

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 243_2939_B KB Home 243.2939.B

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I44459039 thru I44459066

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



January 21,2021

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



		7-3-9	14-4-7				21-8-0				
		7-3-9	7-0-15		1		7-3-9	1			
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 11. TCDL BCLL BCDL	20.0 6/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.36 BC 0.60 WB 0.19 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc -0.08 6 -0.18 6 0.04	c) l/defl 8 >999 8 >999 6 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 99 lb	GRIP 197/144 FT = 20%		
LUMBER- TOP CHORD 2x BOT CHORD 2x WEBS 2x	4 SP No.2 or 3 4 SP No.2 or 3 4 SP No.3	2x4 SPF No.2 2x4 SPF No.2	BR TO BO	ACING- P CHORD T CHORD	Structural wo Rigid ceiling	od sheathir directly app	ng directly appl lied or 10-0-0 o	ied or 4-5-13 oc purl oc bracing.	ins.		
REACTIONS. M M M	(size) 2=0- lax Horz 2=-7 lax Uplift 2=-8 lax Grav 2=97	3-8, 6=0-3-8 5(LC 17) 4(LC 16), 6=-84(LC 17) 6(LC 2), 6=976(LC 2)									
FORCES. (lb) - M TOP CHORD 2 BOT CHORD 2 WEBS 2	FORCES. (Ib) - Max. Comp./Max. Ten All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-1579/99, 3-4=-1386/98, 4-5=-1386/98, 5-6=-1579/100 BOT CHORD 2-10=-97/1373, 8-10=0/949, 6-8=-33/1373 WEBS 4-8=-32/465, 5-8=-279/144, 4-10=-32/465, 3-10=-279/144										
NOTES- 1) Unbalanced roc 2) Wind: ASCE 7- gable end zone 23-6-8 zone; ca reactions showi 3) TCLL: ASCE 7- roof snow: Lum 4) Unbalanced smo 5) This truss has b non-concurrent 6) This truss has b 7) * This truss has b 7) * This truss has b 8) One H2.5A Sim connection is for	of live loads ha 10; Vult=120m and C-C Extent antilever left ar n; Lumber DO 10; Pr=20.0 p ber DOL=1.15 ow loads have been designed with other live been designed been designed	ave been considered for this design. ph Vasd=95mph; TCDL=6.0psf; BCDL= prior(2) -1-10-8 to 1-1-8, Interior(1) 1-1-8 nd right exposed ; end vertical left and right L=1.60 plate grip DOL=1.60 sf (roof live load: Lumber DOL=1.15 Platics is Plate DOL=1.15); Category II; Exp B; F been considered for this design. I for greater of min roof live load of 12.0 be loads. I for a 10.0 psf bottom chord live load no rod and any other members. Fie connectors recommended to connect and does not consider lateral forces.	=6.0psf; h=30ft; Cat. II; Ex to 10-10-0, Exterior(2) 10 ght exposed;C-C for memil te DOL=1.15); Pg=15.0 ps Partially Exp.; Ct=1.10 psf or 1.00 times flat roof I onconcurrent with any othe n chord in all areas where t truss to bearing walls du	o B; Enclosed; M -10-0 to 13-10-0 bers and forces & of (ground snow) oad of 11.6 psf o r live loads. a rectangle 3-6- e to UPLIFT at jt	IWFRS (enve , Interior(1) 1 & MWFRS fo ; Pf=11.6 psf on overhangs 0 tall by 2-0-1 (s) 2 and 6. 1	lope) 3-10-0 to (flat) wide his	South	TH CAR	Kier		



SEFORE USE. conent, not to the overall manent bracing the CSI Building Component 818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



(lb) -Max Horz 2=-75(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 21, 22, 23, 17, 16, 15, 14, 12

All reactions 250 lb or less at joint(s) 18, 20, 21, 22, 23, 17, 16, 15, 14 except 2=257(LC 2), Max Grav 12=257(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-10-8 to 1-1-8, Exterior(2) 1-1-8 to 10-10-0, Corner(3) 10-10-0 to 13-10-0, Exterior(2) 13-10-0 to 23-6-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 7) All plates are 1.5x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 11) will fit between the bottom chord and any other members.



818 Soundside Road

Edenton, NC 27932

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED WITHS KRETERENCE PAGE MIL-74/3 fev. or 19/2/2/2 DEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





10-10	-11	2	21-6-0		- 3	32-1-5			43-0-0	
10-10 Plate Offsets (X,Y) [2:0-3-6,	-11 ,Edge], [8:0-3-6,Edge		0-7-5 [13:0-4-0,0-4-	-8]	<u> </u>	10-7-5			10-10-11	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DC Lumber DOL Rep Stress Ir Code IRC20	2-0-0 DL 1.15 1.15 ccr YES 15/TPI2014	CSI. TC BC WB Matrix	0.87 0.99 0.95 <-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.26 12-13 -0.57 12-13 0.18 8	l/defl >999 >904 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 263 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3					BRACING- TOP CHORD BOT CHORD WEBS	Structural wood Rigid ceiling diru 1 Row at midpt	sheathin ectly appl	g directly a lied or 2-2-(7-12, 3-	applied or 2-2-0 oc purlins. 0 oc bracing. -12	
REACTIONS. (size) 2=0 Max Horz 2=1 Max Uplift 2=-1 Max Grav 2=1	-3-8, 8=0-3-8 06(LC 20) 161(LC 12), 8=-161(L 770(LC 2), 8=1770(L	C 13) C 2)								
FORCES. (lb) - Max. Comp./h TOP CHORD 2-3=-4272/30 BOT CHORD 2-13=-286/39 WEBS 5-12=0/1159,	Max. Ten All forces 7, 3-5=-2850/205, 5- ⁻ 59, 12-13=-288/3955 7-12=-1473/262, 7-1	250 (lb) or less exc 7=-2850/205, 7-8=-4 , 10-12=-207/3955, 0=0/455, 3-12=-147	ept when sho 4272/308 8-10=-204/3 73/261, 3-13=	own. 959 =0/455						
NOTES- 1) Unbalanced roof live loads h 2) Wind: ASCE 7-10; Vult=120r gable end zone and C-C Ext 43-10-8 zone; cantilever left reactions shown; Lumber DC 3) TCLL: ASCE 7-10: Pr=20.0 r	ave been considered mph Vasd=95mph; T(erior(2) -0-10-8 to 3-5 and right exposed ; e DL=1.60 plate grip DC	for this design. CDL=6.0psf; BCDL= 5-2, Interior(1) 3-5-2 and vertical left and DL=1.60 mber DOL=1 15 Pla	=6.0psf; h=30 2 to 21-6-0, E: right exposed	Dft; Cat. II; xterior(2) 2 d;C-C for n	Exp B; Enclosed; N 21-6-0 to 25-9-10, Ir nembers and forces	IWFRS (envelop nterior(1) 25-9-1 s & MWFRS for	oe) 0 to			

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.



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		9-7-9	. 19	-0-7	21-8-	0	28-8-0	
		9-7-9	9-4	-15	2-7-	9	7-0-0	-
Plate Offsets (X,Y)	- [2:0-0-0,0-0-11], [9:0-0-0,0-0-11]						
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 11.6/ TCDL BCLL BCDL	20.0 /15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TFI2014	CSI. TC 0.70 BC 0.85 WB 0.82 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.27 14-16 -0.56 14-16 0.05 9	l/defl L/c >953 240 >462 180 n/a n/a	d PLATES D MT20 a Weight: 161 lb	GRIP 197/144 FT = 20%
LUMBER-BRACING-TOP CHORD2x4 SP No.2 or 2x4 SPF No.2TOP CHORDStructural wood sheathing directly applied or 3-7-0 oc purlins.BOT CHORD2x4 SP No.1BOT CHORDRigid ceiling directly applied or 10-0-0 oc bracing.WEBS2x4 SP No.3JOINTS1 Brace at Jt(s): 28OTHERS2x4 SP No.3Image: Comparison of the comparison of t								
REACTIONS. All (Ib) - Maa Maa Maa	l bearings 7-3-8 e: x Horz 2=-96(LC x Uplift All uplift x Grav All reaction 14=830(LC	xcept (jt=length) 2=0-3-8. 21) 100 lb or less at joint(s) 11, 9 ons 250 lb or less at joint(s) 7 C 1)	except 2=-115(LC 16), 13=- 12, 11 except 2=1091(LC 2),	722(LC 7) 9=688(LC 2), 14=	1134(LC 7),			
FORCES.(lb) - MaTOP CHORD2-BOT CHORD2-9-WEBS16	ax. Comp./Max. To 3=-1786/165, 3-5= 16=-163/1554, 14 11=-32/906 5-28=-20/702, 6-28	en All forces 250 (lb) or les 1522/137, 5-6=-1398/136, i -16=-32/906, 13-14=-32/906 3=-13/699, 3-16=-385/185, 8-	is except when shown. 6-8=-1076/156, 8-9=-1071/9; , 12-13=-32/906, 11-12=-32/9 -14=-465/216	3 906,				
NOTES- 1) Unbalanced roof 2) Wind: ASCE 7-10 gable end zone a 30-6-8 zone; cani reactions shown; 3) Truss designed fo Gable End Detail 4) TCLL: ASCE 7-10 roof snow: Lumbe 5) Unbalanced snow 6) This truss has be non-concurrent w 7) All plates are 1.55	live loads have be b; Vult=120mph Va and C-C Exterior(2 tilever left and righ Lumber DOL=1.6 or wind loads in th s as applicable, oi 0; Pr=20.0 psf (roc er DOL=1.15 Plate v loads have been een designed for g vith other live loads x4 MT20 unless o	een considered for this design asd=95mph; TCDL=6.0psf; E) -1-10-8 to 1-1-8, Interior(1) tt exposed ; end vertical left a 0 plate grip DOL=1.60 e plane of the truss only. For consult qualified building de of live load: Lumber DOL=1.1 e DOL=1.15); Category II; Ex considered for this design. reater of min roof live load of s.	n. 3CDL=6.0psf; h=30ft; Cat. II; 1-1-8 to 14-4-0, Exterior(2) ' and right exposed;C-C for more signer as per ANSI/TPI 1. 15 Plate DOL=1.15); Pg=15.0; p B; Partially Exp.; Ct=1.10 12.0 psf or 1.00 times flat recommended 12.0 psf or 1.00 times flat recommended 13.0 times flat recommended 14.0 times flat recommended 14.0 times flat recommended 15.0 time	Exp B; Enclosed; I I4-4-0 to 17-4-0, In embers and forces mal to the face), so 0 psf (ground snow of load of 11.6 psf	MWFRS (envelop terior(1) 17-4-0 to & MWFRS for ee Standard Indu); Pf=11.6 psf (fla on overhangs	be) o stry tt	SEAL	Will we

8) Gable studs spaced at 2-0-0 oc.

- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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Plate Offsets (X,Y) [3:0-1-12	.,0-1-8], [8:0-5-13,0-1-0], [10:0-6-0,0-6-4], [12:0-6-0,0-6-4]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.92 BC 0.71 WB 0.98 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.15 1-12 -0.29 1-12 0.04 9	l/defl >999 >899 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 370 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or BOT CHORD 2x8 SP DSS WEBS 2x4 SP No.3 *E 3-12: 2x4 SP No REACTIONS. (size) 1=(0	2x4 SPF No.2 xcept* .2 or 2x4 SPF No.2 -3-8 + TBE4 Simpson Strong-Tie) (req.	BF TC BC 0-4-6), 7=0-3-8, 9=0-3-8	RACING- DP CHORD S DT CHORD F (req. 0-8-1)	Structural woo Rigid ceiling di SUPPLEMEN OTHER MEAN	d sheathin rectly appl TARY BEAF	g directly app ied or 6-0-0 o RING PLATES, DW FOR THE M	lied or 2-8-3 oc purlins c bracing. SPECIAL ANCHORAGE IINIMUM REQUIRED SU	, OR IPPORT
Max Horz 1=-8 Max Uplift 1=-2 Max Grav 1=55	59(LC 12), 7=-301(LC 31), 9=-581(LC 1 59(LC 3), 7=55(LC 27), 9=10259(LC 2)	3)		WIDTH (SUCH ARE THE RES OR THE BUIL	HAS COLU SPONSIBILI DING DESI	MN CAPS, BEA TY OF THE TR GNER.	ARING BLOCKS, ETC.) USS MANUFACTURER	
FORCES. (lb) - Max. Comp./N TOP CHORD 1-2=-10214/52 6-7=-116/194' BOT CHORD 1-12=-520/93' WEBS 3-12=-158/500 6-9=-1229/172	ax. Ten All forces 250 (lb) or less exc 3, 2-3=-10111/488, 3-4=-4457/283, 4-5 9, 10-12=-349/6919, 9-10=-8/782, 8-9= 30, 3-10=-3838/245, 4-10=-157/3177, 5- 2, 6-8=-64/1481	ept when shown. =-4457/295, 5-6=-173/35/ -2529/150, 7-8=-1750/11 10=-215/4497, 5-9=-7418	42, 5 5/407,					
 NOTES- 1) 2-ply truss to be connected as foll Bottom chords connected as foll Bottom chords connected as foll Webs connected as follows: <i>2</i> 2) All loads are considered equa ply connections have been pl 3) Unbalanced roof live loads hat 4) Wind: ASCE 7-10; Vult=120m gable end zone; cantilever lef 5) TCLL: ASCE 7-10; Vult=20m roof snow: Lumber DOL=1.15 6) Unbalanced snow loads have 7) This truss has been designed will fit between the bottom ch 9) WARNING: Required bearing 10) TBE4 Simpson Strong-Tie of Continued if mature and does not of 	igether with 10d (0.131"x3") nails as foll Jws: 2x4 - 1 row at 0-9-0 oc. follows: 2x8 - 2 rows staggered at 0-9-0 2x4 - 1 row at 0-9-0 oc. Illy applied to all plies, except if noted as ovided to distribute only loads noted as ive been considered for this design. uph Vasd=95mph; TCDL=6.0psf; BCDL= t and right exposed ; end vertical left an sf (roof live load: Lumber DOL=1.15 Plate i Plate DOL=1.15); Category II; Exp B; I been considered for this design. I for a 10.0 psf bottom chord live load no ad for a live load of 20.0psf on the botton ord and any other members. g size at joint(s) 9 greater than input beat connectors recommended to connect tru consider lateral forces.	ows: I oc. s front (F) or back (B) face (F) or (B), unless otherwi =6.0psf; h=30ft; Cat. II; E> d right exposed; Lumber ite DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10 proconcurrent with any other m chord in all areas where ring size. ss to bearing walls due to	e in the LOAD CAS se indicated. xp B; Enclosed; Mi DOL=1.60 plate gi sf (ground snow); er live loads. e a rectangle 3-6-0 uUPLIFT at jt(s) 1.	SE(S) section WFRS (enveli rip DOL=1.60 Pf=11.6 psf (⁻ 0 tall by 2-0-0 . This connect	. Ply to ope) flat wide ion is	Contraction of the second seco	SEAL 044925 MGINEEF January 21,2	
WARNING - Verify design para Design valid for use only with MiT a truss system. Before use, the bi building design. Bracing indicate is always required for stability and fabrication, storage, delivery, erec Safety Information available for	neters and READ NOTES ON THIS AND INCLUDE ek® connectors. This design is based only upon p juliding designer must verify the applicability of desi d is to prevent buckling of individual truss web and. I to prevent collapse with possible personal injury a tion and bracing of trusses and truss systems, see m Truss Plate Institute, 2670 Crain Highway, Suite	D MITEK REFERENCE PAGE MI arameters shown, and is for an in gn parameters and properly inco (or chord members only. Additio and property damage. For gener e ANS/TPI1 Quality Cr 2 203 Waldorf, MD 20601	I-7473 rev. 5/19/2020 B ndividual building comp ryporate this design into nal temporary and perr al guidance regarding i riteria, DSB-89 and BC	BEFORE USE. ponent, not o the overall manent bracing the CSI Building Cor	nponent		ENGINEERING BY A Miltek 818 Soundside Road Edenton, NC 27932	

Job	Truss	Truss Type	Qty	Ply	KB Home 243.2939.B	
						144459043
243_2939_B	GG	COMMON GIRDER	1	2		
				_	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.4	130 s Nov	30 2020 MiTek Industries, Inc. Wed Jan 20 13:34:41 2021	Page 2
		ID:XZs	siAHNe IV	plcdoAjGo	3ztm9T-DCWDijBX7d9TppdWhNSAad0WmyMtpRyy7gAEF	P7ztS_i

NOTES-

11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.

- 12) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9. This connection is for uplift only and does not consider lateral forces.
- 13) Use Simpson Strong-Tie LUS26 (4-SD9112 Girder, 4-SD9212 Truss, Single Ply Girder) or equivalent spaced at 2-3-0 oc max. starting at 2-0-12 from the left end to 12-2-4 to connect truss(es) to back face of bottom chord.
- Use Simpson Strong-Tie LUS26 (4-10d Girder, 4-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 12-11-4 from the left end to 26-11-4 to connect truss(es) to back face of bottom chord.
- 15) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

- Uniform Loads (plf)
- Vert: 1-4=-43, 4-7=-43, 1-7=-20 Concentrated Loads (lb)

Vert: 11=-722(B) 8=-722(B) 15=-762(B) 16=-762(B) 17=-762(B) 18=-762(B) 19=-762(B) 20=-762(B) 21=-722(B) 22=-722(B) 23=-722(B) 24=-722(B) 25=-722(B) 26=-722(B) 26=-72(B) 26=-72(B) 26=-72(B) 26=-72(B) 26=-72(B) 26=-72(B) 2

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7-6-0	7-0-0	7-0-0	26-2-0	28-6-0 35-6	-0 +	43-0-0	
Plate Offsets (X,Y) [5:0-4-0,	0-4-8], [11:0-5-0,0-0-0], [15:0-5-0,0-0-0]	100	400	240 10		100	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDI 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.92 BC 0.52 WB 0.85 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defi -0.73 13 >704 -1.41 13 >364 0.20 8 n/a	L/d 240 180 a n/a	PLATES MT20 MT18HS Weight: 250 lb	GRIP 244/190 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP DSS *E 3-5,5-7: 2x6 SP BOT CHORD 2x6 SP DSS WEBS 2x4 SP No.3 REACTIONS. (size) 2=0 Max Horz 2=-3 Max Uplift 2=-2 Max Grav 2=1 FORCES. (lb) - Max. Comp.// TOP CHORD 2-3=-4529/47 7-8=-4529/47 BOT CHORD 2-16=-417/42 8-10=-386/42 WEBS 3-16=-27/116 7-10=-27/116	xcept* No.2 -3-8, 8=0-3-8 38(LC 21) 216(LC 12), 8=-216(LC 13) 770(LC 2), 8=1770(LC 2) Max. Ten All forces 250 (lb) or less exc 6, 3-4=-4155/474, 4-5=-7831/811, 5-6=-7 6 14, 14-16=-694/7029, 13-14=-694/7029, 214 7, 4-16=-3080/349, 4-13=-104/877, 6-13 7, 5-13=-424/141, 4-14=0/272, 6-12=0/2	BI TC BC W 2831/811, 6-7=-4155/475 12-13=-663/7029, 10-12 =-105/877, 6-10=-3080/3 72	RACING- DP CHORD DT CHORD EBS 663/7029, 48,	Structural wood sheath 2-0-0 oc purlins (2-1-1 Rigid ceiling directly ap 2 Rows at 1/3 pts	iing directly appli 5 max.): 3-7. oplied or 10-0-0 o 4-16, 6-10	ed or 2-2-0 oc purlins	, except
 NOTES- 1) Unbalanced roof live loads h 2) Wind: ASCE 7-10; Vult=120r gable end zone and C-C Ext , Exterior(2) 35-6-0 to 41-7-0 exposed;C-C for members a 3) TCLL: ASCE 7-10; Pr=20.0 p roof snow: Lumber DOL=1.1 governs. Rain surcharge ap 4) Unbalanced snow loads haw 5) This truss has been designee non-concurrent with other liv 6) Provide adequate drainage to 7) All plates are MT20 plates ur 8) This truss has been designee 9) * This truss has been designed 9) * This truss has been designed 9) * This truss has been designed 10) One H2.5A Simpson Strong connection is for uplift only 11) Graphical purlin representa 	ave been considered for this design. mph Vasd=95mph; TCDL=6.0psf; BCDL= erior(2) -0-10-8 to 3-5-2, Interior(1) 3-5-2 , Interior(1) 41-7-0 to 43-10-8 zone; cant nd forces & MWFRS for reactions showr s6 (roof live load: Lumber DOL=1.15 Pla 5 Plate DOL=1.15); Category II; Exp B; F plied to all exposed surfaces with slopes a been considered for this design. d for greater of min roof live load of 12.0 e loads. o prevent water ponding. nless otherwise indicated. d for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the bottor lord and any other members. J-Tie connectors recommended to conne and does not consider lateral forces. tion does not depict the size or the orien	6.0psf; h=30ft; Cat. II; E: to 7-6-0, Exterior(2) 7-6- ilever left and right expos ; Lumber DOL=1.60 plate te DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10, Lu less than 0.500/12 in acc psf or 1.00 times flat roof inconcurrent with any oth n chord in all areas where ct truss to bearing walls of ation of the purlin along f	xp B; Enclosed; N 0 to 13-7-0, Inter sed; end vertical e grip DOL=1.60 ssf (ground snow u=50-0-0; Min. fla cordance with IB0 i load of 11.6 psf e r live loads. e a rectangle 3-6 due to UPLIFT at the top and/or bo	MWFRS (envelope) ior(1) 13-7-0 to 35-6-0 left and right); Pf=16.5 psf (flat tt roof snow load C 1608.3.4. on overhangs -0 tall by 2-0-0 wide t jt(s) 2 and 8. This ttom chord.	Contraction of the second seco	SEAL 044925	

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A MiTek Affi 818 Soundside Road Edenton, NC 27932



F	9-6-0		17-6-9	2	5-5-7	33-6-0			43-0-0	
	9-6-0		8-0-9	7-	10-13	8-0-9		1	9-6-0	·
LOADING (psi TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip D Lumber DO Rep Stress Code IRC2	2-0-0 DOL 1.15 L 1.15 Incr YES 015/TPI2014	CSI. TC 0.72 BC 0.43 WB 0.92 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT	in (loc) -0.48 14-15 -0.96 14-15) 0.16 10	l/defl >999 >535 n/a	L/d 240 180 n/a	PLATES MT20 MT18HS Weight: 257 lb	GRIP 197/144 244/190 FT = 20%
LUMBER- TOP CHORD	2x4 SP No.2 or 2x4 4-6,6-8: 2x6 SP No.2	SPF No.2 *Exc 2	cept*		BRACING- TOP CHORD	Structural wood except	sheathing	directly app	lied or 2-3-10 oc purlin	IS,

	4-6,6-8: 2x6 SP No.2		except			
BOT CHORD	2x6 SP DSS		2-0-0 oc purlins (2-7-14 ma	ax.): 4-8.		
WEBS	2x4 SP No.3	BOT CHORD	ORD Rigid ceiling directly applied or 10-0-0 oc bracing.			
		WEBS	1 Row at midpt	5-17, 7-15, 7-12		
REACTIONS.	(size) 2=0-3-8, 10=0-3-8					
	Max Horz 2=48(LC 20)					

Max Uplift 2=-212(LC 12), 10=-212(LC 13) Max Grav 2=1770(LC 2), 10=1770(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-4340/517, 3-4=-4206/433, 4-5=-3938/429, 5-7=-5894/631, 7-8=-3936/428, 8-9=-4204/433, 9-10=-4339/518
- BOT CHORD
 2-17=-478/4024, 15-17=-560/5894, 14-15=-533/5907, 12-14=-533/5907, 10-12=-440/4023

 WEBS
 3-17=-241/255, 4-17=-6/918, 5-17=-2214/284, 5-15=0/305, 7-14=0/308, 7-12=-2229/286, 8-12=-6/919, 9-12=-242/254

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-5-2, Interior(1) 3-5-2 to 9-6-0, Exterior(2) 9-6-0 to 15-7-0, Interior(1) 15-7-0 to 33-6-0, Exterior(2) 33-6-0 to 39-7-0, Interior(1) 39-7-0 to 43-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This
- connection is for uplify only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	5-10-11	11-6-0	21-6-0		3.	1-6-0		37-1-5	43-0-0		
	5-10-11	5-7-5	10-0-0	I	1(0-0-0		5-7-5	5-10-11	1	
Plate Offsets	(X,Y) [2:0-0-15	,0-0-9], [5:0-4-0,0-4-8], [8	0-0-15,0-0-9]								
	()										
	SI)	SPACING-	2-0-0 CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
ICLL (roof)	20.0	Plate Grin DOI	1 15 TC	0.93	Vert(LL)	-0.44 1.3	>999	240	MT20	197/144	
Snow (Pf/Pg)	16.5/15.0	Lumber DOL	1.15	0.00	Vort(CT)	0.97 12 15	- 505	190	11120	13//144	
TCDL	10.0	Lumber DOL	1.15 BC	0.99	Ven(CT)	-0.87 13-15	>586	180			
BCLL	00 *	Rep Stress Incr	YES WB	0.92	Horz(CT)	0.20 8	n/a	n/a			
BCDI	10.0	Code IRC2015/TP	I2014 Matri	x-S					Weight: 254 lb	FT = 20%	
BCDL	10.0										
LUMBER-				B	RACING-						
TOP CHORD	2x4 SP No 2 or 3	2x4 SPE No 2 *Except*		т	OP CHORD	Structural wood	sheathing	n directly appl	lied or 2-2-0 oc purlins	except	
	4-5 5-6: 2v6 SP	No 2			0. 00	2-0-0 oc purline	(2-2-0 mg	y). 1-6		, oncopt	
	4-3,3-0. 2x0 3i	110.2		D		Z-0-0 00 putitis	(2-2-0 m	an.). 4-0.			
BOICHORD	2X6 SP NO.2			B		Rigia celling air	ectly appli	ed or 2-2-0 of	c bracing.		
WEBS	2x4 SP No.3			W	/EBS	1 Row at midpt		5-15, 5-11			
REACTIONS.	. (size) 2=0-3	3-8, 8=0-3-8									
	Max Horz 2=57	(I C 20)									
	Max Holift 2- 2	(2020)									
		100(10 12), 0=-200(10 13)									
	Max Grav Z=17	70(LC 2), 8=1770(LC 2)									
FORCES. (I	b) - Max. Comp./M	ax. Ten All forces 250 (b) or less except when sh	own.							
TOP CHORD	2-3=-4367/449	. 3-4=-4007/416. 4-5=-37	48/415. 5-6=-3748/414. 6-	7=-4007/415	5.						
	7-8-4367/450)	,,.		,						
	2 16 414/405	, 1 15 16- 414/4051 12 1	E 442/E020 11 12 442	/F020 10 11	270/4054						
BOT CHORD	210=414/4031, 13-10=414/4031, 13-13=442/3030, 11-13=442/3030, 10-11=3/0/4031,										
	8-10=-370/4051										
WEBS	3-15=-485/150), 4-15=0/800, 5-15=-1544	/224, 5-13=0/412, 5-11=-`	1544/224, 6-1	11=0/800,						
	7-11=-485/151										
NOTES-											
	al reaf live leads he		ia design								
1) Unbalance	d roof live loads ha	we been considered for th	is design.								
Wind: ASC	E 7-10; Vult=120m	<pre>iph Vasd=95mph; TCDL=</pre>	6.0psf; BCDL=6.0psf; h=3	0ft; Cat. II; E	xp B; Enclosed; I	/WFRS (envelo	be)				
gable end ;	zone and C-C Exte	erior(2) -0-10-8 to 3-5-2, In	terior(1) 3-5-2 to 11-6-0, E	Exterior(2) 11	-6-0 to 17-7-0, In	terior(1) 17-7-0 t	0				
31-6-0. Ext	terior(2) 31-6-0 to 3	37-7-0. Interior(1) 37-7-0 to	43-10-8 zone: cantilever	left and right	t exposed : end v	ertical left and ri	aht		MILLIN		
exposed C	-C for members an	d forces & MWERS for re	actions shown: Lumber D	$\gamma = 1.60$ plat	a aria DOI = 1.60		J			11.	
				SL = 1.00 plat		DE 40 E 6 /6		1	"AH UARO	14	
3) TULL: ASU	JE 7-10; PI=20.0 p	si (rooi live load: Lumber	JOL=1.15 Plate DOL=1.1	5); Pg=15.0 p	si (ground show); PI=16.5 pSI (II	11	1 1	8	1.4	
roof snow:	Lumber DOL=1.15	Plate DOL=1.15); Catego	ory II; Exp B; Partially Exp	.; Ct=1.10, Lu	u=50-0-0; Min. fla	it roof snow load			SIN		
governs. F	Rain surcharge app	lied to all exposed surface	es with slopes less than 0.	500/12 in acc	cordance with IB	C 1608.3.4.		74		Mon>	
4) Unbalance	d snow loads have	been considered for this	desian.							i wy	
5) This truss I	has been designed	for greater of min roof live	a load of 12 0 psf or 1 00 t	imes flat roof	f load of 11 6 nsf	on overhands		2 2	Q .		
	rrant with other live		c 1000 01 12:0 p31 01 1:00 1	inico naciooi	100000111.0 p3	on overhangs			OFAL		
		ioaus.							SEAL		
b) Provide ad	equate drainage to	prevent water ponding.						- -	044005	: =	
This truss h	has been designed	for a 10.0 psf bottom cho	rd live load nonconcurrent	t with any oth	ner live loads.				044925	1 E	
8) * This truss	s has been designe	ed for a live load of 20.0ps	f on the bottom chord in a	Il areas wher	e a rectangle 3-6	-0 tall by 2-0-0 w	ride			100 C	
will fit betw	een the bottom cho	ord and any other membe	rs.		0					1 S S	
9) One H2 54	Simpson Strong T	Tie connectors recommon	ded to connect trues to bo	aring walls d	ue to LIPLIET of i	t(s) 2 and 8 This		5.0	· A. a	: a :	
5) One 12.0A	is far unlift and			anny wans u		(3) Z anu 0. Thi	,	100	VGINEE!	6.5	
connection	i is for upliπ only an	iu uues not consider latera	al lorces.					110	0	IN ST	
10) Graphical	I purlin representat	ion does not depict the siz	e or the orientation of the	purlin along	the top and/or bo	ttom chord.		11,	TTN CE	11	
									WI. 54	11	



January 21,2021

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F	6-10-11		13-6-0		21-6-0			29-6-0)		36-1	1-5	43-0-0	_
Plate Offsets (X	(,Y) [2:0-0-15	,0-0-9], [5:0-4-0	0,0-4-8], [8:	0-0-15,0-0-9],	[12:0-3-12,0	D-0-0], [14	4:0-3	-12,0-0-0]			0-7	-0	0-10-11	
LOADING (psf TCLL (roof) Snow (Pf/Pg) TCDL	7) 20.0 16.5/15.0 10.0	SPACIN Plate Gr Lumber	IG- ip DOL DOL	2-0-0 1.15 1.15 VES	CSI. TC BC	0.78 0.88 0.67		DEFL. Vert(LL) Vert(CT)	in -0.34 -0.69	(loc) 13 13	l/defl >999 >740	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL BCDI	0.0 * 10.0	Code IF	RC2015/TPI	12014	Matri	0.67 x-S		HOIZ(CT)	0.16	0	n/a	n/a	Weight: 253 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.1 *Ex 4-5,5-6: 2x6 SP 2x6 SP No.2 2x4 SP No.3	xcept* No.2					BR TO BO WE	ACING- P CHORD T CHORD EBS	Structura 2-0-0 oc Rigid cei 1 Row at	al wood purlins ling dire t midpt	sheathin (3-6-2 m ectly appli	g directly app ax.): 4-6. ied or 10-0-0 5-15, 5-11	blied or 2-2-0 oc purlins oc bracing. 1	, except
REACTIONS.	(size) 2=0- Max Horz 2=67 Max Uplift 2=-19 Max Grav 2=17	3-8, 8=0-3-8 7(LC 16) 99(LC 12), 8=- ⁻ 770(LC 2), 8=17	199(LC 13) 770(LC 2)											
FORCES. (Ib) TOP CHORD	- Max. Comp./M 2-3=-4381/430	lax. Ten All fc), 3-4=-3764/38	orces 250 (l 33, 4-5=-350	b) or less exc 05/386, 5-6=-3	ept when sh 8505/386, 6-	own. 7=-3764/	383,							
BOT CHORD	2-16=-397/406) 63, 15-16=-397/ 63	/4063, 13-1	5=-327/4211,	11-13=-327	/4211, 10)-11=	-346/4063,						
WEBS	3-15=-742/159 7-11=-742/160), 4-15=0/766, 5)	5-15=-988/1	154, 5-13=0/3	24, 5-11=-98	38/153, 6	-11=(0/766,						
NOTES- 1) Unbalanced 2) Wind: ASCE gable end zc 29-6-0, Exte exposed;C-C 3) TCLL: ASCE roof snow: L governs. Ra 4) Unbalanced 5) This truss ha non-concurre 6) Provide aded 7) This truss ha non-concurre 6) Provide aded 7) This truss ha 8) * This truss the will fit betwee 9) One H2.5A S connection is 10) Graphical p	roof live loads ha 7-10; Vult=120m one and C-C Exte rior(2) 29-6-0 to 2 for members an 7-10; Pr=20.0 p umber DOL=1.15 in surcharge app snow loads have as been designed ent with other live quate drainage to as been designed ent with other live quate drainage to as been designed en the bottom ch Simpson Strong-T s for uplift only an purlin representat	ave been consider apph Vasd=95mp prior(2) -0-10-8 35-7-0, Interior(di forces & MW sf (roof live load been consider blied to all expo been consider for greater of r loads. prevent water for a 10.0 psf for a live load ord and any oth frie connectors and does not con ion does not de	dered for the ph; TCDL=6 to 3-5-2, Ini 1) 35-7-0 to (FRS for rea d: Lumber I 15); Catego sed surface red for this of min roof live ponding. bottom choo d of 20.0pst bottom choo d of 20.0pst ner member recommend sider latera apict the siz	is design. 6.0psf; BCDL= terior(1) 3-5-2 o 43-10-8 zonv actions shown DOL=1.15 Pla ory II; Exp B; F ss with slopes design. a load of 12.0 rd live load nc f on the bottor rs. ded to connec al forces. e or the orient	6.0psf; h=3 to 13-6-0, E e; cantilever f; Lumber D0 te DOL=1.1 Partially Exp. less than 0. psf or 1.00 t nconcurrent n chord in al t truss to be ation of the	0ft; Cat. I :xterior(2) left and r DL=1.60 J 5); Pg=15 ; Ct=1.10 500/12 in imes flat ; with any I areas w aring wall purlin alo	I; Exp) 13-6 ight c plate 5.0 ps), Lu= 0 a acco roof I o other here Is due	p B; Enclosed; M 6-0 to 19-7-0, In exposed ; end v grip DOL=1.60 sf (ground snow =50-0-0; Min. fla ordance with IB0 load of 11.6 psf er live loads. a rectangle 3-6 e to UPLIFT at j ne top and/or bo	MWFRS (terior(1) 1 ertical lef troof snc C 1608.3. on overha -0 tall by t(s) 2 and ttom choi	envelop 19-7-0 t t and rig 5 psf (fla w load 4. angs 2-0-0 w I 8. This rd.	pe) o ght at ide	Communities State	SEAL 044925	Werner Manning

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ENGINEERING BY A MITek Atfiliate 818 Soundside Road Edenton, NC 27932

January 21,2021





	<u>− 7-10-11</u> 7-10-11		<u>15-6-0</u> 7-7-5	+ 21-	-6-0 D-0		27-6-0			35-1-5 7-7-5		43-0-0	
Plate Offsets	(X,Y) [2:0-0-15	,0-0-9], [10:0-0-15,0	0-0-9]				000					1.0.11	
LOADING (p TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 16.5/15.0 10.0 0.0 *	SPACING- Plate Grip Du Lumber DOL Rep Stress I Code IRC20	2-0-0 OL 1.15 . 1.15 ncr YES 115/TPI2014	CSI. TC BC WB Matrix	0.77 0.91 0.73 x-S		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.30 -0.61 0.17	(loc) 15 15 10	l/defl >999 >844 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 244 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP DSS *Ex 5-7: 2x4 SP No.2 2x6 SP No.2 2x4 SP No.3 . (size) 2=0- Max Horz 2=-7 Max Uplit 2=-17 May Gray 2=-12	xcept* 2 or 2x4 SPF No.2 3-8, 10=0-3-8 7(LC 21) 91(LC 12), 10=-191 720(L 2), 10=-191	(LC 13)			BR. TOI BO WE	ACING- P CHORD T CHORD BS	Structura 2-0-0 oc Rigid cei 1 Row at	al wood purlins ling dire midpt	sheathinı (2-6-11 n ectly appli	g directly app nax.): 5-7. ied or 10-0-0 3-16, 9-14	plied or 2-2-0 oc purlins) oc bracing. 4	, except
FORCES. (TOP CHORD BOT CHORD WEBS	lb) - Max. Comp./M 2-3=-4358/405 9-10=-4358/40 2-18=-375/403 10-12=-317/4 3-18=0/322, 3 9-14=-980/182	lax. Ten All forces 5, 3-5=-3505/341, 5- 36 99, 16-18=-375/4039 039 -16=-980/181, 5-16= 2, 9-12=0/322	: 250 (lb) or less exc: -6=-3239/348, 6-7=-3 9, 15-16=-233/3565, =0/722, 6-16=-607/10	ept when sho 3239/348, 7- 14-15=-233/ 08, 6-14=-60	own. 9=-3505/3 /3565, 12-)7/108, 7- ⁻	341, ∙14≕ 14=0	-317/4039,)/722,						
NOTES- 1) Unbalance 2) Wind: ASC gable end 27-6-0, Ex exposed;C 3) TCLL: ASC roof snow: governs. I 4) Unbalance 5) This truss non-concu 6) Provide ac 7) This truss 8) * This truss will fit betw 9) One H2.5/ connectior 10) Graphica	ed roof live loads ha E 7-10; Vult=120m zone and C-C Exte terior(2) 27-6-0 to 3 -C for members an CE 7-10; Pr=20.0 p Lumber DOL=1.15 Rain surcharge app d snow loads have has been designed rrent with other live lequate drainage to has been designed is has been designed to have the bottom chu A Simpson Strong-1 h is for uplift only ar	ave been considered ph Vasd=95mph; T rrior(2) -0-10-8 to 3- 33-7-0, Interior(1) 33 di forces & MWFRS sf (roof live load: Lu s Plate DOL=1.15); blied to all exposed a b been considered for for a live load of 2 ord and any other m fie connectors reco- nd does not conside ion does not depict	d for this design. 'CDL=6.0psf; BCDL= 5-2, Interior(1) 3-5-2 3-7-0 to 43-10-8 zone is for reactions shown mber DOL=1.15 Pla Category II; Exp B; F surfaces with slopes or this design. oof live load of 12.0 ding. m chord live load no 20.0psf on the bottor nembers. mmended to connec r lateral forces. the size or the orient	=6.0psf; h=30 to 15-6-0, E e; cantilever ; Lumber DC te DOL=1.15 Partially Exp. less than 0.9 psf or 1.00 ti nconcurrent n chord in al t truss to bea ation of the	Oft; Cat. II; xterior(2) left and ri- DL=1.60 p 5); Pg=15; ; Ct=1.10, 500/12 in : with any of l areas wh aring walls purlin alor	; Exp 15-6 ght e late 0 ps , Lu= accc oof l oothe nere s due	b B; Enclosed; M 5-0 to 21-6-0, Int exposed ; end ve grip DOL=1.60 if (ground snow) 50-0-0; Min. flat ordance with IBC oad of 11.6 psf of r live loads. a rectangle 3-6- to UPLIFT at jt le top and/or bot	IWFRS (erior(1) 2 ertical left ; Pf=16.5 t roof snc c 1608.3. on overha 0 tall by (s) 2 and tom chor	envelop 21-6-0 ti t and rig 5 psf (fla w load 4. angs 2-0-0 w I 10. Th rd.	be) o ght at ide is	Summer Street	SEAL 044925	The summer of the second secon

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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L 8-10-	11 17-6-0	25-6-0	34-1-5	43-0-0
8-10-	11 8-7-5	8-0-0	8-7-5	8-10-11
Plate Offsets (X,Y) [2:0-0-15	,0-0-9], [11:0-0-15,0-0-9]			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15	CSI. DEFL. TC 0.76 Vert(LL) BC 0.90 Vert(CT)	in (loc) I/defl L/d -0.31 15 >999 240 -0.64 15-16 >806 180	PLATES GRIP MT20 197/144
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	MB 0.69 Horz(CT) Matrix-S) 0.16 11 n/a n/a	Weight: 252 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 6-7: 2x6 SP No. BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3	2x4 SPF No.2 *Except* 2	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing directly 2-0-0 oc purlins (3-3-1 max.): 6-7. Rigid ceiling directly applied or 10- 1 Row at midpt 7-16	applied or 2-2-0 oc purlins, except -0-0 oc bracing.
REACTIONS. (size) 2=0- Max Horz 2=86 Max Uplift 2=-1 Max Grav 2=17	3-8, 11=0-3-8 (LC 20) 82(LC 12), 11=-182(LC 13) '70(LC 2), 11=1770(LC 2)			
FORCES. (lb) - Max. Comp./M TOP CHORD 2-3=-4358/414 8-10=-4079/32 BOT CHORD 2-18=-402/40 WEBS 3-18=-295/163 8-15=-757/16 NOTES- 1) Unbalanced roof live loads ha 2) Wind: ASCE 7-10; Vult=120n gable end zone and C-C Exte 25-6-0, Exterior(2) 25-6-0 to 3 exposed;C-C for members ar 3) TCLL: ASCE 7-10; PT=20.0 p roof snow: Lumber DDL=1.1f governs. Rain surcharge app 4) Unbalanced snow loads have 5) This truss has been designed non-concurrent with other live 6) Provide adequate drainage to 7) This truss has been designed will fit between the bottom ch 9) One H2.5A Simpson Strong- connection is for uplift only ar 10) Graphical purifin represental	ax. Ten All forces 250 (lb) or less exc. ax. Ten All forces 250 (lb) or less exc. b. 3-5=-4078/344, 5-6=-3257/316, 6-7=-3 c. 4, 10-11=-4358/414 c. 4, 10-11=-4358/414 c. 5-18=0/473, 5-16=-756/161, 6-16=0/6 c. 7, 8-13=0/475, 10-13=-295/164 ave been considered for this design. by Vasd=95mph; TCDL=6.0psf; BCDL= crior(2) -0-10-8 to 3-5-2, Interior(1) 3-5-2 c. 7, Interior(1) 31-7-0 to 43-10-8 zone d forces & MWFRS for reactions shown sf (roof live load: Lumber DOL=1.15 Pla considered for this design. lifed to all exposed surfaces with slopes been considered for this design. for greater of min roof live load of 12.0 considered for this design. for a 10.0 psf bottom chord live load no cd for a live load of 20.0psf on the bottor ord and any other members, with BCDL connectors recommended to connect in does not consider lateral forces.	apt when shown. 1041/325, 7-8=-3256/316, 13-15=-237/3584, 11-13=-337/4044 20, 7-16=-248/251, 7-15=0/619, 16.0psf; h=30ft; Cat. II; Exp B; Enclosed; to 17-6-0, Exterior(2) 17-6-0 to 23-7-0, Ir a; cantilever left and right exposed ; end v ; Lumber DOL=1.60 plate grip DOL=1.60 the DOL=1.15); Pg=15.0 psf (ground snow tartially Exp.; Ct=1.10, Lu=50-0-0; Min. fk less than 0.500/12 in accordance with IB posf or 1.00 times flat roof load of 11.6 psf nconcurrent with any other live loads. n chord in all areas where a rectangle 3-f = 10.0psf. truss to bearing walls due to UPLIFT at ation of the purlin along the top and/or br	MWFRS (envelope) tterior(1) 23-7-0 to vertical left and right); Pf=16.5 psf (flat at roof snow load C 1608.3.4. Ton overhangs 5-0 tall by 2-0-0 wide jt(s) 2 and 11. This	SEAL 044925

January 21,2021

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L 9-1	10-11 19-6-	023-6-	0 33-1-5	1	43-0-0	
	10-11 9-7-5	5 4-0-0) 9-7-5	1	9-10-11	
Plate Offsets (X,Y) [2:0-1-3,	,0-0-9], [11:0-1-3,0-0-9]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDI 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.91 BC 0.94	DEFL. in (I Vert(LL) -0.33 13 Vert(CT) -0.67 13	oc) l/defl L/d .15 >999 240 .15 >760 180	PLATES MT20	GRIP 197/144
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.31 Matrix-S	Horz(CT) 0.16	11 n/a n/a	Weight: 249 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3	2x4 SPF No.2	BR TO BC WE	ACING- P CHORD Structural v 2-0-0 oc pu T CHORD Rigid ceilin EBS 1 Row at m	rood sheathing directly a rlins (3-3-13 max.): 6-7. g directly applied or 2-2- idpt 5-16, 8	applied, except - 0 oc bracing. -15	
Max Horz 2=90 Max Uplift 2=-1 Max Grav 2=1	6(LC 16) 172(LC 12), 11=-172(LC 13) 770(LC 2), 11=1770(LC 2)					
FORCES. (lb) - Max. Comp./M TOP CHORD 2-3=-4364/383 8-10=-4080/30	Max. Ten All forces 250 (lb) or less exc 5, 3-5=-4079/305, 5-6=-3024/281, 6-7=-2 05, 10-11=-4365/385	ept when shown. 2787/292, 7-8=-3022/281,				
BOT CHORD 2-18=-378/400 WEBS 3-18=-357/182 8-13=0/577, 2	67, 16-18=-252/3533, 15-16=-98/2785, 1 2, 5-18=0/575, 5-16=-900/180, 6-16=-3/6 10-13=-356/182	3-15=-196/3533, 11-13=- 662, 7-15=-28/660, 8-15=-	305/4068 902/180,			
 NOTES- 1) Unbalanced roof live loads in: 2) Wind: ASCE 7-10; Vult=120n gable end zone and C-C Ext: 43-10-8 zone; cantilever left : reactions shown; Lumber DC 3) TCLL: ASCE 7-10; Pr=20.0 p roof snow: Lumber DOL=1.1! governs. Rain surcharge app 4) Unbalanced snow loads have 5) This truss has been designed non-concurrent with other live 6) Provide adequate drainage tt 7) This truss has been designed 8) * This truss has been designed 8) * This truss has been designed 9) One H2.5A Simpson Strong-connection is for uplift only at 10) Graphical purlin representation 	ave been considered for this design. mph Vasd=95mph; TCDL=6.0psf; BCDL= erior(2) -0-10-8 to 3-5-2, Interior(1) 3-5-2 and right exposed ; end vertical left and in DL=1.60 plate grip DOL=1.60 sof (roof live load: Lumber DOL=1.15 Plate 5 Plate DOL=1.15); Category II; Exp B; F plied to all exposed surfaces with slopes e been considered for this design. d for greater of min roof live load of 12.0 e loads. o prevent water ponding. d for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the bottor ord and any other members. Tie connectors recommended to connec nd does not consider lateral forces. tion does not depict the size or the orient	=6.0psf; h=30ft; Cat. II; Ex to 19-6-0, Exterior(2) 19-0 right exposed;C-C for mer te DOL=1.15); Pg=15.0 pc Partially Exp.; Ct=1.10, Lu- less than 0.500/12 in accor- psf or 1.00 times flat roof l onconcurrent with any other n chord in all areas where t truss to bearing walls du tation of the purlin along the	p B; Enclosed; MWFRS (en 6-0 to 29-7-0, Interior(1) 29- nbers and forces & MWFRS sf (ground snow); Pf=16.5 p 50-0-0; Min. flat roof snow ordance with IBC 1608.3.4. load of 11.6 psf on overham er live loads. a rectangle 3-6-0 tall by 2-0 e to UPLIFT at jt(s) 2 and 1 ne top and/or bottom chord.	/elope) 7-0 to for sf (flat load gs I-0 wide 1. This	SEAL 044925	Mee.



818 Soundside Road Edenton, NC 27932

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Job	Truss	Truss Type		Qty	Ply	KB Home 243.29	939.B		144450054
243_2939_B	HG1	Hip Girder		1	2				144459051
84 Components (Dunn),	Dunn, NC - 28334,			8.4	130 s Nov 3	Job Reference (o 30 2020 MiTek Ind	ptional) lustries, Inc. We	ed Jan 20 13:35:30 20	21 Page 1
-0-10-8 5-6-0	11-11-14	18-3-15	 24-8-1	D:XZssjAHNe	_IVplcdoAj 31-0-2	Gq3ztm9T-Hdzk_	TnGagKWBLtx3	Aqg3yYuTi6eoeo9Oo	XKcoztRzx
0-10-8 5-6-0	6-5-14	6-4-2	6-4-2		6-4-2		6-5-14	5-6-0	0-10-8
4.00 12 NAIL 8	ED NAIL $x_8 = \frac{2}{3}$ NAILED NAILED 2 $x_{0} = \frac{2}{2}$ NAILED NAILED 2 NAILED	ED NAILED x4 NAILED 4 22 23 24 1 22 24 1 24	NAILED NAILED N/ 8x8 = NAILED $25 \frac{5}{26} 27 \times 28 \times 10^{-10}$	AILED NAILED 2x4 29 630 2 0 0 0 0	NAILED N 31 32	NAILED NAILED $8x12 = NA$	AllED NAILED 3년 25 교 교	NAILED 6x6 = 9	Scale = 1:73.4
	מאי עם עם איי				0.0	Դանս _ տ			
4x6 = 36	¹⁹ 37 38	¹⁸ ³⁹ 17 ⁴⁰ 4	1 42 ¹⁶ 43 44	45 ¹⁵ 46	47	14 48 ¹³ 49	50 51	¹² 52	4x6 =
LUS24	2x4 NAILED NAILED	10x12 MT18HS =	2x4 NAILED	4x8 =	NAILED N	IAILED 2x4 NA	AILED NAILED	6x6 = LUS24	
ſ	NAILED 5x	12 = NAILED	NAILED NAILED NA	AILED NAILED	10x12 M	1T18HS =	N	AILED	
	NAIL	ED NAILED				NAILED			
<u>5-6-0</u> 5-6-0	<u>11-11-14</u> 6-5-14	18-3-15 6-4-2	24-8-1 6-4-2		<u>31-0-2</u> 6-4-2		<u>37-6-0</u> 6-5-14	43-0-0	
Plate Offsets (X,Y) [2:0-	-2-15,0-0-4], [3:0-4-0,0-2-6],	[5:0-2-4,0-4-8], [7:	0-5-12,0-4-8], [9:0-3-0,0-2	2-11], [10:0-3-	3,0-0-4], [12:0-2-4,0-3-0], [18:0-5-4,0-2-0]	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code JBC2015	2-0-0 1.15 1.15 NO TPI2014	CSI. TC 0.92 BC 0.69 WB 0.81 Matrix_S	DEFL. Vert(LL) Vert(CT) Horz(CT	in -0.95 -1.91) 0.20	(loc) l/defl 15-16 >540 15-16 >268 10 n/a	L/d 240 180 n/a	PLATES MT20 MT18HS Weight: 511 lb	GRIP 197/144 244/190 FT = 20%
BCDL 10.0		1712014	Wath - S					weight. 511 b	FT = 2076
TOP CHORD 2x6 SP No. 1-3,9-11: 2: BOT CHORD 2x6 SP DS: WEBS 2x4 SP No. 3-18,7-12: 2: REACTIONS. (size) Max Horz Max Uplift Max Grav FORCES. (lb) - Max. Con TOP CHORD 2-3=-806 7-9=-746 BOT CHORD 2-19=-102 13-15=-21 WEBS 3-19=-102	2 *Except* x4 SP No.2 or 2x4 SPF No.: S 3 *Except* 2x4 SP No.2 or 2x4 SPF Nc 2=0-3-8, 10=0-3-8 2=30(LC 58) 2=-477(LC 8), 10=-478(LC 2=3057(LC 2), 10=3063(LC np./Max. Ten All forces 25 8/1190, 3-4=-13766/2102, 4 99/1137, 9-10=-8165/1202 92/7528, 18-19=-1096/7499 063/13942, 12-13=-2063/13 2: 3.18-11029(6600 4.18	2 9) 2) 0 (lb) or less excep -5=-13762/2100, 5- , 16-18=-2536/169; 942, 10-12=-1082/ -98/2/15, 5-18=-03;	TOP BOT WEB -6=-16759/2545, 6-7=-167 73, 15-16=-2536/16973, 7637 276721 5-16=-0/508	CHORD CHORD S	Structura 2-0-0 oc Rigid cei 1 Row a	al wood sheathin purlins (2-6-7 m ling directly appl t midpt	g directly applid ax.): 3-9. ied or 10-0-0 or 7-12	ed or 3-9-9 oc purlins c bracing.	s, except
5-15=-26	0/49, 6-15=-611/263, 7-15=-	-450/2970, 7-13=0/-	480, 7-12=-6794/1064,						
 5-15=-200 9-12=-201 9-12=-201 9-12=-201 NOTES- 1) 2-ply truss to be connected Bottom chords connected as Bottom chords connected as follo 2) All loads are considered ply connections have be 3) Unbalanced roof live load 4) Wind: ASCE 7-10; Vult= gable end zone; cantilev 5) TCLL: ASCE 7-10; Pr=21 roof snow: Lumber DOL= exposed surfaces with sl 6) Unbalanced snow loads 7) This truss has been desi non-concurrent with other 8) Provide adequate draina 9) All plates are MT20 plate 10) This truss has been det 11) * This truss has been det Marking - Verify design 	20/49, 6-15=-611/263, 7-15=- 7/2237 ted together with 10d (0.131 s follows: 2x4 - 1 row at 0-9 d as follows: 2x6 - 2 rows sf ws: 2x4 - 1 row at 0-9-0 oc. equally applied to all plies, en provided to distribute onl ds have been considered fo 120mph Vasd=95mph; TCD er left and right exposed ; e 0.0 psf (roof live load: Lumb =1.15 Plate DOL=1.15); Cat lopes less than 0.500/12 in have been considered for ti gned for greater of min roof er live loads. rige to prevent water ponding es unless otherwise indicate signed for a 10.0 psf bottom lesigned for a live load of 20 om chord and any other me	"x3") nails as follow 0 oc, 2x6 - 2 rows aggered at 0-9-0 o except if noted as f y loads noted as (F r this design. L=6.0psf; BCDL=6 nd vertical left and er DOL=1.15 Plate egory II; Exp B; Pa accordance with IB is design. live load of 12.0 ps g. d. chord live load noi .0psf on the bottom mbers.	 460, 7-12=-6734/1064, vs: staggered at 0-9-0 oc. c. ront (F) or back (B) face ir r) or (B), unless otherwise .0psf; h=30ft; Cat. II; Exp right exposed; Lumber DC DOL=1.15); Pg=15.0 psf rtially Exp.; Ct=1.10, Lu=5 C 1608.3.4. sf or 1.00 times flat roof loat nconcurrent with any othe n chord in all areas where 	n the LOAD C indicated. B; Enclosed; DL=1.60 plate (ground snov 00-00 Rain s ad of 11.6 psl ad of 11.6 psl r live loads. a rectangle 3	ASE(S) so MWFRS (grip DOL v); Pf=16.5 urcharge f on overh -6-0 tall by	ection. Ply to envelope) =1.60 5 psf (flat applied to all angs y 2-0-0 wide		SEAL 044925 MGINEE January 21,2	WIE HILL
WarkNING - Venty design Design valid for use only wi a truss system. Before use, building design. Bracing in is always required for stabil fabrication, storage, deliver Safety Information availa	I parameters and KEAD NOTES ON th MiTek® connectors. This design the building designer must verify th dicated is to prevent buckling of ind ity and to prevent collapse with pos y, erection and bracing of trusses a ble from Truss Plate Institute, 2670	is based only upon para e applicability of design vidual truss web and/or sible personal injury and nd truss systems, see Crain Highway, Suite 20	MILER REFERENCE PAGE MIL-72 imeters shown, and is for an indiv parameters and properly incorpo chord members only. Additional I property damage. For general <u>c</u> ANS/TP11 Quality Criter 33 Waldorf, MD 20601	*/ 3 rev. 5/19/2021 vidual building co prate this design i temporary and p guidance regardin ria, DSB-89 and	into the overa ermanent brand the BCSI Buildi	SE. at aali acing ing Component		A MiTek 818 Soundside Road Edenton, NC 27932	

Job	Truss	Truss Type	Qty	Ply	KB Home 243.2939.B	
						I44459051
243_2939_B	HG1	Hip Girder	1	2		
				2	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.4	430 s Nov	30 2020 MiTek Industries, Inc. Wed Jan 20 13:35:31 2021	Page 2
		ID:XZ	SsiAHNe	IVplcdoAj	Gq3ztm9T-lqX6BpouL SNpVR7duMvbA53C6RtX52JdSHt9	EztRzw

NOTES-

12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

14) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 35-10-8 oc max. starting at 3-6-12 from the left end to 39-5-4 to connect truss(es) to back face of bottom chord.

15) Fill all nail holes where hanger is in contact with lumber.

16) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-43, 3-9=-53, 9-11=-43, 2-10=-20

Concentrated Loads (lb)

Vert: 3=-67(B) 9=-67(B) 19=-33(B) 12=-33(B) 8=-62(B) 20=-62(B) 21=-62(B) 22=-62(B) 23=-62(B) 24=-62(B) 25=-62(B) 27=-62(B) 28=-62(B) 29=-62(B) 32=-62(B) 32=-62(B) 33=-62(B) 34=-62(B) 35=-62(B) 35=

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			5-6-0					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.51 BC 0.35 WB 0.00 Matrix-P	DEFL. Vert(LL) - Vert(CT) - Horz(CT) -	in (loc) -0.05 2-4 -0.09 2-4 -0.00 3	l/defl >999 >675 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 18 lb	GRIP 197/144 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=70(LC 12) Max Uplift 3=-66(LC 16), 2=-47(LC 12)

Max Grav 3=153(LC 2), 2=279(LC 2), 4=106(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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	ł				3-0-0						
					3-0-0						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI:	2-0-0 1.15 1.15 YES 2014	CSI. TC BC WB Matriz	0.16 0.13 0.00 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.01 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 12 lb	GRIP 197/144 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=48(LC 12) Max Uplift 3=-41(LC 16), 2=-43(LC 12)

Max Grav 3=89(LC 2), 2=202(LC 2), 4=66(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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	<u> </u>	-6-0 -6-0		3-6-0 2-0-0			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.18 WB 0.02 Matrix-P	DEFL. in Vert(LL) -0.01 Vert(CT) -0.02 Horz(CT) 0.01	(loc) l/defl 6 >999 6 >999 4 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 13 lb	GRIP 197/144 FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-6-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=27(LC 13)

Max Uplift 2=-51(LC 12), 4=-19(LC 13) Max Grav 5=74(LC 7), 2=203(LC 36), 4=65(LC 35)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
 One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- 10) One H2.5A Simpson Strong-Tile connectors recommended to connect truss to bearing wails due to OPLIFT at Jt(s) 2. If connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



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		<u>3-6-0</u> 3-6-0		<u>5-6-0</u> 2-0-0		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.37 BC 0.20 WB 0.02 Matrix-S	DEFL. in Vert(LL) -0.01 Vert(CT) -0.03 Horz(CT) 0.00	(loc) l/defl L/d 2-6 >999 240 2-6 >999 180 5 n/a n/a	PLATES MT20 Weight: 26 lb	GRIP 197/144 FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x6 SP No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-6-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=Mechanical, 2=0-3-8 Max Horz 2=50(LC 9) Max Uplift 5=-26(LC 8), 2=-51(LC 8) Max Grav 5=242(LC 2), 2=339(LC 32)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 2. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 3-4=-53, 2-5=-20 Concentrated Loads (lb)

Vert: 3=-16(F) 6=-13(F) 8=-43(F)



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- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 10) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.



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		0-3-8		6-	-8-8				1	
Plate Offsets ((X,Y) [4:Edge,0)-2-0]								
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 11.6/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.26 BC 0.12 WB 0.05 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 -0.01 0.00	(loc) 1 1 5	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 27 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS	2x4 SP No.2 or 2 2x4 SP No.2 or 2 2x4 SP No.3 2x4 SP No.3	2x4 SPF No.2 2x4 SPF No.2	В Т	RACING- OP CHORD OT CHORD	Structura except ei Rigid ceil	I wood nd verti ling dire	sheathin cals. ectly appl	g directly a lied or 10-0	pplied or 6-0-0 oc purlir -0 oc bracing.	S,

REACTIONS. (size) 5=6-8-8, 2=6-8-8, 6=6-8-8 Max Horz 2=46(LC 20) Max Uplift 5=-27(LC 17), 2=-64(LC 12) Max Grav 5=107(LC 2), 2=288(LC 2), 6=266(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-10-8 to 1-1-8, Exterior(2) 1-1-8 to 3-11-0, Corner(3) 3-11-0 to 6-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) n/a

11) Non Standard bearing condition. Review required.



TRENGING BY A Mi Tek Attiliate 818 Soundside Road Edenton, NC 27932

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January 21,2021

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L	8-0-11	15-11-	5			24-0-0		
	8-0-11	7-10-1	0			8-0-11	1	
Plate Offsets (X,Y) [2:0-0-0,0-	-1-8], [2:0-1-8,0-5-3], [6:0-0-0,0-1-8], [6	:0-1-8,0-5-3]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.49 BC 0.70 WB 0.22 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.15 8-10 -0.25 6-8 0.05 6	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 112 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 2: BOT CHORD 2x4 SP No.2 or 2: WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3 , Right: 2x4 SP	x4 SPF No.2 x4 SPF No.2 No.3	BR TC BC	RACING- OP CHORD S OT CHORD F	Structural woo Rigid ceiling di	d sheathing rectly appli	g directly app ed or 10-0-0	lied or 4-1-9 oc purlins oc bracing.	
REACTIONS. (size) 2=0-3 Max Horz 2=-92 Max Uplift 2=-69 Max Grav 2=101	-8, 6=0-3-8 (LC 17) (LC 16), 6=-69(LC 17) 10(LC 2), 6=1010(LC 2)							
FORCES. (lb) - Max. Comp./Ma TOP CHORD 2-3=-1586/112, BOT CHORD 2-10=-114/1343 WEBS 4-8=-59/528, 5	ux. Ten All forces 250 (lb) or less exc 3-4=-1408/134, 4-5=-1408/134, 5-6=-1 3, 8-10=0/906, 6-8=-36/1343 8=-323/174, 4-10=-59/528, 3-10=-323/	ept when shown. 586/112 174						
 NOTES- 1) Unbalanced roof live loads hav 2) Wind: ASCE 7-10; Vult=120mp gable end zone and C-C Exteri 24-10-8 zone; cantilever left ar reactions shown; Lumber DOL- 3) TCLL: ASCE 7-10; Pr=20.0 psi roof snow: Lumber DOL=1.15 I 4) Unbalanced snow loads have t 5) This truss has been designed f non-concurrent with other live I 6) This trues has been designed f 	ve been considered for this design. bh Vasd=95mph; TCDL=6.0psf; BCDL= ior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 hd right exposed ; end vertical left and r =1.60 plate grip DOL=1.60 f (roof live load: Lumber DOL=1.15 Pla Plate DOL=1.15); Category II; Exp B; F been considered for this design. for greater of min roof live load of 12.0 loads.	=6.0psf; h=30ft; Cat. II; Ex to 12-0-0, Exterior(2) 12- ight exposed;C-C for mer te DOL=1.15); Pg=15.0 p 'artially Exp.; Ct=1.10 psf or 1.00 times flat roof	p B; Enclosed; Mi 0-0 to 15-0-0, Inte nbers and forces sf (ground snow); load of 11.6 psf o ar live loads	WFRS (envelo erior(1) 15-0-0 & MWFRS for Pf=11.6 psf (f on overhangs	ope) to 		H CARO	N. R.

- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.





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ENGINEERING BY ERENCO A Millek Attiliate 818 Soundside Road Edenton, NC 27932

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			23	-8-8					1	
Plate Offsets (X,Y) [2:0-0-0,0)-1-0], [2:0-1-8,0-5-3]								
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 11.6/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.05 BC 0.04 WB 0.09 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 1 1 15	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 131 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS	2x4 SP No.2 or 2 2x4 SP No.2 or 2 2x4 SP No.3 2x4 SP No.3 2x4 SP No.3	2x4 SPF No.2 2x4 SPF No.2		BRACING- TOP CHORD BOT CHORD	Structura except e Rigid ce	al wood nd verti iling dire	sheathin cals. ectly appl	g directly ap ied or 10-0-0	oplied or 6-0-0 oc purlins 0 oc bracing.	,

23-8-8

WEDGE Left: 2x4 SP No.3

- REACTIONS. All bearings 23-8-8.
 - (lb) Max Horz 2=96(LC 16) Max Uplift All uplift 100 lb or less at joint(s) 2, 22, 23, 25, 26, 27, 20, 19, 18,
 - 17, 16 Max Grav All reactions 250 lb or less at joint(s) 15, 2, 21, 22, 23, 25, 26, 27, 20, 19, 18, 17, 16

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-0-0, Exterior(2) 2-0-0 to 12-0-0, Corner(3) 12-0-0 to 15-0-0, Exterior(2) 15-0-0 to 23-6-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 1.5x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-9-1 to 4-0-0, Interior(1) 4-0-0 to 6-10-12, Exterior(2) 6-10-12 to 9-9-8, Interior(1) 9-9-8 to 13-0-7 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-9-1 to 3-9-1, Interior(1) 3-9-1 to 4-5-15, Exterior(2) 4-5-15 to 7-5-15, Interior(1) 7-5-15 to 8-2-13 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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