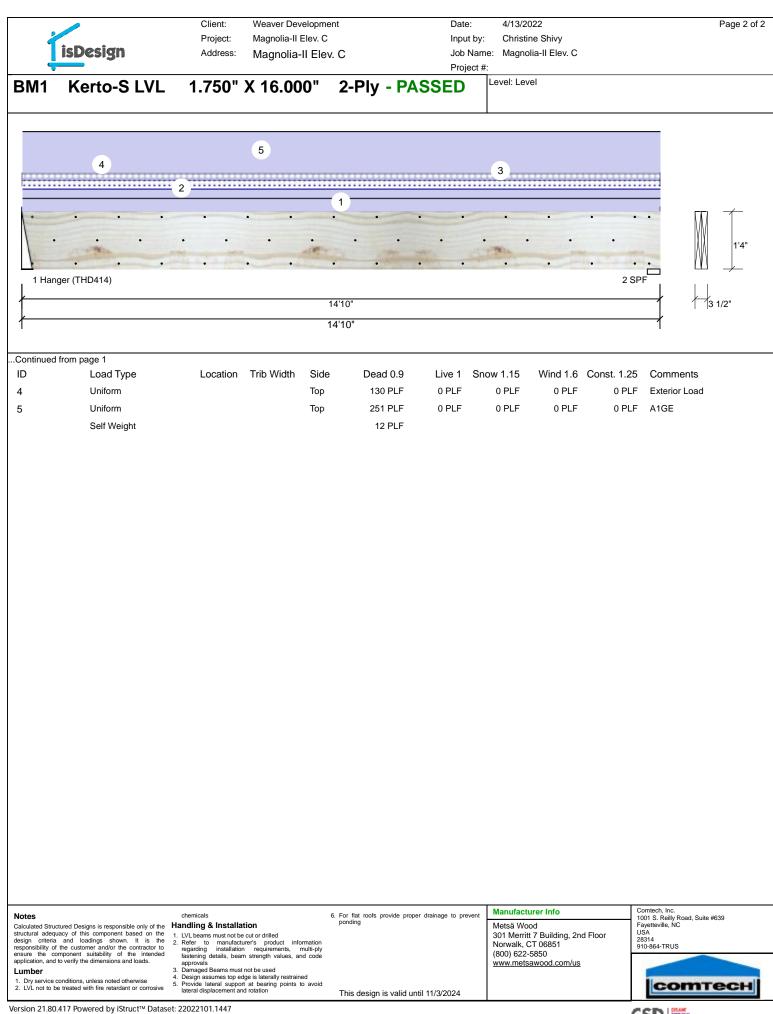


Version 21.80.417 Powered by iStruct™ Dataset: 22022101.1447

CSD 🛤



isDesign		Weaver Development		Date:	4/13/2022	2		Page 1 of
ic Decian	Project:	Magnolia-II Elev. C		Input by	: Christine	Shivy		Ū.
Ispesizii	Address:	Magnolia-II Elev. C	;	Job Nar	•	II Elev. C		
		<u> </u>		Project	#: Level: Level			
BM2 Kerto-S LV	L 1.750"	X 16.000" 2	2-Ply - P	ASSED	Level. Level			
2								
		1						
• • •			•	•	• •		ſ	$\pi$
								WM I
ALL CARE		18.77	17	-	1. Sec. 1.			1'4"
		•	•	•	· ·		l	
1 SPF				2 S				, ,
		10'9 1/2"					1	3 1/2"
ſ		10'9 1/2"			1			
Vember Information	<del>_</del>			-	NPATTERN	ED lb (Uplift)		
Type: Girder Plies: 2		ation: Floor n Method: ASD		Brg Direction	Live	Dead		ind Con
Moisture Condition: Dry	-	n Method: ASD ng Code: IBC/IRC 20	)15	1 Vertical 2 Vertical	3389 3389	1200 1200	0 0	0 0
Deflection LL: 480		Sharing: No		2 vortiour	0000	1200	0	0
Deflection TL: 360	Deck:	Not Checke	ed					
Importance: Normal - II Temperature: Temp <= 100°	F							
				Bearings				
				Bearing Leng	-	Cap. React D/L lb		
				1 - SPF 3.500 2 - SPF 3.500		88% 1200/3389 88% 1200/3389		D+L D+L
Analysis Results				2-377 3.300	y ven	12007 3303	4303 L	DTL
•	Location Allowed	Capacity Comb.						
		0.330 (33%) D+L	L					
Moment 11397 ft-lb	5'4 3/4" 34565 ft-lb	( )	1					
Unbraced 11397 ft-lb	5'4 3/4" 34565 ft-lb 5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb	0.969 (97%) D+L	L					
Unbraced 11397 ft-lb	5'4 3/4" 11764 ft-lb	0.969 (97%) D+L 0.363 (36%) D+L						
Unbraced 11397 ft-lb Shear 4341 lb	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4	0.969 (97%) D+L 0.363 (36%) D+L	L					
Unbraced         11397 ft-lb           Shear         4341 lb           LL Defl inch         0.085 (L/1457)           TL Defl inch         0.115 (L/1076)	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L	L L L					
Unbraced         11397 ft-lb           Shear         4341 lb           LL Defl inch         0.085 (L/1457)	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotai	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L	L L L	-				
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotat rior bearings by the bu	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L iin at the end bearings. La	L L L ateral support					
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of 4 to exceed 6".	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotation for bearings by the bu 10d Box nails (.128x3) s for fasteners require	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La iilding code. ') at 12" o.c. Maximum end d for specified loads.	L L L ateral support	-				
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of 1 to exceed 6". 3 Refer to last page of calculations 4 Girders are designed to be supp	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotati rior bearings by the bu 10d Box nails (.128x3' s for fasteners require ported on the bottom e	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La iilding code. ') at 12" o.c. Maximum end d for specified loads.	L L L ateral support					
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of 4 to exceed 6". 3 Refer to last page of calculations 4 Girders are designed to be supp 5 Top must be laterally braced at 6 6 Bottom must be laterally braced	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotai rior bearings by the bu 10d Box nails (.128x3' s for fasteners require borted on the bottom e end bearings. at end bearings.	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La iilding code. ') at 12" o.c. Maximum end d for specified loads.	L L L ateral support					
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of 4 to exceed 6". 3 Refer to last page of calculations 4 Girders are designed to be supp 5 Top must be laterally braced at 6 6 Bottom must be laterally braced 7 Lateral slenderness ratio based	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotation for bearings by the bu 10d Box nails (.128x3) s for fasteners require ported on the bottom e end bearings. at end bearings. on single ply width.	<ul> <li>0.969 (97%) D+L</li> <li>0.363 (36%) D+L</li> <li>80) 0.329 (33%) L</li> <li>60) 0.335 (33%) D+L</li> <li>iion at the end bearings. La iiding code.</li> <li>i') at 12" o.c. Maximum end</li> <li>d for specified loads.</li> <li>dge only.</li> </ul>	L L ateral support	Live 1 Sr	now 1 15	Nind 1.6 Const 1	25 Comments	3
Unbraced 11397 ft-lb Shear 4341 lb LL Defl inch 0.085 (L/1457) TL Defl inch 0.115 (L/1076) Design Notes 1 Provide support to prevent latera may also be required at the inter 2 Fasten all plies using 3 rows of 4 to exceed 6". 3 Refer to last page of calculations 4 Girders are designed to be supp 5 Top must be laterally braced at e 6 Bottom must be laterally braced	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotai rior bearings by the bu 10d Box nails (.128x3' s for fasteners require borted on the bottom e end bearings. at end bearings.	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La iilding code. ') at 12" o.c. Maximum end d for specified loads.	L L ateral support d distance not	Live 1 Sr 267 PLF	now 1.15 \ 0 PLF	Wind 1.6 Const. 1 0 PLF 0	.25 Comments PLF F4	3
Unbraced       11397 ft-lb         Shear       4341 lb         LL Defl inch       0.085 (L/1457)         TL Defl inch       0.115 (L/1076)         Design Notes         1 Provide support to prevent latera         may also be required at the inter         2 Fasten all plies using 3 rows of to exceed 6".         3 Refer to last page of calculations         4 Girders are designed to be support to prevent based of Lateral slenderness ratio based         Top must be laterally braced at each of Lateral slenderness ratio based         ID       Load Type	5'4 3/4" 11764 ft-lb 1'7 1/2" 11947 lb 5'4 3/4" 0.259 (L/4 5'4 3/4" 0.345 (L/3 al movement and rotation for bearings by the bu 10d Box nails (.128x3) s for fasteners require ported on the bottom e end bearings. at end bearings. on single ply width.	0.969 (97%) D+L 0.363 (36%) D+L 80) 0.329 (33%) L 60) 0.335 (33%) D+L ion at the end bearings. La iiding code. ') at 12" o.c. Maximum end d for specified loads. dge only.	L L ateral support d distance not			0 PLF 0		5

Ti	sDesign	-	agnolia-II Elev. C Iagnolia-II Elev. C			Christine Shivy Magnolia-II Ele				
GDH	Kerto-S LVL	1.750" X	11.875" 2-	-Ply - P	Project #:	Level: Level				
•	nd Grain	2	1	đ	•	- -	2 SPF Enc	· · ·		11 7/8"
/			16'10'					$\longrightarrow$	1 1-	
	nformation	<b>A</b> 12 - 12			Reactions UN			0	1477	
Type: Plies: Moisture Con Deflection Ll Deflection TI mportance:		Application Design Me Building C Load Shar Deck:	ethod: ASD ode: IBC/IRC 201		Brg Direction 1 Vertical 2 Vertical	Live 0 0	Dead 2098 2098	Snow 337 337	Wind 0 0	Cor
emperature	:: Temp <= 100°F				Bearings					
					Bearing Lengt 1 - SPF 3.500" End	h Dir. Cap Vert 24%	React D/L lb 2098 / 337		Ld. Case L	Ld. Com D+S
nalysis R			Oran a site and a sate	0	Grain 2 - SPF 3.500"	Vert 24%	2098 / 337	2434	L	D+S
Analysis Moment Jnbraced Shear LL Defl inch	8354 ft-lb 9694 ft-lb 1788 lb 1 10.070 (L/2809) 8'5	8'5" 17919 ft-lb 8'5" 9704 ft-lb	. ,	Case Uniform L Uniform L	End Grain					
esign No				_	ſ					
1 Provide si may also 2 Fasten all to exceed 3 Refer to la 4 Girders au 5 Top loads 6 Top must 7 Bottom m	upport to prevent lateral m be required at the interior plies using 2 rows of 10d	bearings by the buildin Box nails (.128x3") at r fasteners required for ed on the bottom edge ly by all plies. aximum of 9'6 3/4" o.c. end bearings.	g code. 12" o.c. Maximum end c specified loads.							
D	Load Type	Location Tr	ib Width Side	Dead 0.9			1.6 Const. 1		mments	
2	Uniform Uniform		Тор Тор	200 PLF 40 PLF	0 PLF 0 PLF				erior Loads )" Gable End	
	Self Weight			9 PLF						
uctural adequacy sign criteria a sponsibility of the sure the comp plication, and to v umber	ed Designs is responsible only of the y of this component based on the nd loadings shown. It is the customer and/or the contractor to onent suitability of the intended erify the dimensions and loads. ditions, unless noted otherwise eated with fire retardant or corrosive	chemicals Handling & Installation 1. LVL beams must not be cut on 2. Refer to manufacturers regarding installation re fastening details, beam stre approvals 3. Damaged Beams must not be 4. Design assumes top edge is: 5. Provide lateral support at b	pond drilled product information quirements, multi-ply ngth values, and code used aterally restrained	flat roofs provide pi ing	oper drainage to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Buildin Norwalk, CT 06851 (800) 622-5850 www.metsawood.co	-	Fayettevil USA 28314 910-864-1	teilly Road, Suite # le, NC	