

Trenco RE: 681099_Master_130 - H&H/Jackson/ 818 Soundside Rd Site Information: Edenton, NC 27932 Project Customer: H and H Project Name: Lot/Block: B Subdivision: ALL Model: Address: State: NC City: Fayetteville General Truss Engineering Criteria & Design Loads (Individual Truss Design **Drawings Show Special Loading Conditions):** Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.2 Wind Code: ASCE 7-10 Wind Speed: 130 mph Design Method: MWFRS (Envelope)/C-C hybrid Wind ASCE 7-10 Roof Load: 40.0 psf Floor Load: N/A psf Mean Roof Height (feet): 25 Exposure Category: C No. Seal# Truss Name Date No. Seal# Truss Name Date A01 A02 35 36 5/16/19 137107367 5/16/19 137107401 C25 C26 123456789111111111122222222222333333 137107368 137107402 5/16/19 5/16/19 37 38 D01 D02 137107403 137107404 137107369 A04 5/16/19 5/16/19 A05 137107370 5/16/19 5/16/19 D03 E01 A06 **3**9 137107405 137107371 16/ 19 5/16/19 137107372 **4**0 137107406 A07 16/ 19 5/16/19 41 42 137107373 137107374 I37107407 I37107408 A08 Ē02 '16/19 5/16/19 A09 **F**G01 16/195/16/19 I37107409 I37107410 G01 G02 43 137107375 A10 5/16/19 5/16/19 137107376 A11 44 5/16/19 5/16/19 A20 A21 I37107411 I37107412 137107377 45 G03 5/16/19 5/16/19 137107378 46 Ğ04 5/16/19 5/16/19 137107379 4Ť 137107413 A22 G05 5/16/19 5/16/19 137107380 A23 48 137107414 J01 5/16/19 5/16/19 137107415 137107416 137107381 4ğ ĴŎ2 A24 5/16/19 5/16/19 A25 137107382 50 <u>Ĵ03</u> 5/16/19 5/16/19 51 52 137107383 A26 137107417 J04 5/16/19 5/16/19 137107384 137107418 Ĵ05 /16/19 5/16/19 53 54 55 56 A28 B01 137107419 137107385 J06 5/16/19 5/16/19 137107386 16/19 137107420 ĴŎ7 5/16/19 137107421 137107422 J22 137107387 B02 /16/19 5/16/19 137107388 **B**03 PB0 16/19 5/16/19 57 58 59 137107389 137107423 PB02 B04 /16/19 5/16/19 137107390 137107424 **PB03** C01 16/195/16/19 137107391 137107425 **PB04** 16/19 /16/19 **6**0 137107392 16/19137107426 **PB05** 16/19 61 62 63 137107393 137107427 **PB06** 16/19 /16/19 137107394 C05 16/19 137107428 **PB07** 5/16/19 137107395 137107429 **PB08** 16/19 16/1 137107396 64 137107430 137107397 65 137107431 V02 /16/19 16/1 137107398 g 137107399 137107400 5/16/19

The truss drawing(s) referenced above have been prepared by

Truss Design Engineer's Name: Sevier, Scott My license renewal date for the state of North Carolina is December 31, 2019 **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use the building designer should verify applicability of design parameters and esti-incorporate these designs into the overall building design parameters and esti-



Sevier, Scott

May 16,2019



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Engineering By EREACO A MiTek Atfiliate 818 Soundside Road Edenton, NC 27932

MANUTURI I



		14-4-2				28-6-14		1		42-11-0	
	·	14-4-2		•		14-2-12		·		14-4-2	
Plate Offsets (X	,Y)	[2:0-8-2,0-0-8], [5:0-7-12,	0-4-4], [6:0-7	-12,0-4-4], [9:	0-8-2,0-0-8], [11:0-5-0,0-4-8]					
			•								
LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plate Grip DOL	1.15	TC	0.58	Vert(LL)	-0.43 11-13	>999	360	MT20	244/190
TCDL 10.0		Lumber DOL	1.15	BC	0.79	Vert(CT)	-0.62 11-13	>827	240		
BCLL 0.0) *	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.09 9	n/a	n/a		
BCDL 10.0		Code IRC2015/TF	912014	Matri	k-AS	Wind(LL)	0.27 13-16	>999	240	Weight: 272 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.2	TOP CHORD	Structural wood sheathing directly applied, except
BOT CHORD	2x6 SP No.1		2-0-0 oc purlins (5-1-14 max.): 5-6.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied.

- REACTIONS. (lb/size) 2=1759/0-5-8, 9=1759/0-5-8 Max Horz 2=-279(LC 10) Max Uplift 2=-451(LC 12), 9=-451(LC 13) Max Grav 2=1805(LC 2), 9=1803(LC 2)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-3063/1020, 3-5=-2750/957, 5-6=-1878/846, 6-8=-2746/957, 8-9=-3059/1020
- BOT CHORD 2-13=-740/2754, 11-13=-315/1865, 9-11=-734/2645
- WEBS 3-13=-744/576, 5-13=-265/1128, 6-11=-265/1122, 8-11=-744/576

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=451, 9=451.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

May 16,2019

818 Soundside Road Edenton, NC 27932



Continued on page 2

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REFERENCE FACE INTERCISES. To according to the operation of a second sec fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

May 16,2019

Job	Truss	Truss Type	Qty	Ply	H&H/Jackson/	
					1371	07370
681099_Master_130	A05	PIGGYBACK BASE	10	1		
					Job Reference (optional)	
Builders FirstSource,	Sumter, SC - 29153,		8.	220 s Nov	16 2018 MiTek Industries, Inc. Thu May 16 07:38:54 2019 Page	e 2
		ID:6U sN	IXmsGM6h	NZXeeE IIa	6zQrSu-?z5LYQYOYAzljZRD9DkdkgTpBSSG4bBsVBav3bzG_0	qV

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-5=-60, 5-6=-60, 6-9=-60, 19-22=-20 Concentrated Loads (lb)

Vert: 11=-100 27=-100





- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

Continued on page 2

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. 818 Soundside Road

Edenton, NC 27932

S Μ.

minin May 16,2019

Job	Truss	Truss Type	Qty	Ply	H&H/Jackson/
				-	137107371
681099_Master_130	A06	PIGGYBACK BASE	5	1	
					Job Reference (optional)
Builders FirstSource,	Sumter, SC - 29153,		8.2	220 s Nov	16 2018 MiTek Industries, Inc. Thu May 16 07:38:55 2019 Page 2
		ID:6U	sNXmsGN	//6hZXeeE	Ila6zQrSu-TAejmmY0JU5cKj0QjxFsHt0 xspvp2P?krJSb1zG qU

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-5=-60, 5-6=-60, 6-9=-60, 19-22=-20 Concentrated Loads (lb)

Vert: 11=-100 27=-100





1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Continued on page 2

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

minin May 16,2019

Job	Truss	Truss Type	Qtv	Plv	H&H/Jackson/	
			,	,	107	407070
					137	10/3/2
691000 Master 120	107	DICCYDACK DASE	16	1		
001099_10185181_130	AUT	FIGGTBACK BASE	10			
					Job Reference (optional)	
	0 1 00 00150					
Builders FirstSource,	Sumter, SC - 29153,		8.2	220 S NOV	16 2018 MITEK Industries, Inc. Thu May 16 07:38:57 2019 Pag	ge 2
			machie		A CALL BY TREACTEL KARAGA HKMIEK BEZON VOID A CALL	~~ ~
			TISGINIO	IZAGGE IIG		4S

Uniform Loads (plf) Vert: 1-5=-60, 5-6=-60, 6-10=-60, 20-23=-20 Concentrated Loads (lb) Vert: 12=-100 28=-100





1	14-4-2	28-6-14	42-11-0	1
	14-4-2	14-2-12	14-4-2	1
Plate Offsets (X,Y)	[2:0-8-6,0-0-8], [4:0-4-0,0-4-4], [4:0-0-0,0-2-12], [6:0-4-0,0-3-4], [7:0-0-0,0-2-12], [7:0-4-0,0-4-4], [9:0)-8-2,0-0-8], [11:0-5-0,0-4-8], [26:0-2-0,0-0-0],	
	[44:0-2-0.0-0-0]			

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.47 BC 0.97 WB 0.70 Matrix-AS	DEFL. in (loc) 1// Vert(LL) -0.40 11-13 >4 Vert(CT) -0.61 11-13 >4 Horz(CT) 0.09 9 9 Wind(LL) 0.17 13-55 >1	defi L/d 399 360 345 240 n/a n/a 999 240	PLATES MT20 Weight: 425 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP 11-12: : WEBS 2x4 SP 5-11: 2: OTHERS 2x4 SP REACTIONS. (lb/size Max Hu Max U May U	No.2 No.1 *Except* 2x6 SP No.2 No.3 *Except* x4 SP No.2 No.3 e) 2=1759/0-5-8, 9=1759/0-5-8 forz 2=-279(LC 10) Difft 2=-451(LC 12), 9=-451(LC 13) even 2 4175(LC 2), 9=1777(LC 2)		BRACING- TOP CHORD Structural 2-0-0 oc p BOT CHORD Rigid ceilir WEBS 1 Row at n	wood sheathing directly urlins (5-1-11 max.): 5-6 1g directly applied. nidpt 5-11	applied, except	
FORCES. (lb) - Max. TOP CHORD 2-3= BOT CHORD 2-13=- WEBS 3-13=-	Comp./Max. Ten All forces 250 (lb) or 3022/1020, 3-5=-2708/957, 5-6=-1883/8 -733/2711, 11-13=-314/1830, 9-11=-73 -747/526, 5-13=-248/1108, 5-11=-271/2	less except when shown. 47, 6-8=-2678/958, 8-9=- ¼2587 85, 6-11=-211/998, 8-11=	2992/1020 742/527			
 NOTES- (12) 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) MWFRS for reaction 3) Truss designed for w Gable End Details as 4) Provide adequate dr 5) All plates are 2x4 MT 6) Gable studs spaced 7) This truss has been 8) * This truss has been the b 9) Provide mechanical 2=451, 9=451. 10) This truss design re sheetrock be applie 11) Graphical purlin rep 12) This manufactured particular building is 	loads have been considered for this de- ult=130mph (3-second gust) Vasd=103r gable end zone and C-C Exterior(2) zon s shown; Lumber DOL=1.60 plate grip E vind loads in the plane of the truss only. s applicable, or consult qualified building ainage to prevent water ponding. F20 unless otherwise indicated. at 2-0-0 oc. designed for a 10.0 psf bottom chord live n designed for a number of 20.0psf on to ottom chord and any other members, wi connection (by others) of truss to bearin equires that a minimum of 7/16" structure and directly to the bottom chord. presentation does not depict the size or to truss is designed as an individual building is the responsibility of the building design	sign. nph; TCDL=6.0psf; BCDL e; end vertical left and rig IOL=1.60 For studs exposed to win designer as per ANSI/TF e load nonconcurrent with he bottom chord in all are th BCDL = 10.0psf. g plate capable of withsta al wood sheathing be app he orientation of the purli ig component. The suital er per ANSI TPI 1 as refe	=6.0psf; h=25ft; Cat. II; Exp C; Encl ht exposed;C-C for members and fo d (normal to the face), see Standard 'I 1. any other live loads. as where a rectangle 3-6-0 tall by 2- nding 100 lb uplift at joint(s) except lied directly to the top chord and 1/2 n along the top and/or bottom chord pility and use of this component for a erenced by the building code.	osed; rces & 1 Industry -0-0 wide (jt=lb) " gypsum any	Contraction of the second seco	SEAL 44925 M. SEVIET





I			42-11-0				I
Plate Offsets (X,Y)	[5:0-0-0,0-2-12], [5:0-4-0,0-4-4], [6:0-2-0	,0-0-0], [10:0-1-12,0-0-15]	, [11:0-5-0,0-2-13], [11:	0-0-0,0-2-	12], [16:0-0-0,0-2-	12], [16:0-5-0,0-2-13], [17	:0-1-12
	,0-0-15], [21:0-2-0,0-0-0], [22:0-0-0,0-2-	12], [22:0-4-0,0-4-4], [32:0-	·5-0,0-4-8]				
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.10	DEFL. in Vert(LL) 0.00	(loc) 26	l/defl L/d n/r 120	PLATES MT20	GRIP 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.06	Vert(CT) 0.00	26	n/r 120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.17	Horz(CT) 0.01	25	n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 380 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x6 SF	P No.2		TOP CHORD	Structura	al wood sheathing	directly applied or 6-0-0 o	c purlins, except
BOT CHORD 2x6 SF	P No.2			2-0-0 oc	purlins (6-0-0 max	k.): 11-16.	
OTHERS 2x4 SP	P No.3		BOT CHORD WEBS	Rigid cei 1 Row at	ling directly applie t midpt	d or 10-0-0 oc bracing. 13-36, 12-37, 10-38, 14-	35, 15-34, 17-33
REACTIONS. All b	earings 42-11-0.						

42-11-0

(lb) - Max Horz 2=-245(LC 10)

Max Grav All reactions 250 lb or less at joint(s) 2, 36, 37, 38, 40, 41, 42, 43, 44, 35, 34, 33, 32, 31, 30, 29, 28, 25 except 45=356(LC 19), 27=355(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-262/192, 8-9=-210/267, 9-10=-261/304, 10-11=-245/289, 11-12=-251/301,
 - 12-13--251/301, 13-14--251/301, 14-15--251/301, 15-16--251/301, 16-17--245/289, 17-18--261/304
- WEBS 3-45=-309/225, 24-27=-309/224

NOTES- (12)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 36, 37, 38, 40, 41, 42, 43, 44, 35, 34, 32, 31, 30, 29, 28 except (jt=lb) 45=194, 27=193.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.





Max Uplift All uplift 100 lb or less at joint(s) 2, 36, 37, 38, 40, 41, 42, 43, 44, 35, 34, 32, 31, 30, 29, 28 except 45=-194(LC 12), 27=-193(LC 13)



- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=461, 9=461
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any













	8-5-8	14-3-3	+	28-7-14			40-5-0	42	-11-0	
Plate Offsets (X,Y)	<u>5:0-4-0,0-3-4], [9:0-2-8,0-</u>	·2-0], [12:0-5-0,0·	4-8]	14-4-11			11-9-3	2	-0-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES I2014	CSI. TC 0.50 BC 1.00 WB 0.60 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.42 12-14 -0.75 12-14 0.09 9 0.15 12-14	l/defl >920 >516 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 416 lb	GRIP 244/190 FT = 20%	
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP 6-14: 2 OTHERS 2x4 SP REACTIONS. All be	No.2 No.2 No.3 *Except* x4 SP No.2 No.3 earings 8-1-12 except (it=le	2nath) 9=2-2-4. 1	5=0-3-8.11=0-3-8.	BRACING- TOP CHOR BOT CHOR WEBS	D Structu 2-0-0 o D Rigid c 1 Row	ral wood s c purlins (eiling direc at midpt	sheathing direct 4-8-2 max.): 5-6 ctly applied. 6-14	ly applied, except		
(Ib) - Max H Max U Max G	orz 2=245(LC 11) plift All uplift 100 lb or les 17=-118(LC 12) rav All reactions 250 lb o 1), 11=531(LC 3), 2=	ss at joint(s) 15, 1 or less at joint(s) 1471(LC 1)	1 except 2=-404(LC 1: 16 except 2=1471(LC	2), 9=-387(LC 13) 1), 9=1286(LC 1),	16=-250(LC 1 17=304(LC 1),), 15=252(L	С			
FORCES. (lb) - Max. TOP CHORD 2-3=- BOT CHORD 2-17= 11-1: 3-14=	Comp./Max. Ten All foro 2656/1026, 3-5=-2311/918 -745/2235, 16-17=-745/2 2=-741/2311, 9-11=-741/2 590/467, 5-14=-93/665, 0	ces 250 (lb) or les 8, 5-6=-1779/868 235, 15-16=-745/ 311 6-14=-255/175, 6	s except when shown. , 6-8=-2417/918, 8-9=- 2235, 14-15=-745/223 -12=-126/814, 8-12=-5	2735/1023 5, 12-14=-403/184 66/461	6,					
 NOTES- (12) 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) MWFRS (envelope) MWFRS for reaction 3) Truss designed for w Gable End Details at 4) Provide adequate dr 5) All plates are 2x4 MT 6) Gable studs spaced 7) This truss has been 8) * This truss has been 8) * This truss has been 8) * This truss has been 9) Provide mechanical (jt=lb) 2=404, 9=387 10) This truss design re sheetrock be applie 11) Graphical purlin ref 12) This manufactured particular building is 	e loads have been conside ult=130mph (3-second gu gable end zone and C-C I s shown; Lumber DOL=1. vind loads in the plane of t s applicable, or consult qu ainage to prevent water p T20 unless otherwise indic at 2-0-0 oc. designed for a 10.0 psf bc n designed for a live load d ottom chord and any othe connection (by others) of , 16=250, 17=118, 2=404. equires that a minimum of ed directly to the bottom cd presentation does not dep truss is designed as an in s the responsibility of the l	red for this desig st) Vasd=103mpl Exterior(2) zone; 60 plate grip DO he truss only. Fo alified building de onding. ated. withom chord live lo of 20.0psf on the r members, with l truss to bearing p 7/16" structural v oord. ict the size or the dividual building building designer	n. n; TCDL=6.0psf; BCDL end vertical left and rig =1.60 r studs exposed to wir ssigner as per ANSI/TF bad nonconcurrent with bottom chord in all are 3CDL = 10.0psf. late capable of withsta wood sheathing be app orientation of the purli component. The suita per ANSI TPI 1 as refe	=6.0psf; h=25ft; C ht exposed;C-C fo ad (normal to the f Pl 1. any other live loa as where a rectan inding 100 lb uplift lied directly to the n along the top an bility and use of th erenced by the bui	tat. II; Exp C; E or members and ace), see Stand ds. gle 3-6-0 tall by at joint(s) 15, 7 top chord and d/or bottom chi is component f lding code.	inclosed; d forces & dard Indus y 2-0-0 wid 11 except 1/2" gypsu ord. or any	try de um	Contraction of the second seco	CARO ESSIC SEAL 44925 GINEER M. SEULU	Rummin.
WARNING - Verify Design valid for use or a truss system. Before building design. Braci is always required for fabrication, storage, du Safety Information	design parameters and READ N hy with MiTek® connectors. This use, the building designer must in indicated is to prevent bucklin stability and to prevent collapse v ellivery, erection and bracing of trn valiable from Truss Plate Institut	OTES ON THIS AND design is based only verify the applicability g of individual truss w vith possible personal usses and truss syste e, 218 N. Lee Street,	INCLUDED MITEK REFEREI upon parameters shown, an of design parameters and pr eb and/or chord members or injury and property damage. ms, see ANSUTPI1 Suite 312, Alexandria, VA 22	VCE PAGE MII-7473 re d is for an individual bu roperly incorporate this ly. Additional tempora For general guidance Quality Criteria, DSB 314.	v. 10/03/2015 BEF(ilding component, r design into the over ry and permanent i regarding the -89 and BCSI Buil	DRE USE. not erall bracing ding Compo	onent	818 Soundside Edenton, NC 2	A MiTek Affiliate Road 7932	



F	<u>14-4-13</u> 14-4-13		<u>28-6-3</u> 14-1-5			<u>42-11-0</u> 14-4-13	
Plate Offsets (X,Y)	[3:0-5-0,0-4-8], [4:0-6-11,0-4-7], [5:0-6-1	1,0-4-7], [6:0-5-0,0-4-8], [9	9:0-5-0,0-4-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.61 BC 0.78 WB 0.70 Matrix-AS	DEFL. in Vert(LL) -0.44 Vert(CT) -0.65 Horz(CT) 0.08 Wind(LL) 0.28	(loc) 9-11 = 9-11 = 7 11-14 =	l/defl L/d >999 360 >796 240 n/a n/a >999 240	PLATES MT20 Weight: 274 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF WEBS 2x4 SF	P No.2 P No.1 P No.3		BRACING- TOP CHORD BOT CHORD	Structura 2-0-0 oc Rigid ceil	I wood sheathing d purlins (5-2-3 max. ling directly applied	irectly applied, except): 4-5.	
REACTIONS (Ib/size	2 - 1783/0.5.8 $7 - 1783/0.5.8$						

- REACTIONS. (lb/size) 2=1783/0-5-8, 7=1783/0-5-8 Max Horz 2=-279(LC 10) Max Uplift 2=-459(LC 12), 7=-459(LC 13) Max Grav 2=1826(LC 2), 7=1825(LC 2)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-2987/999, 3-4=-2654/927, 4-5=-1855/842, 5-6=-2645/923, 6-7=-2983/1000

BOT CHORD 2-11=-714/2632, 9-11=-313/1855, 7-9=-708/2534

WEBS 3-11=-688/567, 4-11=-225/1022, 5-9=-221/1017, 6-9=-692/568

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=459, 7=459.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



ENGINEERING BY EREENCO AMITEK Affiliate 818 Soundside Road Edenton, NC 27932



Plate Offsets ((X,Y)	[<u>12:0-1-12,0-0-15], [13:0</u> ·	-5-0,0-2-13], [1	3:0-0-0,0-2-	12], [16:0-5-0	0,0-2-13], [16:0-0-0	,0-2-12], [17:0-	1-12,0-0-	15], [35:0-5-0),0-4-8]	
LOADING (ps TCLL 20 TCDL 10 BCLL 0 BCDL 10	sf)).0).0).0 *).0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TI	2-0-0 1.15 1.15 YES PI2014	CSI. TC BC WB Matri	0.08 0.03 0.18 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.01	(loc) 27 27 27	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 399 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD OTHERS	2x6 SP 2x6 SP 2x4 SP	No.2 No.1 No.3				BRACING- TOP CHOP BOT CHOP WEBS	RD RD	Structu 2-0-0 o Rigid c 1 Row	ral wood c purlins eiling dire at midpt	sheathing dir (6-0-0 max.): ectly applied o 1	 rectly applied or 6-0-0 c 13-16. or 10-0-0 oc bracing. 4-39, 12-40, 11-41, 15- 	oc purlins, except -38, 17-37, 18-36
REACTIONS.	All be	arings 42-11-0.										

(lb) - Max Horz 2=-279(LC 10)

Max Uplift All uplift 100 b or less at joint(s) 2, 27, 39, 40, 41, 43, 44, 45, 46, 47, 48, 38, 36, 35, 34, 33, 32, 31, 30 except 49=-136(LC 12), 29=-127(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 27, 39, 40, 41, 43, 44, 45, 46, 47, 48, 49, 38, 37, 36, 35, 34, 33, 32, 31, 30, 29

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-337/211, 9-10=-193/265, 10-11=-247/303, 11-12=-296/345, 12-13=-276/325, 13-14=-283/337, 14-15=-283/337, 15-16=-283/337, 16-17=-276/325, 17-18=-297/345, 18-19=-246/287

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 27, 39, 40, 41, 43, 44, 45, 46, 47, 48, 38, 36, 35, 34, 33, 32, 31, 30 except (jt=lb) 49=136, 29=127.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

MILLIN \mathbf{C} Annun manut SEAL 044925 S Μ. (IIIIIIIII) May 16,2019

TRENCIO A MITEK Affiliate 818 Soundside Road Edenton, NC 27932



	14-4-13 14-4-13		28-6-3 14-1-5		42-8-0 14-1-13	
Plate Offsets (X,Y)	[3:0-5-0,0-4-8], [4:0-6-11,0-4-7], [5:0-6-1	1,0-4-7], [6:0-5-0,0-4-8], [7	<u>7:0-0-11,0-5-9], [7:0-0-6</u>	6,0-0-10], [8:0-5-0,0-4-8]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.66 BC 0.78 WB 0.70 Matrix-AS	DEFL. in Vert(LL) -0.44 Vert(CT) -0.66 Horz(CT) 0.09 Wind(LL) 0.29	l (loc) l/defl L/d 8-10 >999 360 8-10 >773 240 7 n/a n/a 10-13 >999 240	PLATES MT20 Weight: 270 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 S BOT CHORD 2x6 S WEBS 2x4 S WEDGE Right: 2x4 SP No.3	P No.2 P No.1 P No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathi 2-0-0 oc purlins (5-2-2 r Rigid ceiling directly ap	ng directly applied, except nax.): 4-5. plied.	

REACTIONS. (lb/size) 2=1774/0-5-8, 7=1706/Mechanical Max Horz 2=278(LC 9) Max Uplift 2=-458(LC 12), 7=-418(LC 13) Max Grav 2=1817(LC 2), 7=1762(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2969/994, 3-4=-2635/922, 4-5=-1834/838, 5-6=-2595/910, 6-7=-2908/981
- BOT CHORD 2-10=-736/2607, 8-10=-340/1834, 7-8=-717/2466
- WEBS 3-10=-692/568, 4-10=-225/1027, 5-8=-210/970, 6-8=-647/555

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=458, 7=418.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



818 Soundside Road Edenton, NC 27932



	14-4-13	19-5-8	23-5-8 28	3-6-3	42-8-0			
	14-4-13	5-0-11	4-0-0 5-	0-11	14-1-13			
Plate Offsets (X,Y) [2:0-0-0,0-1-1], [3:0-5-0,0-4-8], [4:0-7-12,0-4-4], [5:0-6-11,0-4-7], [6:0-5-0,0-4-8], [7:0-0-11,0-5-9], [7:0-0-6,0-0-10], [8:0-5-0,0-4-8]								
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.67 BC 0.40 WB 0.68 Matrix-AS	DEFL. in Vert(LL) -0.32 Vert(CT) -0.54 Horz(CT) 0.08 Wind(LL) 0.28	i (loc) l/defi L/d 12-19 >999 360 9-10 >950 240 7 n/a n/a 12-19 >999 240	PLATES GRIP MT20 244/190 Weight: 292 lb FT = 2	20%		
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP DSS WEBS 2x4 SP No.3 *Except*			BRACING- TOP CHORD BOT CHORD	RACING- OP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (4-11-10 max.): 4-5. OT CHORD Rigid ceiling directly applied.				
13-14: 2x4 SP No.2WEBS1 Row at midpt13-14WEDGELeft: 2x4 SP No.3, Right: 2x4 SP No.3								
REACTIONS. (Ib/s	ize) 2=1874/0-5-8, 7=1806/Mechanical							

Max Uplift 2=-358(LC 12), 7=-318(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-3053/776, 3-4=-2654/699, 4-5=-1983/686, 5-6=-2613/691, 6-7=-2989/768
- BOT CHORD
 2-12=-549/2575, 10-12=-185/1884, 9-10=-185/1884, 8-9=-185/1884, 7-8=-537/2517

 WEBS
 3-12=-677/582, 12-13=-120/944, 4-13=-106/970, 5-14=-98/904, 8-14=-105/877,
- 6-8=-629/574

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) 200.0lb AC unit load placed on the bottom chord, 21-4-0 from left end, supported at two points, 5-0-0 apart.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=358, 7=318.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Contraction of the SEAL 044925 //////// May 16,2019





	<u>14-4-13</u> 14-4-13	<u> </u>	<u>23-5-8</u> <u>28-6-</u> 4-0-0 5-0-1	3 1	<u>42-11-0</u> 14-4-13		
Plate Offsets (X,Y)	[2:0-0-0,0-1-1], [3:0-5-0,0-4-8], [4:0-6-11]	0-4-7], [5:0-6-11,0-4-7], [6:0-5-0,0-4-8], [7:Edge	,0-1-1], [9:	:0-5-0,0-4-8]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.66 BC 0.41 WB 0.68 Matrix-AS	DEFL. i Vert(LL) -0.3: Vert(CT) -0.5: Horz(CT) 0.0' Wind(LL) 0.2:	n (loc) 2 9-23 2 10-11 7 7 8 13-20	l/defl L/d >999 360 >981 240 n/a n/a >999 240	PLATES MT20 Weight: 297 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP DSS WEBS 2x4 SP No.3 *Except* 14-15: 2x4 SP No.2 WEDGE Left: 2x4 SP No.3, Right: 2x4 SP No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structur 2-0-0 oc Rigid ce 1 Row a	ral wood sheathing direc c purlins (4-11-12 max.): eiling directly applied. at midpt 14-1	tly applied, except 4-5. 15		

- REACTIONS. (lb/size) 2=1884/0-5-8, 7=1883/0-5-8 Max Horz 2=-279(LC 10) Max Uplift 2=-359(LC 12), 7=-360(LC 13)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-3071/780, 3-4=-2673/703, 4-5=-2003/689, 5-6=-2664/702, 6-7=-3067/783
- BOT CHORD
 2-13=-524/2591, 11-13=-157/1906, 10-11=-157/1906, 9-10=-157/1906, 7-9=-524/2587

 WEBS
 3-13=-673/582, 13-14=-120/937, 4-14=-106/965, 5-15=-105/957, 9-15=-112/930,
- 6-9=-676/584

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) 200.0lb AC unit load placed on the bottom chord, 21-4-0 from left end, supported at two points, 5-0-0 apart.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=359, 7=360.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







			42-11-0				1
Plate Offsets (X,Y)	[2:0-0-0,0-0-1], [2:0-0-1,0-4-6], [2:Edge, [25:Edge,0-0-1], [33:0-5-0,0-4-8]	0-0-1], [5:0-3-0,0-3-0], [11	1:0-3-0,0-1-14], [16:0-3-0),0-1-14], [22:0-3-0,	0-3-0], [25:0-0-1,0)-4-6], [25:0-0-0,	0-0-1],
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.11 BC 0.04 WB 0.14 Matrix-S	DEFL. ir Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.01	n (loc) l/defl 26 n/r 26 n/r 25 n/a	L/d 120 120 n/a	PLATES MT20 Weight: 356 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP OTHERS 2x4 SP WEDGE Left: 2x4 SP No.2, Righ	P No.2 P No.2 P No.3 ht: 2x4 SP No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood s 2-0-0 oc purlins (6 Rigid ceiling direc 1 Row at midpt	heathing directly a 5-0-0 max.): 11-16 tly applied or 10-0 13-37, 17-34,	applied or 6-0-0 (5. 0-0 oc bracing. 12-38, 10-39, 9 18-33	oc purlins, except 41, 14-36, 15-35,
(ib) - Max H (ib) - Max H Max U Max G FORCES. (ib) - Max. TOP CHORD 2-3=- 13-14 24-25 BOT CHORD 2-47= 42-44 36-37 31-31	 Jorz 2=-253(LC 10) Iplift All uplift 100 lb or less at joint(s) 2, 46, 36, 35, 32, 31, 30, 29, 28, 25 ex 27=-136(LC 13) Jrav All reactions 250 lb or less at joint(46, 47, 36, 35, 34, 33, 32, 31, 30, 2 Comp./Max. Ten All forces 250 (lb) or -326/182, 9-10=-235/274, 10-11=-220/25 A=-224/270, 14-15=-224/270, 15-16=-22 S=-252/180 =-173/260, 46-47=-173/260, 45-46=-173, 3=-176/262, 41-42=-176/262, 30-31=-17 7=-176/262, 30-31=-176/262, 30-31=-17 	37, 38, 39, 41, 42, 43, 44 cept 47=-145(LC 12), 33: s) 2, 37, 38, 39, 41, 42, 4 9, 28, 27, 25 less except when shown i9, 11-12=-224/270, 12-1: 4/270, 16-17=-220/260, 1 /260, 44-45=-176/262, 43 6/262, 33-34=-176/262, 3 6/262, 33-34=-176/262, 3	4, 45, =-103(LC 13), 3, 44, 45, - - - - - - - - - - - - -				
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) MWFRS for reaction 3) Truss designed for w Gable End Details a: 4) Provide adequate dr 5) All plates are 2x4 M 6) Gable requires conti 7) Gable studs spaced 8) This truss has been will fit between the b 10) Provide mechanica Contifice#3orf#ade 26.	2=-177203, 23-30=-177203, 23-30=-1772-174/260 a loads have been considered for this de /ult=130mph (3-second gust) Vasd=103r gable end zone and C-C Exterior(2) zor s shown; Lumber DOL=1.60 plate grip I wind loads in the plane of the truss only. Is applicable, or consult qualified building rainage to prevent water ponding. T20 unless otherwise indicated. inuous bottom chord bearing. I at 2-0-0 oc. designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on t pottom chord and any other members. al connection (by others) of truss to beari 36, 35, 32, 31, 30, 29, 28, 25 except (jt=	sign. mph; TCDL=6.0psf; BCDL ie; end vertical left and rig DOL=1.60 For studs exposed to wir designer as per ANSI/Tf e load nonconcurrent with he bottom chord in all are ng plate capable of withs lb) 47=145, 33=103, 27=	L=6.0psf; h=25ft; Cat. II; ht exposed;C-C for men nd (normal to the face), s PI 1. h any other live loads. has where a rectangle 3- tanding 100 lb uplift at jo 136.	Exp C; Enclosed; nbers and forces & see Standard Indust 6-0 tall by 2-0-0 wid pint(s) 2, 37, 38, 39,	iry le 41,		SEAL 944925 M. SEVIEN ay 16,2019
WARNING - Verify Design valid for use of a truss system. Before	r design parameters and READ NOTES ON THIS A nly with MiTek® connectors. This design is based o e use, the building designer must verify the applicat	ND INCLUDED MITEK REFERE Inly upon parameters shown, an ility of design parameters and p	NCE PAGE MII-7473 rev. 10/03 d is for an individual building or roperly incorporate this design	2015 BEFORE USE.			

besign valid to get only with with the contractions. This design is based only door parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	H&H/Jackson/
681099 Master 130	A25	GABLE	1	1	137107382
001000_Waster_100	123	GABLE			Job Reference (optional)
Builders FirstSource,	Sumter, SC - 29153,		8.2	220 s Nov	16 2018 MiTek Industries, Inc. Thu May 16 07:39:12 2019 Page 2
		ID:6U_	sNXmsGM	/6hZXeeE	_lla6zQrSu-TRA8KamgJiECtKphC?2sTSD??iq5luxVe_xsiYzG_qD

NOTES-

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





PHILIPPINI PHILIPPINI minin May 16,2019

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REFERENCE FACE INTERCISES. To according to the operation of a second sec fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Edenton, NC 27932

UNUTURITY I

Job	Truss	Truss Type	Qty	Ply	H&H/Jackson/
004000 Master 400	4.07				137107384
681099_Master_130	A27	GABLE	1	1	Job Reference (optional)
Builders FirstSource,	Sumter, SC - 29153,		8.2	220 s Nov	16 2018 MiTek Industries, Inc. Thu May 16 07:39:16 2019 Page 2
		ID:6U s	NXmsGM	6hZXeeE	IIa6zQrSu-MCQfAypBMxkeMx6SRr7odIObIK?xEZP5Zcv3rJzG q9

NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



May 16,2019



```
Max Grav All reactions 250 lb or less at joint(s) 17, 2, 23, 24, 25, 26, 27, 22, 21, 20, 19 except 28=326(LC 19), 18=299(LC 24), 16=383(LC 1)
```

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS 3-28=-290/214, 15-18=-262/204

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 2, 23, 24, 25, 26, 27, 22, 21, 19 except (jt=lb) 28=184, 18=157, 16=102.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 17.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



ENGINEERING BY REENCO AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932



		<u>12-3-3</u> 12-3-3		<u>22-6-12</u> 10-3-9		28-6-1	4 3	2-3-8 -8-10
Plate Offse	ets (X,Y)	[2:0-0-12,0-5-10], [2:0-0-6,0-0-11], [4:0-	3-0,0-1-14], [5:0-3-0,0-1-14	4], [6:0-4-0,0-3-0], [9:0-3	3-8,0-3-0]			
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 10.0 0.0 * 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.65 BC 0.85 WB 0.72 Matrix-AS	DEFL. ir Vert(LL) -0.36 Vert(CT) -0.68 Horz(CT) 0.06 Wind(LL) 0.07	(loc) //d 9-11 >7 11-14 >4 8 r 11-14 >9	lefi L/d 57 360 100 240 n/a n/a 199 240	PLATES MT20 Weight: 149 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 *Except* 2-10: 2x4 SP No.1 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3				BRACING- TOP CHORD BOT CHORD WEBS	Structural w 2-0-0 oc pu Rigid ceiling 1 Row at m	wood sheathing dire Irlins (6-0-0 max.): - g directly applied. hidpt 5-	ectly applied, except 4-5. 9	
REACTIONS. (lb/size) 2=842/0-5-8, 9=1669/0-5-8, 8=130/0-1-8 Max Horz 2=-255(LC 13) Max Uplift 2=-264(LC 12), 9=-414(LC 13), 8=-56(LC 13) Max Grav 2=859(LC 23), 9=1669(LC 1), 8=133(LC 20)								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1159/458, 3-4=-781/317, 4-5=-661/341, 5-6=-40/838, 6-7=-247/839 BOT CHORD 2-11=-330/1036, 9-11=-105/499, 7-9=-775/385 WEBS 3-11=-559/358, 5-11=-95/500, 5-9=-1472/341, 6-9=-519/384								
NOTES								

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 8.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 2=264, 9=414.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



818 Soundside Road Edenton, NC 27932





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-3=-1448/188, 3-4=-270/96, 4-6=-272/96, 6-8=-1457/187

BOT CHORD 1-11=-28/995, 9-11=-24/1003, 8-9=-24/991

WEBS 6-9=-14/595, 3-6=-901/370, 3-11=-14/601

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces &

MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (5.0 psf) on member(s). 3-6; Wall dead load (5.0 psf) on member(s).6-9, 3-11

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 9-11

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8.

8) Attic room checked for L/360 deflection.

9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



818 Soundside Road Edenton, NC 27932



- will fit between the bottom chord and any other members.
- 5) Ceiling dead load (5.0 psf) on member(s). 2-5; Wall dead load (5.0 psf) on member(s). 5-8, 2-9
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 8-9
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 7.
 8) Attic room checked for L/360 deflection.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.









FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-945/350, 3-4=-723/446, 4-6=-945/350

BOT CHORD 1-7=-131/683, 6-7=-55/627

WEBS 3-7=-224/493, 4-7=-224/493

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=172, 6=172.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.





3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (5.0 psf) on member(s). 3-6; Wall dead load (5.0 psf) on member(s).6-9, 3-11

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 9-11

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8.

8) Attic room checked for L/360 deflection.

9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Ceiling dead load (5.0 psf) on member(s). 4-8; Wall dead load (5.0 psf) on member(s).8-12, 4-14
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 12-14
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

9) Attic room checked for L/360 deflection.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPRETENCE PAGE MIT-1473 TeV. 100322010 SECORE 052. Design valid for use only with MITEK @ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-98 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.




being value to use only wint histo contractions. This designer must early be applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must be applicability of the overall design parameters and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPII Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=171, 7=171.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=164, 7=168.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





Job	Truss	Truss Type	Qty	Ply	H&H/Jackson/		137107400
681099_Master_130	C24	GABLE	1	1	Job Potoronas (anti	al)	137 107400
Builders FirstSource,	Sumter, SC - 29153,	1		8.220 s Nov	16 2018 MiTek Industri	es, Inc. Thu May 16 (17:39:35 2019 Page 1
	-1-3-8	7-11-0	5-۵-۵		21-7-0 7-11-0	22-10-8	_iəbriuuxə?a?i2&_ps
	1-3-8	7-11-0	5-9-0		7-11-0	1-3-6	0
			3 9 3 3-0-0	4x6 \\ 10 11 X		3x6 14 15 16 8 9 6 9 6	Scale = 1:58.0
	29 28 3x8 6x8 =	21 25 25 24 25 24 24 24 25 24 24	20 22	21	20 19 1	4x6	
Plate Offsets (X,Y) [7	:0-2-8,Edge], [10:0-2-8,Edge].	[17:Edge,0-3-8], [28:0-3-8,0-3-1	21-7-0			1	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-C Plate Grip DOL 1.11 Lumber DOL 1.11 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. 5 TC 0.23 5 BC 0.12 5 WB 0.18 Matrix-S	DEFL. Vert(LL) - Vert(CT) - Horz(CT)	in (loc) 0.01 16 0.01 16 0.00 17	l/defl L/d n/r 120 n/r 120 n/a n/a	PLATES MT20 Weight: 191 lt	GRIP 244/190 • FT = 20%
LUMBER- TOP CHORD 2x4 SP N BOT CHORD 2x6 SP N WEBS 2x4 SP N 2-28: 2x4 OTHERS 2x4 SP N REACTIONS. All bear (lb) - Max Hor Max Upli	lo.2 lo.2 lo.2 *Except* I SP No.3 lo.3 rings 21-7-0. z 29=-379(LC 10) ft All uplift 100 lb or less at jc 17=-111(LC 11), 26=-192(L 13), 19=-133(LC 13), 18=-3 v All reactions 250 lb or less 29=557(LC 9), 17=288(LC 1 18=260(LC 11)	bint(s) 23, 25, 22 except 29=-467 C 12), 27=-164(LC 12), 28=-424 75(LC 13) at joint(s) 25, 26, 27, 21, 20, 19 13), 23=285(LC 22), 28=447(LC	BRACING- TOP CHORD BOT CHORD WEBS 7(LC 10), 4(LC 12), 20=-211(LC except 10), 22=281(LC 22),	Structur except e Rigid ce 1 Row a	al wood sheathing dirr end verticals, and 2-0- illing directly applied o at midpt 8-	ectly applied or 6-0-0 0 oc purlins (10-0-0 r 6-0-0 oc bracing. 23, 6-25, 9-22, 11-2) oc purlins, max.): 7-10. 1
FORCES. (lb) - Max. Ct TOP CHORD 2-29=-5 7-8=-22 BOT CHORD 28-29=- WEBS 14-18=	omp./Max. Ten All forces 25 529/447, 2-3=-293/284, 3-4=-2 44/296, 8-9=-244/296, 9-10=-2 -347/325 -267/219, 2-28=-397/434	0 (lb) or less except when show 59/270, 5-6=-278/340, 6-7=-239 44/296, 10-11=-239/278, 11-12=	n. 9/278, =-278/327				
 NOTES- 1) Unbalanced roof live ld 2) Wind: ASCE 7-10; Vul MWFRS (envelope) ga MWFRS for reactions : 3) Truss designed for win Gable End Details as a 4) Provide adequate drai 5) All plates are 2x4 MT2 6) Gable requires continu 7) Truss to be fully sheat 8) Gable studs spaced at 9) This truss has been de 10) * This truss has been will fit between the bo 11) Provide mechanical of (jt=lb) 29=467, 17=1* 12) Graphical purlin repro 	bads have been considered fo t=130mph (3-second gust) Va able end zone and C-C Exterior shown; Lumber DOL=1.60 plat d loads in the plane of the true applicable, or consult qualified nage to prevent water ponding to unless otherwise indicated. Jour bottom chord bearing. hed from one face or securely t 2-0-0 oc. sesigned for a 10.0 psf bottom of d designed for a live load of 20 bottom chord and any other mer connection (by others) of truss 11, 26=192, 27=164, 28=424, esentation does not depict the	r this design. sd=103mph; TCDL=6.0psf; BCD or(2) zone; end vertical left and r te grip DOL=1.60 ss only. For studs exposed to w building designer as per ANSI/7 g. braced against lateral movemen chord live load nonconcurrent wi .0psf on the bottom chord in all a mbers, with BCDL = 10.0psf. to bearing plate capable of with 20=211, 19=133, 18=375. size or the orientation of the pur	DL=6.0psf; h=25ft; Ca right exposed;C-C for vind (normal to the fac TPI 1. nt (i.e. diagonal web). ith any other live loads areas where a rectang astanding 100 lb uplift rlin along the top and/	t. II; Exp C; Er members and e), see Stand s. gle 3-6-0 tall b at joint(s) 23, or bottom cho	nclosed; I forces & ard Industry by 2-0-0 wide 25, 22 except ord.	Annum Human	SEAL 044925 MGINEER M. SEVIER
WARNING - Verify de Design valid for use only a truss system. Before u- building design. Bracing	sign parameters and READ NOTES (with MiTek® connectors. This design se, the building designer must verify th indicated is to prevent buckling of indi	ON THIS AND INCLUDED MITEK REFER is based only upon parameters shown, a e applicability of design parameters and vidual truss web and/or chord members	RENCE PAGE MII-7473 rev. and is for an individual build properly incorporate this de only. Additional temporary	10/03/2015 BEFO ing component, nu sign into the over and permanent b	IRE USE. ot rall racing		

building design. Bracing indicated is to prevent buckling of individual russ web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=166, 7=166.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)

11=176, 7=176.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.











Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

A MITek Affiliat 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H/Jackson/	
					13	37107404
681099_Master_130	D02	Common Girder	5	2		
				-	Job Reference (optional)	
Builders FirstSource,	Sumter, SC - 29153,		8.	220 s Nov	16 2018 MiTek Industries, Inc. Thu May 16 07:39:39 2019 Pa	age 2
		ID:6U	sNXmsG	M6hZXeel	E IIa6zQrSu-AdJL?p4cy?eNcUNtIA1B39raLb f7?kUsh n8Uz0	G po

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 11=-1686(B) 12=-1780(B) 13=-1780(B) 14=-1780(B)



Job	Truss	Truss Type	Qty	Ply	H&H/Jackson/	
681099 Master 130	D03	Common Girder	3			137107405
Puildoro EirotSouroo	Sumter SC 20152			2	Job Reference (option	al)
Builders FirstSource,	Sumer, SC - 29153,	ID:6U_	sNXmsGM	220 S NOV ShZXeeE_	Ila6zQrSu-fqskC95FjJm	nEEdy3rtYQbMNnp?KGsUMd4LjLgwzG_pn
		4-7-0	<u>9-2-</u> 4-7-	0 0	———————————————————————————————————————	
		4x6				Scale = 1:39.3
	I	3				
		12.00 12				
	-4	4x6 1/			4x6 Ň	
	ي 1.	2			4	
	0-1- 0-4-				1-0-1	
			17 HTU26			
		4x12 3x8	111020	4	x12	
		7x10 — H1026		/x1	0 =	
		4-7-0 4-7-0	9-2- 4-7-	0 0		
Plate Offsets (X,Y) [1:0-6-4,0-1-8], [1:0-2-8,0-1-14],	[5:0-6-4,0-1-8], [5:0-2-8,0-1-14], [6:0-5-12,0-1-4	8]			
LOADING (psf)	SPACING- 2-0-0 Plate Grip DOI 1.15	CSI. DEFL.	in	(loc)	I/defl L/d	PLATES GRIP
TCDL 10.0	Lumber DOL 1.15	BC 0.25 Vert(CT	.) 0.03 Г) -0.04	6-9	>999 240	WT20 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr NC Code IRC2015/TPI2014	WB 0.43 Horz(C Matrix-MS	T) 0.01	5	n/a n/a	Weight: 159 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x8 SP WEBS 2x4 SP SLIDER Left 2x8	No.2 DSS No.2 SP DSS 1-11-12, Right 2x8 SF	BRACII TOP CH BOT CH 2 DSS 1-11-12	NG- Hord Hord	Structur Rigid ce	al wood sheathing dire iling directly applied o	ectly applied or 6-0-0 oc purlins. r 10-0-0 oc bracing.
REACTIONS. (Ib/size) Max Ho Max Up) 1=3194/0-5-8, 5=2760/0-5-8 rz 1=145(LC 5) vlift 1=-1202(LC 9), 5=-974(LC 8	3)				
FORCES. (lb) - Max. 0 TOP CHORD 1-3=-2 BOT CHORD 1-6=-6 WEBS 3-6=-1	Comp./Max. Ten All forces 250 2606/1043, 3-5=-2685/1057 363/1842, 5-6=-663/1842 316/3479) (Ib) or less except when shown.				
NOTES- 1) 2-ply truss to be conr Top chords connecte Bottom chords conne Webs connected as f 2) All loads are conside ply connections have 3) Unbalanced roof live	hected together with 10d (0.131' d as follows: 2x6 - 2 rows stagg cted as follows: 2x8 - 2 rows sta ollows: 2x4 - 1 row at 0-9-0 oc. red equally applied to all plies, e been provided to distribute only loads have been considered for	(x3") nails as follows: ered at 0-9-0 oc. aggered at 0-5-0 oc. except if noted as front (F) or back (B) face in th / loads noted as (F) or (B), unless otherwise ind this design.	he LOAD C dicated.	ASE(S) s	ection. Ply to	
 Wind: ASCE 7-10; VI MWFRS (envelope) of 5) This truss has been of 6) * This truss has been will fit between the bo 7) Provide mechanical of 1=1202, 5=974. 	JIT=130mph (3-second gust) Vac gable end zone; end vertical left designed for a 10.0 psf bottom c designed for a live load of 20.0 ottom chord and any other memi connection (by others) of truss to	sd=103mph; 1CDL=6.0pst; BCDL=6.0pst; h=2t and right exposed; Lumber DOL=1.60 plate gri hord live load nonconcurrent with any other live psf on the bottom chord in all areas where a re bers. b bearing plate capable of withstanding 100 lb u	ip DOL=1.(e loads. ctangle 3-6 uplift at joir	Exp C; Er 50 5-0 tall by it(s) excel	ctosed; 2-0-0 wide ot (jt=lb)	OR ESSION NE
 8) Use Simpson Strong- starting at 2-3-4 from 9) Fill all nail holes when 	Tie HTU26 (20-10d Girder, 14- the left end to 6-3-4 to connect re hanger is in contact with lumb	10dx1 1/2 Truss, Single Ply Girder) or equivale truss(es) to back face of bottom chord. ver.	nt spaced	at 2-3-0 o	c max.	SEAL 044925
LOAD CASE(S) Stand	ard					
 Dead + Roof Live (ba Uniform Loads (plf) 	lanced): Lumber Increase=1.15	, Plate Increase=1.15				S WGINEEN W
Vert: 1-3=-60 Concentrated Loads	0, 3-5=-60, 7-11=-20 (lb)					M. SEMIN
Vert: 15=-16	86(B) 16=-1786(B) 17=-1786(B))				May 16 2010
						Way 10,2019



ENGINEERING B





A MiTek Affi 818 Soundside Road Edenton, NC 27932



2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 6=126.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







Vert: 5=-86 11=-78(B) 12=-78(B) 13=-78(B) 14=-78(B) 15=-78(B) 16=-78(B) 17=-78(B) 18=-78(B) 19=-78(B) 20=-78(B) 10=-78(B) 10=-

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

May 16,2019



818 Soundside Road Edenton, NC 27932



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=228. 6=228.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

7) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







818 Soundside Road Edenton, NC 27932



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=240, 6=240.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



L			19-11-0					
			19-11-0					
Plate Offsets (X,Y)	[5:0-1-12,0-0-15], [6:0-3-0,0-1-14], [6:0-0	<u> </u>	4], [9:0-0-0,0-1-12], [10:	0-1-12,0	-0-15]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.16 BC 0.05 WB 0.06 Matrix-S	DEFL. in Vert(LL) -0.01 Vert(CT) -0.01 Horz(CT) 0.00	(loc) 14 14 15	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 114 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S OTHERS 2x4 S	P No.2 P No.2 P No.3 P No.3		BRACING- TOP CHORD BOT CHORD	Structu except Rigid c	ral wood end verti eiling dire	sheathing dire cals, and 2-0- ctly applied o	ectly applied or 6-0-0 c 0 oc purlins (6-0-0 ma r 6-0-0 oc bracing.	oc purlins, x.): 6-9.

REACTIONS. All bearings 19-11-0.

(lb) - Max Horz 25=-159(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 25, 15, 21, 22, 23, 20, 17 except 24=-128(LC 12), 16=-121(LC 13) Max Grav All reactions 250 lb or less at joint(s) 25, 15, 21, 22, 23, 24, 20, 18, 17, 16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Provide adequate drainage to prevent water ponding.
- All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 15, 21, 22, 23, 20, 17 except (jt=lb) 24=128, 16=121.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







Horz(CT)

BRACING-

BOT CHORD

-0.00

n/a

Rigid ceiling directly applied.

2

n/a

Weight: 23 lb

Structural wood sheathing directly applied, except end verticals.

FT = 20%

BCDL	10.0

LUMBER-

BCLL

TOP CHORD2x6 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

0.0

REACTIONS. (lb/size) 2=257/0-3-0, 4=164/0-1-8 Max Horz 2=73(LC 8) Max Uplift 2=-170(LC 8), 4=-118(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Rep Stress Incr

Code IRC2015/TPI2014

YES

NOTES- (8)

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

WB

Matrix-AS

0.00

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=170, 4=118.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







		0-0	-0									
Plate Offs	ets (X,Y)	[2:0-2-1,0-0-1]										
LOADING TCLL	i (psf) 20.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC	0.23	DEFL. Vert(LL)	in 0.07	(loc) 4-9	l/defl >992	L/d 240	PLATES MT20	GRIP 244/190
TCDL	10.0	Lumber DOL Rep Stress Incr	1.15 VES	BC	0.28	Vert(CT)	-0.05	4-9 2	>999	240 n/a		
BCDL	10.0	Code IRC2015/TF	912014	Matri	x-AS	1012(01)	-0.00	2	n/a	Π/a	Weight: 27 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

Pl

TOP CHORD 2x6 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=295/0-3-0, 4=205/0-1-8 Max Horz 2=86(LC 8) Max Uplift 2=-193(LC 8), 4=-148(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

0-6-8

NOTES-(8)

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=193. 4=148.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MILER KETEKERICE FAGE MILETATION, INVERTIGATION AND INVERTI AND INVERTIGATION AND INVERTIGATION AND INVERTIGATION AND fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





4x6 =

ł

Plate Offsets (X V) [3.0-0-11 0-1-8]

Fible Offsets (A, I)	[3.0-0-11,0-1-0]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.82 BC 0.57 WB 0.00 Matrix-S	DEFL. ir Vert(LL) -0.02 Vert(CT) 0.04 Horz(CT) 0.00	(loc) l/defl 1 n/r 1 n/r 4 n/a	L/d 120 120 n/a	PLATES MT20 Weight: 29 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF	2 No.2 2 No.2		BRACING- TOP CHORD	Structural wood except end verti	sheathing dir	ectly applied or 5-3-4	oc purlins,

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS 2x6 SP No.2 REACTIONS. (lb/size) 4=306/7-11-8, 2=365/7-11-8 Max Horz 2=113(LC 11)

Max Uplift 4=-114(LC 12), 2=-156(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(8)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=114. 2=156.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







	1				7-11-8						1
	Γ				7-11-8						1
Plate Offsets (X,Y)	[2:0-1-0,Edge]										
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.51 0.41 0.18	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.19 -0.14 -0.01	(loc) 5-8 5-8 5	l/defl >493 >676 n/a	L/d 240 240 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2	2014	Matrix	k-AS						Weight: 33 lb	FT = 20%
LUMBER-					BRACING						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

- 2x6 SP No.2 *Except* 3-5: 2x4 SP No.3
- REACTIONS. (lb/size) 2=365/0-3-0, 5=306/0-1-8 Max Horz 2=116(LC 8) Max Uplift 2=-245(LC 8), 5=-215(LC 8)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-469/548
- BOT CHORD 2-5=-635/444
- WFBS 3-5=-450/596

NOTES-(8)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=245, 5=215.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPRETENCE PAGE MIT-1473 TeV. 100322010 SECORE 052. Design valid for use only with MITEK @ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-98 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





3x8 =

Plate Offsets (X,Y)-- [2:0-4-0,0-2-1], [3:0-3-3,Edge], [4:Edge,0-1-8]

ł

	x / 1		0 1/1 0	1								
LOADIN TCLL TCDL	G (psf) 20.0 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI. TC BC	0.87 0.61	DEFL. Vert(LL) Vert(CT)	in -0.02 0.04	(loc) 1 1	l/defl n/r n/r	L/d 120 120	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2015/TI	YES PI2014	WB Matri	0.00 x-S	Horz(CT)	0.00	4	n/a	n/a	Weight: 28 lb	FT = 20%
	R- 0RD 2y4 SE	2 No 2					20	Structu	ral wood	sheathing di	rectly applied or 2-2-0	

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

REACTIONS. (lb/size) 4=310/7-11-8, 2=368/7-11-8 Max Horz 2=154(LC 8) Max Uplift 4=-133(LC 12), 2=-139(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-297/71

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=133, 2=139.
- 9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 4.







2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=371, 5=411.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







.

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	*	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matrix	0.60 0.40 0.14 <-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.02 0.03 -0.00	(loc) 1 1 7	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 54 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2 BOT CHORD 2 WEBS 2 OTHERS 2	2x4 SP 2x4 SP 2x4 SP 2x4 SP	No.1 No.1 No.3 No.3				BRACING- TOP CHOF BOT CHOF	RD RD	Structu except Rigid ce	ral wood end verti eiling dire	sheathing dir cals. ectly applied o	ectly applied or 6-0-0 or 6-0-0 oc bracing.	oc purlins,

REACTIONS. All bearings 13-11-8.

(lb) - Max Horz 2=259(LC 8)

3-10=-537/485

Max Uplift All uplift 100 lb or less at joint(s) 2, 7 except 8=-105(LC 12), 9=-224(LC 1), 10=-327(LC 12) Max Grav All reactions 250 lb or less at joint(s) 7, 9 except 2=309(LC 1), 8=263(LC 1), 10=766(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Bearing at joint(s) 2, 7, 8, 9, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7 except (jt=lb) 8=105, 9=224, 10=327.

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 8, 9, 10.







LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	-0.02	4-9	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	-0.05	4-9	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code IRC2015/TP	12014	Matri	x-AS	Wind(LL)	0.02	4-9	>999	240	Weight: 27 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

REACTIONS. (lb/size) 2=295/0-3-0, 4=205/0-1-8 Max Horz 2=90(LC 11) Max Uplift 2=-120(LC 8), 4=-78(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This term has been designed forced 0.0 active.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=120.

- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.







Scale = 1:7.8



2-0-0 2-0-0 Plate Offsets (X Y)--- [3:0-3-0 Edge]

LOADING) (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.02	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2	2014	Matrix	x-S						Weight: 5 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

0-1-12

REACTIONS. (lb/size) 1=-1/2-0-0, 5=45/2-0-0, 2=87/2-0-0 Max Horz 1=26(LC 11) Max Uplift 1=-18(LC 8), 5=-13(LC 13), 2=-26(LC 12) Max Grav 1=16(LC 11), 5=45(LC 1), 2=94(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

0-0-

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 2.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.









Scale = 1:7.8

2x4 = 2x4 =

Plate Offsets (X,Y)-- [3:0-3-0.Edge]

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

0-1-12

REACTIONS. (lb/size) 1=-1/2-0-0, 5=45/2-0-0, 2=87/2-0-0 Max Horz 1=26(LC 11) Max Uplift 1=-18(LC 8), 5=-13(LC 13), 2=-26(LC 12) Max Grav 1=16(LC 11), 5=45(LC 1), 2=94(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

0-0-

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 2.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







2x4 =

2x4 =

Structural wood sheathing directly applied or 4-4-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

4-4-9 4-4-9 Plate Offsets (X,Y)--[3:0-3-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in l/defl L/d PLATES GRIP (loc) TCLL 20.0 Plate Grip DOL 1.15 тс 0.03 Vert(LL) n/a 999 MT20 244/190 n/a TCDL 10.0 Lumber DOL 1.15 BC 0.07 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WВ 0.00 Horz(CT) 0.00 5 n/a n/a Code IRC2015/TPI2014 BCDL Matrix-P Weight: 11 lb FT = 20% 10.0 BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. All bearings 4-4-9.

Max Horz 1=26(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 2, 4 Max Grav All reactions 250 lb or less at joint(s) 1, 5, 2, 4

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing. 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 2, 4.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







2x4 =

2x4 =

Structural wood sheathing directly applied or 4-4-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

4-4-9 4-4-9 Plate Offsets (X,Y)--[3:0-3-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in l/defl L/d PLATES GRIP (loc) TCLL 20.0 Plate Grip DOL 1.15 тс 0.03 Vert(LL) n/a 999 MT20 244/190 n/a TCDL 10.0 Lumber DOL 1.15 BC 0.07 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WВ 0.00 Horz(CT) 0.00 5 n/a n/a Code IRC2015/TPI2014 BCDL Matrix-P Weight: 11 lb FT = 20% 10.0 BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. All bearings 4-4-9.

Max Horz 1=26(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 2, 4 Max Grav All reactions 250 lb or less at joint(s) 1, 5, 2, 4

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 2, 4.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



818 Soundside Road Edenton, NC 27932



2x4 =

2x4 =

Structural wood sheathing directly applied or 4-4-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

4-4-9 4-4-9 Plate Offsets (X,Y)--[3:0-3-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in l/defl L/d PLATES GRIP (loc) TCLL 20.0 Plate Grip DOL 1.15 тс 0.03 Vert(LL) n/a 999 MT20 244/190 n/a TCDL 10.0 Lumber DOL 1.15 BC 0.07 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WВ 0.00 Horz(CT) 0.00 5 n/a n/a Code IRC2015/TPI2014 FT = 20% BCDL Matrix-P Weight: 11 lb 10.0 BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. All bearings 4-4-9.

Max Horz 1=26(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 2, 4 Max Grav All reactions 250 lb or less at joint(s) 1, 5, 2, 4

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing. 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 2, 4.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.























	T	T		DL		
Job	Iruss	Truss Type	Qty	Ply	H&H/Jackson/	137107430
681099_Master_130	V01	GABLE	6	1	Job Reference (optiona	al)
Builders FirstSource,	Sumter, SC - 29153,	$5-11-4 \\ 5-11-4 \\ 3x6 = \\ 2.00 12 \\ 4 \\ 4 \\ 3 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1$	8.2 ID:6U_sNXmsGM6I <u>16-9-11</u> 10-10-7	220 s Nov nZXeeE_1	16 2018 MiTek Industrie la6zQrSu-XsegcKLQmlf	es, Inc. Thu May 16 07:40:01 2019 Page 1 PFFs35aoQLyoINLSXWH1Hjv6JyvCzG_pS Scale = 1:65.4
	0-101 8-11- 8-11- 10- 10- 10- 10- 12-1 12-1	6x8 × 18 17	16 15 <u>16-9-11</u> 11-10-8	10	11 12 3x4 ×	2
Plate Offsets (X,Y) [6:	0-3-0,Edge]	1				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.13 BC 0.10 WB 0.25 Matrix-S	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.02	(loc) - - 12	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES GRIP MT20 244/190 Weight: 126 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP N BOT CHORD 2x4 SP N OTHERS 2x4 SP N	BRACING-TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins.BOT CHORDRigid ceiling directly applied or 6-0-0 oc bracing.WEBS1 Row at midpt5-18, 7-17					
REACTIONS. All bearings 16-9-11. (lb) - Max Horz 1=-376(LC 13) Max Uplift All uplift 100 lb or less at joint(s) 1 except 12=-115(LC 11), 2=-109(LC 10), 18=-332(LC 13), 19=-196(LC 12), 20=-225(LC 12), 16=-197(LC 13), 15=-154(LC 13), 14=-163(LC 13), 13=-156(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 18, 19, 20, 17, 16, 15, 14, 13 except 12=371(LC 13), 2=447(LC 12) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-291/404, 2-3=-375/234, 9-10=-262/183, 10-11=-412/302, 11-12=-547/410 BOT CHORD 2-20=-440/577, 19-20=-448/596, 18-19=-447/601, 17-18=-306/416, 16-17=-306/416, 15-16=-306/416, 14-15=-306/416, 13-14=-306/416, 12-13=-306/416						
 NOTES- (10) 1) Unbalanced roof live lo 2) Wind: ASCE 7-10; Vult MWFRS (envelope) ga MWFRS for reactions s 3) All plates are 2x4 MT24 4) Gable requires continu 5) This truss has been de 6) * This truss has been de 6) * This truss has been de 6) * This truss has been de 7) Bearing at joint(s) 1, 2 capacity of bearing sur 8) Provide mechanical co 12=115, 2=109, 18=33 9) Beveled plate or shim in 10) This manufactured truparticular building is time 	ads have been considered for =130mph (3-second gust) Vas ble end zone and C-C Exterio shown; Lumber DOL=1.60 plat 0 unless otherwise indicated. ous bottom chord bearing. signed for a 10.0 psf bottom c lesigned for a 10.0 psf bottom c lesigned for a live load of 20.0 om chord and any other memi considers parallel to grain valu face. nnection (by others) of truss to 2, 19=196, 20=225, 16=197, 1 required to provide full bearing uss is designed as an individue he responsibility of the building	this design. id=103mph; TCDL=6.0psf; BCDL=6.0ps r(2) zone; end vertical left and right exp e grip DOL=1.60 hord live load nonconcurrent with any o psf on the bottom chord in all areas who pers. te using ANSI/TPI 1 angle to grain form b bearing plate capable of withstanding 5=154, 14=163, 13=156. surface with truss chord at joint(s) 1, 2 a building component. The suitability a g designer per ANSI TPI 1 as reference	sf; h=25ft; Cat. II; I osed;C-C for merr ther live loads. ere a rectangle 3-6 ula. Building desi 100 lb uplift at join , 19, 20. nd use of this com d by the building c	Exp C; Er ibers and 3-0 tall by gner shou it(s) 1 exc ponent fo code.	nclosed; forces & 2-0-0 wide uld verify sept (jt=lb) or any	SEAL 044925

SEAL 044925 MGINEER, HR. M. SEVILIUM May 16,2010

CO

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

ENGINEERING BY



V2015 BEFORE USE. pmponent, not into the overall permanent bracing ng the I BCSI Building Component 818 Soundside Road Edenton, NC 27932
