

Trenco 818 Soundside Rd Edenton, NC 27932

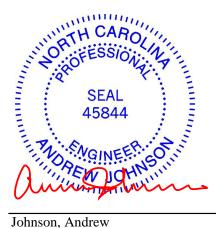
Re: 22020129-A 106 Farm at Neills Creek-Cooper 3-Roof

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: I53118391 thru I53118391

My license renewal date for the state of North Carolina is December 31, 2022.

North Carolina COA: C-0844



July 15,2022

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

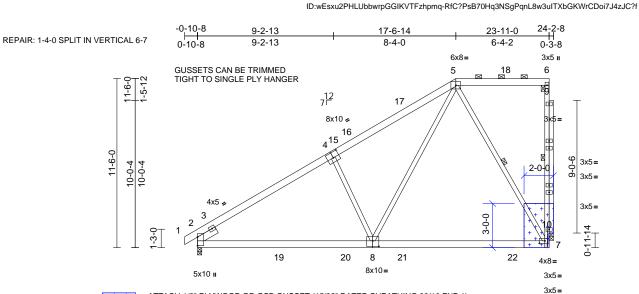
Job	Truss C01	Truss Type	Qty	Ply	106 Farm at Neills Creek-Cooper 3-Roof				
22020129-A		Piggyback Base	3	1	Job Reference (optional)	153118391			

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24.2.0

Page: 1

Carter Components (Sanford), Sanford, NC - 27332,



 ATTACH 1/2" PLYWOOD OR OSB GUSSET (15/32" RATED SHEATHING 32/16 EXP 1)
 TO EACH FACE OF TRUSS WITH (0.131" X 2.5" MIN.) NAILS PER THE FOLLOWING NAIL SCHEDULE: 2 X 3'S - 2 ROWS, 2 X 4'S - 3 ROWS, 2 X 6'S AND LARGER - 4 ROWS: SPACED @ 4" O.C.
 MAILS TO BE DRIVEN FROM BOTH FACES. STAGGER SPACING FROM FRONT TO BACK FACE FOR A NET 2" O.C. SPACING IN EACH COVERED TRUSS MEMBER. USE 2" MEMBER END DISTANCE.

				11-1	1-0	1		23-9-4		24	-2-8 H		
0 1 1 70 1				11-1	1-0	1		11-10-4		0-	-5-4		
Scale = 1:78.4													
Plate Offsets (X, Y): [4:0-5-0,0-4-8],	[7:0-4-8,0-2-0], [8:0-	5-0,0-4-8									-	
Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.62	Vert(LL)	-0.26	7-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.79	Vert(CT)	-0.40	7-8	>717	180		
TCDL	10.0	Rep Stress Incr	YES		WB	0.72	Horz(CT)	-0.05	10	n/a	n/a		
BCLL	0.0*	Code	IRC201	8/TPI2014	Matrix-MSH								
BCDL	10.0											Weight: 199 lb	FT = 20%
			2)		E 7-16; Pr=20.0 ps	f (roof L		-1 15					
LUMBER TOP CHORD	2x6 SP No.2		3)		:1.15); Pf=20.0 psf								
BOT CHORD					Is=1.0; Rough Ca								
WEBS	2x4 SP No.2 *Excep	1* 4-8-2v4 SP No 3		Cs=1.00; C		. 2,	_, 00 0.	,					
OTHERS	2x4 SP No.3			,	snow loads have	been co	sidered for t	this					
SLIDER	Left 2x6 SP No.2 1	1-6-0		design.									
BRACING			5)		as been designed								
TOP CHORD	Structural wood she	athing directly applie	d or		psf or 1.00 times			osf on					
5-8-0 oc purlins, except end verticals, and				overhangs non-concurrent with other live loads.Provide adequate drainage to prevent water ponding.									
	2-0-0 oc purlins (6-0												
BOT CHORD	0 0 ,	applied or 10-0-0 oc	()		as been designed ad nonconcurrent								
	bracing.		8)		has been designe								
WEBS	1 Row at midpt 6-7, 5-7				om chord in all area			.0031					
REACTIONS (lb/size) 2=1004/0-3-8, 10=950/0-3-8					by 2-00-00 wide w			tom					
	Max Horiz 2=396 (LC			chord and a	any other members	, with BC	DL = 10.0ps	sf.					
	Max Uplift 2=-107 (L Max Grav 2=1246 (L				oint(s) 10 consider			ue					
	,		24)		TPI 1 angle to gra							minin	110.
FORCES	(lb) - Maximum Compression/Maximum Tension				ould verify capacit						6	I'L CA	Pall
TOP CHORD)/232 5-6=-159/164	10		Simpson Strong-T led to connect trus			a to		~	1	A	19 March
	7-10=-90/1109, 6-10	, ,			t(s) 2 and 10. This						5.	O'. FESS	IO: NE
BOT CHORD	2-7=-311/1366			,	es not consider lat						C/V	many	zunn
WEBS	5-8=-164/1246, 5-7=	-1011/183, 4-8=-597	/313 11		s designed in acco							:2	N 1 3
NOTES				International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPL1								1 1 1	
 Unbalanced roof live loads have been considered for 					and referenced sta					-			• •
this design	ז.	12	12) Graphical purlin representation does not depict the size 45844										
	ASCE 7-16; Vult=130mph (3-second gust)			or the orientation of the purlin along the top and/or								1 E -	
	=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft;			bottom chord.									
· · ·	Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior			LOAD CASE(S) Standard							Encols		
	zone and C-C Exterior(2E) -0-10-8 to 2-6-5, Interior (1) 2-6-5 to 14-2-1, Exterior(2R) 14-2-1 to 20-11-11, Interior			A CONTRACTOR OF ALLER AND							T. NS I'		
	-11 to 23-9-4 zone; cantilever left and right			EW JOH W							OHIM		
exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown;										THEW JOHNING			
										July 15,2022			
Lumber D	OL=1.60 plate grip DO	DL=1.60										50.7	-,



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

