

RE: J0422-2019 Cav&Cates/Lot 52 Anderson Creek Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J0422-2019 Lot/Block: Address: City:

Model: Subdivision: State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 19 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3 4 5 6 7 8 9 10 11 12 13 14	Seal# I50280782 I50280783 I50280784 I50280785 I50280786 I50280787 I50280788 I50280789 I50280790 I50280791 I50280793 I50280793 I50280795	Truss Name A1 A1A A1GE A2 A2GE B1 B1GE C1GE C2 C3 M1 M1GE P1 P1GE	Date 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022 2/17/2022
15 16 17 18	I50280796 I50280797 I50280798 I50280799	VA1 VA2 VA3 VA4	2/17/2022 2/17/2022 2/17/2022 2/17/2022
19	150280800	VA5	2/17/2022

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

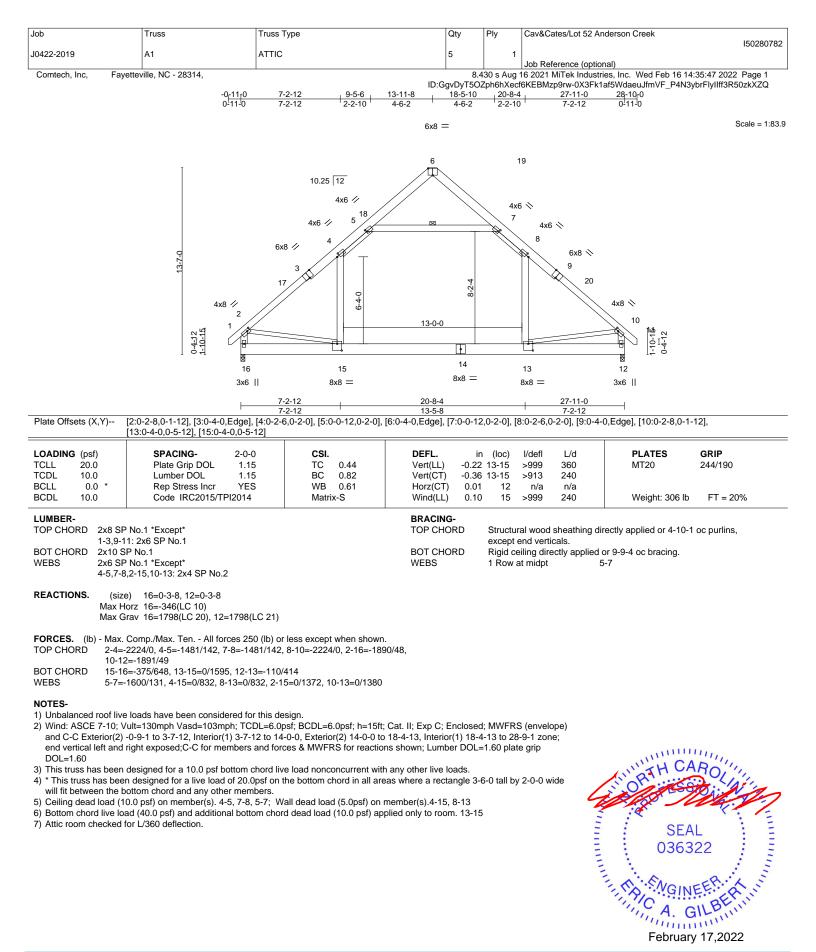
My license renewal date for the state of North Carolina is December 31, 2022

North Carolina COA: C-0844

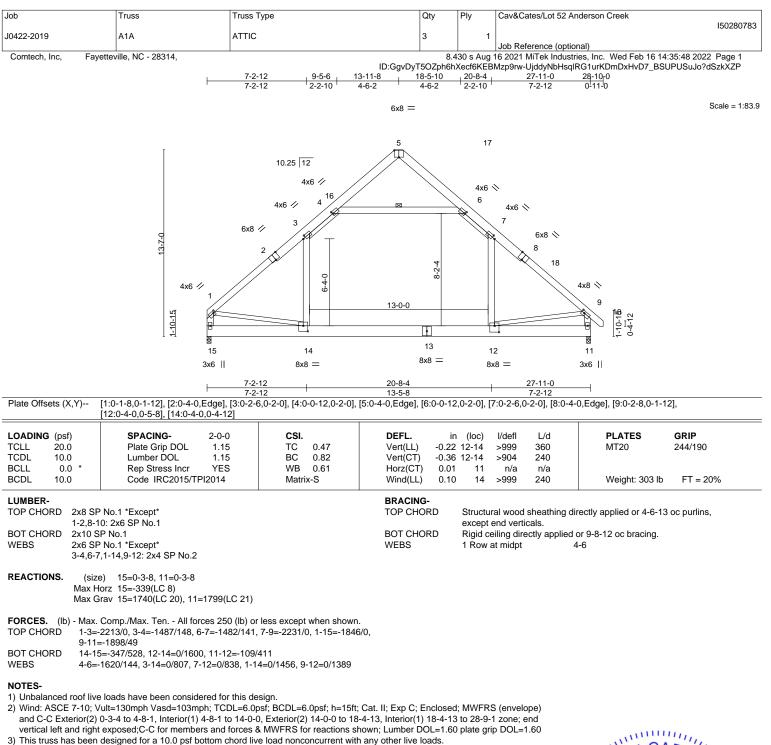
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

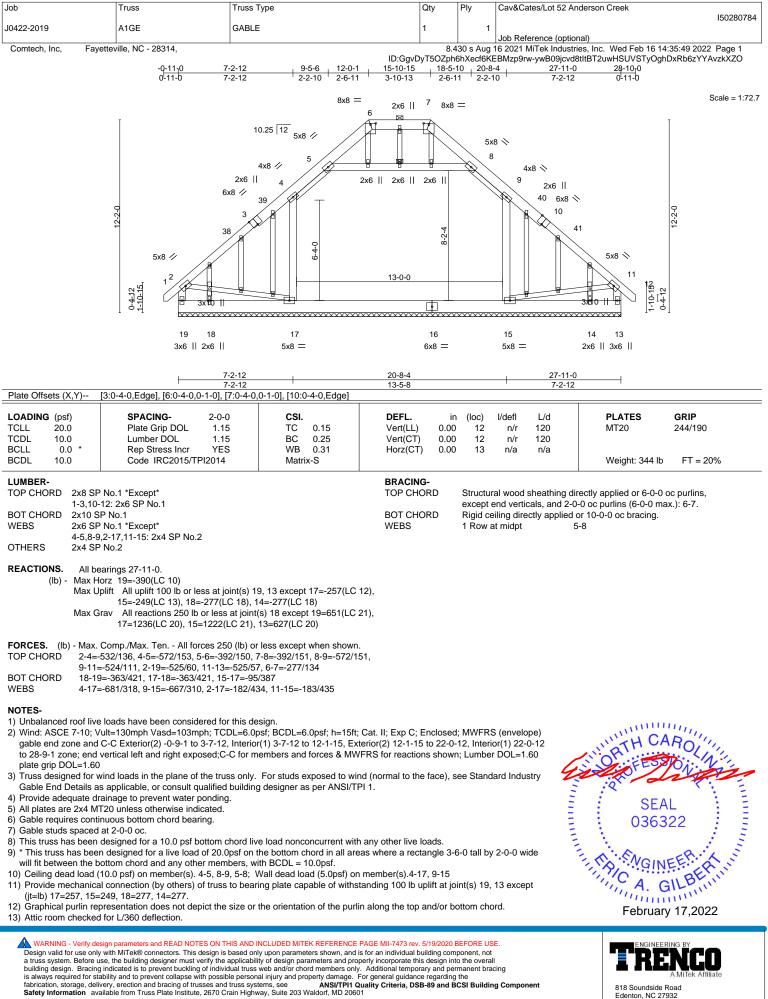
5) Ceiling dead load (10.0 psf) on member(s). 3-4, 6-7, 4-6; Wall dead load (5.0psf) on member(s).3-14, 7-12

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14

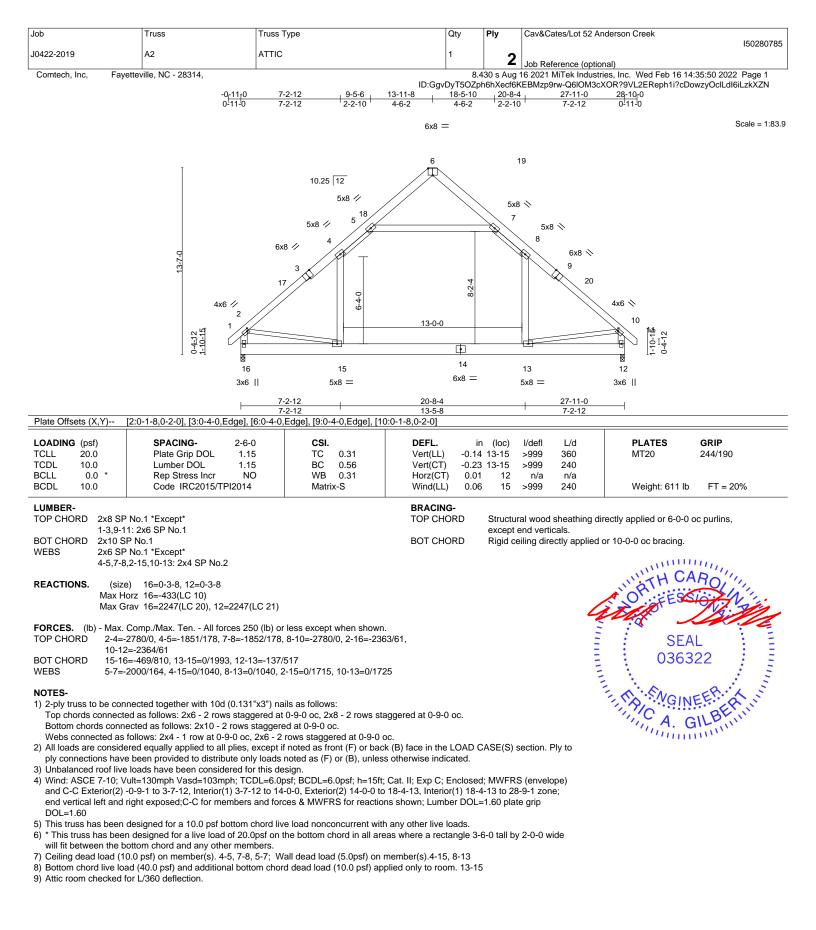
7) Attic room checked for L/360 deflection.





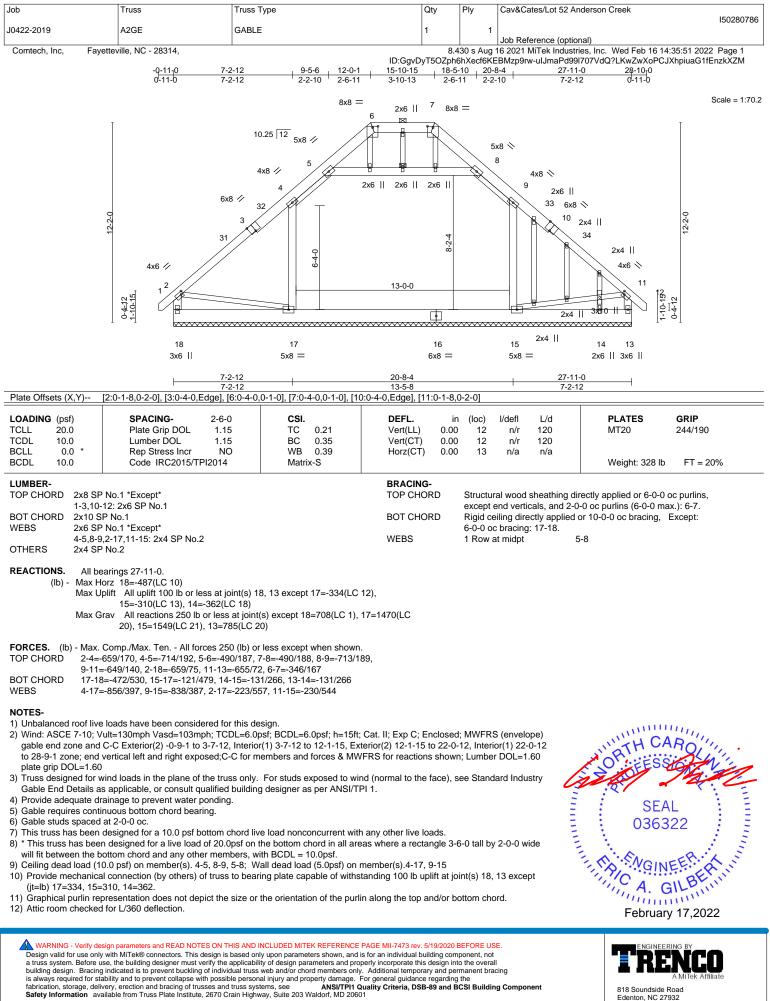


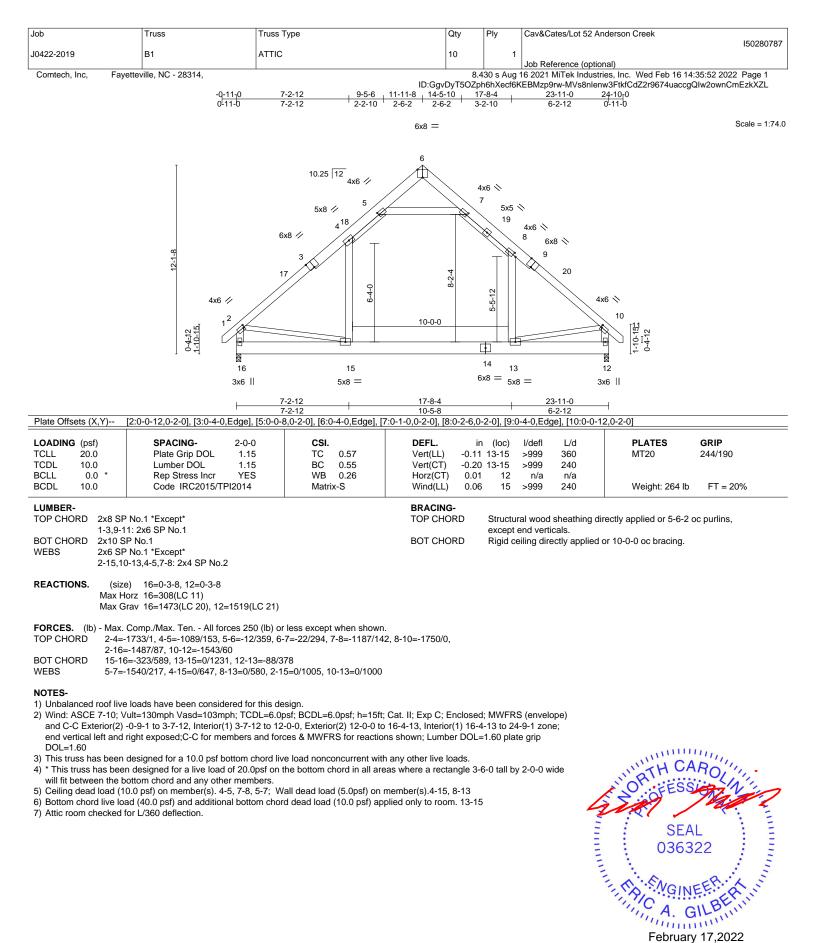
Edenton, NC 27932



February 17,2022

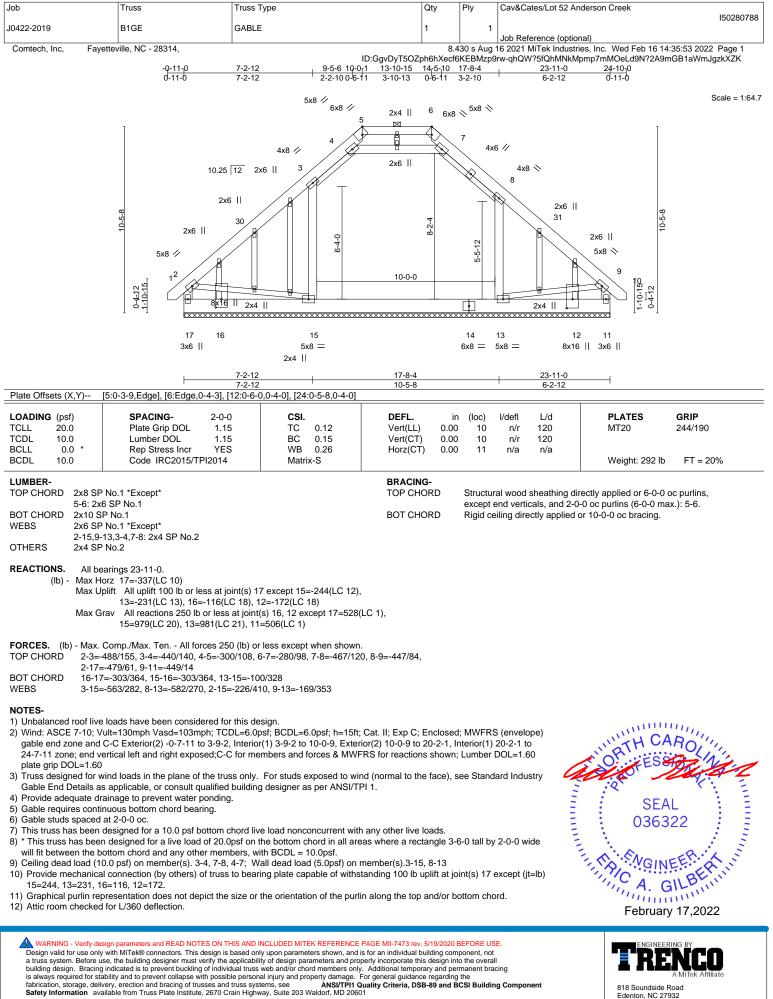


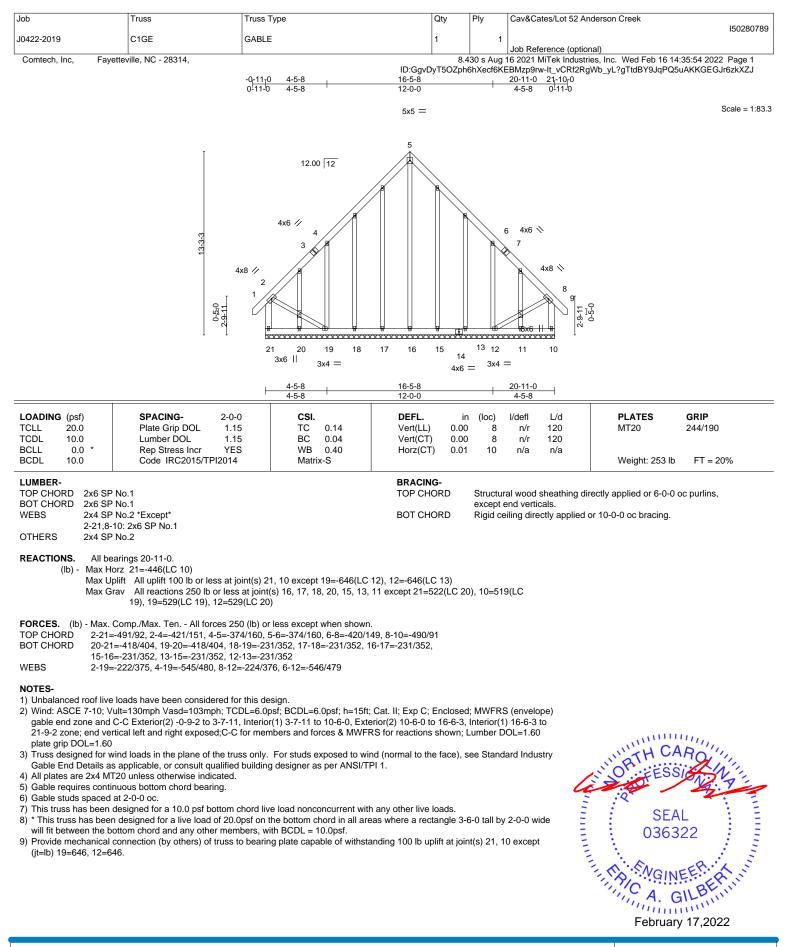




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 TRENGINEERING BY A MiTek Atfiliate 818 Soundside Road

Edenton, NC 27932







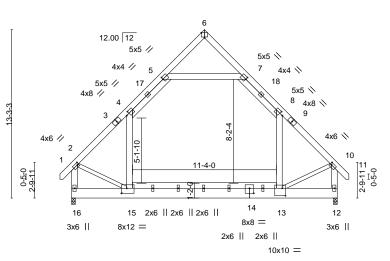
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Cav&Cates/Lot 52 Anderson Creek
					150280790
J0422-2019	C2	ATTIC	5	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8.4	30 s Aug 1	6 2021 MiTek Industries, Inc. Wed Feb 16 14:35:55 2022 Page 1

# -0<sub>1</sub>11<sub>1</sub>0 4-6-12 7-5-1 10-5-8 13-5-15 16-4-4 20-11-0 21<sub>1</sub>-10 0111<sub>2</sub>0 4-6-12 2-10-5 3-0-7 2-10-5 4-6-12 0-11-0



Scale = 1:90.7



		1 4-6-12 I	16-4-4	20-11-0	
		4-6-12	11-9-8	4-6-12	
Plate Offsets (X,Y)	[2:0-1-4,0-2-0], [4:0-3-2,0-2-4], [5:0-0-2,0	-2-4], [6:0-3-0,Ed	ge], [7:0-0-2,0-2-4], [8:0-3-2,0-2-4],	[10:0-1-4,0-2-0	], [13:0-3-0,0-7-0], [14:0-4-0,0-4-12],
	[15:0-5-0,0-4-12]				

LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.67 BC 0.65 WB 0.28 Matrix-S	Vert(LL) -0.17 Vert(CT) -0.28 Horz(CT) 0.01	n (loc) l/defl 7 13-15 >999 8 13-15 >888 12 n/a 5 13-15 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 271 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x10 S			BRACING- TOP CHORD	Structural wood	0	rectly applied or 5-3-6	oc purlins,
WEBS 2x6 SP	P No.1 *Except* 0-13: 2x4 SP No.2		BOT CHORD			or 10-0-0 oc bracing.	
	e) 16=0-3-8, 12=0-3-8 orz 16=-351(LC 10) rav 16=1457(LC 21), 12=1457(LC 20)						

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1461/0, 4-5=-920/156, 7-8=-920/156, 8-10=-1461/0, 2-16=-1642/9, 10-12=-1642/9

BOT CHORD 15-16=-315/413. 13-15=0/927

WEBS 5-7=-973/190, 4-15=-9/605, 8-13=-8/605, 2-15=0/926, 10-13=0/929

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 10-6-0, Exterior(2) 10-6-0 to 14-10-13, Interior(1) 14-10-13 to 21-9-2 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 4-5, 7-8, 5-7; Wall dead load (5.0psf) on member(s).4-15, 8-13

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15

7) Attic room checked for L/360 deflection.



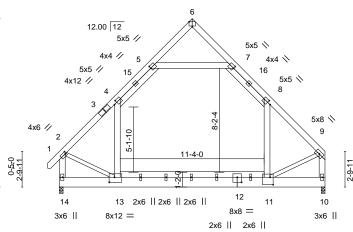




#### ID:GgvDyT5O2ph6hXecf6KEBMzp9rw-FG6fd6hl2HmIDGVOouv5GzFXwDx4M6edjYlQv?z -0-11-0 4-6-12 7-5-1 10-5-8 13-5-15 16-4-4 20-11-0 0-111-0 4-6-12 2-10-5 3-0-7 3-0-7 2-10-5 4-6-12



Scale = 1:90.7



10×10 -

				10x10 =			
Plate Offsets (X,Y)	[2:0-1-0,0-2-0], [4:0-3-2,0-2-4], [5:0-0-2	4-6-12 4-6-12 0-2-41 [6:0-3-0 Edge] [7:0	<u>16-4-4</u> <u>11-9-8</u> )-0-2 0-2-4] [8:0-3-2 0-2	20-11-0 4-6-12 2-41 [11:0-3-0 0-7		0-4-12] [13:0-5-0 0-4-	12]
		,0 2 4], [0.0 0 0,⊏uge], [7.0	0 2,0 2 4], [0.0 0 2,0 2	. +j, [11.0 0 0,0 1	0], [12.0 4 0	;0 + 12]; [10:0 0 0;0 +	12]
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. ir	(loc) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.68	Vert(LL) -0.17	11-13 >999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.65	Vert(CT) -0.28	11-13 >881	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.28	Horz(CT) 0.01	10 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.05	11-13 >999	240	Weight: 268 lb	FT = 20%
LUMBER-	1		BRACING-				
TOP CHORD 2x6 SF	P No.1		TOP CHORD	Structural wood	l sheathing di	rectly applied or 5-2-5 o	oc purlins.
	SP No.1			except end ver	0		· · · · · · · · · · · · · · · · · · ·
			BOT CHORD			or 10-0-0 oc bracing	
WEBS 2x6 SF	P No.1 *Except*		BOT CHORD	Rigid ceiling dir	ectly applied	or 10-0-0 oc bracing.	

REACTIONS. (size) 14=0-3-8, 10=0-3-8 Max Horz 14=-318(LC 10)

2-13,9-11: 2x4 SP No.2

Max Grav 14=1455(LC 21), 10=1415(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-1460/0, 4-5=-917/148, 7-8=-924/159, 8-9=-1452/0, 2-14=-1641/0, 9-10=-1596/0

3-3-3

BOT CHORD 13-14=-297/380, 11-13=0/906

WEBS 5-7=-982/196, 4-13=-5/609, 8-11=-20/581, 2-13=0/925, 9-11=0/949

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 10-6-0, Exterior(2) 10-6-0 to 14-10-13, Interior(1) 14-10-13 to 20-8-12 zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

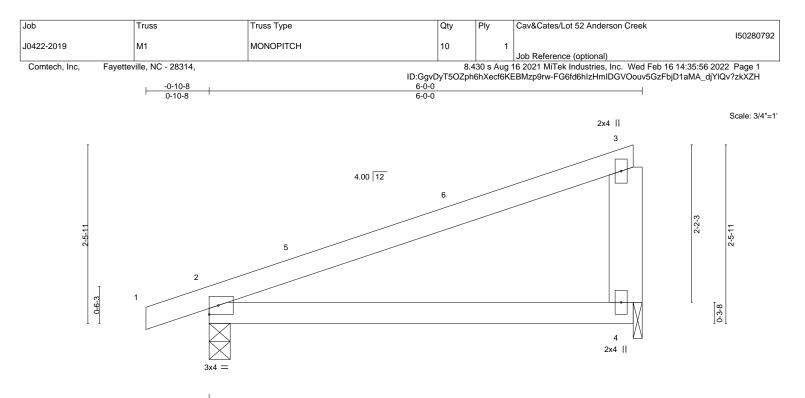
5) Ceiling dead load (10.0 psf) on member(s). 4-5, 7-8, 5-7; Wall dead load (5.0psf) on member(s).4-13, 8-11

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

7) Attic room checked for L/360 deflection.



818 Soundside Road Edenton, NC 27932



OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.43	Vert(LL) -(	0.05 2-4	>999	360	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.30	Vert(CT) -(	0.11 2-4	>642	240		
SCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00	n/a	n/a		
SCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.12 2-4	>579	240	Weight: 24 lb	FT = 20%

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=73(LC 8) Max Uplift 2=-114(LC 8), 4=-97(LC 8)

Max Grav 2=292(LC 1), 4=219(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=114.

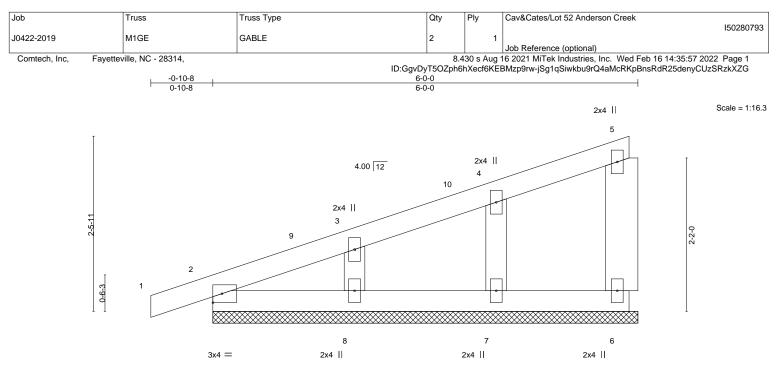


Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





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LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.05 BC 0.02 WB 0.04 Matrix-P	Vert(CT) 0.	in (loc) 00 1 00 1 00	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 27 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF			BRACING- TOP CHORD				rectly applied or 6-0-0	oc purlins,
BOT CHORD 2x4 SF WEBS 2x6 SF			BOT CHORD		t end verti ceiling dire		or 10-0-0 oc bracing.	

OTHERS 2x4 SP No.2

REACTIONS. All bearings 6-0-0.

(lb) -Max Horz 2=105(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 6, 2, 7, 8 Max Grav All reactions 250 lb or less at joint(s) 6, 2, 7, 8

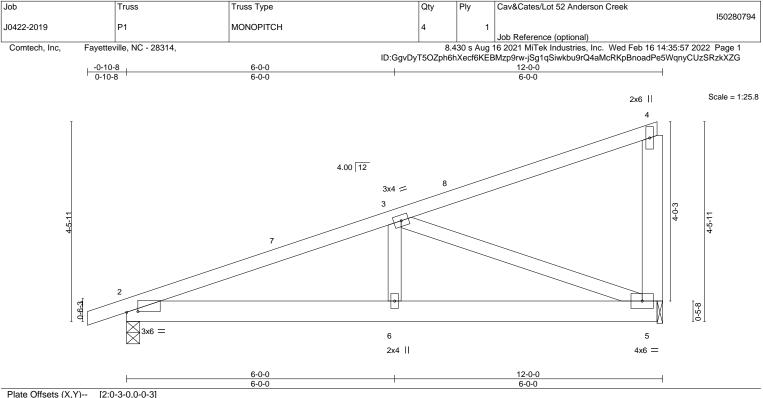
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 7, 8.







OADING         (psf)           CLL         20.0           CDL         10.0           CLL         0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.30 BC 0.18 WB 0.47		in -0.02 -0.04 0.01	(loc) 2-6 2-6 5	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.01	2-6	>999	240	Weight: 68 lb	FT = 20%
UMBER- OP CHORD 2x4 SI 30T CHORD 2x6 SI			BRACING- TOP CHORI			iral wood end verti	0	irectly applied or 6-0-0	) oc purlins,

BOT CHORD

Rigid ceiling directly applied or 8-5-11 oc bracing.

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2 \*Except\*

 4-5: 2x6 SP No.1

REACTIONS. (size) 2=0-3-8, 5=0-1-8 Max Horz 2=140(LC 8) Max Uplift 2=-195(LC 8), 5=-201(LC 8) Max Grav 2=529(LC 1), 5=462(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-839/715

 TOP CHORD
 2-3=-839/715

 BOT CHORD
 2-6=-808/736, 5-6=-808/736

WEBS 3-6=-336/267, 3-5=-755/825

## NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 11-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.

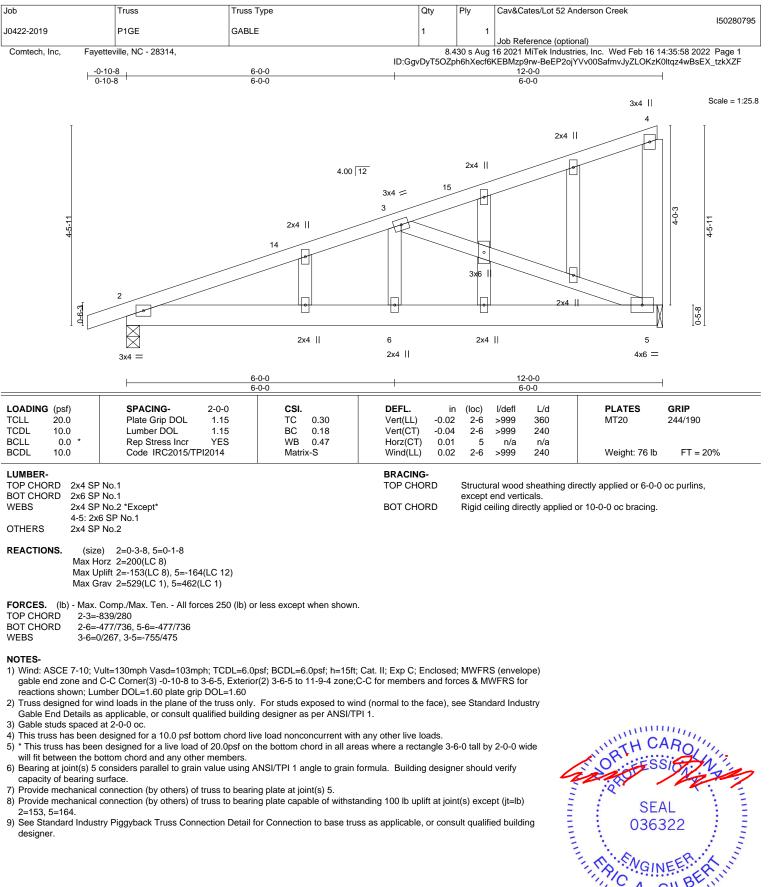
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=195, 5=201.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



TRENGINEERING BY A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

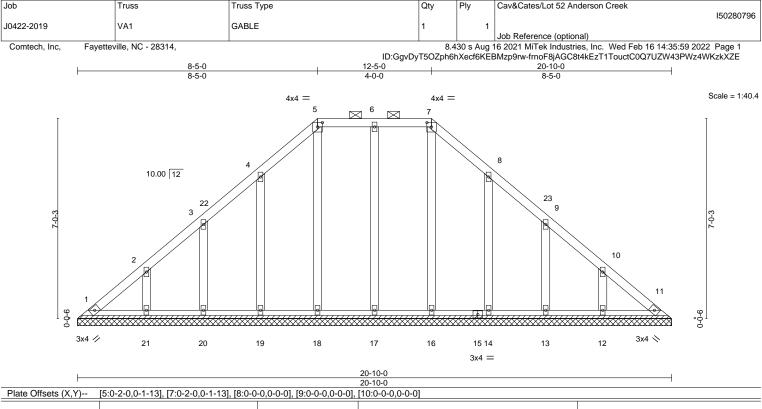


9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

February 17,2022

818 Soundside Road Edenton, NC 27932

G 40000



LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.05 BC 0.03 WB 0.11 Matrix-S	DEFL. i Vert(LL) n/ Vert(CT) n/ Horz(CT) 0.0	a -	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 125 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP OTHERS 2x4 SP	' No.1		BRACING- TOP CHORD BOT CHORD	2-0-0 0	oc purlins	(6-0-0 max.)	irectly applied or 6-0-0 o ): 5-7. or 10-0-0 oc bracing.	oc purlins, except

REACTIONS. All bearings 20-10-0.

(lb) - Max Horz 1=-201(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 11, 17, 18 except 19=-119(LC 12), 20=-107(LC 12), 21=-126(LC 12), 14=-118(LC 13), 13=-107(LC 13), 12=-125(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 11, 17, 18, 19, 20, 21, 16, 14, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 8-5-0, Exterior(2) 8-5-0 to 18-5-0, Interior(1) 18-5-0 to 20-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

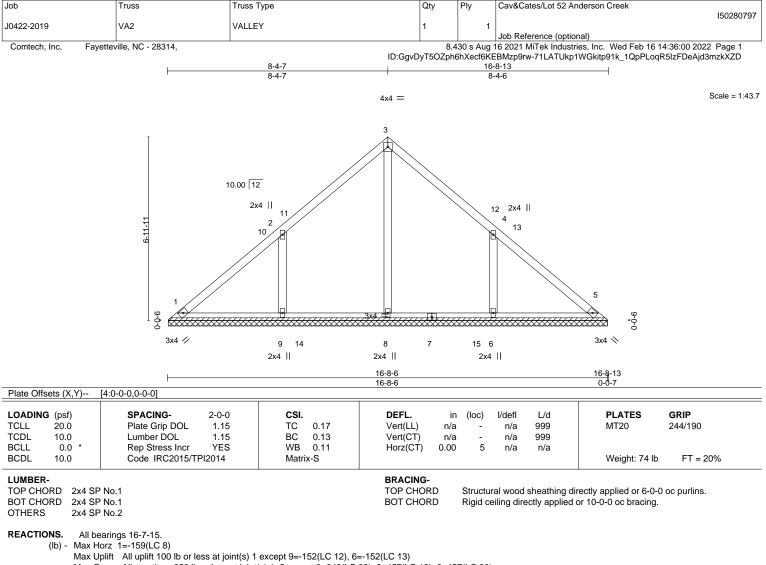
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11, 17, 18 except (jt=lb) 19=119, 20=107, 21=126, 14=118, 13=107, 12=125.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=329(LC 22), 9=457(LC 19), 6=457(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-9=-379/265, 4-6=-379/265

## NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 8-4-7, Exterior(2) 8-4-7 to 12-9-3, Interior(1) 12-9-3 to 16-4-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

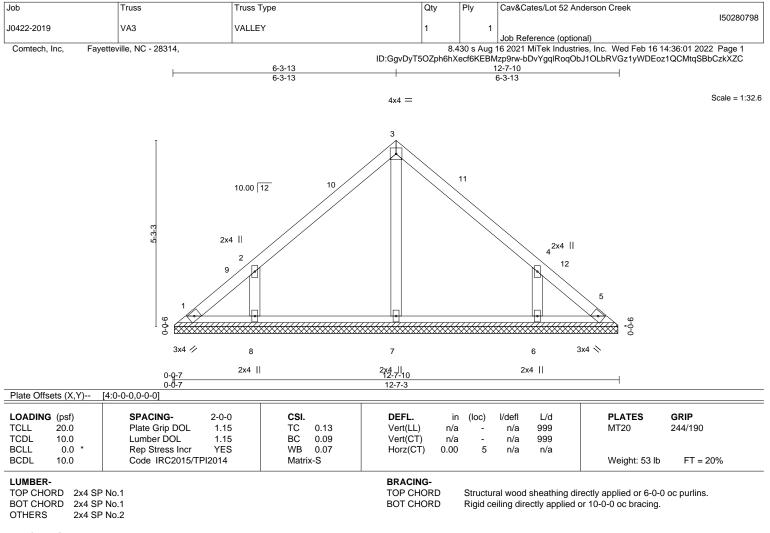
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=152, 6=152.





<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



**REACTIONS.** All bearings 12-6-11.

(lb) - Max Horz 1=-118(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=327(LC 19), 6=327(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-312/240, 4-6=-312/240

#### NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-3-13, Exterior(2) 6-3-13 to 10-8-10, Interior(1) 10-8-10 to 12-2-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

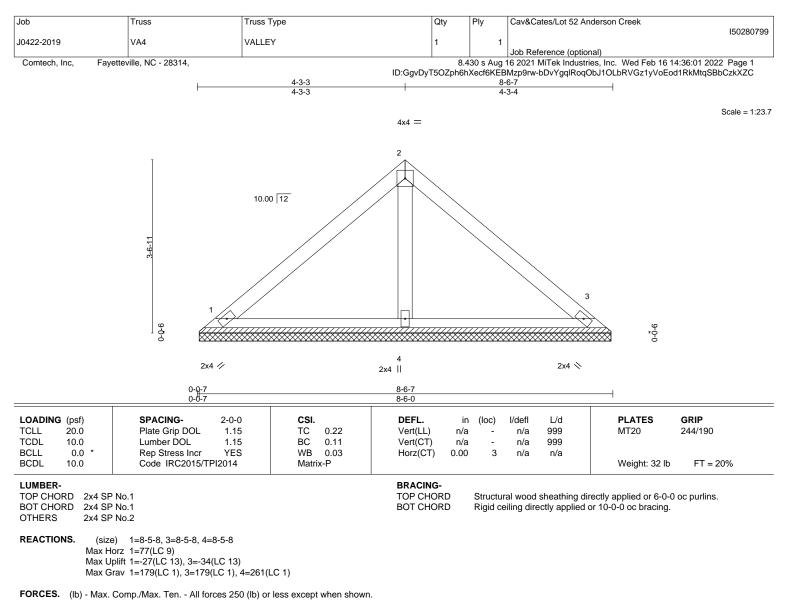
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.





<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

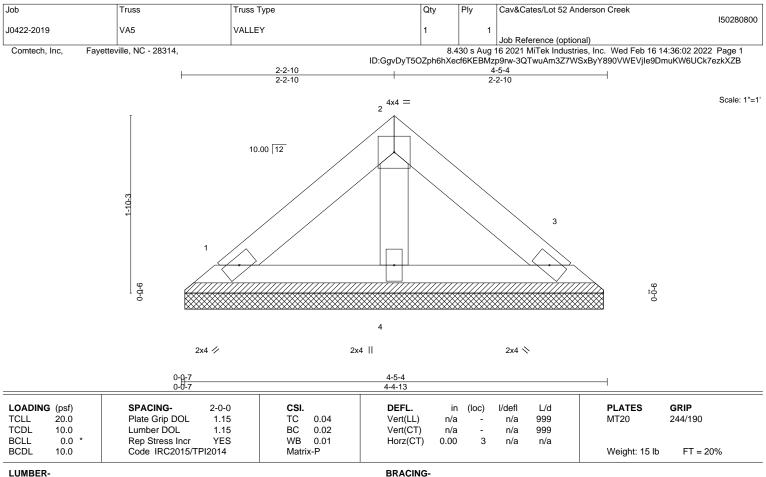
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.







TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=4-4-5, 3=4-4-5, 4=4-4-5

Max Horz 1=-36(LC 8)

Max Uplift 1=-13(LC 13), 3=-16(LC 13)

Max Grav 1=84(LC 1), 3=84(LC 1), 4=123(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



Structural wood sheathing directly applied or 4-5-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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