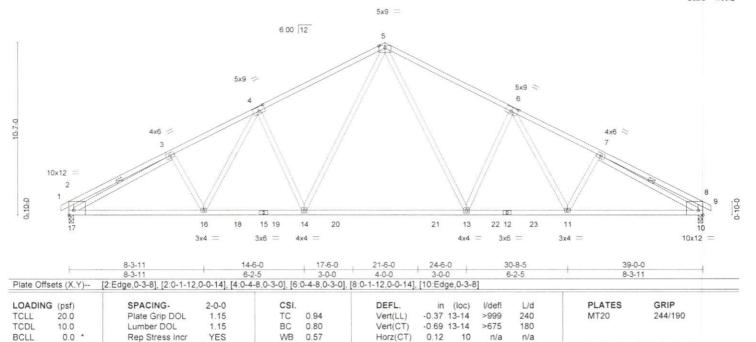
J	ob	Truss	Truss Type		Qty	Ply	Cali NP Vault Master		
1	8030341-MASTER	T1	COMMON		13	1			139464279
		1					Job Reference (optional)		
	84 Components (Dunn),	Dunn, NC - 28334,			8.	320 s Nov	19 2019 MiTek Industries, Inc. Mor	n Dec 2 14:54:39 201	19 Page 1
				ID:20kN0	Rrr?KSPF	RIXgb91IA	Py8hTY-IsOVCqv2Lc9YnyXJVBEzo	IVU6wDV1WhqCpbm	ONyCzXk
	-0-6-0	6-4-8	3-9	19-6-0	27	-3-7	32-7-8	39-0-0	39 ₆ -0
	0-6-0	6-4-8 5-4	-1	7-9-7	7.	9-7	5-4-1	6-4-8	0.6-0

Scale = 1:68.2



BRACING-TOP CHORD BOT CHORD

WEBS

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2

10.0

BOT CHORD 2x4 SP No.2 *Except*

12-15: 2x4 SP No.1

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 17=1587/0-3-8, 10=1587/0-3-8

Max Horz 17=126(LC 12)

Max Uplift 17=-95(LC 12), 10=-95(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

Code IRC2015/TPI2014

TOP CHORD 2-3=-820/199, 3-4=-2450/468, 4-5=-2174/485, 5-6=-2174/485, 6-7=-2450/468,

7-8=-820/199, 2-17=-575/185, 8-10=-575/185

16-17=-307/2189, 14-16=-219/2047, 13-14=-70/1508, 11-13=-219/2047, 10-11=-306/2189 **BOT CHORD**

5-13=-125/842, 6-13=-563/234, 6-11=-43/284, 5-14=-125/842, 4-14=-563/234, WEBS

4-16=-43/284, 3-17=-1829/257, 7-10=-1829/257

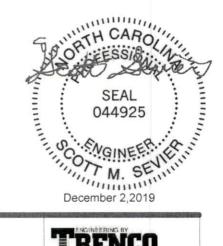
NOTES-

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

Matrix-AS

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 10.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Weight: 231 lb

Structural wood sheathing directly applied, except end verticals.

3-17, 7-10

Rigid ceiling directly applied.

1 Row at midpt

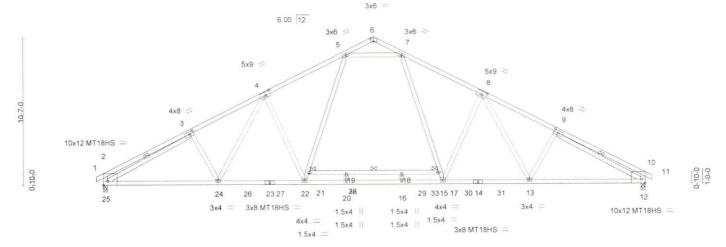
FT = 20%

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE Mil-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTER® connectors. This design is based only upon parameters have reported to the connectors. This design is based only upon parameters and property incorporate this design individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Eracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent uccliapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITH1 Quality Criteria, DSB-89 and BCSI Building Component Safety information, available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Cali NP Vault Master Truss Type Qty Job Truss 139464280 18030341-MASTER T1A **ROOF TRUSS** Job Reference (optional) Dunn NC - 28334 8.320 s Nov 19 2019 MiTek Industries, Inc. Mon Dec 2 14:54:41 2019 Page 1 84 Components (Dunn) ID.20kNGRrr?KSPRIXgb91IA?y8hTY-FFWGdWwJtEPG0FhiccHRtAbpBkrBVMS7f74tSGyCzXi 0-6-0 11-8-9 17-6-0 19-6-0 21-6-0 2-0-0 2-0-0 32-7-8 6-4-8

Scale = 1:79.4



		8-3-11		6-2-5	3-0-0	4-0-0	3-0-0		6-2-5		8-3-11	
Plate Offs	ets (X,Y)	[2:Edge,0-3-8], [2:0-1-12	,0-0-14], [3:0-	3-8,0-2-0], [4:	0-4-8,0-3-0], [6	6:0-3-0,Edge], [8	:0-4-8,0-	-3-0], [9	:0-3-8,0-	2-0], [10:0-1-	12,0-0-14], [12:Edge,0-	3-8]
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.52	16-20	>886	240	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.98	Vert(CT)	-1.06	16-20	>436	180	MT18HS	244/190
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.81	Horz(CT)	0.14	12	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matrix	c-AS						Weight: 247 lb	FT = 20%

17-6-0 21-6-0 24-6-0

BRACING-

WEBS

TOP CHORD

BOT CHORD

30-8-5

6-0-0 oc bracing: 17-21

1 Row at midpt

Rigid ceiling directly applied. Except:

Structural wood sheathing directly applied, except end verticals.

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

17-21: 2x4 SP No.1, 14-23: 2x4 SP DSS

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 25=1758/0-3-8, 12=1758/0-3-8 Max Horz 25=126(LC 16)

Max Grav 25=1835(LC 2), 12=1835(LC 2)

8-3-11

6-4-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

2-3=-866/142, 3-4=-2994/219, 4-5=-2783/176, 7-8=-2783/176, 8-9=-2994/219, TOP CHORD

9-10=-866/142, 2-25=-594/155, 10-12=-594/155

24-25=-95/2652, 22-24=0/2568, 20-22=0/2172, 16-20=0/2172, 15-16=0/2172, **BOT CHORD** 13-15=0/2568, 12-13=-93/2652

7-17=0/1064, 15-17=-12/908, 8-15=-552/249, 8-13=-107/257, 21-22=-12/908 5-21=0/1064, 4-22=-552/249, 4-24=-107/257, 3-25=-2285/66, 9-12=-2285/66,

5-7=-2090/264

NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) 150.0lb AC unit load placed on the bottom chord, 19-6-0 from left end, supported at two points, 4-0-0 apart.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

14-6-0

- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters as hown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see Austral 1817. Alexandria, VA 22314.

Safety information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Scale = 1:71.1 5x9 = 6 00 12 5 5x9 5x9 < 4x6 4x6 10x12 = 2 0-10-D 0-10-0 図10 16 40 15 41 43 44 45 1346 3x4 10x12 3x4 3x6 4x4 4x4 3x8 MT18HS = 17-6-0 14-6-0 8-3-11 8-3-11

Plate Offsets (X,Y)-	[2:Edge,0-3-8], [2:0-1-12	2,0-0-14], [4:0-4	-8,0-3-0], [5:	0-2-0,0-0-0],	[6:0-4-8,0-3-0], [8	:0-1-12	0-0-14],	[10:Edg	e,0-3-8]		
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.99	Vert(LL)	-0.44	13-14	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.99	Vert(CT)	-0.83	13-14	>559	180	MT18HS	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.63	Horz(CT)	0.13	10	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matri	x-AS						Weight: 327 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

12-15: 2x4 SP No.1

WEBS 2x4 SP No 3

2x4 SP No.3 **OTHERS**

(lb/size) 17=1677/0-3-8, 10=1719/0-3-8 REACTIONS.

Max Horz 17=126(LC 39)

Max Uplift 17=-141(LC 12), 10=-162(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-851/215, 3-4=-2610/554, 4-5=-2338/583, 5-6=-2416/630, 6-7=-2690/595, TOP CHORD

7-8=-866/222, 2-17=-590/193, 8-10=-597/196

16-17=-382/2335, 14-16=-304/2188, 13-14=-152/1645, 11-13=-343/2255 **BOT CHORD**

10-11=-415/2404

5-13=-233/1023, 6-13=-538/250, 6-11=-72/255, 5-14=-138/864, 4-14=-546/245, WEBS

4-16=-62/262, 3-17=-1947/328, 7-10=-2003/361

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be User Defined crushing capacity of 425 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 17=141, 10=162.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 74 lb down and 45 lb up at 21-3-4, and 74 lb down and 45 lb up at 23-2-12, and 74 lb down and 45 lb up at 25-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

Continued on page 2 LOAD CASE(S) Standard

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314



Structural wood sheathing directly applied, except end verticals.

3-17, 7-10

Rigid ceiling directly applied.

1 Row at midpt

Job	Truss	Truss Type	Qty	Ply	Cali NP Vault Master
18030341-MASTER	T1GE	GABLE	1	1	Job Reference (optional)

84 Components (Dunn),

Dunn, NC - 28334,

8.320 s Nov 19 2019 MiTek Industries, Inc. Mon Dec 2 14:54:43 2019 Page 2 ID:2okNGRrr?KSPRIXgb91IA?y8hTY-Bee02CyZOrf_FZr4k1Jvybg9RXXTzJqQ7RZzX8yCzXg

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-2=-60, 2-5=-60, 5-8=-60, 8-9=-60, 10-17=-20

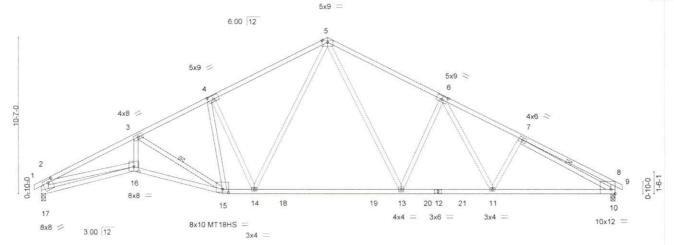
Concentrated Loads (lb)

Vert: 43=-74(F) 45=-74(F) 46=-74(F)



Job	Truss		Truss Type		Qty	Ply	Cali NP Vault Master			1000
18030341-MASTER	T1V		COMMON		8	1				139464282
							Job Reference (option	nal)		
84 Components (Dunn),	Dunn, NC - 2	8334,				8.320 s Nov	19 2019 MiTek Industr	ries, Inc. Mon Dec	2 14:54:44 2019	Page 1
					ID:2okNC	Rrr?KSPRIX	(gb91IA?y8hTY-fqCOF	YzB99nrtjPHlkq8Vp	DKwxt2ikYaL5IX	3byCzXf
-0 ₇ 6 ₇ 0	6-3-12	11-8-9	12-4-0	19-6-0	27-3-7		32-7-8	39-0-0	39-6-0	8
0-6-0	6-3-12	5-4-13	0-7-7	7-2-0	7-9-7		5-4-1	6-4-8	0-6-0	

Scale = 1:75.3



	1	6-3-12	12-4-0	14-6-0	24-6-0)		30-8-5		39-0-0	_
		6-3-12	6-0-4	2-2-0	10-0-0)		6-2-5		8-3-11	
Plate Offse	ets (X,Y)	[4:0-4-8,0-3-0], [6:0-4-8,0	0-3-0], [8:0-1-	12,0-0-14], [10	D:Edge,0-3-8], [15:0	-5-0,0-2-4], [16:0	4-0,0-3-	10], [17:0-	2-12,0-0-10]	, [17:0-3-8,0-2-4]	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	n (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.94	Vert(LL) -0.4	13-14	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.97	Vert(CT) -0.7	7 13-14	>604	180	MT18HS	244/190
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(CT) 0.2	2 10	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-AS					Weight: 238 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

Rigid ceiling directly applied.

1 Row at midpt

Structural wood sheathing directly applied, except end verticals.

3-15, 7-10

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

12-15,10-12: 2x4 SP No.1

WEBS 2x4 SP No.3 *Except*

2-17: 2x6 SP No.2, 2-16: 2x4 SP No.2

REACTIONS. (lb/size) 17=1589/0-3-8, 10=1584/0-3-8

Max Horz 17=-125(LC 17)

Max Uplift 17=-96(LC 12), 10=-95(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD $2\text{-}3\text{=-}4164/623,\ 3\text{-}4\text{=-}2254/441,\ 4\text{-}5\text{=-}2101/482,\ 5\text{-}6\text{=-}2123/484,\ 6\text{-}7\text{=-}2433/467,}$

7-8=-825/201, 2-17=-1680/339, 8-10=-577/186

16-17=-243/746, 15-16=-484/3645, 14-15=-219/2001, 13-14=-68/1464, 11-13=-218/2003, **BOT CHORD**

10-11=-305/2184

3-16=-120/1568, 3-15=-1828/293, 4-14=-561/239, 5-14=-121/785, 5-13=-127/840, WEBS

6-13=-563/233, 6-11=-42/285, 2-16=-309/2952, 7-10=-1783/254

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 10.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



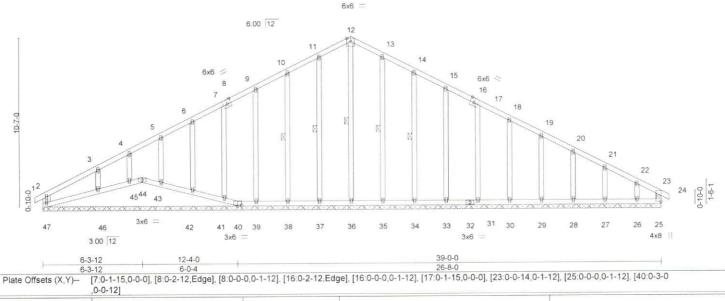
🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road Edenton, NC 27932

Qty Cali NP Vault Master Truss Type Job Truss 139464283 18030341-MASTER T1VGE GABLE Job Reference (optional) 8.320 s Nov 19 2019 MiTek Industries, Inc. Mon Dec 2 14:54:45 2019 Page 1 Dunn NC - 28334 84 Components (Dunn), ID.2okNGRrr?KSPRIXgb91IA?y8hTY-70mmTuzpwSviVt_TrSLN10liiLRDRLBjal24b1yCzXe 39-0-0 0-6-0 19-6-0 7-9-7 0-6-0 11-8-9

Scale = 1:69.6



LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.		(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	-0.00	23	n/r	120	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	0.00	23	n/r	90		
CLL	0.0 *	Rep Stress Incr	YES	WB	0.12	Horz(CT)	0.01	25	n/a	n/a	TWO IN THE CONTRACTOR	
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-R	10000-000					Weight: 269 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS

23-25: 2x4 SP No.3

2x4 SP No.3 **OTHERS**

2x6 SP No.2 *Except*

BRACING-

BOT CHORD

WEBS

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

12-36, 11-37, 10-38, 13-35, 14-34

REACTIONS. All bearings 39-0-0

(lb) - Max Horz 47=-125(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 47, 44, 40, 25, 37, 38, 39, 41, 42, 43, 45, 35, 34, 33, 31, 30, 29, 28, 27, 26 except 46=-102(LC 12) Max Grav All reactions 250 lb or less at joint(s) 47, 44, 40, 25, 36, 37, 38, 39,

41, 42, 43, 45, 46, 35, 34, 33, 31, 30, 29, 28, 27, 26

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 10-11=-120/273, 11-12=-133/309, 12-13=-133/309, 13-14=-120/273

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 47, 44, 40, 25, 37, 38, 39, 41, 42, 43, 45, 35, 34, 33, 31, 30, 29, 28, 27, 26 except (jt=lb) 46=102.
- 12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 44, 41, 42, 43, 45, 46.

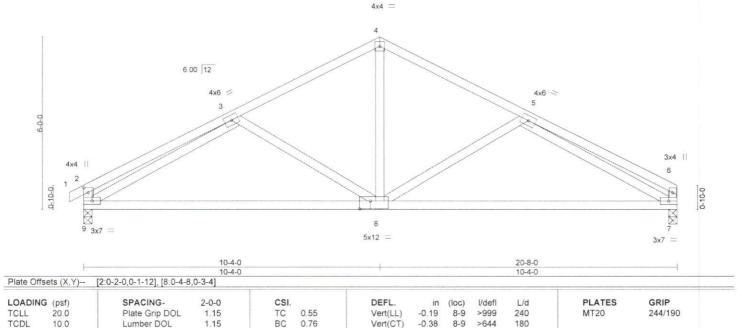


🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see AISI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply Cali NP Vault Master 139464284 18030341-MASTER T2 Common Job Reference (optional) 84 Components (Dunn) Dunn, NC - 28334 8.320 s Nov 19 2019 MiTek Industries, Inc. Mon Dec 2 14:54:46 2019 Page 1 ID 2okNGRrr?KSPRIXgb91IA?y8hTY-bDJ9gE_Rhm1Y61ZfP9scaEImXlcsAg?spPne7TyCzXd 15-4-4 10-4-0 20-8-0 5-0-4 5-0-4 5-3-12

Scale = 1:38.6



LUMBER-

BCLL

BCDL

TOP CHORD 2x4 SP No.2

0.0

10.0

BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.3 BRACING-

Horz(CT)

0.03

TOP CHORD

Structural wood sheathing directly applied or 5-7-13 oc purlins,

Weight: 110 lb

FT = 20%

except end verticals.

n/a

n/a

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. (lb/size) 9=854/0-3-8, 7=814/0-3-8

Max Horz 9=76(LC 9)

Max Uplift 9=-54(LC 12), 7=-44(LC 13)

Rep Stress Incr

Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-444/92, 3-4=-944/187, 4-5=-945/187, 5-6=-377/50, 2-9=-356/116, 6-7=-276/69

YES

WB 0.60

Matrix-S

BOT CHORD 8-9=-175/996, 7-8=-175/1002

4-8=-43/530, 5-8=-290/179, 3-8=-284/177, 3-9=-775/175, 5-7=-850/218 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 7.

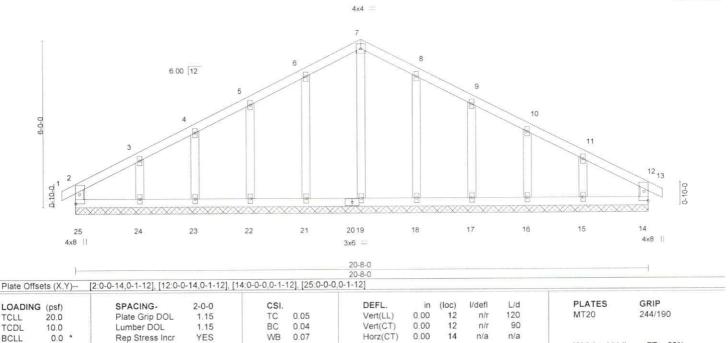


🛦 WARNING - Verify dosign parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	Cali NP Vault Master	139464285
18030341-MASTER	T2GE	Common	1	1		100404200
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,				19 2019 MiTek Industries, Inc. Mon 11A?y8hTY-3PtXuZ?4S49PkA8sztOr7	
-0-6-0		10-4-0	100000000000000000000000000000000000000		20-8-0	21-2-0
0-6-0		10-4-0	Walland World and the Control of the		10-4-0	0-6-0

Scale = 1:39.8



LUMBER-

TCLL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

10.0

WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Weight: 111 lb

except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD**

REACTIONS. All bearings 20-8-0

(lb) - Max Horz 25=75(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 25, 14, 21, 22, 23, 24, 18, 17, 16, 15 Max Grav All reactions 250 lb or less at joint(s) 25, 14, 19, 21, 22, 23, 24, 18, 17, 16, 15

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

NOTES-

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

Matrix-R

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 14, 21, 22, 23, 24, 18, 17, 16, 15.



FT = 20%

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property design. Property and personnent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road Edenton, NC 27932

Qty Cali NP Vault Master Job Truss Type Truss 139464286 Common 18030341-MASTER ТЗ Job Reference (optional) 8.320 s Nov 19 2019 MiTek Industries, Inc. Mon Dec 2 14:54:48 2019 Page 1 84 Components (Dunn) Dunn NC - 28334 ID:20kNGRrr?KSPRIXgb91IA?y8hTY-XbRv5v0iDNHGMKj2Xav4ffN20YN?eiF9GjGkCMyCzXb 13-4-0 6-8-0 6-8-0 Scale = 1:26.8 4x6 = 6.00 12 6 8x8 13-4-0 6-8-0 [5:Edge,0-5-13], [5:0-1-12,0-0-0], [7:0-1-12,0-0-0], [7:Edge,0-5-13] Plate Offsets (X,Y)--DEFL. PLATES GRIP in (loc) I/defl L/d LOADING (psf) SPACING-2-0-0 CSI 240 MT20 244/190 0.80 -0.04 6-7 >999 TCLL 20.0 Plate Grip DOL 1.15 TC Vert(LL) 180 BC -0.08 6-7 >999 TCDL 10.0 Lumber DOL 1 15 0.40 Vert(CT) 0.0 * BCLL Rep Stress Incr YES WB 0.10 Horz(CT) 0.01 5 n/a n/a FT = 20% Weight: 68 lb Code IRC2015/TPI2014 BCDL 10.0 Matrix-S BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD**

2x4 SP No.3 WEBS

REACTIONS. (lb/size) 7=561/0-3-8, 5=521/0-3-8 Max Horz 7=57(LC 11)

Max Uplift 7=-38(LC 12), 5=-28(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-646/135, 3-4=-641/132, 2-7=-504/165, 4-5=-463/135 TOP CHORD

6-7=-192/415, 5-6=-103/291 BOT CHORD

3-6=0/262 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

17) Orbital for the data have been contained for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed: C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5.



Structural wood sheathing directly applied or 5-6-4 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing

except end verticals.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for usery resign parameters and READ NOTES UNITHS AND INCLUDED MITER REFERENCE PAGE MIN-473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property manage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

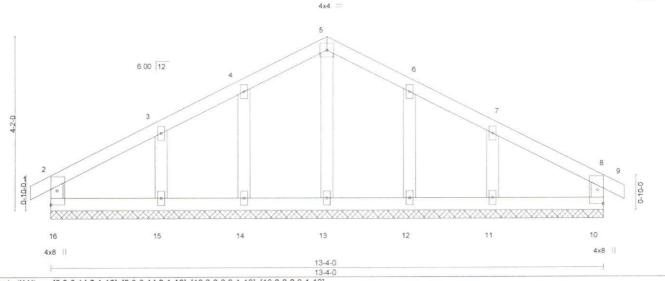
ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Cali NP Vault Master	
		0.00				139464287
18030341-MASTER	T3GE	Common	1	1		
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8	320 s Nov	19 2019 MiTek Industries, Inc. Mon Dec 2	14:54:49 2019 Page 1
		ID:2	kNGRrr?KS	PRIXgb911	A?y8hTY-0o?HJF1K_hP7zUIE4HQJCswPC	DypgNAfJVN0lkoyCzXa
₇ 0-6-0		6-8-0			13-4-0	13-10-0
0-6-0		6-8-0			6-8-0	0-6-0

Scale = 1.26.6



LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	0.00	8	n/r	120	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	0.00	9	n/r	90		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	10	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-R						Weight: 63 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

OTHERS 2x4 SP No.3 BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-4-0

(lb) - Max Horz 16=56(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11 Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MIT eNB connectors. This design is based only upon parameters had reference PAGE MIT-4/3 rev. 1002/2018 BEFORE USE. Design valid for use only with MIT eNB connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

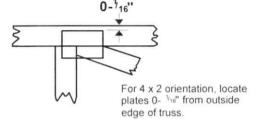


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.





* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

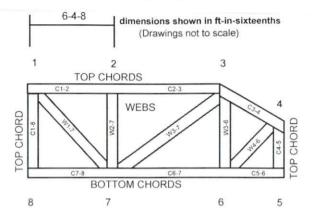
DSB-89: Design Standard for Bracing.

BCSI:

Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

of Metal Plate
MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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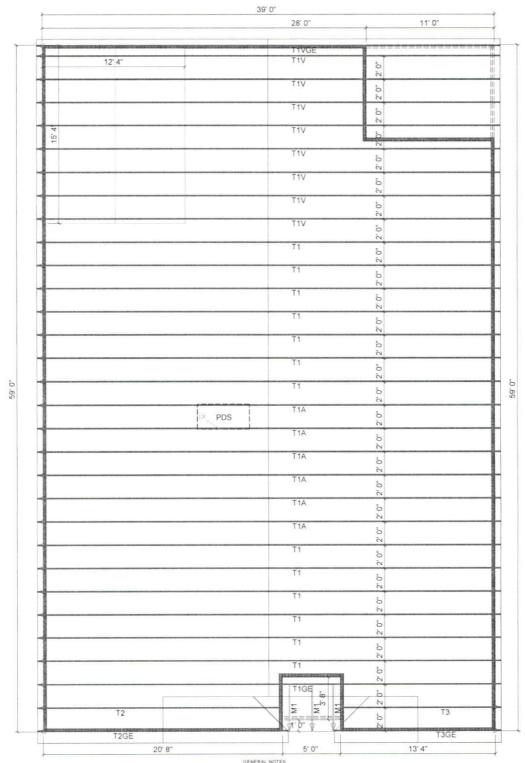
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

001397

MASTER



DO NOT CUT OR MODIFY TRUSSES

TRUSSES ARE SPACED 24" ON CENTER UNLESS NOTED OTHERWISE

△ - INDICATES LEFT END OF TRUSS.

REFER TO THE INDIVIDUAL TRUSS DESIGN DRAWINGS FOR THE LOCATION OF LATERAL BRACING AND MULTI-PLY AND/OR SCAB CONNECTION REQUIREMENTS.

PER ANSI TRI 1-2002 THE TRUSS ENGINEER IS RESPONSIBLE FOR TRUSS TO TRUSS CONNECTIONS AND TRUSS PLY TO PLY CONNECTIONS. THIS TRUSS PLACEMENT PLAN REGCOMENDS TRUSS TO BEARING CONNECTIONS AND TRUSS TO BEAR CONNECTIONS WHICH SHALL BE REVIEWED BY THE SUILDING DESIGNER. IT IS THE RESPONSIBILITY OF THE SUILDING DESIGNER TO RESOLVE ALL ROOF PORCES ADEQUATE TO THE FOUNDATION.

A	
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В	
C	
D	
80	H2.5A



DEDICATED TO QUALITY & EXCELLENCE 420 DIXON DAIRY ROAD KINGS MOUNTAIN, NORTH CAROLINA PHONE: 704.937.3712 FAX: 877.807.7983

Cali - NP Vault MBR (MASTER) EXPRESS HOMES

NP Vault MBR (MASTER)

		/	,
Scale:	NOT TO SCALE	P.O. Number:	Order:
Quote by:	Jason Ledbetter		MASTER
Design by		Ship Date:	Project
Salespers	° Terry Gonya		001397