

RE: J0322-1504 Cates\Lot 737 Lexington Plantation Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J0322-1504 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.

No	Seal#	Truss Name	Date
1	150319666	Δ1	2/10/2022
2	150319000	۸ <u>۵</u>	2/10/2022
2	150519007	AZ	2/19/2022
3	150319668	B1	2/19/2022
4	150319669	B2	2/19/2022
5	150319670	B3	2/19/2022
6	150319671	C1	2/19/2022
7	150319672	C2	2/19/2022
8	150319673	M1	2/19/2022
9	150319674	M2	2/19/2022
10	150319675	P1	2/19/2022
11	150319676	P2	2/19/2022
12	150319677	V1	2/19/2022
13	150319678	V2	2/19/2022
14	150319679	V3	2/19/2022
15	150319680	\/4	2/19/2022

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Johnson, Andrew

My license renewal date for the state of North Carolina is December 31, 2022

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Johnson, Andrew









WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

February 19,2022

minim







- BOT CHORD 2-15=-57/2004, 12-15=0/1343, 10-12=-51/1796
- WEBS 4-15=-514/355, 6-15=-72/1086, 6-12=-72/1087, 8-12=-514/355

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-15 to 3-7-14, Interior(1) 3-7-14 to 18-0-0, Exterior(2) 18-0-0 to 22-4-13, Interior(1) 22-4-13 to 36-8-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) 200.0lb AC unit load placed on the bottom chord, 18-0-0 from left end, supported at two points, 5-0-0 apart.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Cates\Lot 737 Lexington Plantation
					150319671
J0322-1504	C1	ATTIC	1	1	
					Job Reference (optional)
Comtech, Inc, Fayette	/ille, NC - 28314,		8	3.430 s Au	g 16 2021 MiTek Industries, Inc. Fri Feb 18 09:46:43 2022 Page 1

ID:eo_4ms57ZKhslxK1ijvAQdyiOuh-JjK1YrvI_ORBs8a3Z2E_xbuMcrssLw03iXQVKYzjxcQ -0-10-8 3-11-12 0-10-8 3-11-12 7-9-4 9-2-11 12-0-0 14-9-5 16-2-12 20-0-4 1-5-7 2-9-5 2-9-5 1-5-7 3-9-8 24-0-0 24-10-8 3-11-12 0-10-8 3-9-8



Scale = 1:89.0



8x8 =

		<u>3-11-12</u> 7-9-4 3-11-12 3-9-8	<u>16-2-12</u> <u>20-0-4</u> <u>24-0-0</u> 8-5-8 <u>3-9-8</u> <u>3-11-1</u>	2
Plate Offsets (X,Y)	[2:0-2-8,0-2-8], [6:0-3-0,Edge], [10:0-2-8,0-2-8], [12:0-2-8,0-4-	2], [13:0-4-0,0-4-12], [14:0-4-0,0-4-12]	
LOADING (psf)	SPACING- 2-0-0 Plate Grip DOI 11) CSI.	DEFL. in (loc) I/defl	L/d PLATES GRIP 360 MT20 244/190

	i (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL 1.15	10 0.56	Vert(LL) -0.0	12-14	>999	360	M120	244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.36	Vert(CT) -0.1	4 12-14	>999	240			
BCLL	0.0 "	Rep Stress Incr YES	VVB 0.29	Horz(CT) 0.0	10	n/a	n/a		FT 000/	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	VVIND(LL) 0.1	0 2-14	>999	240	vveight: 250 lb	FT = 20%	
LUMBER	-			BRACING-						_

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No 1 BOT CHORD 2x10 SP No.1 WFBS 2x6 SP No 1 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-4=-1944/69, 4-5=-1032/198, 7-8=-1032/199, 8-10=-1944/68
- BOT CHORD 2-14=0/1250, 12-14=0/1250, 10-12=0/1250

WEBS 5-7=-1324/350, 4-14=-32/817, 8-12=-32/816

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-14 to 3-7-15, Exterior(2) 3-7-15 to 12-0-0, Corner(3) 12-0-0 to 16-2-12, Exterior(2) 16-2-12 to 24-8-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Ceiling dead load (10.0 psf) on member(s). 4-5, 7-8, 5-7; Wall dead load (5.0psf) on member(s). 4-14, 8-12

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14

7) Attic room checked for L/360 deflection.



Structural wood sheathing directly applied or 5-3-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=382(LC 11) Max Grav 2=1512(LC 20), 10=1512(LC 21)

Job	Truss	Truss Type	Qty	Ply	Cates\Lot 737 Lexington Plantation
					150319672
J0322-1504	C2	ATTIC	3	1	
					Job Reference (optional)
Comtech, Inc, Fayett	eville, NC - 28314,			3.430 s Au	g 16 2021 MiTek Industries, Inc. Fri Feb 18 09:46:44 2022 Page 1

ID:eo_4ms57ZKhslxK1jyAQdyiOuh-nvtPlBwNlha2Tl9F7llDTpQX3FC14NDDxB93t?zjxcP -0_10_8 3-11-12 7-9-4 9-2-11 12-0-0 14-9-5 16-2-12 20-0-4 24-0-0 0_10_8 3-11-12 3-9-8 1-5-7 2-9-5 1-5-7 3-9-8 3-11-12



Scale = 1:89.0



Plate Offs	sets (X,Y)	[2:0-2-8,0-2-8], [6:0-3-0,	<u></u>	- <u>11-12 7</u> - <u>11-12 3</u> 8,Edge], [11:0	7-9-4 3-9-8 D-2-8,0-3-0],	<u>16-2-12</u> <u>8-5-8</u> [12:0-4-0,0-4-12]	<u>20-0-4</u> <u>3-9-8</u>	24-0 3-11-	+0 -12		
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL TCDL	20.0 10.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC BC	0.58 0.37	Vert(LL) Vert(CT)	-0.08 11-13 -0.14 11-13	>999 >999	360 240	MT20	244/190
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.29	Horz(CT)	0.01 10	n/a	n/a		

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

0.07 2-13

>999

240

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 5-1-12 oc purlins.

Weight: 247 lb

FT = 20%

BCDL	10.	0		Code	IRC2015/TPI2014
LUMBER TOP CH	₹- ORD	2x6 SP	No.1		

IDP CHORD2X6 SP No.1BOT CHORD2x10 SP No.1WEBS2x6 SP No.1WEDGELeft: 2x4 SP No.3

REACTIONS.	(size)	2=0-3-8, 10=0-3-8
	Max Horz	2=302(LC 9)
	Max Grav	2=1519(LC 20), 10=1475(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-4=-1929/4, 4-5=-1029/149, 7-8=-1036/161, 8-10=-1912/0
- BOT CHORD 2-13=0/1220, 11-13=0/1220, 10-11=0/1220

WEBS 5-7=-1346/268, 4-13=0/800, 8-11=-0/767

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-14 to 3-7-15, Interior(1) 3-7-15 to 12-0-0, Exterior(2) 12-0-0 to 16-2-12, Interior(1) 16-2-12 to 23-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-S

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Ceiling dead load (10.0 psf) on member(s). 4-5, 7-8, 5-7; Wall dead load (5.0psf) on member(s). 4-13, 8-11

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

7) Attic room checked for L/360 deflection.







	6-0-0
	6-0-0
LOADING (psf)	SPACING- 2-0-0 CSI. DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 TC 0.55 Vert(LL) -0.06 2-4 >999 360 M120 244/190 Lumber DOL 1.15 BC 0.50 Vert(CT) -0.11 2-4 >615 240
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES WB 0.00 Horz(CT) -0.00 4 n/a n/a Code IRC2015/TPI2014 Matrix-P Wind(LL) 0.00 2 **** 240 Weight: 24 lb FT = 20%
LUMBER-	BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
WEBS	2x4 SP No.2
OTHERS	2x4 SP No 2

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=109(LC 8) Max Uplift 2=-105(LC 8), 4=-78(LC 12) Max Grav 2=295(LC 1), 4=221(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-4=-164/281

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 5-9-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=105.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





		F				6-0-0						
LOADING TCLL TCDL	G (psf) 20.0 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 - 1.15 1.15	CSI. TC BC	0.44 0.31	DEFL. Vert(LL) Vert(CT)	in -0.06 -0.11	(loc) 2-4 2-4	l/defl >999 >615	L/d 360 240	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Inc Code IRC2015	r YES 5/TPI2014	WB Matrix	0.00 -P	Horz(CT) Wind(LL)	0.00 0.12	4 2-4	n/a >554	n/a 240	Weight: 22 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1WEBS2x4 SP No.2

WEBS 2x4 SP No.2 REACTIONS. (size) 2=0-3-

FIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=77(LC 8) Max Uplift 2=-117(LC 8), 4=-95(LC 8)

Max Grav 2=295(LC 1), 4=221(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 5-9-15 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=117.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





Scale = 1:22.0



 	<u> 6-0-0</u> 6-0-0			<u>12-0-0</u> 6-0-0		
Plate Offsets (X,Y)	[2:0-3-0,Edge], [4:0-3-0,Edge]				-	
LOADING(psf)TCLL20.0TCDL10.0BCLL0.0BCDL10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.46 BC 0.30 WB 0.06 Matrix-S	DEFL. in Vert(LL) 0.09 Vert(CT) -0.07 Horz(CT) -0.01	l (loc) l/defl L/d 4-6 >999 240 2-6 >999 240 4 n/a n/a	PLATES MT20 Weight: 46 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S OTHERS 2x4 S	SP No.1 SP No.1 SP No.2 SP No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing a Rigid ceiling directly applied	directly applied or 6-0- d or 6-0-14 oc bracing.	0 oc purlins.
REACTIONS. (si Max Max Max	ze) 2=0-3-8, 4=0-3-8 Horz 2=-46(LC 17) Uplift 2=-293(LC 8), 4=-293(LC 9) Grav 2=530(LC 1), 4=530(LC 1)					
FORCES.(lb) - MaxTOP CHORD2-33BOT CHORD2-63WEBS3-63	 Comp./Max. Ten All forces 250 (lb) or -859/1155, 3-4=-859/1155 -975/759, 4-6=-975/759 -342/281 	r less except when shown.				
NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-10:	ve loads have been considered for this de Vult=130mph Vasd=103mph: TCDL=6.0t	esign. osf: BCDL=6.0psf: h=15ft: Ca	t. II: Exp C: Enclosed	: MWFRS (envelope)		

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 6-0-0, Corner(3) 6-0-0 to 10-4-13, Exterior(2) 10-4-13 to 12-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=293, 4=293.







6-0-0			12-0-0					
I	6-0-0		6-0-0					
Plate Offsets (X,Y)	[2:0-2-0,Edge], [4:0-2-0,Edge]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.37 BC 0.30 WB 0.06 Matrix-S	DEFL. in (loc) //defl L/d Vert(LL) 0.08 4-6 >999 240 Vert(CT) -0.07 2-6 >999 240 Horz(CT) 0.01 4 n/a n/a Weight: 42 lb FT = 20%					
LUMBER- TOP CHORD 2x4 3 BOT CHORD 2x4 3 WEBS 2x4 3	SP No.1 SP No.1 SP No.2		BRACING-TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins.BOT CHORDRigid ceiling directly applied or 6-8-11 oc bracing.					
REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=-27(LC 17) Max Uplift 2=-205(LC 8), 4=-205(LC 9) Max Grav 2=530(LC 1), 4=530(LC 1)								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-859/932, 3-4=-859/932 BOT CHORD 2-6=-802/759, 4-6=-802/759 WEBS 3-6=-357/281								
NOTES-								

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 6-0-0, Exterior(2) 6-0-0 to 10-4-13, Interior(1) 10-4-13 to 12-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=205, 4=205.







- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-5-8, Exterior(2) 5-5-8 to 9-10-5, Interior(1) 9-10-5 to 10-6-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 11=150, 12=121, 9=149, 8=122.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



REACTIONS. (size) 1=8-2-15, 3=8-2-15, 4=8-2-15 Max Horz 1=-90(LC 8)

Max Uplift 1=-33(LC 13), 3=-33(LC 13)

Max Grav 1=184(LC 1), 3=184(LC 1), 4=236(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. (size) 1=5-6-15, 3=5-6-15, 4=5-6-15 Max Horz 1=-58(LC 8)

Max Holz 1=-58(LC 8) Max Uplift 1=-21(LC 13), 3=-21(LC 13)

Max Grav 1=119(LC 1), 3=119(LC 1), 4=153(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.02 BC 0.04 WB 0.00	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n (loc) l a - a -) 3	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P				Weight: 9 lb	FI = 20%
LUMBER- TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1			BRACING- TOP CHORD BOT CHORD	Structural Rigid ceili	I wood sheathing dire	ectly applied or 2-10 r 10-0-0 oc bracing.	-15 oc purlins.

REACTIONS. (size) 1=2-10-15, 3=2-10-15

Max Horz 1=-26(LC 8) Max Uplift 1=-3(LC 13), 3=-3(LC 13)

Max Grav 1=88(LC 1), 3=88(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Rigid ceiling directly applied or 10-0-0 oc bracing.



