1359, 6-12=-1704/542,

2-17=-1001/179, 8-12=-549/464

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 10-11-13, Exterior(2) 10-11-13 to 17-2-7, Interior(1) 17-2-7 to 24-11-3, Exterior(2) 24-11-3 to 31-1-14, Interior(1) 31-1-14 to 41-8-7 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 12, 21 lb uplift at joint 17 and 118 lb uplift at joint 9.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 43-11-0.

- (lb) Max Horz 2=346(LC 11)
 - Max Uplift All uplift 100 lb or less at joint(s) 2, 54, 55, 56, 57, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 39, 73, 50, 49, 48, 47, 46, 45, 44, 43, 42 except 41=-101(LC 13)
 - Max Grav All reactions 250 lb or less at joint(s) 2, 53, 54, 55, 56, 57, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 39, 73, 52, 50, 49, 48, 47, 46, 45, 44, 43, 42, 41

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-312/200, 3-4=-265/211, 13-14=-184/257, 14-15=-227/288, 15-16=-266/317, 16-17=-272/320, 17-18=-255/305, 18-19=-255/305, 19-20=-255/305, 20-21=-255/305,
 - 21-22=-255/305, 22-23=-255/305, 23-24=-255/305, 24-25=-255/305, 25-26=-255/305, 26-27=-272/320, 27-28=-266/313, 28-29=-227/265, 38-39=-255/179

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 54, 55, 56, 57, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 39, 73, 50, 49, 48, 47, 46, 45, 44, 43, 42 except (jt=lb) 41=101. OdhtGrashinabagelig representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 16 Williams Farms	
						163945390
J0224-1259	B1-GE	GABLE	1	1		
					Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,			8.430 s Jar	6 2022 MiTek Industries, Inc. Thu Feb 29 10:20:22 2024	Page 2

ID:2GNsYO62BI49KgBFP3SImayOXVO-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

NOTES-

12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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F	4-3-8	11-1-10	17-11-13	27-11-4		35-9)-2	43-10-7	43-11-0
	4-3-8	6-10-2	6-10-2	9-11-7		7-9-	14 '	8-1-5	0-0-10
Plate Offsets (X, Y)	[6:0-5-4,0-2-12]			1					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip Lumber DC Rep Stress Code IRC	- 2-0-0 DOL 1.15 DL 1.15 6 Incr YES 2015/TPI2014	CSI. TC 0.59 BC 0.55 WB 0.37 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (lc -0.20 14- -0.34 14- 0.10 0.08 17-	bc) I/defl 15 >999 15 >999 10 n/a 18 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 335 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2X6 S BOT CHORD 2X6 S WEBS 2X4 S	P No.1 P No.1 P No.2			BRACING- TOP CHORE BOT CHORE WEBS	D Stru exc 2-0 D Rig 1 R	uctural wood cept -0 oc purlins id ceiling dire cow at midpt	sheathing dir (4-10-3 max.) ectly applied c 3	ectly applied or 3-9-12): 6-7. or 9-9-2 oc bracing. -17, 4-15, 6-14, 9-14	oc purlins,
REACTIONS. (sia Max Max Max	ze) 2=0-3-8, 10= Horz 2=272(LC 11 Uplift 2=-92(LC 12) Grav 2=1848(LC 2	0-2-5) , 10=-77(LC 13)), 10=1951(LC 20)						, ,,,,,,,	
FORCES. (lb) - Max TOP CHORD 2-3= 9-10	. Comp./Max. Ten. =-4045/768, 3-4=-3)=-2903/555	- All forces 250 (lb) or 046/625, 4-6=-2376/59	less except when shown 98, 6-7=-1810/553, 7-9=-2	ı. 2279/574,					
BOT CHORD 2-18 10-	2-18=-641/3816, 17-18=-648/3813, 15-17=-338/2617, 14-15=-128/1944, 12-14=-321/2304, 10-12=-321/2304								
WEBS 3-17 7-14	3-17=-1242/328, 4-17=-12/557, 4-15=-930/286, 6-15=-88/1044, 6-14=-266/145, 7-14=-52/804, 9-14=-766/246, 9-12=0/414								
NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-10; and C-C Exterior(2)	ve loads have been Vult=130mph Vasd	considered for this de	sign. osf; BCDL=6.0psf; h=15ft 1-13_Exterior(2) 17-11-13	; Cat. II; Exp C; Encl	losed; MV	VFRS (envelo	ope)		

Exterior(2) 27-11-4 to 32-4-0, Interior(1) 32-4-0 to 44-8-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 10.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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(lb) - Max Horz 2=524(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 24, 25, 2, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42 except 43=-146(LC 12) Max Grav All reactions 250 lb or less at joint(s) 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43 except 2=342(LC 12)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-616/494, 3-4=-508/401, 4-5=-457/362, 5-6=-408/323, 6-7=-358/284, 7-8=-308/245, 8-9=-257/206

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 25, 2, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42 except (jt=lb) 43=146.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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Edenton, NC 27932



2) Provide adequate drainage to prevent water ponding.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 9=124.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=124.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=183.



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5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) Ceiling dead load (10.0 psf) on member(s). 2-3, 7-8, 3-15, 7-15; Wall dead load (5.0psf) on member(s).2-13, 8-11
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Attic room checked for L/360 deflection.



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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-7 to 3-7-6, Interior(1) 3-7-6 to 9-3-0, Exterior(2) 9-3-0 to 13-7-13, Interior(1) 13-7-13 to 19-3-7 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=115, 2=115.



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818 Soundside Road

Edenton, NC 27932



Wind: ASCE 7-10; Vuit=130mph Vasd=103mph; 1CDL=6.0pst; BCDL=6.0pst; h=15t; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-7 to 3-7-6, Interior(1) 3-7-6 to 9-3-0, Exterior(2) 9-3-0 to 13-7-13, Interior(1) 13-7-13 to 18-4-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=110, 2=115.



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() * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=111, 3=111.

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9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11, 2, 10, 16, 17, 18, 14, 13, 12.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 8) will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 2, 8, 13, 14, 11.10.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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and C-C Exterior(2) 0-3-2 to 4-7-15, Interior(1) 4-7-15 to 4-11-11, Exterior(2) 4-11-11 to 9-2-5, Interior(1) 9-2-5 to 9-8-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint 1, 244 lb uplift at joint 5, 206 lb uplift at joint 2 and 192 lb uplift at joint 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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LOADING (psf) SPACING- Plate Grip DOL 2-0-0 CSI. DEI TCLL 20.0 Plate Grip DOL 1.15 TC 0.05 Ver TCDL 10.0 Lumber DOL 1.15 BC 0.02 Ver BCLL 0.0 * Rep Stress Incr YES WB 0.03 Hor BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Matrix-S Hor	EFL. in (loc) l/defl L/d PLATES GRIP ert(LL) n/a - n/a 999 MT20 244/190 ert(CT) n/a - n/a 999 Weight: 55 lb FT = 20%
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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.1

 BOT CHORD
 2x4 SP No.1

 WEBS
 2x4 SP No.2

 OTHERS
 2x4 SP No.2

2x4 SP No.2

REACTIONS. All bearings 9-11-11.

(lb) - Max Horz 1=159(LC 12)

 Max Uplift
 All uplift 100 lb or less at joint(s) 1, 9, 2, 12, 13, 14, 10

 Max Grav
 All reactions 250 lb or less at joint(s) 1, 9, 2, 11, 12, 13, 14, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 1-4-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 1, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 2, 12, 13, 14, 10.

11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Max Uplift All uplift 100 lb or less at joint(s) 5 except 1=-109(LC 19), 2=-167(LC 12), 4=-138(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 2, 4, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 1, 5, 2, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 1=109, 2=167, 4=138.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

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