PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE**

MEAN ROOF HEIGHT: 19'-8	3"	HEIGHT TO R	RIDGE: 27'-4"							
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A							
FENESTRATION U-FACTOR	0.35	0.35	0.35							
SKYLIGHT U-FACTOR	0.55	0.55	0.55							
GLAZED FENESTRATION SHGC	0.30	0.30	0.30							
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci							
WALL R-VALUE	15	15	19							
FLOOR R-VALUE	19	19	30							
* BASEMENT WALL R-VALUE	5/13	10/15	10/15							
** SLAB R-VALUE	0	10	10							
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19							
* "10/13" MEANS D-10 SHEATHING INS	* "10/12" MEANS D-10 SHEATHING INSULATION OD D-12 CAVITY INSULATION									

10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATIO ** INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL

DESIGNED FOR WIND SPEED OF 120 MPH, 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "B"											
COMPONENT & CLADDING DESIGNED FOR THE FOLLOWING LOADS											
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'			
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8			
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2			
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2			
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9			
ZONE 5 15.5 -20.0 16.3 -21.0 16.9 -21.8 17.4 -22.4											
DESIGNED FOR WIND SPEED OF 130 MPH, 3 SECOND GUST (101 FASTEST MILE) EXPOSURE "B"											

COMPONENT	. & CLA	DDING	DESIG	NED FC	DR THE	FOLLO	WING	_OADS
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35' - 1"	TO 40'	40'-1"	TO 45'
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6	18.7	-20.2
ZONE 2	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3
ZONE 5	18.2	-24.0	19.1	-25.2	19.8	-26.2	20.4	-26.9

ROOF VENTILATION

SECTION R806

R806.1 Ventilation required. Enclosed *attics* and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than 1/4 inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware cloth, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802 7

R806.2 Minimum area. The total net free ventilating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross-ventilation area may be reduced to 1/300 when a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling. Exceptions:

1. Enclosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only. 2. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,619 SQ.FT.

NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 17.46 SQ.FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 8.73 SQ.FT.

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. *Guards* shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or *grade* below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a *guard*.

R312.2 Height. Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads. Exceptions:

1. *Guards* on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the treads.

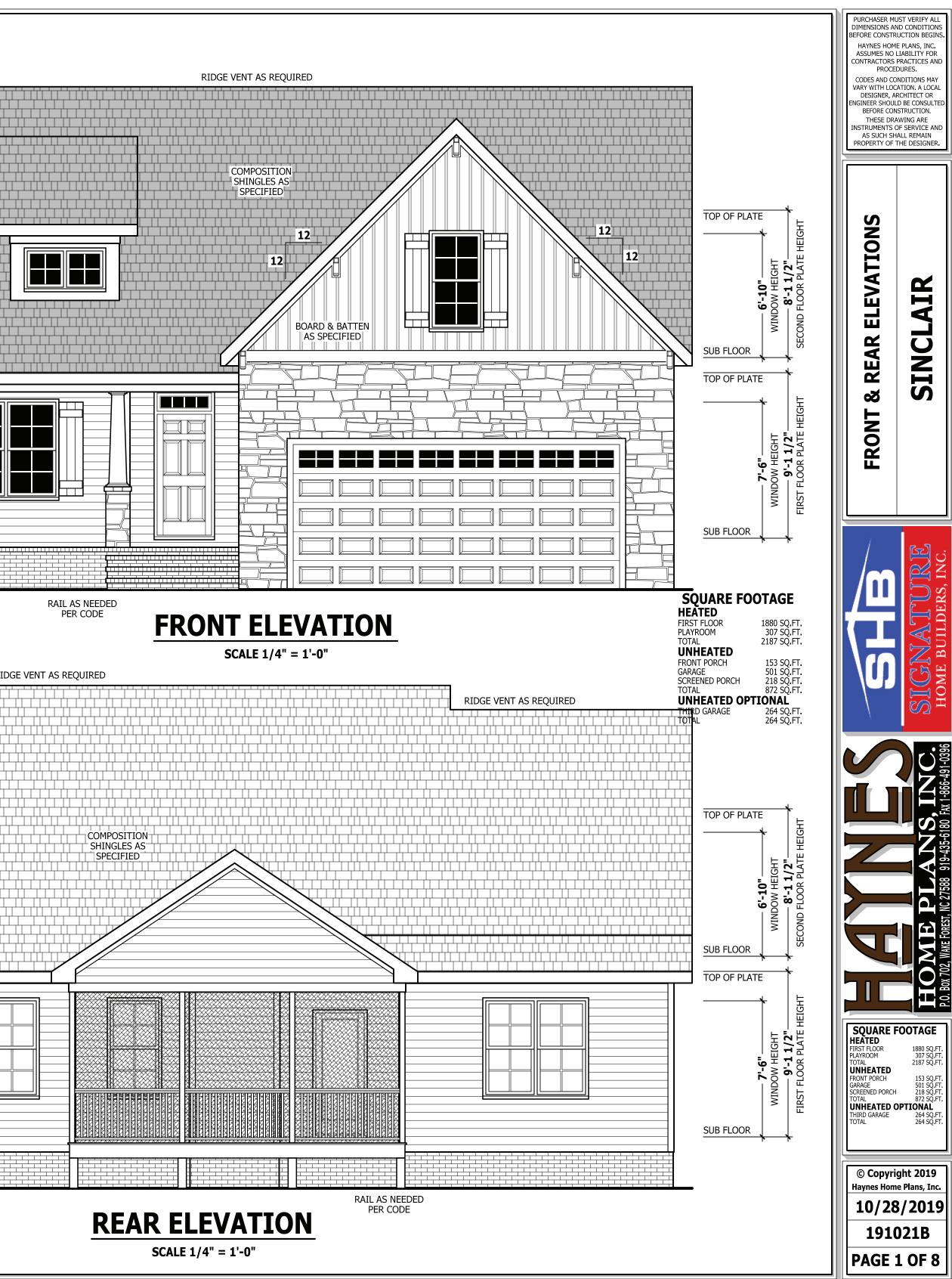
2. Where the top of the *guard* also serves as a handrail on the open sides of stairs, the top of the *guard* shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312.3 Opening limitations. Required *guards* shall not have openings from the walking surface to the required *guard* height which allow passage of a sphere 4 inches (102 mm)in diameter. Exceptions:

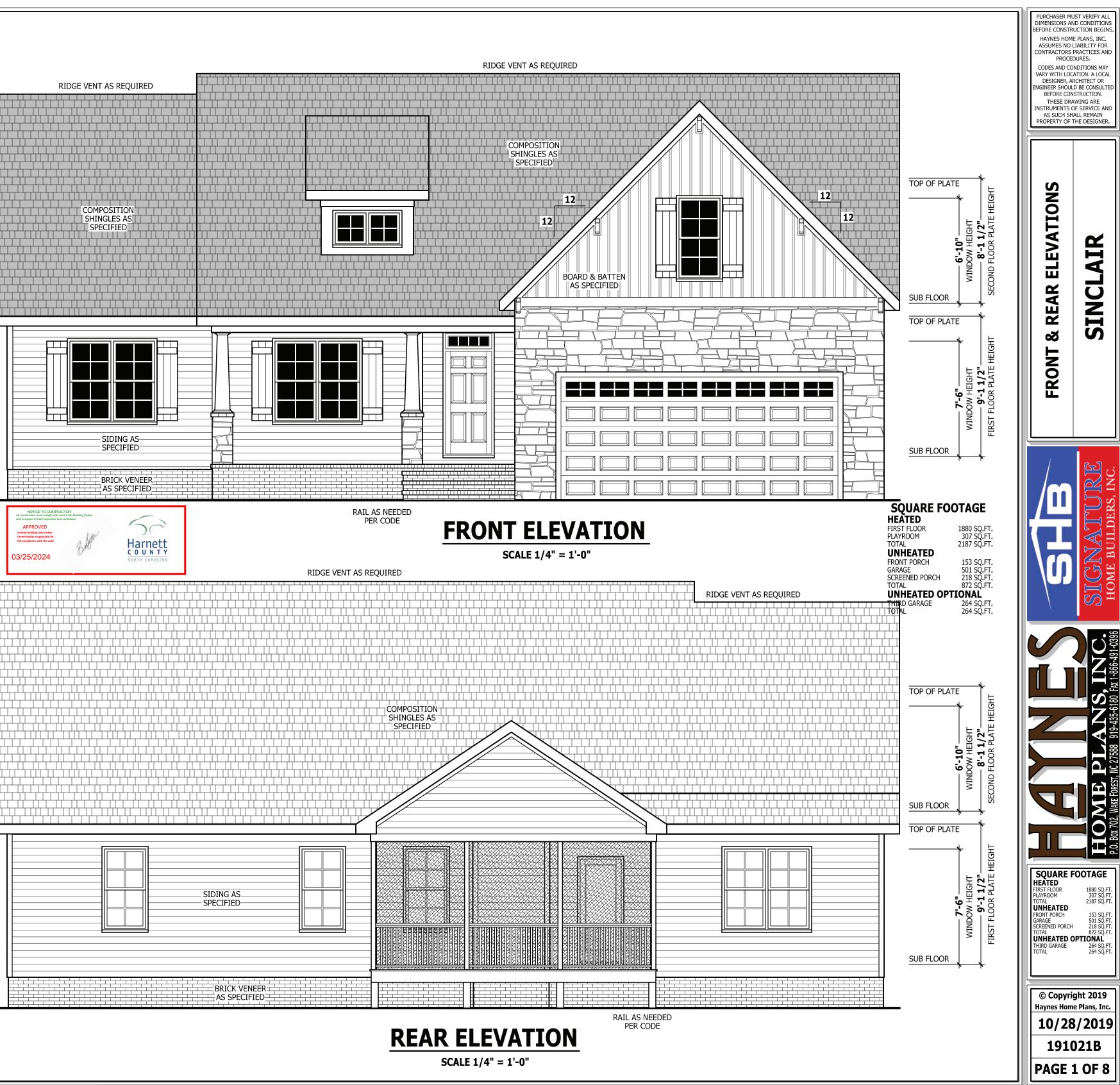
1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a *guard*, shall not allow passage of a sphere 6 inches (153 mm) in diameter.

2. *Guards* on the open sides of stairs shall not have openings which allow passage of a sphere 43/8 inches (111 mm) in diameter.

COMPOSITION SHINGLES AS SPECIFIED



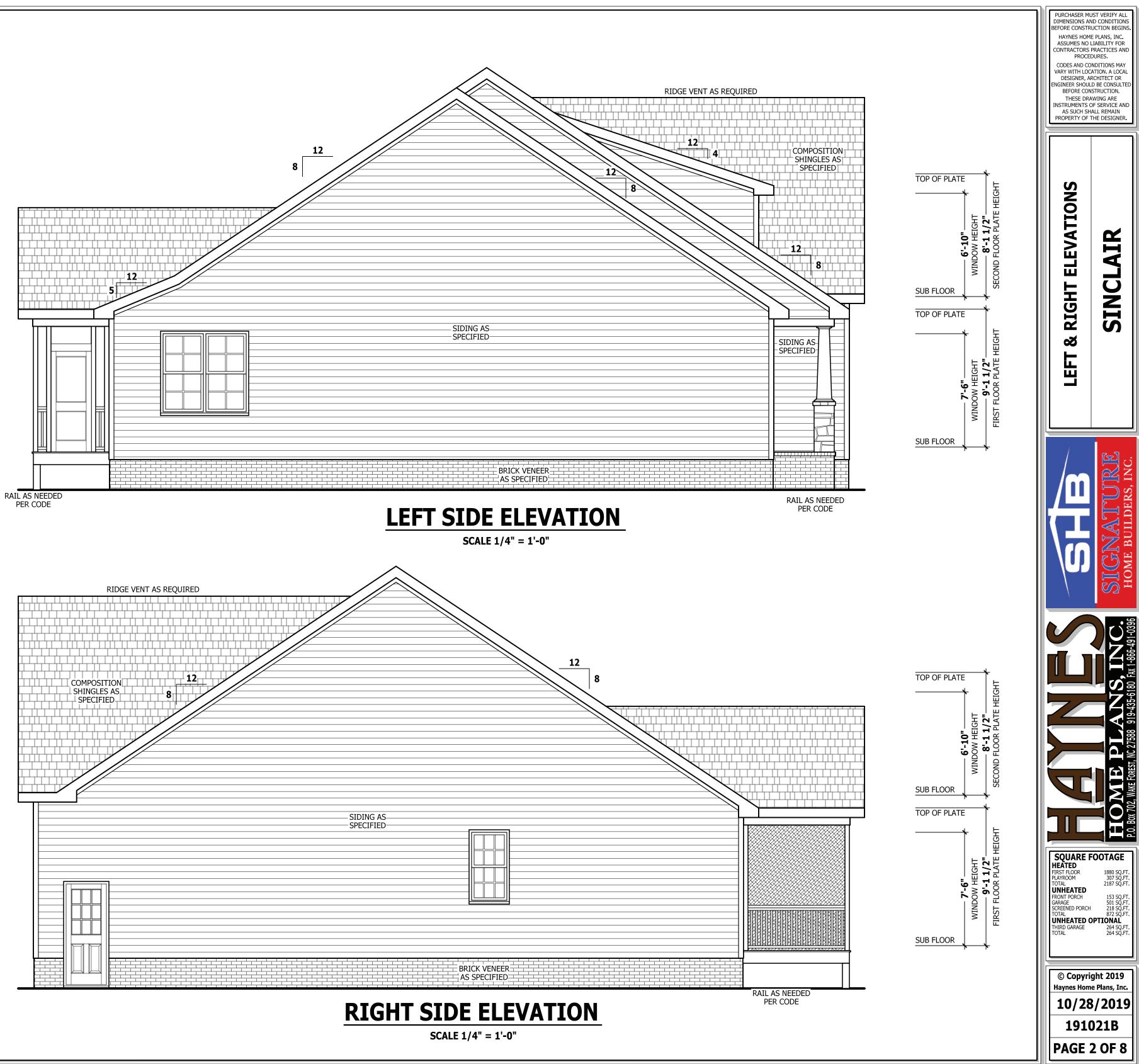


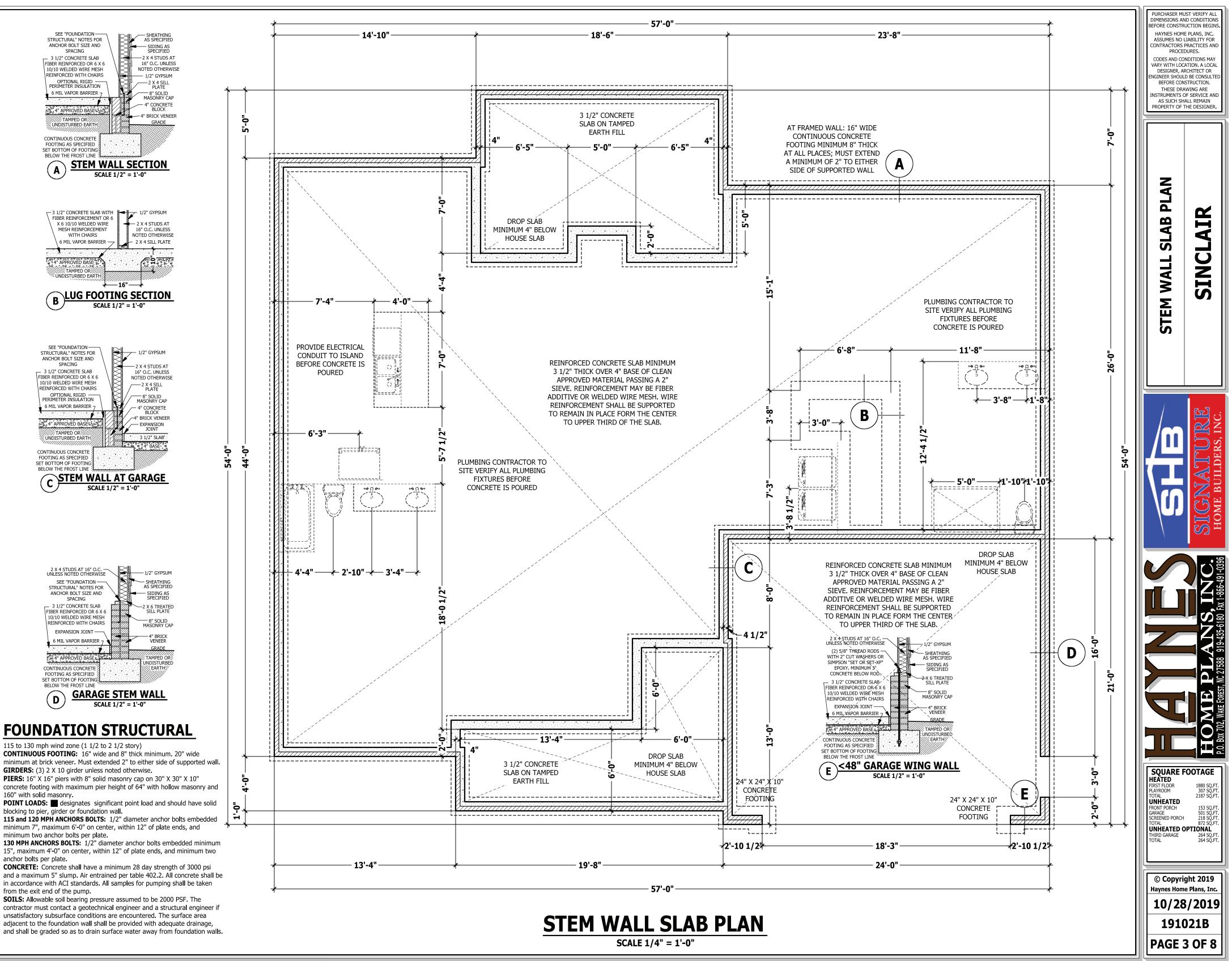


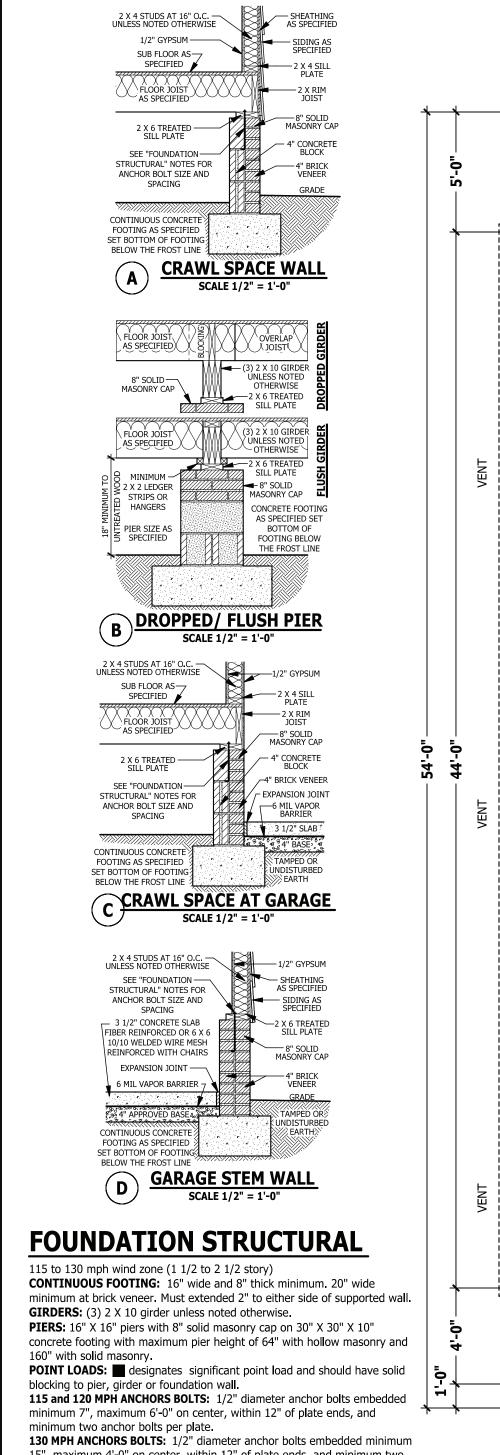


SIDING AS SPECIFIED		<u></u>
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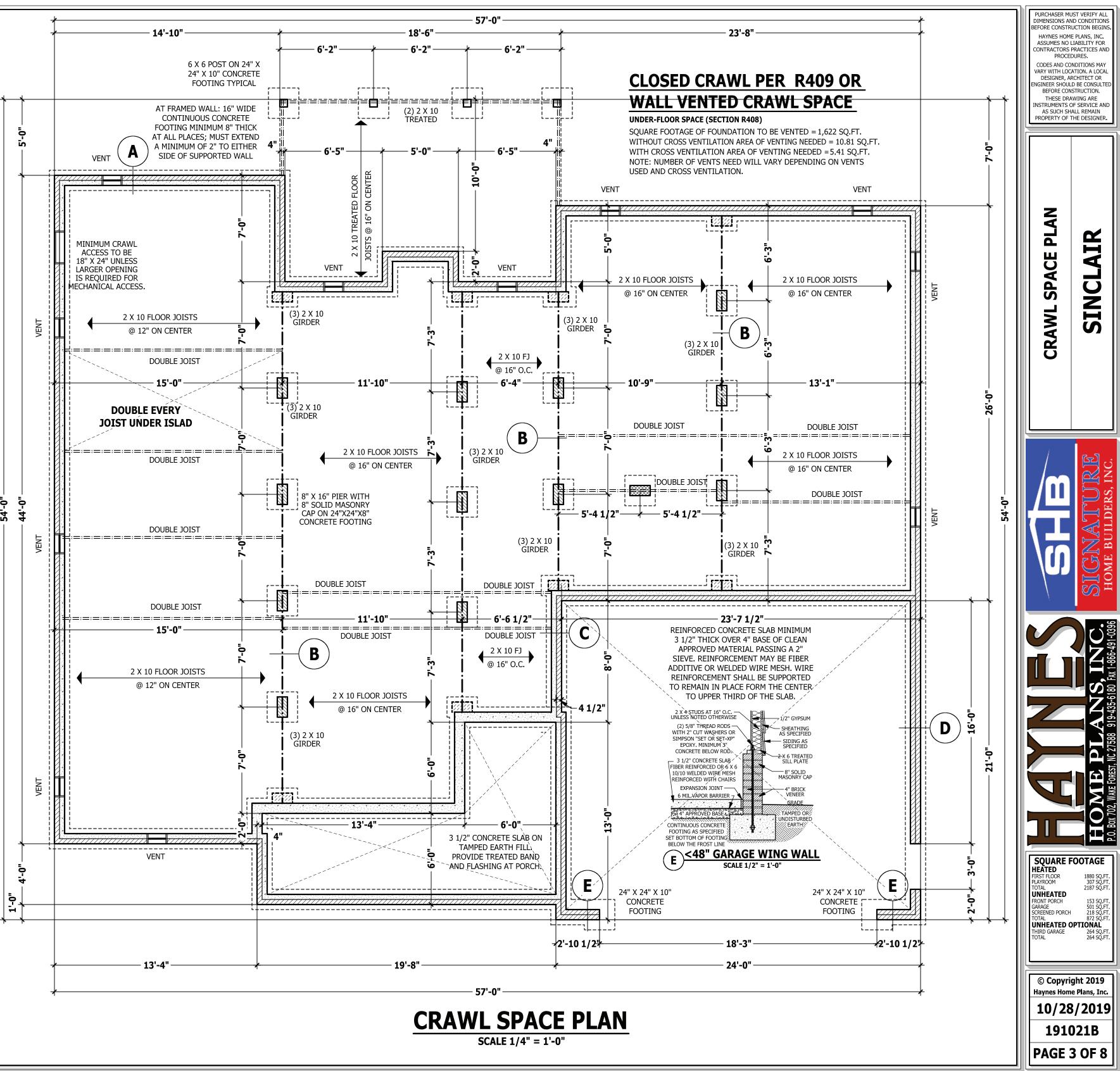


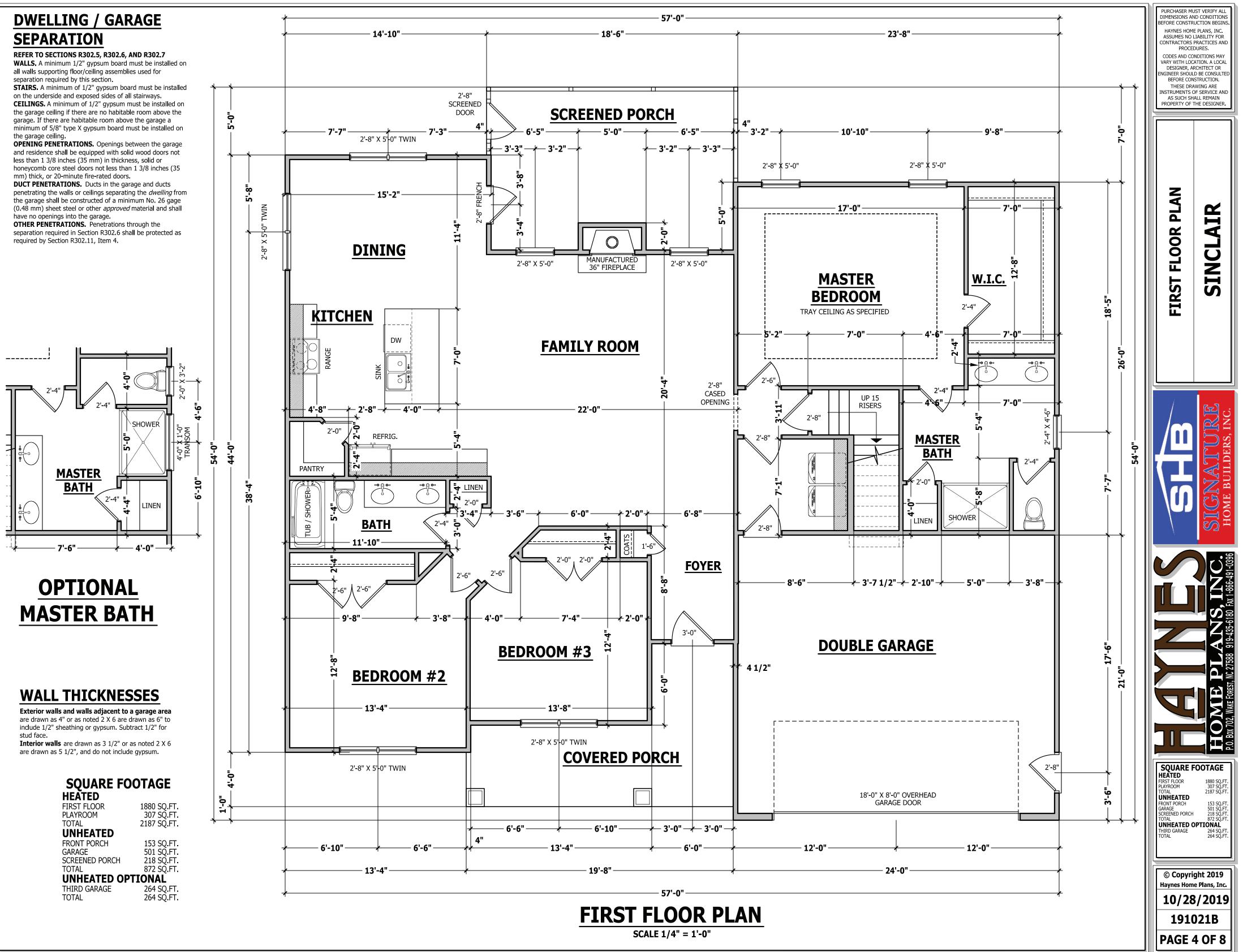


15", maximum 4'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.

CONCRETE: Concrete shall have a minimum 28 day strength of 3000 psi and a maximum 5" slump. Air entrained per table 402.2. All concrete shall be in accordance with ACI standards. All samples for pumping shall be taken from the exit end of the pump.

SOILS: Allowable soil bearing pressure assumed to be 2000 PSF. The contractor must contact a geotechnical engineer and a structural engineer if unsatisfactory subsurface conditions are encountered. The surface area adjacent to the foundation wall shall be provided with adequate drainage, and shall be graded so as to drain surface water away from foundation walls.





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STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise. **ENGINEERED WOOD BEAMS**:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10⁶ PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x10⁶ PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x10⁶ PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted **atterwise.** 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" **bolts** at 2'-0" on center for spans up to 18'-0" unless noted otherwise. **FLOOR SHEATHING:** OSB or CDX floor sheathing minimum 1/2" thick for

FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center jois: spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing.

spacing, and minimum 3/4" thick for 24" on center joist spacing. **ROOF SHEATHING:** OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters. **CONCRETE AND SOILS:** See foundation notes.

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using

method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1. **REQUIRED LENGTH OF BRACING:** Required brace wall length for each side of the circumscribed rectangle are interpolated

per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5.1's actual length.

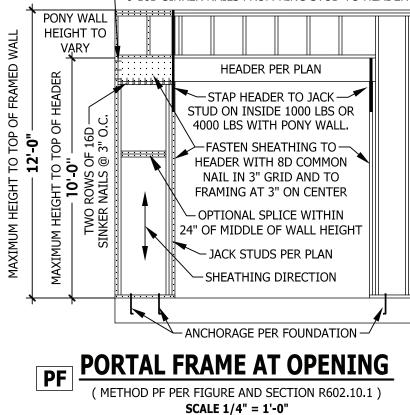
Hethod PF contributes 1.5 times its actual length.
 HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

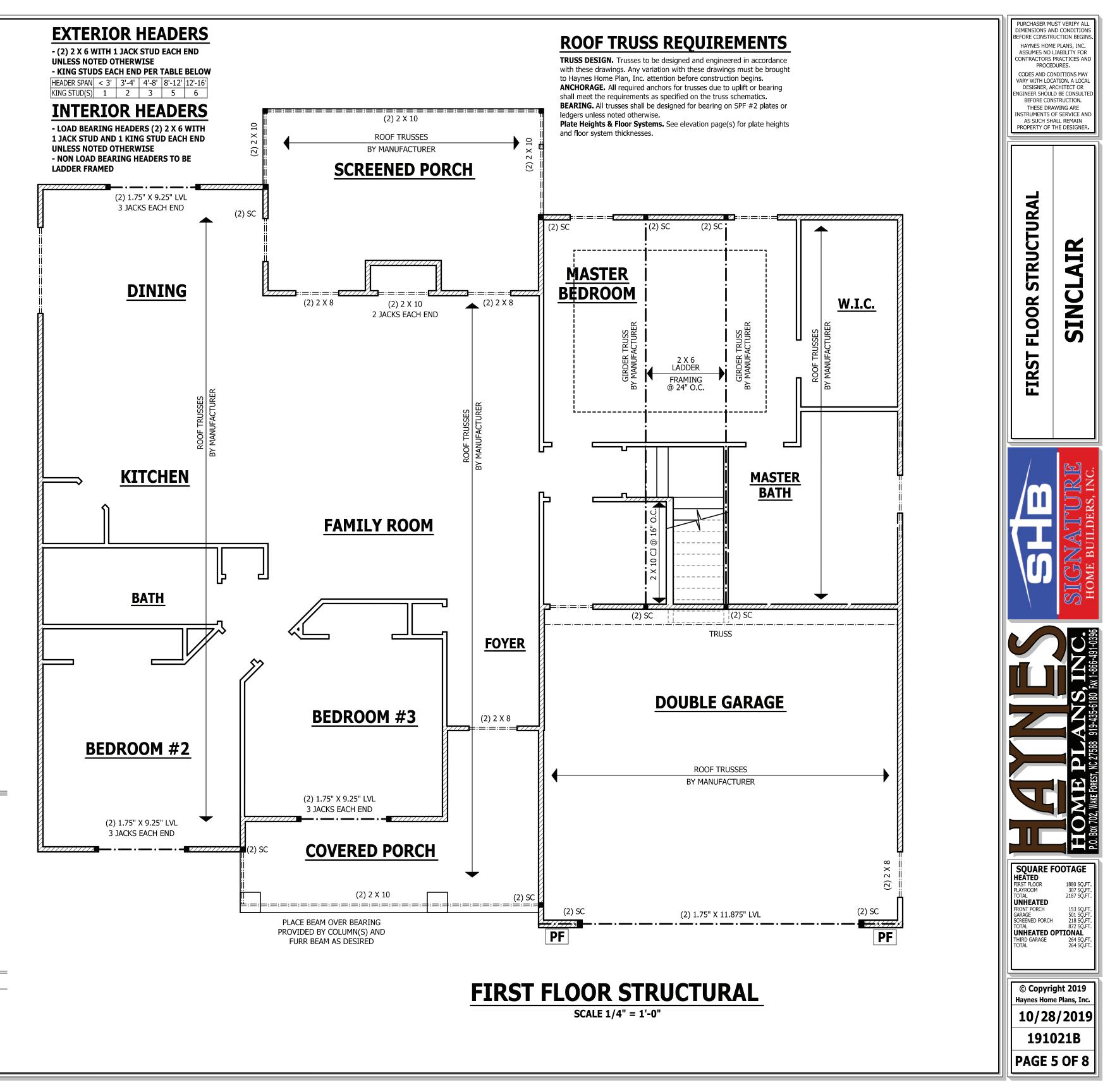
Methods Per Table R602.10.1 **CS-WSP**: Shall be minimum 3/8" OSB or CDX nailed at 6" on

center at edges and 12" on center at intermediate supports with 6d common nails or $8d(2 \ 1/2" \log x \ 0.113" diameter)$. **CS-SFB:** Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing nails.

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. **PF:** Portal fame per figure R602.10.1







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DESIGN LOADS LIVE LOAD DEAD LOAD DEFLECTION (PSF) (PSF) (LL) USE 10 10 L/240 Attics without storage Attics with limited storage 20 10 L/360 Attics with fixed stairs 40 | 10 | L/360 Balconies and decks 40 | 10 | L/360 Fire escapes 40 | 10 | L/360 200 --Guardrails and handrails --Guardrail in-fill components 50 ----Passenger vehicle garages 50 10 L/360 Rooms other than sleeping 40 10 L/360 10 L/360 Sleeping rooms 30 Stairs 40 --L/360 Snow 20 --

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS :

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Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

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1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4"

thick for 24" on center joist spacing. **ROOF SHEATHING:** OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters.

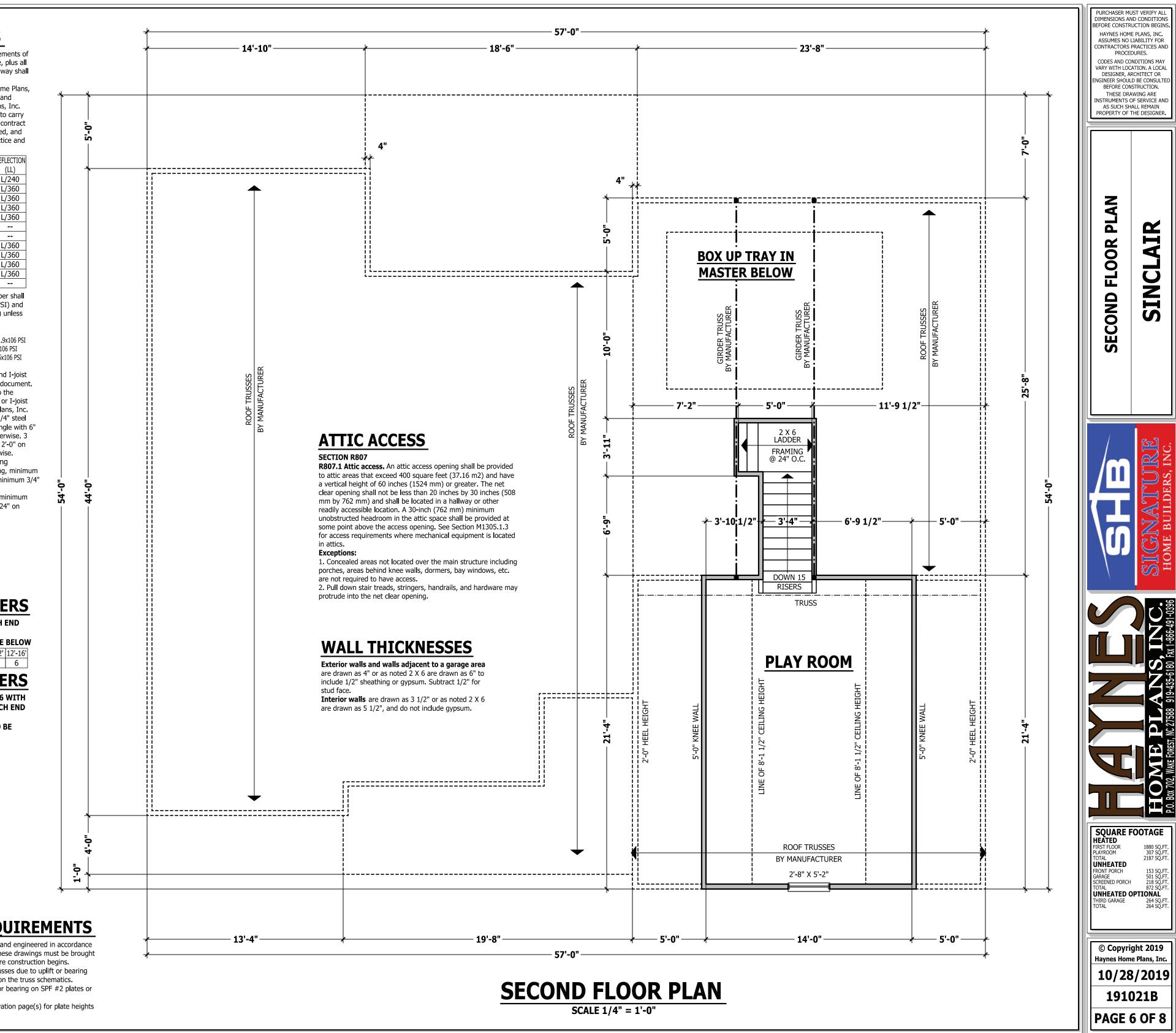
CONCRETE AND SOILS: See foundation notes.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE - KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

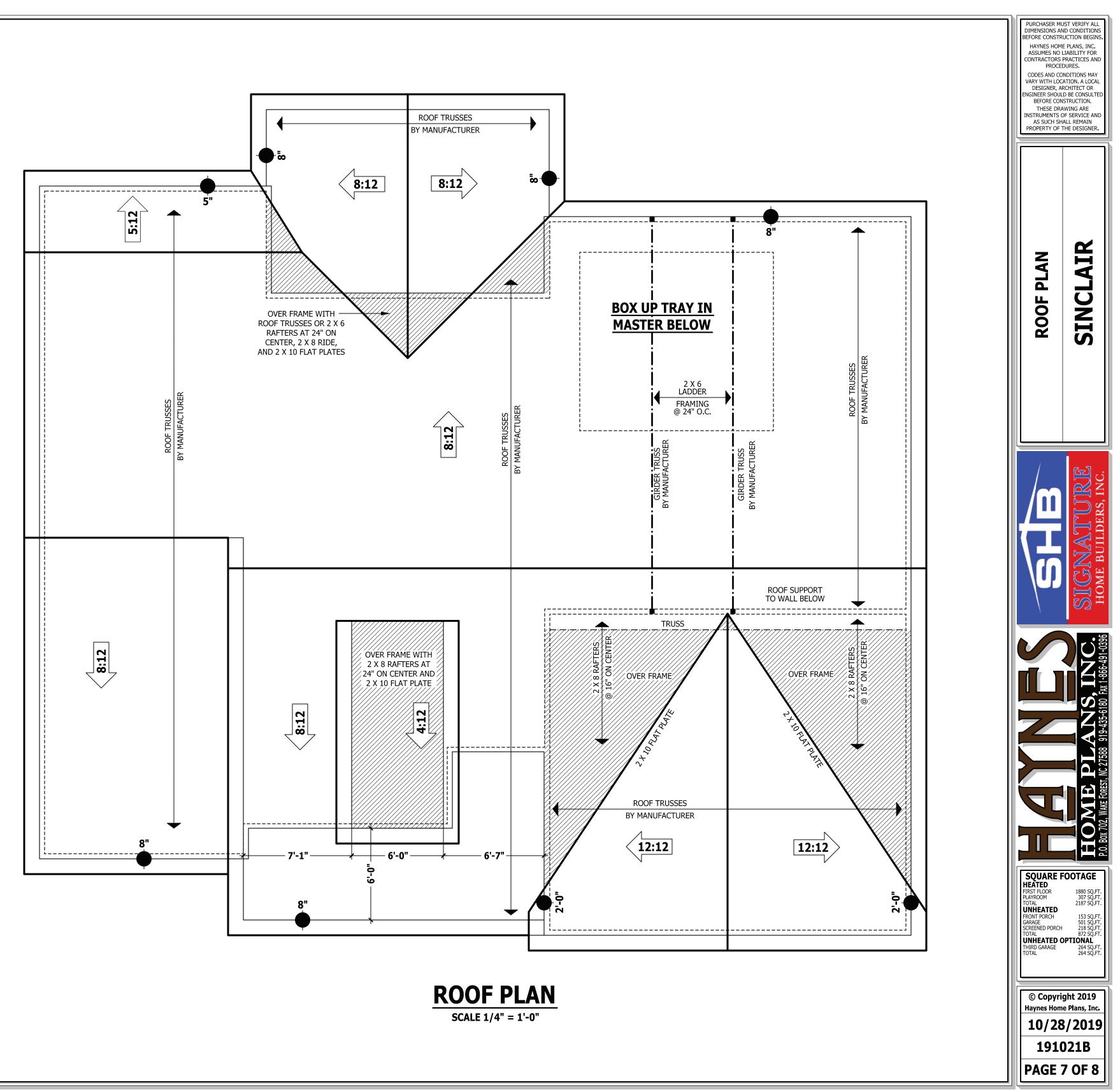
- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED



ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **ANCHORAGE.** All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.



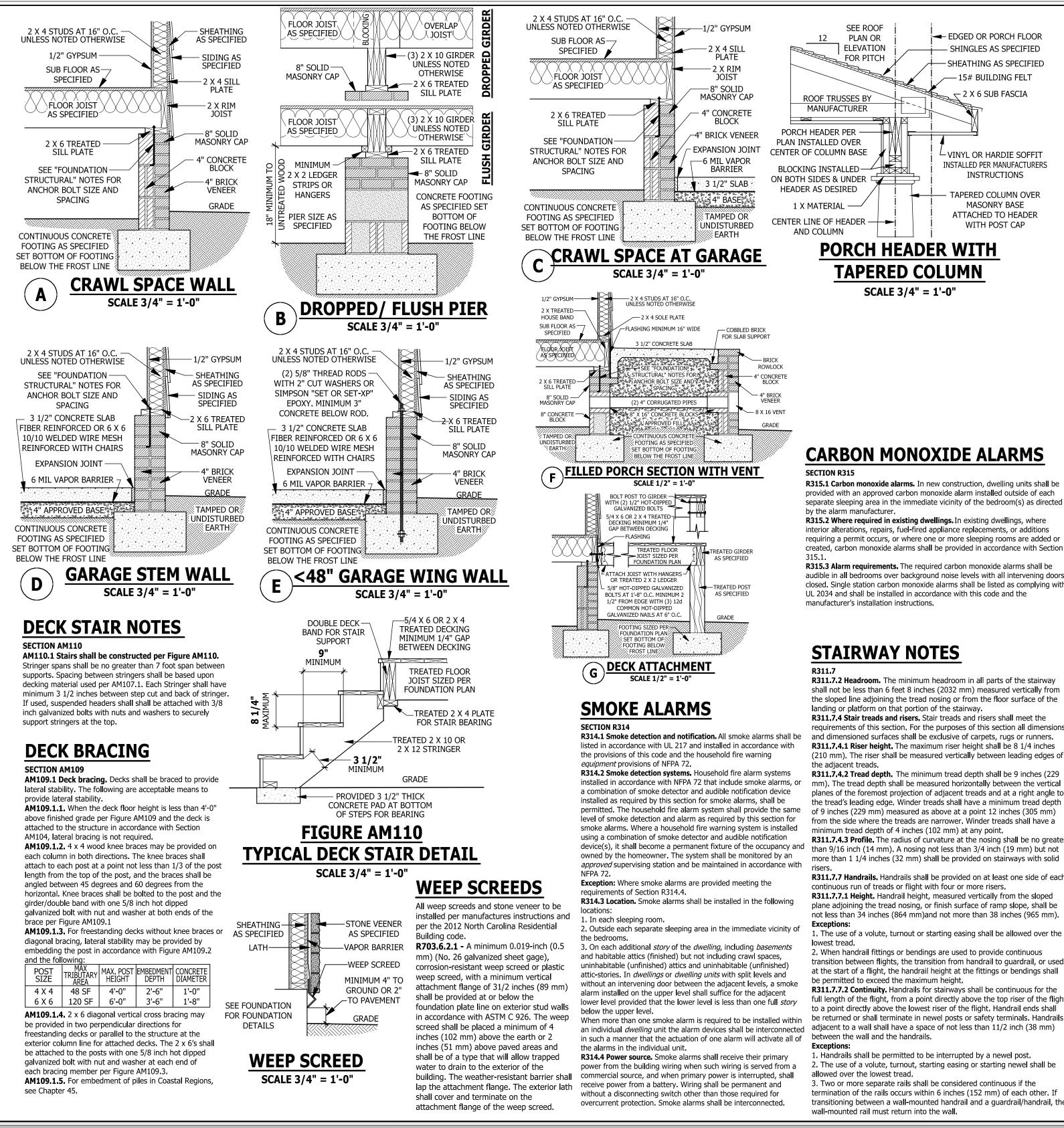
ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

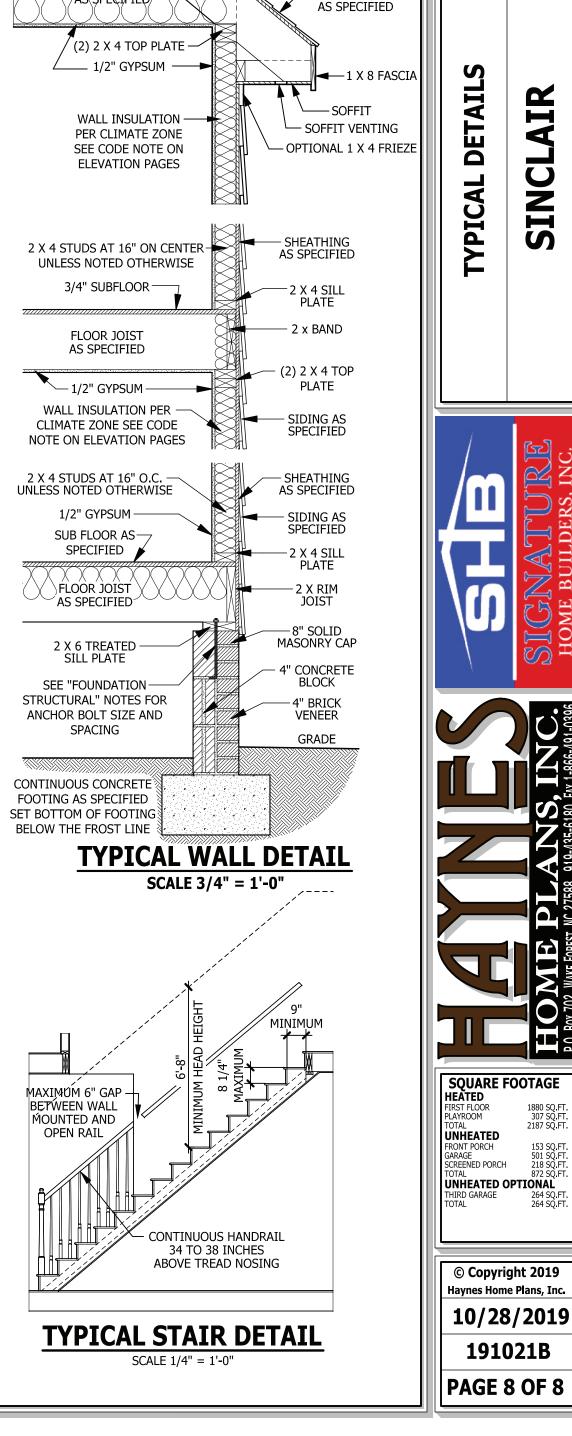
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Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

+ HEEL HEIGHT ABOVE FIRST FLOOR PLATE HEEL HEIGHT ABOVE



interior alterations, repairs, fuel-fired appliance replacements, or additions



12

CÉILING JOISTS

/AS\SPECIFIÈD/

PITCH PER ROOF PLAN

OR ELEVATIONS

- SHINGLES AS SPECIFIED

-15# BUILDING FELT

SHEATHING

RAFTERS AS 50.

ROOF INSULATION

PER CLIMATE ZONE

SEE CODE NOTE ON

ELEVATION PAGES

PURCHASER MUST VERIFY ALL

IMENSIONS AND CONDITIONS

EFORE CONSTRUCTION BEGINS

HAYNES HOME PLANS, INC.

ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND

PROCEDURES.

CODES AND CONDITIONS MAY

DESIGNER, ARCHITECT OR

BEFORE CONSTRUCTION.

THESE DRAWING ARE

NSTRUMENTS OF SERVICE AND

AS SUCH SHALL REMAIN

PROPERTY OF THE DESIGNER.

IR

SINCL

1880 SQ.FT 307 SQ.FT 2187 SQ.FT

153 SQ.FT 501 SQ.FT 218 SQ.FT 872 SQ.FT

264 SQ FT 264 SQ FT

ARY WITH LOCATION. A LOCAL

IGINEER SHOULD BE CONSULTED

separate sleeping area in the immediate vicinity of the bedroom(s) as directed

requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section

audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with

shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the

requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches

R311.7.4.2 Tread depth. The minimum tread depth shall be 9 inches (229

planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

R311.7.7 Handrails. Handrails shall be provided on at least one side of each

plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm).

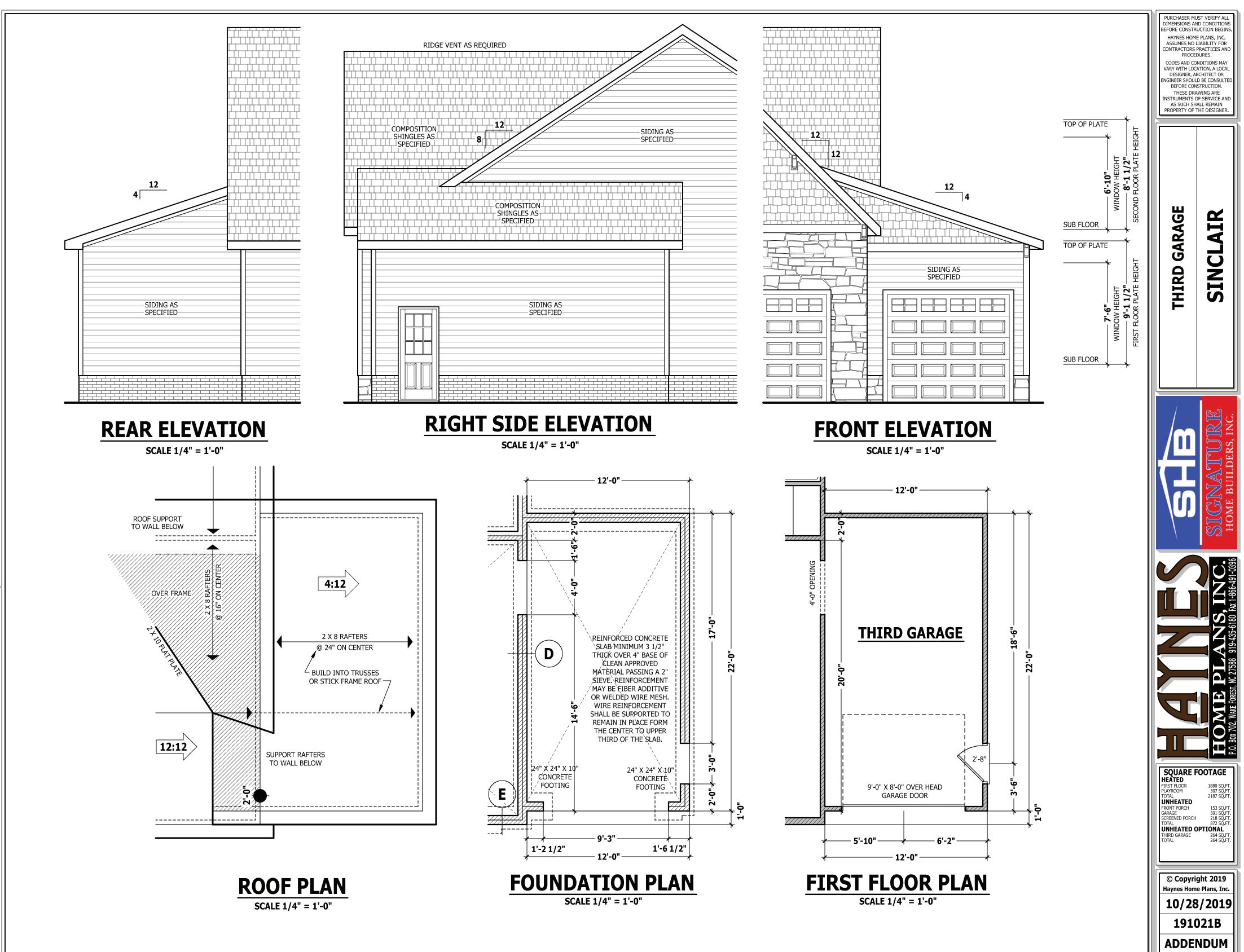
1. The use of a volute, turnout or starting easing shall be allowed over the

transition between flights, the transition from handrail to guardrail, or used at the start of a flight, the handrail height at the fittings or bendings shall

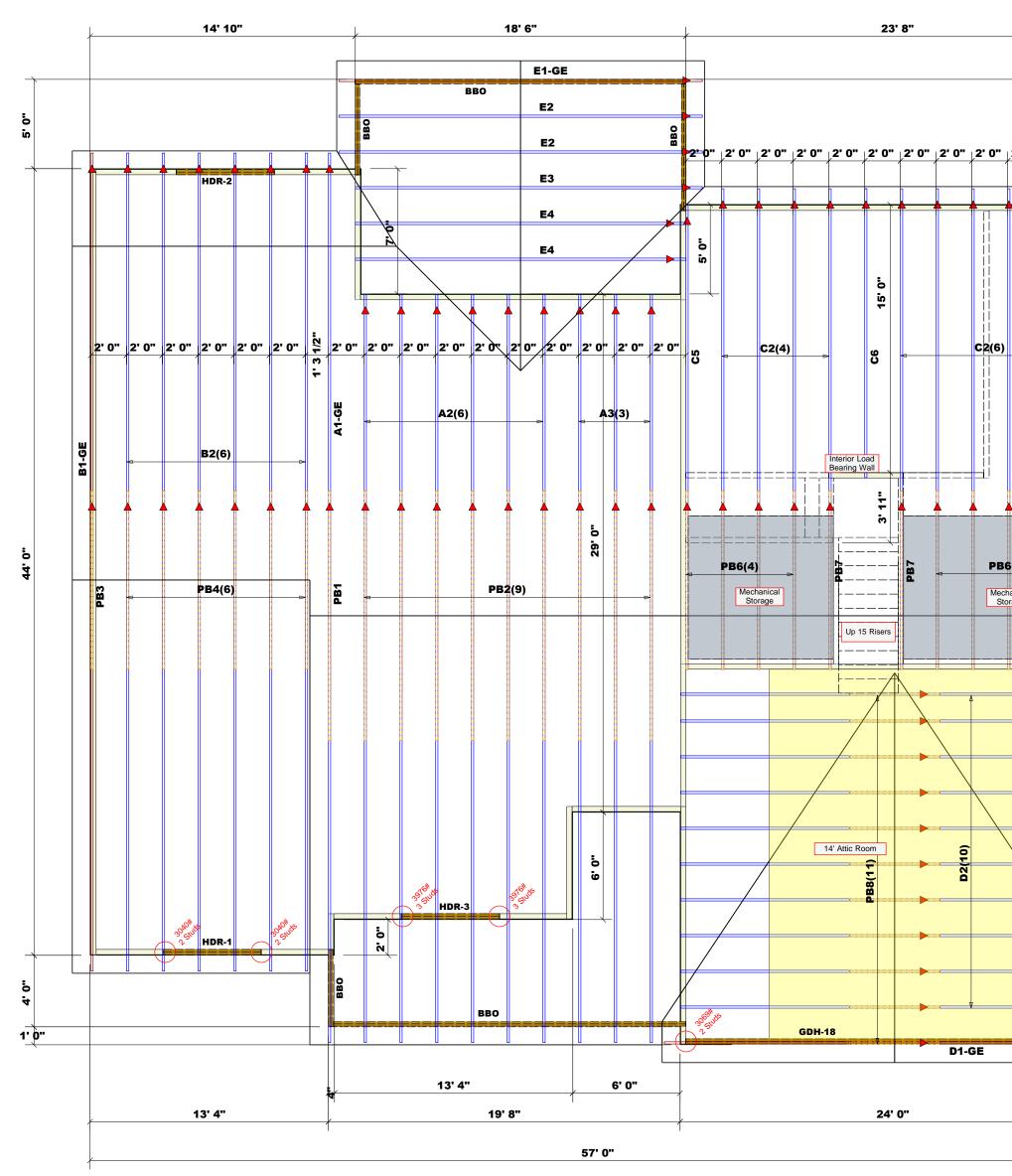
full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails

2. The use of a volute, turnout, starting easing or starting newel shall be

termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the



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					F Bearin deemo requir	Fax ng reactioned to com ements. T	OF & SES coad Ir teville e: (910) (910) ms less th ply with t	& FL & B ndustr , N.C. 0) 864 864-4	OOF EAN ial Pau 28309 -8787 i444 ial to 300 iptive Co iperive Co	A AS K W M K M K M K M K K K K K K K K K K K	
0" 2'	0"	2' 0"			Code found requir but no profes suppo those registe design excee	ed Tables requireme ation size ed to sup ot greater sional sh rrt system specified ared desig n the supp d 15000#. Signature	ents) to d and num port react than 1500 all be reta for any r in the att gn profess port syste	etermine ber of wo ions grea 00#. A reg ined to d eaction tl ached Ta sional shi m for all	the minir od studs iter than 3 jistered d lesign the nat excee bles. A all be reta reactions	num 3000# esign ds ined to that	
						AD CHI (BASEI WBER OF JI	ON TABL	ES R502.5(l) & (b))		
(6)			61-6E	▲ = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards Roof Area = 3966.79 sq.ft. Ridge Line = 98.13 ft. Hip Line = 0 ft. Horiz. OH = 136.09 ft. Raked OH = 259.03 ft. Decking = 136 sheets	RE (01 2400 5100 6800 8500 10200 11900 13600 15300	3 4 5 6 7 8	2550 5100 12750 15300	0 1 2 2 3 3 0 4 0 5	136	00 1	
PB6(5	cal e 1'			All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise. Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs BEAM SCHEDULE PlotID Length Product Plies Net Qty Fab Type HDR-1 6'0" 1-3/4"x 9-1/4" LVL Kerto-S 2 2 FF HDR-2 6'0" 1-3/4"x 9-1/4" LVL Kerto-S 2 2 FF HDR-3 6'0" 1-3/4"x 9-1/4" LVL Kerto-S 2 2 FF GDH-9 12'0" 1-3/4"x 11-7/8" LVL Kerto-S 2 2 FF GDH-18 24'0" 1-3/4"x 14" LVL Kerto-S 2 2 FF	Erwin / Harnett County	Lot 16 Williams Farms / Erwin, NC	Roof	2/29/24	Anthony Williams	Anthony Williams	
		2. 0 2. 0			COUNTY	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALESMAN	
		Z: 0 Z: 0 Z: 0			Signature Home Builders	Lot 16 Williams Farms	HHP / The Sinclair (191021B) - 2-Car	Plan Date: 10/28/19	NA	J0224-1259	
					BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #	
					These to compo design See ind identifit design permar for the suppor and co design consult	A TRUSS trusses an nents to b at the spi lividual de ed on the er is respi nent braci overall st t structur lumns is t er. For ge t BCSI-B1 elivery pa	e designe be incorpo ecification esign she placemen onsible fo ng of the ructure. 1 e includin he respon neral guio and BCS	ed as indi prated int n of the b ets for ea nt drawing r tempor roof and 'he desig g header nsibility c lance reg I-B3 prov	vidual bu o the buil uilding de ch truss g. The bu ary and floor syst n of the tri s, beams, f the buil arding br ided with	ilding ding esigner. design ilding russ walls, ding acing, the	

HDR-1	Kerto-S L	VL 1					ne: Lot 16 W			
			.750" X	9.250" 2	2-Ply - I	Project #	#: J0224-12 Level: Level	59		
	2									
			1							
	- 10-	•		Marga		•••				9
1 SPF End	Grain 0-3-0				2 SPF	End Grain 0-3-0				
1			5'6"			1	,		1	3 1/2"
1			6'				1			
ember Infor	rmation					Reactions UN	NPATTERN	ED lb (Uplift)		
Гуре:	Girder		Application:	Floor		Brg Direction	Live	Dead		/ind Con
Plies: <i>I</i> loisture Conditio	2 nn: Drv		Design Metho Building Code		5	1 Vertical	0	1576	1464	0
eflection LL:	480		Load Sharing:		0	2 Vertical	0	1576	1464	0
Deflection TL:	360		Deck:	Not Checked	ł					
nportance:	Normal - II									
emperature:	Temp <= 100°F	:								
						Bearings				
						Bearing Leng	th Dir.	Cap. React D/L lt	o Total Ld. Ca	ase Ld. Coml
						1 - SPF 3.000)" Vert	34% 1576 / 1464	4 3040 L	D+S
abycic Boch	ltc					End Grain				
nalysis Resul		anation All	awad Car	casity Camp	C = = = =	2 - SPF 3.000)" Vert	34% 1576 / 1464	4 3040 L	D+S
,	.ctual L 007 ft-lb	ocation All		pacity Comb.	Case	End				
	007 ft-lb			78 (28%) D+S 66 (37%) D+S	L	Grain				
	007 It-lb 011 lb	3 10: 1' 1/4" 794			L					
				53 (25%) D+S	L					
L Defl inch 0.	. ,		41 (L/480) 0.2 88 (L/360) 0.3		L					
L Defl inch 0.		3 0.1	88 (L/360) 0.34	+0 (34%) D+S	L	4				
esign Notes						4				
	ort to prevent lateral equired at the interio				teral support					
	s using 2 rows of 10	0d Box nails (.128x3") at 12"	o.c. Maximum end	distance not					
to exceed 6". 3 Refer to last pa	age of calculations	for fasteners	required for spe	cified loads						
	signed to be suppo									
	t be supported equ		es.							
	aterally braced at er be laterally braced a	-	IS.							
	rness ratio based o	-								
D	Load Type	Loc	cation Trib V	/idth Side	Dead 0.9	Live 1 Sn	now 1.15	Wind 1.6 Const.	1.25 Comments	6
l	Uniform			Тор	488 PLF	0 PLF	488 PLF	0 PLF 0	PLF B2 TRUSS	
2	Uniform			Тор	30 PLF	0 PLF	0 PLF	0 PLF 0	PLF WALL	
	Self Weight				7 PLF					
otes		chemicals				roper drainage to prevent	Manufacture	r Info		
alculated Structured Desig	igns is responsible only of th is component based on th	ne Handling &		pond			Metsä Wood	Building and Elect		
esign criteria and loa	adings shown. It is the	ne 2. Refer to	must not be cut or drille manufacturer's proc installation requiren	luct information			Norwalk, CT			
pplication, and to verify the	suitability of the intende	ed fastening d approvals	etails, beam strength	values, and code			(800) 622-58 www.metsaw	50		
umber . Dry service conditions, u	unless noted otherwise	 Damaged B Design assu 	eams must not be used umes top edge is lateral	ly restrained						
. LVL not to be treated w	with fire retardant or corrosiv	ve 5. Provide late lateral displa	eral support at bearing acement and rotation		s design is valid	until 6/28/2026				

	•	Client: Si	gnature Homes		Date:	2/29/2024	Page 2 of 8
		Project:			Input by:	Anthony Williams	
ÍS	Design	Address:			Job Name		
-					Project #:	J0224-1259	
HDR-1	Kerto-S LVL	1.750	X 9.250"	2-Ply - PASSI	ED	Level: Level	
				•			
•	•	•	•	• •	•		$\overline{\mathbf{M}}$ 1
						1/2"	
						$\frac{1}{M}$	9 1/
•	•	•	•	• •	• —		
	nd Grain 0-3-0			2 SPF End Grain (Λ	/
		5101			/-0-0		
		5'6'			1	,	1 13 1/2"
1		6'			1		
Multi-Ply A	nalvsis						
		Od Boy pails (13	28v2") at 12" o	.c Maximum end dist	anco no	nt to avcoad 6"	
Capacity	0.0 %						
Load	0.0 P						
Yield Limit per F		7 PLF					
Yield Limit per F	astener 81.9 1	lb.					
Yield Mode	IV						
Edge Distance	1 1/2	"					
Min. End Distand Load Combination							
Duration Factor	1.00						
Notes		chemicals	e	. For flat roofs provide proper drainage	to prevent	Manufacturer Info	
Notes Calculated Structured		andling & Installation		ponding	- provoitt	Metsä Wood	1
design criteria and	of this component based on the 1. loadings shown. It is the 2. customer and/or the contractor to	LVL beams must not be cut or Refer to manufacturer's	product information			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	
ensure the compon	ent suitability of the intended ify the dimensions and loads.	regarding installation reg fastening details, beam stren approvals	igth values, and code			(800) 622-5850 www.metsawood.com/us	
Lumber	3. 4.	Damaged Beams must not be Design assumes top edge is la	aterally restrained				
2. LVL not to be trea	ted with fire retardant or corrosive 5.	Provide lateral support at be lateral displacement and rotati	earing points to avoid	This design is valid until 6/28/20	26		

1			Project:	Signature Ho	mes		In	ate: iput by:		y William				Page 3 of 8
ÍS	Design		Address:					ob Name		Williams	Farms			
HDR-2	Korto S		1 750"	V 0 26				roject #:	J0224-					
1UK-2	Kerto-S		1.750	A J.Z	50 Z	-r iy - i	-A33E							
	2													
				1										
														7 1
													IVIN	
	Coltra .	and a second	1000		String The	-		-					$ \Lambda $	9
		•		•	A REAL PROPERTY.	and a charge of	and the second	•					V V	
1 SPF E	nd Grain 0-3-0					2 SPF	End Grain 0-	3-0						
/			5'	6"				-t					1	3 1/2"
/			6	6'					•					
								·						
lember Inf	ormation						Reaction	ns UNF	PATTER	NFD II	o (Uplift)			
Туре:	Girder		Applicatio	on: F	loor		1	ection	Liv		Dead	Snow	Wind	Con
Plies:	2		Design N		SD		1 Ver	tical		0	1498	1386	0	
Moisture Cond			Building		3C/IRC 2015		2 Ver	tical		0	1498	1386	0	
Deflection LL:	480		Load Sha	•	lo lat Chaskad									
Deflection TL:	360 Normal II		Deck:	N	lot Checked									
mportance: ſemperature:	Normal - II Temp <= 10	no∘⊨												
iemperature.	Temp <= 10						Bearing	s						
							Bearing		n Dir.	Cap.	React D/L lb	o Total	Ld. Case	Ld. Comb
							1 - SPF	•	Vert	33%	1498 / 1386			D+S
							End	0.000		0070	100,1000	2001	-	2.0
nalysis Res	sults						Grain	0.000"	. <i>.</i> .	000/				D .0
Analysis	Actual	Location		Capacity	Comb.	Case	2 - SPF End	3.000	Vert	33%	1498 / 1386	2884	L	D+S
Moment	3802 ft-lb		14423 ft-lb	0.264 (26%		L	Grain							
Jnbraced	3802 ft-lb		10944 ft-lb	0.347 (35%		L								
Shear	1908 lb	1' 1/4"		0.240 (24%	,	L								
	0.029 (L/2324)		0.141 (L/480)			L								
TL Defl inch	0.060 (L/1117)	3'	0.188 (L/360)	0.322 (32%) D+S	L	1							
esign Note							1							
	port to prevent la required at the ir				earings. Late	eral support								
	lies using 2 rows	•		•	kimum end di	istance not								
to exceed 6		ana far faatan	ana na muina difa	n an a sifi a d la	ada									
	t page of calculati designed to be su			-	aus.									
	ust be supported		-	,										
	e laterally braced	•												
	t be laterally brac derness ratio bas		•											
D	Load Type		-	rib Width	Side	Dead 0.9	Live	1 Sno	w 1.15	Wind	1.6 Const.	1.25 Co	omments	
1	Uniform				Тор	462 PLF	0 PL	F 4	62 PLF	0 F	PLF 0	PLF B2	TRUSS	
<u>2</u>	Uniform				Тор	30 PLF	0 PL		0 PLF			PLF WA		
	Self Weight					7 PLF	0.2		/	51	Ũ			
						7 I LF								
									Marriel	and the f				
lotes alculated Structured [Designs is responsible on	chemic lv of the Handlin	als g & Installatio	n	For flat pondin	at roofs provide p ng	roper drainage to	prevent	Manufactu Metsä Woo			_		
tructural adequacy of esign criteria and	f this component based loadings shown. It	on the 1. LVL be is the 2. Refer	ams must not be cut to manufacturer	or drilled	nation				301 Merritt	7 Buildin	g, 2nd Floor			
esponsibility of the cu nsure the component	ustomer and/or the contr ent suitability of the i	actor to regardi ntended fastoni	to manufacturers ng installation ing details, beam st	requirements, m	ulti-ply				Norwalk, C (800) 622-	5850				
pplication, and to verif .umber	fy the dimensions and load	is. approv 3. Damag	als ed Beams must not	be used					www.metsa	awood.co	m/us			
. Dry service conditio	ons, unless noted otherwis ed with fire retardant or c	5. Provide	assumes top edge is alteral support at	bearing points to	avoid									
		lateral	displacement and rol	adun	This	design is valid	until 6/28/202	6						

Γ		•	Client:	Signature Homes		Date:	2/29/2024	Page 4 of 8
	Tis	Design	Project: Address:			Input by: Job Name	Anthony Williams Lot 16 Williams Farms	
						Project #:	J0224-1259	
	HDR-2	Kerto-S L\	/L 1.750	" X 9.250"	2-Ply - PASS	SED ^L	evel: Level	
	•	•	•	•	• •	•	1/2	
							<1 1/2"	9 1/4
	•	•	•	•	• •	•	— <u>↓</u>	
	1 SPF E	End Grain 0-3-0			2 SPF End Grain	0-3-0		
	1			5'6"		1		3 1/2"
	1			6'		1		
L								
	Multi-Ply A							
	Fasten all pli Capacity		f 10d Box nails	(.128x3") at 12" c	.c Maximum end dis	stance no	t to exceed 6".	
L	Load	0.	0 PLF					
	Yield Limit per F Yield Limit per F		63.7 PLF 1.9 lb.					
C	См	1						
	Yield Mode Edge Distance	IV 1	1/2"					
ſ	Vin. End Distan	ce 3'						
	Duration Factor		00					
$\left \right $			about 1		The Ark made		Manufacturer Info	
	Notes Calculated Structured	Designs is responsible only of the of this component based on the		ion	. For flat roofs provide proper drainag ponding	e to prevent	Metsä Wood	1
	design criteria and responsibility of the c	I loadings shown. It is the customer and/or the contractor to	2. Refer to manufactur	cut or drilled er's product information requirements, multi-ply strength values, and code			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	
	ensure the compon application, and to ver Lumber	ent suitability of the intended ify the dimensions and loads.	approvals 3. Damaged Beams must r	ot be used			(800) 622-5850 www.metsawood.com/us	
	1. Dry service conditi	ions, unless noted otherwise ted with fire retardant or corrosive	 Design assumes top edg Brouide lateral support 	e is laterally restrained at bearing points to avoid	This design is valid until 6/28/2	2026		
					mis acaign is valla unul 0/20/	2020		

is	Design		ent: Signatu nject: dress:	re Homes		Date: Input by: Job Nam	e: Lot 16 William			Page 5 of 8
HDR-3	Kerto-S	LVL 1	.750" X 9	.250"	2-Ply -	Project #	: J0224-1259 Level: Level			
	2		1							
				all a gra	•					9
	nd Grain 0-3-0				2 SPF I	End Grain 0-3-0				
1			5'6"			1				3 1/2"
ł			6'				1			
lember Inf						Reactions UN		lh (I mlift)		
Type:	Girder		Application:	Floor		Brg Direction		Dead	Snow V	Nind Con
Plies:	2		Design Method:	ASD		1 Vertical	0	2044	1932	0
Noisture Cond	lition: Dry		Building Code:	IBC/IRC 20	15	2 Vertical	0	2044	1932	0
Deflection LL:	480		Load Sharing:	No						
Deflection TL:	360		Deck:	Not Checke	ed					
mportance:	Normal - II	-								
Temperature:	Temp <= 100°	Έ				Bearings				
								Popot D/L lb	Total I.d. C	Casa Id Camb
						Bearing Lengt 1 - SPF 3.000"	•	. React D/L lb		Case Ld. Comb D+S
						1 - SPF 3.000" End	veit 45%	u ∠044/1932	3910 L	5+0
nalysis Re	sults					Grain				
Analysis		Location Allo	owed Capa	city Comb.	Case	2-SPF 3.000"	" Vert 45%	2044 / 1932	3976 L	D+S
Moment	5241 ft-lb	3' 144	123 ft-lb 0.363	(36%) D+S	L	End Grain				
Jnbraced	5241 ft-lb	3' 109	944 ft-lb 0.479	(48%) D+S	L	- ···				
Shear	2628 lb	1' 1/4" 794	43 lb 0.331	(33%) D+S	L					
L Defl inch	0.040 (L/1667)	3' 0.1	41 (L/480) 0.288	(29%) S	L					
TL Defl inch	0.083 (L/810)	3' 0.1	88 (L/360) 0.444	(44%) D+S	L					
esign Not	es					1				
 may also be Fasten all p to exceed 6 Refer to las Girders are Top loads m Top must be Bottom must 	port to prevent later e required at the inte lies using 2 rows of ". t page of calculation designed to be supp rust be supported ec e laterally braced at st be laterally braced derness ratio based	rior bearings by 10d Box nails (s for fasteners ported on the bo qually by all plie end bearings. I at end bearing	/ the building code .128x3") at 12" o. required for speci ottom edge only. .s. s.	e. c. Maximum end						
D	Load Type		cation Trib Wi	dth Side	Dead 0.9	Live 1 Sno	ow 1.15 Wind	11.6 Const. 1	1.25 Comment	ts
l	Uniform			Тор	644 PLF	0 PLF	644 PLF 0	PLF 0	PLF A2 TRUSS	6
2	Uniform			Тор	30 PLF	0 PLF	0 PLF 0	PLF 0	PLF WALL	
	Self Weight				7 PLF					
otoo		chemicals		6 F	r flat roofe provide	roper drainage to prevent	Manufacturer Info	,		
ructural adequacy c esign criteria and sponsibility of the c isure the compone oplication, and to veri	Designs is responsible only of of this component based on loadings shown. It is ustomer and/or the contractor ent suitability of the inter fy the dimensions and loads.	f the the pr to nded Handling & 1. LVL beams r 2. Refer to regarding fastening de approvals	must not be cut or drilled manufacturer's produc installation requiremer etails, beam strength value	por t information ts, multi-ply	r flat roofs provide p nding	opor orannage to prevent	Metsä Wood 301 Merritt 7 Buildi Norwalk, CT 06851 (800) 622-5850 www.metsawood.c	ng, 2nd Floor	- 	
umber	and unless poted otherwise	Design assu	eams must not be used imes top edge is laterally r eral support at bearing p	estrained						
Dry service condition	ted with fire retardant or corro									

	•	Client:	Signature Homes		Date:	2/29/2024	Page 6 of 8
Tis	Design	Project: Address:			Input by: Job Name	Anthony Williams Lot 16 Williams Farms	
-	-				Project #:	J0224-1259	
HDR-3	Kerto-S LV	L 1.750	" X 9.250"	2-Ply - PASS	SED ^L	evel: Level	
	•	•	•	• •	•		
	•	•	•		•	1/2"	
						<1 1/2"	9 1/4
	•	•	•	• •	•	$\overline{}$	
	End Grain 0-3-0			2 SPF End Grain	0-3-0		
			5'6"		1		3 1/2"
1			6'		1		
Multi-Ply A							
Fasten all pl Capacity	ies using 2 rows of 0.0		(.128x3") at 12" c	o.c Maximum end dis	stance no	t to exceed 6".	
Load	0.0	PLF					
Yield Limit per F Yield Limit per F		.7 PLF) Ib					
См	1						
Yield Mode Edge Distance	IV 1 1/	2"					
Min. End Distan Load Combinati	ce 3"						
Duration Factor	1.00)					
						Manufacturor Info	
Notes Calculated Structured		chemicals Handling & Installat	ion	i. For flat roofs provide proper drainage ponding	e to prevent	Manufacturer Info Metsä Wood	4
structural adequacy design criteria and responsibility of the	of this component based on the I loadings shown. It is the customer and/or the contractor to	 LVL beams must not be Refer to manufacture regarding installation 	rer's product information			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	
ensure the compon application, and to ver	ent suitability of the intended ify the dimensions and loads.	fastening details, beam approvals 3. Damaged Beams must r	requirements, multi-ply strength values, and code			(800) 622-5850 www.metsawood.com/us	
1. Dry service condit 2. LVL not to be treat		 Design assumes top edg Provide lateral support 	e is laterally restrained at bearing points to avoid				
		lateral displacement and	roiduuri	This design is valid until 6/28/2	2026		

			Client: Sig Project:	nature Homes		Date Input		29/2024 thony Willian	าร			Page 7 of 8
IS	Design	1	Address:					t 16 Williams	Farms			
						Proje		224-1259				
GDH-18	Kerto-S	LVL	1.750" X	【 14.000 "	2-Ply - I	PASSED	Level	Level				
											0 0 tal Ld. Case Ld. Co 69 L D+0.75	
		2										
•		• •			1	·		·	•	• •	M	$\overline{1}$
	d Grain 0-3-8	- Maria					a the		SPF End Gr		W	1'2"
	Grain 0-5-6							2	SFF Ella Gi	ain 0-3-6		
1				18	3'3"					ſ	13	1/2"
<u> </u>				18	'10"					1	,	
Member Inf						Reactions			-			
Type: Plies:	Girder 2		Application: Design Met	Floor nod: ASD		Brg Directi		Live	Dead	Snow		Con
Moisture Cond			Building Co		2015	1 Vertica		377	2504	377		
Deflection LL:	480		Load Sharin		2015	2 Vertica		377	2504	377	0	
Deflection TL:			Deck:	9. Not Check	ked							
Importance:	Normal - II											
Temperature:	Temp <= 100	°F										
						Bearings						
						Bearing Le	ength D	ir. Cap.	React D/L	lb Total	Ld. Case	Ld. Com
						1-SPF 3.	-	ert 30%	2504 / 50			D+0.75(L
						End						(-
Analysis Re	sults					Grain						
Analysis	Actual	Location A	Allowed C	apacity Comb	o. Case	2 - SPF 3.	500" Ve	ert 30%	2504 / 56	3069	L	D+0.75(L
Moment	12910 ft-lb	9'5" 2	26999 ft-lb 0	.478 (48%) D+L	L	End Grain						
Unbraced	13754 ft-lb	9'5"			5(L+S) L	_						
				100%)								
Shear	2447 lb	1'5 1/2"		.234 (23%) D+L	L							
	0.102 (L/2160)			.222 (22%) 0.75(L								
TL Defl inch	0.555 (L/398)	9'5 1/16" (0.612 (L/360) 0	.905 (91%) D+0.7	5(L+S) L	1						
Design Not	es											
may also be	port to prevent later e required at the inte lies using 3 rows of	erior bearings	by the building	code.]						
to exceed 6)".		. ,									
	t page of calculation											
	designed to be sup nust be supported e		-	iiy.								
	e laterally braced at											
	st be laterally braced		0									
	derness ratio based		-			<u> </u>						
ID	Load Type	l	Location Trib	Width Side	Dead 0.9		Snow 1.1		1.6 Const		mments	
1	Uniform			Тор	55 PLF	40 PLF	40 PL			0 PLF R +	۴F	
2	Uniform			Тор	200 PLF	0 PLF	0 PL	.F 01	PLF	0 PLF WA	ALL	
	Self Weight				11 PLF							
Notes		chemica	lls	6	For flat roofs provide p	roper drainage to prev	vent Manu	facturer Info				
Calculated Structured	Designs is responsible only o of this component based on	of the Handling	g & Installation		ponding	. <u>.</u>	Metsa	i Wood				
design criteria and	loadings shown. It is	the 2. Refer	ms must not be cut or d to manufacturer's p installation requ	product information			Norw	1erritt 7 Buildin alk, CT 06851	y, ∠nd Fíoor			
ensure the compon	ent suitability of the inter ify the dimensions and loads.		ig installation requ g details, beam streng Is	th values, and code				622-5850 metsawood.co	m/us			
Lumber		 Damage Design : 	ed Beams must not be u assumes top edge is lat	erally restrained								
	ons, unless noted otherwise ted with fire retardant or corro	5. Provide	lateral support at bea isplacement and rotation	ring points to avoid	This design is valid	until 6/28/2026						
					acorgin io vallu	STU 0/20/2020						

	esign	Client: Signature Homes Project: Address:		Date: Input by: Job Name: Project #:	J0224-1259	Pag
GDH-18	Kerto-S LVL	1.750" X 14.000	" 2-Ply - PASS	ED	evel: Level	
· · ·	· · · ·	· · · · ·	· · · · ·	· · ·	· · · · · ·	;;[] M V
1 SPF End G	rain 0-3-8			-	2 SPF End Grain	
/			18'3"			3 1/2"
1			18'10"			
lulti-Ply Ana	-)d Box nails (.128x3") at 12'	' o.c. Maximum end dis	tance no	t to exceed 6"	
pacity	0.0 %		o.c Maximum end dis			
ad Id Limit per Foot	0.0 PL 245.6					
ld Limit per Fast	ener 81.9 lb 1).				
ld Mode	IV					
ge Distance . End Distance	1 1/2" 3"					
ad Combination	5					
ration Factor	1.00					
				<u> </u>	Manufacturer Info	
otes alculated Structured Desi	gns is responsible only of the Ha		 For flat roofs provide proper drainage ponding 	to prevent	Metsä Wood	4
uctural adequacy of th sign criteria and lo	is component based on the 1. I adings shown. It is the 2. P	.VL beams must not be cut or drilled Refer to manufacturer's product information			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	
sure the component	suitability of the intended	egarding installation requirements, multi-ply astening details, beam strength values, and code approvals			(800) 622-5850 www.metsawood.com/us	
Umber Dry service conditions,	3. [4. [Damaged Beams must not be used Design assumes top edge is laterally restrained				
	unless noted otherwise 5. F	Provide lateral support at bearing points to avoid ateral displacement and rotation	This design is valid until 6/28/2	0.00		

This design is valid until 6/28/2026