Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 32224 JOB: 22-2664-R01 JOB NAME: LOT 49 CROSSING @ ANDERSON CRE Wind Code: 37 Wind Speed: Vult= 130mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 33 Truss Design(s)

Trusses:

M01, M02, M03, M04, M05, M06, M08, M09, PB01, PB02, PB03, PB04, PB05, R01, R02, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, VT01, VT02, VT03, VT04, VT05, VT06, VT07,



Warning !--- Verify design parameters and read notes before use.



| BCLL BCDL | 0.0 * 10.0 | Rep Stress Incr YES Code IRC2018/TPI2014 | WB 0.04 Matrix-R | Horz(CT) | -0.00 | 8 | n/a | n/a | Weight: 43 lb | FT = 0% |
|-----------------------------------|--------------------------------|---|---------------------|-----------------------|---------------------|-------------------|-----------------|---------------------------------|---------------------------|--------------------------|
| LUMBER- TOP CHORI BOT CHORI | D 2x4 SP No.2 D 2x4 SP No.3 | | | BRACING- TOP CHORD | Structur end ver | ral wo ticals. | od shea | athing direc | lly applied or 6-0-0 oc l | purlins, except |
| WEBS | 2x4 SP No.3 | | | BOT CHORD | Rigid ce | eiling | directly | applied or 6 | 6-0-0 oc bracing. | |
| OTHERS | 2x4 SP No.3 | | | | MiTek be ins | recoi | nmend during | s that Stabil truss erection | izers and required cros | ss bracing Stabilizer |

Installation guide

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REACTIONS. All bearings 8-11-8.

(lb) - Max Horz 13=111(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 13, 8, 12, 11, 10, 9 Max Grav All reactions 250 lb or less at joint(s) 13, 8, 12, 11, 10, 9

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14-15)

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

Gable studs spaced at 2-0-0 oc.

- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will the second and any other members. Provide mechanical connection (by others) of truss to bearing plate copeling of the second seco 11)
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 8, 12, 11, 10, 9
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates ALL RANK that the member must be braced.
- 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard





13) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates ALA KK that the member must be braced.

14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard

Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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| <u>- 1 1010 0110010 (7,17)</u> [2.0 2 | <u>, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u> | 20] | | | | | | |
|--|---|--|--|--|--|--|--|-----------------------------------|
| LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014 | CSI. TC 0.84 BC 0.41 WB 0.00 Matrix-R | DEFL. Vert(LL) Vert(CT) Horz(CT) | in (loc) 0.08 4-5 -0.15 4-5 0.00 4 | l/defl >999 >544 n/a | L/d 240 180 n/a | PLATES MT20 Weight: 26 lb | GRIP 244/190 FT = 0% |
| LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 | | E | BRACING- TOP CHORD BOT CHORD | Structural v end vertica Rigid ceilin MiTek rec | vood she ls. g directly commend | athing direct applied or 1 s that Stabil | ly applied or 5-9-13 o 0-0-0 oc bracing. izers and required cr | oc purlins, except |

be installed during truss erection, in accordance with Stabilizer

and a second second

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Installation guide.

REACTIONS. (lb/size) 5=334/0-3-8 (min. 0-1-8), 4=264/0-1-8 (min. 0-1-8) Max Horz 5=93(LC 11) Max Uplift5=-145(LC 10), 4=-114(LC 10) Max Grav 5=427(LC 21), 4=342(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 3-4=-253/83. 2-5=-381/130

NOTES-(11-12)

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- e) Frovide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=145
 10) This truss is designed in accordance with the 2018 International Residential Code sections PE02.44.
 11) Granbicol was to the section of the se

- 11) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the member must be braced.
- 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard





(lb) - Max Horz 15=129(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 15, 9, 14, 13, 12, 11, 10 Max Grav All reactions 250 lb or less at joint(s) 15, 9, 14, 13, 12, 11, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14-15)

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

Gable studs spaced at 2-0-0 oc.

- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11)
- * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will Frovide mechanical connection (by others) of truss to bearing plate capable of with the first of the fir 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 9, 14, 13, 12, 😤

LOAD CASE(S) Standard

MORPHS INTERNAL MORPHS INTERNA Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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- standard ANSI/TPI 1. 13) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates
- ALA KK that the member must be braced.
- 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard

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Max Horz 5=55(LC 11) Max Uplift4=-21(LC 14), 5=-63(LC 10) Max Grav 4=118(LC 21), 5=232(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10-11)

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the member must be braced.
- 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard











vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is to fail individual building component to be instance and loaded vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is to fail individual component to be instance and loaded of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Trusse Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDERSON CREE | K TBD PINNACLE DRIVE SPRING LA |
|-------------|-------|------------|---------------|-------------|---|----------------------------------|
| 22-2664-R01 | R01 | GABLE | 2 | 1 | Job Reference (optional) | # 32224 |
| | | Run: | 8.430 s Feb 1 | 2 2021 Prin | t: 8.430 s Feb 12 2021 MiTek Industries, Inc. | Thu May 5 17:32:30 2022 Page 2 |

ID:NQLeVDd4PWwD4SswrTSDr_y95ve-jCXbxbA?GrqQ9AmIUXm62onc7HWs9PemQgSqdWzJV2F 15) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the member must be braced. 16) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard





vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

| Job | Truss | Truss Type | Qtv | Plv | LOT 49 CROSSING @ ANDE | RSON CREEK TBD PINNACLE DRIVE SPRING LA |
|---|--|---|--|-------------------------|---|--|
| 22 2664 P01 | P03 | Pirguback Base | 8 | | | # 22224 |
| 22-2004-1101 | 105 | | | <u> </u> | Job Reference (optional) | # 32224 |
| | | | ID:NQLeVDd4F | 2 2021 Print WwD4Ssv | vrTSDr_y95ve-fbeLLGBFoS | S48OUw7byoa7Dtpl5_FdBc3uzxwiPzJV2D |
| -0-10 ₇ 8 0-10-8 | 6-10-12 | 13-6-0 | 20-1-0 | | 26-5-0 6-4-0 | 33-3-8 |
| 0-10-8 | 0-10-12 | 0-7-4 | 0-7-0 | | 0-4-0 | 0-10-8 |
| | | 4x6 = | 5 | /9 — | | Scale = 1:66.0 |
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| | | T2 | | \parallel \gg | | |
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| | | | | | 5x6 ≷ | |
| 9 | 3 | | W6// | | 7 | o. |
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| | I | | | // | \ // `` | 3x4 |
| 5x5 🥢 | W | з 🛛 🗌 🥢 | | \ | /////////////////////////////////////// | 8 |
| | | | | | $\mathbb{N}//$ | |
| | W2 B1 💿 | | B2 | | В3 | |
| | 5 4 | 16 13 12 | 17 | 11 | 10 18 19 | |
| 3x4 | 1 1 414 | -3x8 = 4x8 = -3x8 | | | $4x4 \equiv$ | 9 4x4 = |
| | 4X4 | 4x8 — | | 3x8 = | = | |
| | | | | | | |
| | 6-10-12 | 13-6-0 | 23-5-0 | | 33-3- | 8 |
| | 6-10-12 | 6-7-4 | 9-11-0 | 0 4 0 0 0 | 9-10- | 8 |
| | -2-0,0-1-12], [3:0-4-0,0-3-0] | <u> , [4:0-3-12,0-2-0], [5:0-5-12,0-2-0],</u> | [7:0-2-7,0-3-0], [9:0 | 0-1-8,0-2- | 0] | |
| LOADING (psf) TCLL (roof) 20.0 | SPACING- | 2-0-0 CSI . | DEFL. | in (lo | oc) l/defl L/d | PLATES GRIP |
| Snow (Pf) 20.0 | Plate Grip DOL | 1.15 TC 0.72 1.15 BC 1.00 | Vert(LL) | -0.50 10- | 12 >790 240 12 >574 180 | MT20 244/190 |
| TCDL 10.0 | Rep Stress Incr | YES WB 0.64 | Horz(CT) | 0.06 | 9 n/a n/a | |
| BCDL 10.0 | Code IRC2018/TP | I2014 Matrix-SH | | | | Weight: 207 lb FT = 0% |
| LUMBER- | | · | BRACING- | | | |
| TOP CHORD 2x4 SP No | .2 | | TOP CHORD | Structur | al wood sheathing directl | y applied or 3-10-0 oc purlins, except |
| BOT CHORD 2x4 SP No | 0.2 *Except* | | | end vert | icals. iling directly applied or 2. | -2-0 oc bracing Except: |
| WEBS 2x4 SP No | .3 | | BOT CHORD | 10-0-0 o | c bracing: 14-15. | -2-0 oc bracing, Except. |
| | | | WEBS | 1 Row a | t midpt 3-12, 5 | -12, 7-9 |
| | | | | MiTek | recommends that Stabili | zers and required cross bracing |
| | | | | Installa | ition guide. | |
| REACTIONS. (lb/size) | 15=1382/0-3-8 (min. 0-1-1 | 1), 9=1319/Mechanical | | | | |
| Max Horz Max Uplift | 15=243(LC 9) 15=-170(I C 12) 9=-147(I (| 2 13) | | | | |
| Max Grav | 15=1406(LC 20), 9=1419(L | C 3) | | | | |
| EORCES (Ib) Max Co | mp /Max Ten All forces ? | 250 (lb) or less except when shown | | | | |
| TOP CHORD 2-3=-185 | 6/207, 3-4=-1546/211, 4-5= | -1211/230, 5-6=-1774/270, 6-7=-1 | 769/216, | | | |
| 7-8=-421 | /136, 8-9=-365/138, 2-15=- | 1335/201 | 4 40 47- 04/4404 | | | |
| BUT CHURD 14-15=-2 11-17=-2 | 4/1191, 10-11=-205/1574, 4/1191, 10-11=-24/1191, 1 | 13-16=-205/1574, 12-13=-205/1574 0-18=-101/1467, 18-19=-101/1467 | 4, 12-17=-24/1191, . 9-19=-101/1467 | | | |
| WEBS 3-12=-44 | 2/217, 4-12=-37/524, 5-10= | 120/722, 6-10=-248/255, 7-9=-15 | 88/82, 2-14=0/1184 | 4 | | |
| NOTES (11 12) | | | | | | |
| 1) Unbalanced roof live lo | ads have been considered | for this design. | | | | |
| 2) Wind: ASCE 7-16; Vult | =130mph (3-second gust) | Vasd=103mph; TCDL=5.0psf; BCD | 0L=5.0psf; h=23ft; 0 | Cat. II; Exp | B; Enclosed; MWFRS | |
| reactions shown: Lumb | one and C-C Exterior(2) zo per DOL=1.60 plate grip DC | ne; end vertical left and right expos | sed;C-C for membe | ers and foi | rces & MWFRS for | |
| 3) TCLL: ASCE 7-16; Pr= | 20.0 psf (roof LL: Lum DOL | _=1.15 Plate DOL=1.15); Pf=20.0 p | sf (Lum DOL=1.15 | Plate DO | L=1.15); ls=1.0; Rough | ANNIHIGG. |
| Cat B; Partially Exp.; C | e=1.0; Cs=1.00; Ct=1.10 | of live load of 12.0 psf or 2.00 time | s flat roof load of 2 | 0 0 pef or | overbangs | WINTH CARO |
| non-concurrent with oth | ner live loads. | | | 0.0 p31 01 | i overnangs | SEESSIA Nolling |
| 5) Provide adequate drair | hage to prevent water pond | ing. | 4 | t | Inn | ALL AND THE |
| 7) * This truss has been de | lesigned for a 10.0 psi bollor | 0.0psf on the bottom chord in all ar | reas where a rectar | aas. 1ale 3-6-0 | tall by 1-0-0 wide will fit | SEAL |
| between the bottom ch | ord and any other members | s, with BCDL = 10.0psf. | | 0 | III | 28147 |
| 8) Refer to girder(s) for true | uss to truss connections. | s to bearing plate capable of withst | anding 100 lb unlif | t at ioint/s |) except (it=lb) 15-170 | |
| 9=147. | | o to boaring plate capable of WIIIS | | . a. joint(S | , choope (it=ib) 10=170, | AN NOINEER |
| 10) This truss is designed | in accordance with the 20 | 18 International Residential Code s | sections R502.11.1 | and R802 | 2.10.2 and referenced 3 | ARE |
| 11) Graphical web bracing | a representation does not d | lepict the size type or the orientation | on of the brace on t | he web S | Symbol only indicates | Managementing |
| that the member mus | t be braced. | , | | | ,,, | 5/1/2022 |
| 12) Bearing symbols are of | only graphical representation | ons of a possible bearing condition. | Bearing symbols a | are not co | nsidered in the | J/4/2U22 |
| Continued on page 2 Vertically. Applicability of | design parameters and proper in | corporation of component is responsibili | n parameters shown, a ty of building designer | nu is for an | designer or truss engineer | an to be instance and loaded Bracing shown is for lateral support |
| of individual web members | only. Additional temporary bra | cing to ensure stability during construction | on is the responsibility | of the erec | tor. Additional permanent br | acing of the overall structure is the |
| responsibility of the building | g designer. For general guidanc | e regarding fabrication, quality control, s | torage, delivery, erecti | ion and bra | cing, consult ANSI/TPI 1 Na | tional Design Standard for Metal |
| Plate Connected Wood Trus | ss Construction and BCSI 1-03 | Guide to Good Practice for Handling, I | nstalling & Bracing of | f Metal Pla | te Connected Wood Trusses | from Truss Plate Institute, 583 |
| D'Unotrio Drive, Madison, | w1 53/19. | | | | | |

| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDERSON CREEK TBD PINNACLE | DRIVE SPRING LA |
|-------------|-------|--------------------|-----------------------|-------------------------|---|--------------------------------|
| 22-2664-R01 | R03 | Piggyback Base | 8 | 1 | Job Reference (optional) # 32. | 224 |
| | | Run: 8.4: ID:NQ | 30 s Feb 1 LeVDd4P | 2 2021 Print WwD4Ssv | 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 5 17:32:3 vrTSDr_y95ve-fbeLLGBFoS48OUw7byoa7DtpI5_FdBc3 | 32 2022 Page 2 3uzxwiPzJV2D |

LOAD CASE(S) Standard



| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDE | RSON CREEK TBD PINNACLE DRIVE SPRING LA |
|---|--|--|------------------------|------------------------|--|--|
| 22-2664-R01 | R04 | Piggyback Base | 3 | 1 | | # 32224 |
| | | | Run: 8.430 s Feb 12 | 2 2021 Print: | Job Reference (optional) 8.430 s Feb 12 2021 MiTek li | ndustries, Inc. Thu May 5 17:32:32 2022 Page 1 |
| | 1-0-0 | | ID:NQLeVDd4P | WwD4Ssw | rTSDr_y95ve-fbeLLGBFo | S48OUw7byoa7Dtpl5?Cd8x3uzxwiPzJV2D |
| -Q <u>-10-</u> 0-10- | <u>18 2-3-8 7-10-12</u> 8 1-3-8 5-7-4 | <u>13-6-0</u> 20- 5-7-4 6-1 | 1-0 7-0 | <u>26-5-0</u> 6-4-0 | 33-3- | <u>8</u> |
| 0.10 | 1-0-0 | 4×6 — | | 0.0 | 0.10 | Scale = 1:71.1 |
| | | 4x0 — | 5x8 = | - | | |
| | | 8.00 12 7 T | 3 8 | | | |
| | | A Contraction of the second se | | | | ſ |
| | 1/6 | | | \searrow | | |
| | 3x8 🖉 | | | 12 | < | |
| | | 6 | | \ | 5x6 🕅 | |
| q | 5 | | ¥¥9// \ | | 1 0 | o, |
| 0-0 | | | 19 / | Mada | 27 | 0-0 |
| Ì | 5x6 🖉 26 | | | | 1043 | , |
| 5x8 | HW1 TT | W5 W6 | | // | W12 | 3x4 |
| | 3W1 W0 | | | | | 11 |
| P^{1} | PT W4 | | | | / | |
| | <u>₽ 1987 6</u> 20 Ø 6x8 = | 22 28 6x8 = W8 10 | B5 V@16 | | B/ | |
| | 25 24 | $4x4 = 20^{19}$ | 29 33 17 ¹ | 14 30 1 34 | 15 31 32 | 12 |
| 4x | (8 3x4 | 3x4 | 4x | 3 = 4x4 | - | 4x6 = |
| | $4x4 \equiv$ | 6x6 = | 2x4 | 2x4 — | - | |
| | | 2x4 = | 284 | | | |
| | | | | | | |
| | <u>1-0-02-3-8</u> 7-10-12 1-0-01-3-85-7-4 | <u>13-3-8 14-3-8 2</u> 5-4-12 1-0-0 5 | 0-1-0 2 5-9-8 3 | 3-5-0 3-4-0 | <u>33-3-8</u> 9-10-8 | |
| Plate Offsets (X,Y) [3:0- | -1-12,0-2-4], [7:0-3-8,0-1-12 | 2], [8:0-5-12,0-2-0], [10:0-2-7,0-3-0], | [21:0-2-12,0-2-4], | [23:0-6-4 | ,0-3-8] | |
| LOADING (psf) | SPACING- | 2-0-0 CSI . | DEFL. | in (lo | c) l/defl L/d | PLATES GRIP |
| TCLL (roof) 20.0 Snow (Pf) 20.0 | Plate Grip DOL | 1.25 TC 0.72 | Vert(LL) | -0.42 16-1 | 18 >941 240 | MT20 244/190 |
| TCDL 10.0 | Lumber DOL Rep Stress Incr | 1.25 BC 0.94 | Vert(CT) | -0.65 16-1 | 18 >609 180 | |
| BCLL 0.0 * | Code IRC2018/TP | I2014 Matrix-SH | 11012(01) | 0.19 | 12 11/a 11/a | Weight: 244 lb FT = 20% |
| | | | PRACINIC | | | |
| TOP CHORD 2x4 SP No. | .2 | | TOP CHORD | Structura | I wood sheathing direct | ly applied or 2-11-12 oc purlins, |
| BOT CHORD 2x4 SP No. | .2 *Except* | | | except er | nd verticals. | · · · · · · · · · · · · · · · · · · · |
| WEBS 2x4 SP No. | SP No.3, B5,B7: 2x4 SP S | S | BOT CHORD | Rigid ceil | ling directly applied or 1 | 0-0-0 oc bracing, Except: |
| SLIDER Left 2x6 SF | P No.2 - 1-2-11 | | | 6-0-0 oc | bracing: 20-21,19-20. | |
| | | | WERE | 6-0-0 oc | bracing: 15-18 | 0.12 |
| | | | WEBS | MiTek r | recommends that Stabil | zers and required cross bracing |
| | | | | be insta | alled during truss erection | on, in accordance with Stabilizer |
| | 2-14E2/0.2.9 (min 0.1.12) | 10-1402/Machaniaal | | Installat | tion guide. | |
| Max Horz 2 | 2=1455/0-5-6 (mm. 0-1-15) 2=219(LC 9) |), 12–1423/Mechanical | | | | |
| Max Uplift2 | 2=-131(LC 12), 12=-98(LC | 13) | | | | |
| Max Grav2 | 2=1522(LC 20), 12=1618(L | C 3) | | | | |
| FORCES. (Ib) - Max. Cor | np./Max. Ten All forces 2 | 50 (lb) or less except when shown. | | | | |
| TOP CHORD 2-3=-1758 6-7=-193 | 8/151, 3-4=-3661/441, 4-26 2/267 7-8=-1520/268 8-9= |)=-2427/228, 5-26=-2359/244, 5-6=-2 =-2088/269, 9-10=-2085/219, 10-27= | 2283/250, -258/144 | | | |
| 11-27=-42 | 20/124, 11-12=-365/143 | -2000/203, 3-10-2003/213, 10-27- | -200/144, | | | |
| BOT CHORD 2-25=-19 | 7/1138, 4-23=-92/770, 22-2 | 23=-549/3398, 22-28=-177/2067, 21- | 28=-177/2067, | | | |
| 13-30=0/2 | 20/0, 7-211/751, 19-29-0 1430, 13-31=-107/1713, 31 | -32=-107/1713, 12-32=-107/1713 |), 14-30-0/1430, | | | |
| WEBS 3-25=-993 | 3/173, 4-22=-1354/378, 6-2 | 2=0/383, 6-21=-684/226, 19-21=0/1 | 811, 8-21=-181/7 | 53, | | |
| 18-19=-6 10-12=-1 | 655/81, 8-18=-610/116, 8-1 896/118 23-25=-220/1338 | 5=-86/857, 13-15=-108/757, 9-13=-2 3-23=-308/2089 | 24/264, | | | |
| | | 0 20 000,2000 | | | | antilline. |
| NOTES- (11-14) | ade have been considered | for this design | | | | WINNTH CARO |
| 2) Wind: ASCE 7-16; Vult | =130mph (3-second gust) | /asd=103mph; TCDL=5.0psf; BCDL | =5.0psf; h=23ft; C | Cat. II; Exp | B; Enclosed; MWFRS | SEESSIA NELL |
| (envelope) gable end ze | one and C-C Exterior(2E) - | 0-10-8 to 3-11-2, Interior(1) 3-11-2 to | 6-8-9, Exterior(2 | R) 6-8-9 to | 26-7-14, Interior(1) | ART PARTY IN |
| plate grip DOL=1.60 | 1101(2E) 20-4-2 to 33-1-12 2 | cone,C-C for members and forces & | INIVERS IOF react | | II, LUIIIDEI DOL-1.00 | SEAL |
| 3) TCLL: ASCE 7-16; Pr= | 20.0 psf (roof LL: Lum DOL | =1.25 Plate DOL=1.25); Pf=20.0 psf | (Lum DOL=1.15 | Plate DOI | _=1.15); ls=1.0; Rough | 28147 |
| Cat B; Partially Exp.; Co 4) This truss has been dee | e=1.0; Cs=1.00; Ct=1.10 signed for greater of min ro | of live load of 12.0 nst or 2.00 times | flat roof load of 2 | () () nsf on | overhangs | |
| non-concurrent with oth | ner live loads. | | | | | 15 ANOINEER |
| 5) Provide adequate drain | age to prevent water pond | ing. | any other live les | de | 3 | ARK K MORRIGIN |
| 7) * This truss has been d | esigned for a live load of 3 | 0.0psf on the bottom chord in all area | any other live loa | ngle 3-6-0 t | tall by 1-0-0 wide will fit | Managementality |
| between the bottom cho | ord and any other members | s, with BCDL = 10.0psf. | | | - | 5/4/2022 |
| 8) Refer to girder(s) for true Warning !—Verify design | iss to truss connections. parameters and read notes by | efore use. This design is based only upon r | parameters shown a | nd is for an | individual building compon | ent to be installed and loaded |
| Continued on page 2 vertically. Applicability of d | lesign parameters and proper in | corporation of component is responsibility | of building designer | – not truss | designer or truss engineer. | Bracing shown is for lateral support |
| of individual web members of | only. Additional temporary brac | cing to ensure stability during construction | is the responsibility | of the erecto | or. Additional permanent b | racing of the overall structure is the |
| responsibility of the building | g designer. For general guidance | e regarding fabrication, quality control, stor | rage, delivery, erecti | ion and brac | ing, consult ANSI/TPI 1 No | tional Design Standard for Metal |
| riule Connected wood Irus | s construction and BCSI 1-03 | Guide to Good Fractice for Hanaling, Ins | uning a pracing of | metat Plate | e connectea wood trusses | nom muss riate mistitute, 365 |

D'Onofrio Drive, Madison, WI 53719.

| Job | Truss | Truss Type | Qty Ply | LOT 49 CROSSING @ ANDERSO | N CREEK TBD PINNACLE DRIVE SPRING LA |
|-------------|-------|----------------|--|--|--|
| 22-2664-R01 | R04 | Piggyback Base | 3 | 1 Job Reference (optional) | # 32224 |
| | | | Run: 8.430 s Feb 12 202 ID:NQLeVDd4PWwD4S | 21 Print: 8.430 s Feb 12 2021 MiTek Indust SswrTSDr y95ve-7nCkZcCtZmC?0eL | ries, Inc. Thu May 5 17:32:33 2022 Page 2 JK9fJpqQP 2ULRMbBC7dhUErzJV2C |

NOTES- (11-14)

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 2=131.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- (Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 (SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



| Job | Truss | Truss Type | | Qty | Ply L | OT 49 CROSSING @ AN | IDERSON CREEK TBD PI | NNACLE DRIVE SPRING LA |
|---|---|---|---------------------------------------|------------------------------|----------------------------|--|-----------------------------------|--------------------------------|
| 22-2664-R01 | R05 | PIGGYBACK BASE | | 1 | 1 | | - | # 32224 |
| | | | Run: | 8.430 s Feb 1 | 2 2021 Print: 8 | Job Reference (optiona 8.430 s Feb 12 2021 MiTe | il) k Industries, Inc. Thu May | 5 17:32:33 2022 Page 1 |
| | 1.0.0 | | ID:NC | LeVDd4PW | wD4SswrTS | Dr_y95ve-7nCkZcCtZ | mC?0eUK9fJpgQP_hÚL | .gMb3C7dhUErzJV2C |
| -0 <u>-10</u> | <u>-8 2-3-8 7-10-12</u> 8 1-3-8 5-7-4 | 13-6-0 | <u>20-1-0</u> 6-7-0 | | 26-5-0 | 33 | -7-0 | |
| 0-10- | 1-0-0 | 5-7-4 | 0-7-0 | | 0-4-0 | | 2-0 | Scale = 1.71.5 |
| | | 5X6 — | | 5x8 = | | | | 00010 - 1.7 1.0 |
| | | 8.00 12 7 | тз | 8 | | | | |
| T | | | 10 | - AR | | | | Ţ |
| | 4×6 | // | | | | | | |
| | 3x8 🖉 | | | | 12 | | | |
| | | 6 | | // // | \ | 5x6 📎 | | |
| q | 5 | | , XV9/ | \ |) | 9 10 | | ρ |
| 10-0 | | | | v | NAA | W N | 13 | 10-0 |
| 518 | 5x6 / | Inte | | | | | 1 4 | |
| 0.00 | HW1 | W5 VVO | | | | W12 | 3x4 | |
| | 3W1 W2 | | | D. | | | 11 | |
| | | 21 | B5 | Walitos | | B7 | | |
| | 6x8 = | 22 26 6x8 = | W8 19 18 27 30 | | <u>. </u> 면 1/ 31 13 14 | 5 28 20 | | 1 |
| | 25 24 | 4x4 = 2 | 0 19 10 27 30 | ′ 17 3 | x8 = 4x4 = | 20 29 | 12 | |
| 4x | (8 3x4 | 3x4 | 4 | 2x4 | 2x4 = | | 4x6 = | |
| | $4x4 \equiv$ | | 6x6 = | 2x4 | | | | |
| | | | 2X4 — | | | | | |
| | 1-0-02-3-8 7-10-12 | . 13-3-8 1/ | -3-8 20-1-0 | . 2 | 3-5-0 | 33-7-0 | | |
| | 1-0-01-3-8 5-7-4 | 5-4-12 1- | -0-0 <u>5-9-8</u> | | 9-4-0 | 10-2-0 | | |
| Plate Offsets (X,Y) [3:0- | <u>-1-12,0-2-4], [7:0-4-4,0-2-4]</u> | , [8:0-5-12,0-2-0], [10:0- | 3-0,0-3-0], [21:0-2 | 2-12,0-2-4], | [23:0-6-4,0- | -3-12] | | |
| LOADING (psf) TCLL (roof) 20.0 | SPACING- | 2-0-0 CSI . | | DEFL. | in (loc | c) I/defl L/d | PLATES | GRIP |
| Snow (Pf) 20.0 | Plate Grip DOL | 1.15 TC | 0.74 | Vert(LL) | -0.42 16-1 | 8 >963 240 8 >620 180 | MT20 | 244/190 |
| TCDL 10.0 | Rep Stress Incr | YES WB | 0.88 | Horz(CT) | 0.20 1 | 2 n/a n/a | | |
| BCDL 10.0 | Code IRC2018/TP | I2014 Matrix | k-SH | | | | Weight: 245 | lb FT = 20% |
| LUMBER- | | | BRA | CING- | | | | |
| TOP CHORD 2x4 SP No | .2 | | TOP | CHORD | Structural | wood sheathing dire | ectly applied or 2-11-9 | oc purlins, except |
| BOT CHORD 2x4 SP No B2.B4: 2x4 | SP No.3. B5.B7: 2x4 SP S | S | вот | CHORD | Riaid ceili | ais. ing directly applied o | r 10-0-0 oc bracing. | Except: |
| WEBS 2x4 SP No | .3 | | | | 8-3-0 oc b | pracing: 22-23 | | · |
| SLIDER Left 2x6 SF | P No.2 -A 1-2-11 | | | | 6-0-0 oc t | pracing: 20-21,19-20 pracing: 15-18 | | |
| | | | WEE | S | 1 Row at | midpt 8-19 |), 10-12 | |
| | | | | | MiTek re | ecommends that Sta | bilizers and required c | ross bracing |
| | | | | | Installati | ion quide. | stion, in accordance w | |
| REACTIONS. (lb/size) | 2=1465/0-3-8 (min. 0-1-13 |), 12=1434/0-3-8 (min. (|)-1-15) | | | | | |
| Max Horz Max Unliff | 2=231(LC 11) 2=-132(I C 12)_12=-101/I (| : 13) | | | | | | |
| Max Grav | 2=1533(LC 20), 12=1622(L | C 3) | | | | | | |
| FORCES (Ib) - Max Cor | mp /May Ten - All forces 2 | 150 (lb) or less except w | nen shown | | | | | |
| TOP CHORD 2-3=-177 | 3/121, 3-4=-3696/421, 4-5= | -2449/172, 5-6=-2306/1 | 89, 6-7=-1954/16 | 7, | | | | |
| 7-8=-154 POT CHORD 2 25- 19 | 7/184, 8-9=-2139/197, 9-10 |)=-2131/142, 10-11=-564 | 4/161, 11-12=-453 | 8/156 | | | | |
| 20-21=-7 | 12/0, 7-21=-2/762, 19-27=0 |)/1455, 17-27=0/1455, 1 | 4-17=0/1455, 13- | 14=0/1455, | | | | |
| 13-28=-3 | 8/1780, 28-29=-38/1780, 1 | 2-29=-38/1780 | 02 10 01-0/1000 | | | | | |
| 8-21=-18 | 9/759, 18-19=-675/49, 8-18 | 3=-630/86, 8-15=-91/905 | , 13-15=-112/807 | 1 | | | | |
| 9-13=-27 | 3/270, 10-12=-1819/0, 23-2 | 25=-199/1355, 3-23=-290 | 0/2111 | | | | | |
| NOTES- (10-13) | | | | | | | | |
| 1) Unbalanced roof live lo | ads have been considered | for this design. | | | | | WINNING CAR | 1111 |
| 2) Wind: ASCE 7-16; Vult (envelope) gable end z | =130mph (3-second gust) one and C-C Exterior(2) zo | vasd=103mph; TCDL=5 ne: end vertical left and | .0pst; BCDL=5.0p right exposed:C-0 | sf; h=23ft; (C for membe | ers and forc | B; Enclosed; MWFR es & MWFRS for | Sun All CESOIA | INUL |
| reactions shown; Lumb | per DOL=1.60 plate grip DC | L=1.60 | | | | | ROFLOPA | A INT |
| 3) TCLL: ASCE 7-16; Pr= | 20.0 psf (roof LL: Lum DOL | _=1.15 Plate DOL=1.15); | ; Pf=20.0 psf (Lun | n DOL=1.15 | Plate DOL | .=1.15); ls=1.0; Roug | h SEAL | |
| 4) This truss has been de | signed for greater of min ro | of live load of 12.0 psf o | r 2.00 times flat ro | oof load of 2 | 0.0 psf on | overhangs 🗧 | 28147 | 1111 |
| non-concurrent with oth | ner live loads. | ing | | | | HIT | | |
| 6) This truss has been de | signed for a 10.0 psf bottor | n chord live load noncor | current with any o | other live loa | ads. | 3 | A ANDINEER | 1. 1 |
| 7) * This truss has been d | lesigned for a live load of 3 | 0.0psf on the bottom cho | ord in all areas wh | ere a rectar | ngle 3-6-0 t | all by 1-0-0 wide will | fit ARK | Al Punt |
| a) Provide mechanical con | nnection (by others) of trus | s, with DODL = 10.0pst. s to bearing plate capab | le of withstanding | 100 lb uplif | t at joint(s) | except (it=lb) 2=132 | Manager MU | Or. |
| 12=101. | | <u> </u> | | | , | | 5/4/202 | , |
| 9) This truss is designed i | n accordance with the 201 parameters and read notes by | 8 International Residenti efore use. This design is has | al Code sections | K502.11.1 a | and R802.1 | U.2 and referenced ndividual building comp | $\frac{J}{T}$ | oaded |
| Continued on page 2 vertically. Applicability of c | design parameters and proper in | corporation of component is | responsibility of bui | lding designer | r – not truss d | lesigner or truss enginee | r. Bracing shown is for la | ateral support |
| of individual web members | only. Additional temporary bra | cing to ensure stability durin | g construction is the | responsibility | of the erecto | r. Additional permanen | t bracing of the overall str | ucture is the |
| responsibility of the building | g designer. For general guidance | e regarding fabrication, qual Guide to Good Practice for | ty control, storage, o | elivery, erect | ion and braci | ng, consult ANSI/TPI 1 | National Design Standar | <i>a for Metal</i> tute 583 |
| i une connected wood Irus | s construction and BCSI 1-05 | Same to Good I fuctice for | manung, msiunng | a bracing 0 | , menu rule | Connecteu wood Trus | ico nom muss riate misti | |

D'Onofrio Drive, Madison, WI 53719.

| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDERSON CREEK TBD PINNACLE DRIVE SPRING LA |
|-------------|-------|----------------|-------------|--------------|--|
| 22-2664-R01 | R05 | PIGGYBACK BASE | 1 | 1 | Job Reference (optional) # 32224 |
| | | Run: 8.4 | 30 s Feb 12 | 2 2021 Print | t: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 5 17:32:33 2022 Page 2 |

ID:NQLeVDd4PWwD4SswrTSD_y95ve-7nCkZcCtZmC?0eUK9fJpgQP_hULgMb3C7dhUErzJV2C

Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



| Job | Truss | Truss Type | | Qty | Ply LC | OT 49 CROSSING @ ANDE | RSON CREEK TBD PIN | NACLE DRIVE SPRING LAI |
|---|--|--|------------------------------------|------------------------|--|--|---|---|
| 22-2664-R01 | R06 | Piggyback Base | | 1 | 1 | | 4 | + 27774 |
| 22 2004 101 | 100 | | | <u> </u> | Jo Jo | b Reference (optional) | # | 52224 |
| | | | Run: 8.43 ID:NQLeVE | 30 s ⊦eb 12 Dd4PWwD | 2 2021 Print: 8. D4SswrTSDr | .430 s Feb 12 2021 MiTek In y95ve-b m6myDWK4K | ndustries, Inc. Thu May 5 ren3WjNg2Dey8duhw | 17:32:34 2022 Page 1 52IMLHQ1mHzJV2B |
| 0.10 | 1-0-0 | 12.6.0 | 20.1.0 | | 26.5.0 | 22.7.0 | 2459 | |
| 0-10- | -8 1-3-8 5-7-4 | 5-7-4 | 6-7-0 | | 6-4-0 | 7-2-0 | 0-10-8 | |
| | 1-0-0 | $4x6 \equiv$ | | | | | | Scale = 1:72.2 |
| | | | | 5x8 = | | | | |
| | : | 3.00 12 7 | тз | 8 | | | | |
|] | | | | TA | | | | [|
| | | | | | \sim | | | |
| | 4X6 | T2 | | / \\ | 72 | | | |
| | 3X8 1/2 | | | // | | 5x6 <> | | |
| | 5 | 6 | xxy9// | // | | 9 | | |
| 0-0 | L9 | В4 | / wet | // | \backslash | 10 | | 0-0 |
| -01 | | | //1/ | w | 44 | W13 | | 10- |
| 518 | 5x6 / 2/ | | // // | | | | | |
| JAC DAC | HW1 | W5 VV0 | | | 11 16 | | | |
| | 3W1 40 | | | | | | 4,0 | |
| | W4 | | 86 | - B - | | | WG14. 12 | |
|] <u>9</u> Z | | | B5 | | <u>. </u> | B7 | | |
| | 26 25 OX0 — | $4x4 = 29 0x8 - 21^{20}$ | 19 30 33 | 18 1 | 5 34 14 16 | 31 32 | ⊠ 13 | |
| 4 | (8 3×4 | 3×1 | | 3x | 8 = 4x4 = | | 1x6 — | |
| | 4-4- | 0x+ 11 | _ | 2x4 | 2x4 = | | 470 — | |
| | 484 — | 0x0 | | 2x4 | | | | |
| | | 28 | 4 — | | | | | |
| | | | | | | | | |
| | 1-0-02-3-8 7-10-12 1-0-01-3-8 5-7-4 | 13-3-8 14-3-8 | 20-1-0 | | -5-0 | 33-7-0 | | |
| Plate Offsets (X,Y) [3:0- | -1-12,0-2-4], [7:0-3-8,0-1-12 | 2], [8:0-5-12,0-2-0], [10:0-3-0 | ,0-3-0], [11:0-3-0 |),Edge], [| 22:0-2-12,0 | -2-4], [24:0-6-4,0-3-12 | 2] | |
| LOADING (nsf) | | | | | | | | |
| TCLL (roof) 20.0 | SPACING- | 2-0-0 CSI . | | EFL. | in (loc) |) I/defl L/d | PLATES | GRIP |
| Snow (Pf) 20.0 | Lumber DOL | 1.25 IC 0.8 | 2 Ve | ert(CT) | -0.42 17-19 | >904 240 | 101120 | 244/190 |
| ICDL 10.0 | Rep Stress Incr | YES WB 0.8 | B Ho | orz(CT) | 0.20 13 | n/a n/a | | |
| BCDL 10.0 | Code IRC2018/TP | 2014 Matrix-SH | | | | | Weight: 246 I | b FT = 20% |
| | | | BRACIN | IG- | | | | |
| TOP CHORD 2x4 SP No | .2 | | TOP CH | | Structural | wood sheathing direct | ly applied or 2-11-9 | oc purlins, except |
| BOT CHORD 2x4 SP No | .2 *Except* | | | | end vertica | als. | , , , | 1 , 1 |
| B2,B4: 2x4 | SP No.3, B5,B7: 2x4 SP S | S | BOT CH | IORD | Rigid ceilir | ng directly applied or 1 | 0-0-0 oc bracing, E | xcept: |
| WEBS 2x4 SP No | .3 2 No 2 À 1 2 11 | | | | 8-2-7 oc br | racing: 23-24 | | |
| SLIDER Leit 230 Sr | - INU.2 -A 1-2-11 | | | | 6-0-0 oc bi | acing: 21-22,20-21. | | |
| | | | WEBS | | 1 Row at n | nidpt 8-20, 1 | 10-13 | |
| | | | | | MiTek ree | commends that Stabili | izers and required cr | oss bracing |
| | | | | | be install | ed during truss erection | on, in accordance wit | h Stabilizer |
| | 2 = 1464/0.2.9 (min 0.1.12) | 12-1406/0.2.9 (min 0.2.0 | | | Installatio | on guide. | | |
| Max Horz | 2=1464/0-3-6 (mm. 0-1-13) 2=-224(LC 10) |), 13-1490/0-3-0 (11111. 0-2-0 |) | | | | | |
| Max Uplift | 2=-132(LC 12), 13=-122(LC | 313) | | | | | | |
| Max Grav | 2=1533(LC 20), 13=1675(L | C 21) | | | | | | |
| | nn Max Tan All faraaa | 50 (lb) or loss avaant when a | hown | | | | | |
| TOP CHORD 2-3=-177 | 2/150. 3-4=-3698/424. 4-27 | '=-2448/219, 5-27=-2380/23 | 5. 5-6=-2304/241 | Ι. | | | | |
| 6-7=-195 | 3/261, 7-8=-1546/263, 8-9= | -2132/263, 9-10=-2125/214, | 10-28=-578/242 | <u>2</u> , | | | | |
| 11-28=-6 | 83/223, 11-13=-589/229 | | | | | | | |
| BOT CHORD 2-26=-18 21-22=-7 | 3/1155, 4-24=-82/782, 23-2 12/0_7_22=_0/762_20_30=(| 24=-520/3439, 23-29=-161/20 0/1453 18-30=0/1453 15-18 | J94, 22-29=-161/ =0/1453_14-15= | /2094, -0/1//53 | | | | |
| 14-31=-7 | 0/1772.31-32=-70/1772.1 | 3-32=-70/1772 | -0/1400, 14-10- | -0/1400, | | | | |
| WEBS 3-26=-10 | 08/161, 4-23=-1368/365, 6 | -23=0/384, 6-22=-686/224, 2 | 0-22=0/1826, | | | | | |
| 8-22=-17 | 6/759, 19-20=-674/59, 8-19 | 9=-630/92, 8-16=-89/896, 14- | 16=-110/798, | | | | | |
| 9-14=-26 | 2/267, 10-13=-1707/64, 24 | -26=-203/1358, 3-24=-293/2 | 113 | | | | | |
| NOTES- (10-13) | | | | | | | AND ALL MANDER IN THE PARTY OF | a contraction of the second |
| 1) Unbalanced roof live lo | ads have been considered | for this design. | | | | | IN BTH LARO | 1111 |
| 2) Wind: ASCE 7-16; Vult | =130mph (3-second gust) | /asd=103mph; TCDL=5.0ps | f; BCDL=5.0psf; | h=23ft; C | Cat. II; Exp E | 3; Enclosed; MWFRS | OFESSIDA | Valle |
| (envelope) gable end z | one and U-U Exterior(2E) - $erior(2E) 29-7-14$ to $34-5-8$ | 2010-8 to 3-11-2, Interior(1) 3 | 5-11-2 to 6-8-9, E | S for rea | (R) 6-8-9 [0] | 26-7-14, Interior(1) | and the | |
| plate grip DOL=1.60 | | zone, o-o ior members and | | o loi ica | | | SEAL | |
| 3) TCLL: ASCE 7-16; Pr= | 20.0 psf (roof LL: Lum DOL | =1.25 Plate DOL=1.25); Pf= | 20.0 psf (Lum D | OL=1.15 | Plate DOL= | 1.15); ls=1.0; Rough | 28147 | III |
| Cat B; Partially Exp.; C | e=1.0; Cs=1.00; Ct=1.10 | | | | | (HIII) | 2 | 1 5 |
| I his truss has been de non concurrent with eth | signed for greater of min ro | of live load of 12.0 pst or 2.0 | 0 times flat roof | load of 2 | 0.0 pst on o | verhangs | 3. Su. A. | |
| 5) Provide adequate drain | age to prevent water pond | na. | | | | in the second seco | GINEE | Sant |
| 6) This truss has been de | signed for a 10.0 psf bottor | n chord live load nonconcurr | ent with any othe | er live loa | ads. | | MAK K. MORY | man |
| 7) * This truss has been d | esigned for a live load of 3 | 0.0psf on the bottom chord in | n all areas where | e a rectan | ngle 3-6-0 ta | ll by 1-0-0 wide will fit | All the for the state the | 5 · · · · · |
| between the bottom ch | ord and any other members | s, with BCDL = 10.0psf. | withstanding 10 | | t at ioint(a) a | vcent (it-lh) 2-122 | 5/4/2022 | |
| Warning ! | parameters and read notes b | efore use. This design is based or | ily upon parameters | s shown. a | nd is for an ind | dividual building component | ent to be installed and lo | aded |
| Continued on page 2 vertically. Applicability of c | lesign parameters and proper in | corporation of component is resp | onsibility of buildin | g designer | – not truss de | esigner or truss engineer. | Bracing shown is for lat | eral support |
| of individual web members | only. Additional temporary bra | cing to ensure stability during con | struction is the resp | oonsibility | of the erector. | . Additional permanent b | racing of the overall stru | cture is the |
| responsibility of the building | g designer. For general guidanc | e regarding fabrication, quality co | ntrol, storage, deliv | very, erecti | ion and bracin | g, consult ANSI/TPI 1 No | ational Design Standard | for Metal |
| Plate Connected Wood Trus | s Construction and BCSI 1-03 | Guide to Good Practice for Han | dling, Installing & | Bracing of | f Metal Plate | Connected Wood Trusses | from Truss Plate Institu | ite, 583 |

D'Onofrio Drive, Madison, WI 53719.

| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDERSON CREEK | TBD PINNACLE DRIVE SPRING LA |
|-------------|-------|----------------|--------------|-------------|--|------------------------------|
| 22-2664-R01 | R06 | Piggyback Base | 1 | 1 | Job Reference (optional) | # 32224 |
| | · | Run: 8 | .430 s Feb 1 | 2 2021 Prin | t: 8.430 s Feb 12 2021 MiTek Industries, Inc. Th | u May 5 17:32:34 2022 Page 2 |

NOTES- (10-13)

- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 1) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENCINEER FOR ADDITIONAL BRACING CONSIDERATIONS ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDERSON CREEK TBD PINN | ACLE DRIVE SPRING LA |
|-------------|-------|--------------------|-------------------------|-------------------------|---|--|
| 22-2664-R01 | R07 | Piggyback Base | 1 | 1 | Job Reference (optional) # | 32224 |
| | | Run: 8.4: ID:NO | 30 s Feb 12 QLeVDd4I | 2 2021 Print PWwD4Ss | t: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 5 1 swrTSDr_y95ve-3AKU_IE85NSiFxeiH4MHIrVJMI22 | 17:32:35 2022 Page 2 ZqXpVaxAalkzJV2A |

10) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the member must be braced. 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard





D'Onofrio Drive, Madison, WI 53719.

| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDERSON CREEK | TBD PINNACLE DRIVE SPRING LA |
|---|-------|-----------------------|-----|-----|----------------------------------|------------------------------|
| 22-2664-R01 | R08 | Piggyback Base Girder | 1 | 2 | Job Reference (optional) | # 32224 |
| Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 5 17:32:37 2022 Page 2 ID:NQLeVDd4PWwD4SswrTSDr y95ve-?YSEP FOd?iQVFo5OVOlqGalr6lqIPHo1FfhNczJV28 | | | | | | |

NOTES- (14-17)

- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 16=573, 11=919, 9=228.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 12) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 1-6-4 from the left end to 19-6-4 to
- connect truss(es) R03 (1 ply 2x4 SP), R04 (1 ply 2x4 SP) to back face of bottom chord. 13) Fill all nail holes where hanger is in contact with lumber.
- 14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
- SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-4=-60, 4-6=-60, 6-8=-60, 15-16=-20, 9-15=-20

Concentrated Loads (lb)

Vert: 11=-1299(B) 13=-1299(B) 10=-1403(B) 17=-1299(B) 18=-1299(B) 19=-1299(B) 20=-1299(B) 21=-1299(B) 23=-1299(B) 24=-1403(B) 25=-1404(B)





| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDERSON CREEK TBD PINNACLE DRIVE SPRING | G LA |
|---|-------|----------------|-----|-----|--|------|
| 22-2664-R01 | R09 | Piggyback Base | 1 | 1 | Job Reference (optional) # 32224 | |
| Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 5 17:32:37 2022 Page 2 ID:NQLeVDd4PWwD4SswrTSDr_y95ve-?YSEP_FOd?iQVFo5OVOlqGaew6qsIVWo1FfhNczJV28 | | | | | | 5 |

LOAD CASE(S) Standard





Warning !---Verify design parameters and read notes before use. This design is based only upon parameters shown, and is tot an increased of the sector. Bracing shown is for lateral support Continued on page 2. Shown is for lateral support vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss designer. Bracing shown is for lateral support vertically. Applicability of the sector. Additional permanent bracing of the overall structure is the of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDERSON CREEK TBD PINNACLE DRIVE SPRING LA |
|-------------|-------|--|------------------------|--------------------------|--|
| 22-2664-R01 | R10 | Piggyback Base Structural Gable COMMON I I Gable I G | able | 1 | Job Reference (optional) # 32224 |
| | | Run: 8.4: ID:NQL | 30 s Feb 12 eVDd4PW | 2 2021 Print /wD4Sswr | t: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 5 17:32:38 2022 Page 2 TSDr_y95ve-UI0dcKG0OIqH6PNHyDv_NU7piV441ynxGvOFv2zJV27 |

NOTES- (15-16)

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

15) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the member must be braced.
16) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard





Vertically. Applete Dilty of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

| Job | Truss | Truss Type | Qty | Ply | LOT 49 CROSSING @ ANDERSON CREEK TBD PINNACLE DRIVE SPRING LA |
|-------------|-------|------------------|-------------------------|-------------------------|---|
| 22-2664-R01 | R12 | GABLE | 1 | 1 | Job Reference (optional) # 32224 |
| | | Run: 8. ID:NQ | 430 s Feb 1 LeVDd4PW | 2 2021 Prin /wD4Sswr | It: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu May 5 17:32:39 2022 Page 2 TSDr_y95ve-yxa?pfHe9cy8kZyTWwQDwhf7rvZomTD5VZ8oRVzJV26 |

NOTES- (15-16)

- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 15) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the member must be braced.
 16) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard

| 22464-861 V15 Very # 32224 # 32224 Image: A set of the set of | Job | Truss | Truss Type | Qtv | Plv LOT 49 CRO | SSING @ ANDERSO | ON CREEK TBD PINNACLE DRIVE SPRING | |
|---|---|--|---|------------------------|---|--|---|--|
| Image: Add p: For direct Provide State and State International Internation Internation International Inte | 22-2664-R01 | VT05 | Valley | 1 | | | # 32224 | |
| $\frac{5+2}{5+2} \qquad (16.4)$ $\frac{5+2}{5+2} \qquad (16.5)$ | | | | Run: 8.430 s Feb 1 | 2 2021 Print: 8.430 s Feb VD4SswrTSDr v95ve-I | nce (optional) 12 2021 MiTek Indus MWF7Sh.IXRXKib(| tries, Inc. Thu May 5 17:32:42 2022 Page 1 0g2B2 wXKHe27akzr6XBXMS2gz.IV23 | |
| Jule | | | 5-1-2 | 10 | -2-4 -1-2 | in a chora cargo. | | |
| $\frac{1}{100000} = \frac{1}{10000000000000000000000000000000000$ | | | | 4x4 == | | | Scale = 1:44.0 | |
| Image: constraint of the constraint | | | | | | | | |
| $I_{10} = \frac{1}{10^{2}} \frac{1}{10$ | | | _ | 3 | | | | |
| Place Offsets (CV)= [60-28.0-10] Catal is a problem of the state of th | | | | | | | | |
| Image: stand Fig: stand | | | | | | | | |
| Image: space of the set | | | 16.00 12 | | | | | |
| Image: Second | | | φ T1 | 1 | 1 | | | |
| 2x4 // 8 9 7 10 6 24 // 5 2x4 // 8 9 7 10 6 24 // 5 2x4 // 8 9 7 10 6 24 // 5 2x4 // 8 9 7 10 6 24 // 5 2x4 // 8 9 7 10 6 24 // 5 1924 1 1924 1 24 // 5 24 // 5 1924 1 1924 1 1924 1 1924 1054 100.0 115 100.4 1924 1 1924 100.1 100.0 Repercent 200.0 15 100.0 | | | 9 | ST2 | $\langle \rangle$ | | | |
| $\frac{1}{244}$ $\frac{1}{224}$ $\frac{1}$ | | | 2x4 | | 2x4 | | | |
| Add Age 9 7 10 6 24 ii 24 ii 1024 24 ii 24 iii 24 iii 1024 1024 1024 1024 Charles (KY)- (60-28.0-10) (80-28.0-10) 1024 1024 Charles (100) 20.0 SPACING 2.0.20.01 1024 1024 Charles (100) 20.0 SPACING 2.0.20.01 115 EX 0.28 Veri(L1) n/a i n/a 999 MIZe 2.441/90 Charles (100) 10.0 Lumber DOL 1.15 EX 0.28 Veri(C1) n/a i n/a 999 Mize 2.441/90 BCL 10.0 Code (RC2018/TPI2014 Matrix/SH Mize 2.445 Mize 2.441/90 DCH CHOR 2.44 SP No.3 TOP CHORD 3 Stoctural wood sheathing directly applied or 60-00 cp purlis. DCT CHOR 2.44 SP No.3 TOP CHORD 3 Stoctural wood sheathing directly applied or 60-00 cp purlis. DCT CHOR 2.44 SP No.3 Stoctural wood sheathing directly applied or 60-00 cp purlis. DCT CHOR 2.44 SP No.3 Stoctural wood sheathing directly applied or 60-00 cp purlis. RECTONS. All bearings 10-24. Stoctural wood sheathing directly applied or 60-00 cp purlis. RCT 2.56 (b) - Max. Comp.Max. Ten All forces 250 (b) or less at joint(s) except 1=241(LC 12), 5=271(LC 13), 5=352(LC 12), 6=352(LC 13), 6=452(LC 13 | | | | | 4 SH1 5 | | | |
| 2x4 / g 9 7 10 6 2x4 / k 2x4 / g 9 7 10 6 2x4 / k 2x4 / l 2x4 / l 2x4 / k 2x4 / k 2x4 / k 2x4 / l 2x4 / k 2x4 / k 2x4 / k 2x4 / k 2x4 / l 2x4 / k 2x4 / k 2x4 / k 2x4 / k 2x4 / k 2x4 / k 2x4 / k 2x4 / k 2x4 / k 1024 1024 1024 1024 1024 LOADING (n5) SPACING- 2:0-0 11:5 BC 0:2.8 Vert(CT) n/a n/a 999 MT20 2:44/190 Structural wood sheathing directly applied or 6:0-0 oc purlins. BCC Code IRC2018/TPI2014 Matrix-SH BRACING- TOP CHORD Risterious and a structural wood sheathing directly applied or 6:0-0 oc purlins. BCT CHORD 2:44 SP No.3 Structural wood sheathing directly applied or 6:0-0 oc purlins. Risterious and a structural wood sheathing directly applied or 6:0-0 oc purlins. REACTONS. All bearings 10-2-4. Image: Structural wood sheathing directly applied or 6:0-0 oc purlins. Risterious and a structural wood sheathing directly applied or 6:0-0 oc purlins. Risteria structural wood sheathing directly applied | | | | B I | | | | |
| $\begin{array}{c c c c c c c c c c c c c c c c c c c $ | | | | ~~~~~ | | | | |
| Interpretation of the second part of the secon | | | ^{2x4} // 8 9 2x4 | 7 10 2x4 | 6 ^{∠x4} ∖∖ 2x4 | | | |
| Interviewend <th colspan="</td> <td></td> <td></td> <td></td> <td>10-2-4</td> <td></td> <td></td> <td></td> | | | | 10-2-4 | | | | |
| LODING (psr) TCLL (root) SPACING- 20.0 2-0-0 Plate Grip DoL CSI. DEFL vert(L1) in (loc) I/deft L/d PLATES GRIP MT20 Show (P) 20.0 Lumber DOL 1.15 BC 0.26 Vert(C1) n/a - n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.13 Horz(CT) 0.00 5 n/a n/a BCDL 10.0 Rep Stress Incr YES WB 0.13 Horz(CT) 0.00 5 n/a n/a BCDL 10.0 Code IRC2018/TPI2014 Matrix-SH BRACING- TOP CHORD 2x4 SP No.3 Structural wood sheathing directly applied or 6-0-0 cc purlins. BOT CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 Matrix-SH BRACING- TOP CHORD Rigit celling directly applied or 6-0-0 cc baraing. Max Forz Al reactions 250 to rises at joint(s) except 1=-217(LC 10), 5=-195(LC 11), 8=-352(LC 12), 6=-352(LC 13), Max Grav Al reactions 250 to rises at joint(s) except 1=-247(LC 12), 5=-271(LC 13), 7=-336(LC 19), 8=-462(LC 19), 6=-461(LC 20) Mixer Air Least As 22/426, 4-6-429/426 Matrix-SH Hore Second 200, 10 10.00 10.00 10 | Plate Offsets (X,Y) [6:0 |)-2-8,0-1-0], [8:0-2-8,0-1-0] | | 10-2-4 | | | | |
| Current (con) 200 Plate Grip DOL 1.15 TC 0.26 Vert(CT) n/a - n/a 999 MT20 244/190 TCDL 10.0 Rep Stress Incr YES WB 0.13 Horz(CT) 0.00 5 n/a n/a 999 MT20 244/190 BCLL 10.0 Code IRC2018/TPI2014 Matrix-SH Weight: 51 Ib FT = 0% BCLL 10.0 Code IRC2018/TPI2014 Matrix-SH BRACINC- TOP CHORD 2x4 SP No.2 Weight: 51 Ib FT = 0% BOT CHORD 2x4 SP No.3 BRACINC TOP CHORD 3x4 SP No.3 BOT CHORD 2x4 SP No.3 BRACINC- CTUBER 2x4 SP No.3 BRACINC TOP CHORD 1x0 Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 1x4 SPN A.3 BOT CHORD 3x4 SP No.2 BRACINC- Installation guide. Immed and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS. All bearings 10-24. (b) - Max Horz 1=159(LC 1), 5=-195(LC 1), 5=-352(LC 12), 6=-352(LC 12), 6=-352(LC 12), 6=-352(LC 12), 6=-352(LC 12), 6=-461(LC 20) FORCES. (b) - Max Comp.AMAX Tan - All forces 250 (b) or less except when shown. TOP CHORD 1/2=-342/251, 4-5=-325/251 </td <td>LOADING (psf)</td> <td>SPACING-</td> <td>2-0-0 CSI.</td> <td>DEFL.</td> <td>in (loc) l/defl</td> <td>L/d</td> <td>PLATES GRIP</td> | LOADING (psf) | SPACING- | 2-0-0 CSI . | DEFL. | in (loc) l/defl | L/d | PLATES GRIP | |
| Hold 0.0 Rep Stress Incr YES WB 0.13 Horz(CT) 0.0 5 n/a Weight: 51 lb FT = 0% BCDL 10.0 Code IRC2018/TPI2014 Matrix-SH BRACINC- TOP CHORD 2x4 SP No.2 Structural wood sheathing directly applied or 6-0-0 oc parcing. BOT CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 Structural wood sheathing directly applied or 6-0-0 oc bracing. OTHERS 2x4 SP No.3 Structural wood sheathing directly applied or 6-0-0 oc bracing. OTHERS 2x4 SP No.3 Structural wood sheathing directly applied or 6-0-0 oc bracing. REACTIONS. All bearings 10-2-4. Image: Structural wood sheathing directly applied or 0-0-0 oc bracing. (Ib)- Max Horz 1=159(LC 0) max care value reactions 250 lb or less at joint(s) except 1=-217(LC 10), 5=-195(LC 11), 8=-352(LC 12), 6=-352(LC 13). Max Care value reactions 250 lb or less at joint(s) except 1=284(LC 12), 5=271(LC 13), 7=336(LC 19), 8=462(LC 19), 6=462(LC 19), 6=461(LC 20) FORCES. (b)- Max Comp Max. Tam - All forces 250 (b) or less except when shown. UP PCHORD 1/2=-342/25, 1/4=-325/251 Villes 2/2=-482/426, 4/2=-482/426 NOTES- (9-10) 1) Unbalanced roof live loads have been considered for this design. 2) 2) Wind: ASCE 7-16; Vill= | Snow (Pf) 20.0 | Plate Grip DOL Lumber DOL | 1.15 TC 0.26 1.15 BC 0.28 | Vert(LL) Vert(CT) | n/a - n/a n/a - n/a | 999 999 | MT20 244/190 | |
| DODE 100 1 BRACING- TOP CHORD 2x4 SP No.3 DOT CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 Structural wood sheathing directly applied or 16-0-0 oc bracing. DOT CHORD 2x4 SP No.3 BOT CHORD 3x4 SP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS. All bearings 10-2-4. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS. All bearings 10-2-4. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS. All bearings 10-2-4. MiTek recommends that Stabilizers and required cross bracing. ((b) - Max Comp./Max. Ten All forces 250 (lb) or less except when shown. 5271(LC 13), 7=336(LC 19), 8=462(LC 19), 6=461(LC 20) FORCES. (b) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. 000 UPO PCHORD 1.2=342(L2 1.4=5-3425(251 45-342(26) VWIDE 3.262 T-16; VUIE=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23f; Cat. II; Exp B; Enclosed; MWFRS (or reactions shown; Lumber DUL==1.00 Let and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DUL==1.00 Let and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DUL==1.00 Let and ri | BCLL 0.0 * | Rep Stress Incr Code IRC2018/TF | YES WB 0.13 Pl2014 Matrix-SH | Horz(CT) | 0.00 5 n/a | n/a | Weight: 51 lb FT = 0% | |
| TOP CHORD 2x4 SP No.2 TOP AND Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Structural wood sheathing directly applied or 6-0-0 co traing. OTHERS 2x4 SP No.3 BOT CHORD Structural wood sheathing directly applied or 6-0-0 co traing. Rigid celling directly applied or 10-0-0 co traing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS . All bearings 10-2-4. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS . All reactions 250 lb or less at joint(s) except 1=-217(LC 10), 5=-195(LC 11), 8=-352(LC 12), 6=-352(LC 13), 6=-467(LC 20) FORCES . (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD TOP CHORD 1-2=-342/251, 4-5=-325/251 View S VEBS 2-8=-482/426, 4-6=-482/426 NOTES . (g-10) 1) Unbalanced roof live loads have been considered for this design. 1) 2) Wind: ASCE 7-16; Pre-20 op sf (or LL: Lum DOL=-1.15); Pf=20.0 psf, IC LL: SCE 7-16; Pre-20 op sf (or LL: Lum DOL=-1.15); Pf=20.0 psf (LL: DL DOL=-1.15); Pf=20.0 psf (LL: DL DOL=-1.15); Pf=20.0 psf (LL: DL DOL=-1.15); Pf=20.0 psf (or LL: DL DOL=-1.15); Pf=20.0 psf (LL: DL DOL=-1.15); Pf=20.0 psf (or LL: DL DOL=-1.15); Pf=20.0 psf (LL: DL DOL=-1.15); Pf=20 | LUMBER- | | | BRACING- | | | | |
| OTHERS 2x4 SP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS. All bearings 10-24. ((b) - Max Horz 1=159(LC 9) Max Grav All reactions 250 lb or less at joint(s) except 1=-217(LC 10), 5=-195(LC 11), 8=-352(LC 12), 6=-352(LC 13) Max Grav All reactions 250 lb or less at joint(s) except 1=284(LC 12), 5=271(LC 13), 7=336(LC 19), 6=461(LC 20) FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 1-2=-342/251, 4-5=-325/251 WEBS 2-8=-482/426, 4-6=-482/426 NOTES (g-10) 1) Unbalanced roof live loads have been considered for this design. 2) Winch XSCE 7-16, Full=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23f; Cat. II; Exp B; Enclosed; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16, Full=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp; Ce=1.0; Cs=1.00; Cs=1.00 4) Gable requires continuous bottom chord hearing. 5) This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 1.0.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 322 lb | TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 | | TOF BOT | TOP CHORD BOT CHORD | Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. | | | |
| REACTIONS. All bearings 10-2-4. (b) - Max Horz 1=159(LC 9) Max Upilit All upilit 100 ib or less at joint(s) except 1=-217(LC 10), 5=-195(LC 11), 8=-352(LC 12), 6=-352(LC 13), Max Grav All reactions 250 ib or less at joint(s) except 1=284(LC 12), 5=271(LC 13), 7=336(LC 19), 8=462(LC 19), 6=461(LC 20) FORCES. (b) - Max. Comp./Max. Ten All forces 250 (ib) or less except when shown. TOP CHORD 1-2=-342/251, 4-5=-325/251 WEBS 2-8-482/426, 4-6-482/426 NOTES- (9-10) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pre-20.0 psf (root L:: Lum DDL=1.15); Pf=20.0 psf (Lum DDL=1.15 Plate DDL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing. 5) This truss has been designed for a 10.0 psf bottom chord in ela areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 4, 195 lb uplift at joint 5, 352 lb uplift at joint 6 and 352 lb uplift at joint 6. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP11. 9) Graphical web bracing representation does not depict the size, type or the orientation of the bace on the web. Symbol only indicates that | OTHERS 2x4 SP No | 5.3 | | | MiTek recomment be installed during | ds that Stabilizer g truss erection, i | s and required cross bracing n accordance with Stabilizer | |
| (lb) - Max Horz 1=159(LC 9) Max Uplift All uplift 100 lb or less at joint(s) except 1=-217(LC 10), 5=-195(LC 11), 8=-352(LC 12), 6=-352(LC 13), Max Grav All reactions 250 lb or less at joint(s) except 1=284(LC 12), 5=271(LC 13), 7=336(LC 19), 8=462(LC 19), 6=461(LC 20) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-342/251, 4-5=-325/251 WEBS 2-8=-482/426, 4-6=-482/426 NOTES- (9-10) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23f; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp; Ce=1.0; CS=1.00; CC=1.00 4) Gable requires continuous bottom chord bearing. 5) This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANS/ITP1 1. 9) Graphical web braing representation does not depict the size, type or the orientation of the bace on the web. Symbol only indicates that | REACTIONS. All bearing | ngs 10-2-4. | | | Installation guide. | - | | |
| Max Grav All reactions 250 lb or less at joint(s) except 1=284(LC 12), 5=271(LC 13), 7=336(LC 19), 8=462(LC 19), 6=461(LC 20) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-342/251, 4-5=-325/251 WEBS 2-8=-482/426, 4-6=-482/426 NOTES- (9-10) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15; Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.0; Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Pr=20.0 psf (roof LL: Lum DOL=0.0; Pb obtiom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a 10.0 psf obtiom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a live load of 30.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0 psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift a | (lb) - Max Horz Max Uplifi | 1=159(LC 9) t All uplift 100 lb or less at | joint(s) except 1=-217(LC 10), 5=-19 | 95(LC 11), 8=-352(| LC 12), 6=-352(LC 1 | 3) | | |
| FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-342/251, 4-5=-325/251 WEBS 2-8=-482/426, 4-6=-482/426 NOTES- (9-10) Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 Gable requires continuous botom chord bearing. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1 1. Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that | Max Grav | All reactions 250 lb or les 6=461(LC 20) | s at joint(s) except 1=284(LC 12), 5 | =271(LC 13), 7=33 | 6(LC 19), 8=462(LC | 19), | | |
| TOP CHORD 1-2=-342/251, 4-5=-325/251 WEBS 2-8=-482/426, 4-6=-482/426 NOTES- (9-10) Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (or reactions shown; Lumber DOL=1.60 plate grip DOL=1.60) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 Gable requires continuous bottom chord bearing. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a 10.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1 1. Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the standard ANSI/TP1 1. | FORCES. (lb) - Max. Co | mp./Max. Ten All forces | 250 (lb) or less except when shown. | | | | | |
| NOTES- (9-10) 1) Uhbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1 1. 9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that | TOP CHORD 1-2=-342 WEBS 2-8=-482 | 2/251, 4-5=-325/251 2/426, 4-6=-482/426 | | | | | | |
| Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 Gable requires continuous bottom chord bearing. This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the standard ANSI/TPI 1. | NOTES- (9-10) | | | | | | | |
| (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the standard ANSI/TPI 1. | Unbalanced roof live lo Wind: ASCE 7-16; Vul | oads have been considered t=130mph (3-second gust) | l for this design. Vasd=103mph; TCDL=5.0psf; BCD | L=5.0psf; h=23ft; (| Cat. II; Exp B; Enclos | ed; MWFRS | | |
| 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; CS=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing. 5) This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that | (envelope) gable end z reactions shown; Luml | zone and C-C Exterior(2) zo ber DOL=1.60 plate grip D0 | one; end vertical left and right expos DL=1.60 | ed;C-C for membe | ers and forces & MW | FRS for | | |
| 4) Gable requires continuous bottom chord bearing. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the true of the size of the size of the orientation of the brace on the web. Symbol only indicates that the true of the size of the size of the orientation of the brace on the web. Symbol only indicates that the true of the size of the size of the size of the orientation of the brace on the web. Symbol only indicates that the true of the size of t | TCLL: ASCE 7-16; Pr= Cat B; Partially Exp.; C | =20.0 psf (roof LL: Lum DO Ce=1.0; Cs=1.00; Ct=1.10 | L=1.15 Plate DOL=1.15); Pf=20.0 p | sf (Lum DOL=1.15 | Plate DOL=1.15); Is | =1.0; Rough | | |
| 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the standard ANSI/TPI 1. | 4) Gable requires continu5) This truss has been de | ious bottom chord bearing. esigned for a 10.0 psf botto | m chord live load nonconcurrent wit | h any other live loa | ads. | | | |
| 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 1, 195 lb uplift at joint 5, 352 lb uplift at joint 8 and 352 lb uplift at joint 6. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the standard at t | 6) * This truss has been of between the bottom ch | designed for a live load of 3 hord and any other member | 30.0psf on the bottom chord in all an s, with BCDL = 10.0psf. | eas where a rectar | ngle 3-6-0 tall by 1-0- | 0 wide will fit | multing | |
| 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the size of the size of the size of the orientation of the brace on the web. Symbol only indicates that the size of the size | Provide mechanical co 352 lb uplift at joint 8 a | onnection (by others) of trus and 352 lb uplift at joint 6. | ss to bearing plate capable of withst | anding 217 lb uplif | t at joint 1, 195 lb upl | lift at joint 5, | ATH CARO | |
| 9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that | This truss is designed standard ANSI/TPI 1. | in accordance with the 201 | 8 International Residential Code se | ctions R502.11.1 a | and R802.10.2 and re | eferenced | POFESSION THE | |
| the member must be braced. | 9) Graphical web bracing the member must be b | representation does not de | epict the size, type or the orientation | of the brace on th | e web. Symbol only | indicates that | SEAL | |
| 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated. | 10) Bearing symbols are structural design of th | only graphical representati ne truss to support the load | ons of a possible bearing condition. s indicated. | Bearing symbols a | are not considered in | the | 28147 | |
| LOAD CASE(S) Standard | LOAD CASE(S) Standard | d | | | | A A A A A A A A A A A A A A A A A A A | MOINEER | |
| AN K MORALIN | | | | | | Contra International Contraction of the Contraction | At K. MORALIN | |
| 5/4/2022 | | | | | | | 5/4/2022 | |

LAł

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 1 and 52 lb uplift at joint 3. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced

standard ANSI/TPI 1.

9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the member must be braced.

10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1 and 35 lb uplift at joint 3.
 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

9) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the member must be braced.

10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

in (loc)

n/a

n/a

0.00

I/defl

n/a

n/a

n/a

4

I/d

999

999

n/a

 LUMBER BRACING

 TOP CHORD 2x4 SP No.2
 TOP CHORD 2x4 SP No.3
 Structural wood sheathing directly applied or 4-3-12 oc purlins.

 BOT CHORD 2x4 SP No.3
 BOT CHORD 2x4 SP No.3
 Structural wood sheathing directly applied or 10-0-0 oc bracing.

 REACTIONS. (lb/size) 1=149/4-3-12 (min. 0-1-8), 4=149/4-3-12 (min. 0-1-8)
 Min. 0-1-8)
 Min. 0-1-8)

2-0-0

1.15

1.15

YES

Max Horz 1=-18(LC 8) Max Uplift1=-19(LC 9), 4=-19(LC 8)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2018/TPI2014

Lumber DOL

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10-11)

LOADING (psf)

20.0

20.0

10.0

10.0

0.0

TCLL (roof)

Snow (Pf)

TCDL

BCLL

BCDL

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

CSI.

тс

BC

WB 0.00

Matrix-R

0.09

0.16

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Provide adequate drainage to prevent water ponding

5) Gable requires continuous bottom chord bearing.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1 and 19 lb uplift at joint 4.
 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced

standard ANSI/TPI 1.

10) Graphical web bracing representation does not depict the size, type or the orientation of the brace on the web. Symbol only indicates that the member must be braced.

11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

LOAD CASE(S) Standard

will fit ed ate SEAL 28147 SEAL 2915 SEAL

PLATES

Weight: 13 lb

MT20

GRIP

244/190

FT = 0%