

RE: J0322-1383 Southern Touch/Lot 14 Michell Manor

# Site Information:

Customer: Project Name: J0322-1383 Lot/Block: Address: City:

Model: Subdivision: State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

Trenco

818 Soundside Rd

Edenton, NC 27932

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.

	<b>a</b>				<b>a</b>	<b>_</b>	
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	150575528	A1	3/4/2022	21	150575548	VA7	3/4/2022
2	150575529	A1A	3/4/2022	22	150575549	VA8	3/4/2022
3	150575530	A1GE	3/4/2022	23	150575550	VA9	3/4/2022
4	150575531	A2	3/4/2022	24	150575551	VA10	3/4/2022
5	150575532	A3	3/4/2022	25	150575552	VA11	3/4/2022
6	150575533	A4	3/4/2022	26	150575553	VP1	3/4/2022
7	150575534	A4A	3/4/2022	27	150575554	VP2	3/4/2022
8	150575535	A4GE	3/4/2022	28	150575555	VP3	3/4/2022
9	150575536	G1	3/4/2022	29	150575556	VP4	3/4/2022
10	150575537	G1GE	3/4/2022				
11	150575538	P1	3/4/2022				
12	150575539	P1GE	3/4/2022				
13	150575540	PB1	3/4/2022				
14	150575541	PB1GE	3/4/2022				
15	150575542	VA1	3/4/2022				
16	150575543	VA2	3/4/2022				
17	150575544	VA3	3/4/2022				
18	150575545	VA4	3/4/2022				
19	150575546	VA5	3/4/2022				
20	150575547	VA6	3/4/2022				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

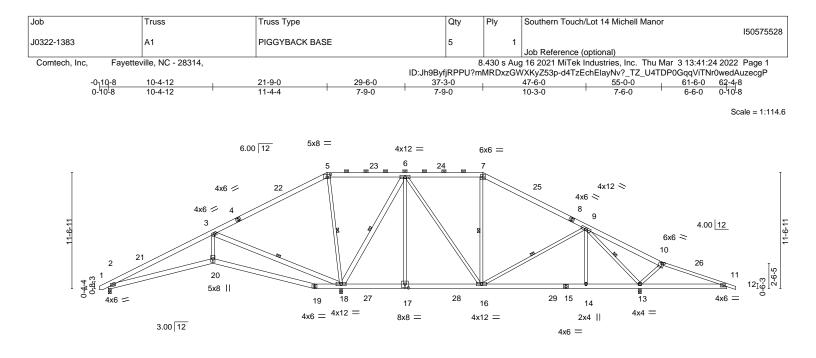
My license renewal date for the state of North Carolina is December 31, 2022

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



	10-4-12	20-6-0	23-1-12	29-6-0	37-3-0		47-6-0	52-10-4	61-6-0	
Plate Offsets (X,			- · · · -	0-4-4	7-9-0		10-3-0	J-4-4	0-7-12	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0		2-0-0 1.15 1.15 YES PI2014	<b>CSI.</b> TC 0.6 BC 0.3 WB 0.6 Matrix-S	5	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.12 19-20 -0.24 19-20 0.04 18 0.05 2-20	l/defl L/d >999 360 >999 240 n/a n/a >999 240	N	PLATES //T20 Veight: 471 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD	2x6 SP No.1 *Except* 10-12: 2x4 SP No.1 2x6 SP No.1 2x4 SP No.2 *Except* 5-18,3-18: 2x6 SP No.1				BRACING- TOP CHORI BOT CHORI WEBS	2-0-0 c D Rigid c	ural wood sheathi oc purlins (6-0-0 r ceiling directly app at midpt	nax.): 5-7. blied or 6-0-0 5-18 6-1	oc bracing. 8 9-16 7-16 3	-18 9-13
	(size) 2=0-3-8, 18=0-3-8, 7 Max Horz 2=-149(LC 10) Max Uplift 2=-13(LC 13), 18=-2 Max Grav 2=495(LC 23), 18=2	37(LC 12), 13=						(i)	HTH CA	No sin
FORCES. (Ib) TOP CHORD	- Max. Comp./Max. Ten All fo 2-3=-623/0, 3-5=-195/1123, 5 9-10=-854/1241, 10-11=-844/	6=-90/1009, 6-							SEA	
BOT CHORD	2-20=0/602, 19-20=0/601, 18-		8=-338/345, 16- <sup>-</sup>	17=-338/34	15,			Ξ.	0363	22 : E
WEBS	14-16=-10/496, 13-14=-10/49 5-18=-971/342, 6-18=-1481/3 7-16=-427/228, 3-18=-1394/2	00, 9-14=0/288,	6-17=0/365, 6-1		9,			Community Community	NGIN	L 22 ER RUIN
NOTES-								11	CAC	II BEIIN
2) Wind: ASCE and C-C Exte Exterior(2) 37 reactions sho	oof live loads have been consic 7-10; Vult=130mph Vasd=103m rior(2) -0-8-10 to 3-8-3, Interior -3-0 to 41-7-12, Interior(1) 41-7 wn; Lumber DOL=1.60 plate gr	ph; TCDL=6.0p 1) 3-8-3 to 21-9 -12 to 62-4-8 zc p DOL=1.60	sf; BCDL=6.0psl I-0, Exterior(2) 2 one; cantilever riç	1-9-0 to 26 ght expose	-1-13, Interior(1) d ;C-C for memb	26-1-13 to 37 ers and forces	RS (envelope) -3-0, s & MWFRS for			inin'
handling and Trusses ("BC qualified regis	This long span truss requires ex erection guidance, see Guide tr SI"), jointly produced by SBCA tered design professional for th dividual truss member restraint/	o Good Practice and TPI. The bu e design and in	for Handling, In ilding owner or t spection of the te	stalling & E he owner's emporary i	Bracing of Metal F s authorized ager Installation restrai	Plate Connect at shall contra nt/bracing and	ed Wood ct with a d the			
	uate drainage to prevent water									
	been designed for a 10.0 psf b						w 2.0.0 wide			
/	as been designed for a live load n the bottom chord and any oth				s where a rectan	yie 3-6-0 tall b	y ∠-0-0 wide			
	nt(s) 2 considers parallel to grai				rmula. Building d	lesigner shoul	d verify			

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 18=237, 13=203.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building

Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 March 4,2022



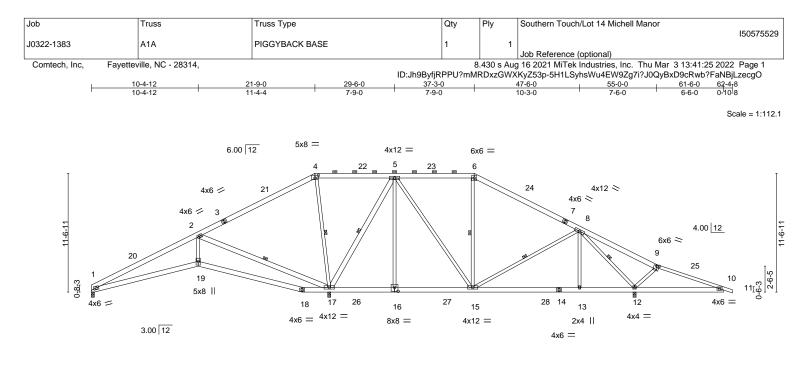
Job	Truss	Truss Type	Qty	Ply	Southern Touch/Lot 14 Michell Manor
J0322-1383	A1	PIGGYBACK BASE	5	1	150575528
00022 1000			Ŭ		Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,		8	.430 s Aug	16 2021 MiTek Industries, Inc. Thu Mar 3 13:41:24 2022 Page 2
		ID:Jh9By	fjRPPU?m	MRDxzGV	VXKyZ53p-d4TzEchElayNv?_TZ_U4TDP0GqqViTNr0wedAuzecgP

## NOTES-

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		10.1.10		23-1-1						17.0.0		50.40.4	55.0.0		
		10-4-12 10-4-12	<u>20-6-0</u> 10-1-4	21-9-0 1-3-0	29-6-0 6-4-4		37-3-0 7-9-0			47-6-0 10-3-0		52-10-4 5-4-4	2-1-12	61-6-0 6-6-0	
Plate Offsets ()		4:0-4-0,0-3-4], [9:0-3-0	0 0 121 [16:0 4	1-4-12	<u>:</u>										
	<u>, , , , , -                           </u>	4.0-4-0,0-3-4], [9.0-3-0	,0-0-12 <u>]</u> , [10.0-4	-0,0-4-6]											
LOADING (psi	f)	SPACING-	2-0-0	CSI.			DEFL.	in	(loc)	l/defl	L/d		PLATES	GRIP	
TCLL 20.0		Plate Grip DOL	1.15	TC	0.63		Vert(LL)		18-19	>999	360		MT20	244/19	0
TCDL 10.0	-	Lumber DOL	1.15	BC	0.36		Vert(CT)		18-19	>999	240				
BCLL 0.	0 *	Rep Stress Incr	YES	WB	0.68		Horz(CT)	0.04	17	n/a	n/a				
BCDL 10.0	0	Code IRC2015/	TPI2014	Matrix	<-S		Wind(LL)	0.05	1-19	>999	240		Weight: 469	lb FT =	20%
UMBER-						-	BRACING-								
TOP CHORD	2v6 SP	No.1 *Except*					TOP CHOR	D	Structu	ural wood	cheathing	directly	applied or 6-0-		evcent
OF CHOILD		4 SP No.1						D			(6-0-0 max				s, except
BOT CHORD							BOT CHOR	D					-0 oc bracing.		
WEBS		No.2 *Except*					WEBS			at midpt			-17, 8-15, 6-1	5 2-17 8-1	2
VEBO		7: 2x6 SP No.1					WEBO		1100	at mapt		4 17, 0	17,010,01	0, 2 17, 0 1	2
	J., L.														
REACTIONS.	(size)	1=0-3-8, 17=0-3-8,	12=0-3-8												
	· · ·	rz 1=-149(LC 10)													
		lift 1=-12(LC 13), 17=-	238(LC 12), 12=	-203(LC 13)											
		av 1=440(LC 23), 17=	· //	· · · ·											
TOP CHORD	9-10= 1-19=( 13-15= 4-17=-	22/0, 2-4=-195/1126, -844/958 0/601, 18-19=0/600, 1 10/496, 12-13=-10/4 971/341, 5-17=-1482/ 428/231, 2-17=-1399/	7-18=0/529, 16-1 96, 10-12=-846/8 307, 8-13=0/288	7=-340/345, 353 , 5-16=0/365	, 15-16=-340 , 5-15=-122/	)/345,	=-854/1241,								
NOTES-															
	roof live	loads have been cons	idered for this de	sign.											
		It=130mph Vasd=103			.0psf; h=15ft	t; Cat.	II; Exp C; En	closed	; MWFR	S (envelo	ope)				
and C-C Ext	terior(2) 0	-1-12 to 4-6-9, Interior	(1) 4-6-9 to 21-9	-0, Exterior(2	2) 21-9-0 to 2	26-1-1:	3, Interior(1)	26-1-1	3 to 37-3	3-0, Exte	rior(2)			A	
37-3-0 to 41	-7-12, Int	erior(1) 41-7-12 to 62-	4-8 zone; cantile	ver right exp	osed ;C-C fc	or men	bers and for	ces &	MWFRS	for reac	tions		"TH C	ABO	11,
		=1.60 plate grip DOL=										A.	ORTHO	K. JL	In's
		g span truss requires e										6.	U. FES	Oldi	This.
		guidance, see Guide									6	1	11		201
		ntly produced by SBCA									_			4	A 5
qualified reg	istered de	esign professional for t	he design and ir	spection of t	he temporar	ry insta	Illation restra	int/bra	cing and	d the		- :	CE	AL	1 =
	ndividual	truss member restrain	t/bracing. MiTek	assumes no	o responsibili	ity for	truss manufa	cture,	handling	g, erectio		: :			: =
bracing.												= :	036	322	1 2
		inage to prevent wate													1 2
,		lesigned for a 10.0 psf										1	N		1 2
		designed for a live loa				eas wh	nere a rectar	gle 3-6	5-0 tall b	y 2-0-0 w	vide	1. 1	N. En-	-cR.	· ~
		ttom chord and any ot										25	S. GI	NEF	03
, ,	( )	onsiders parallel to gra	ain value using A	NSI/TPI 1 a	ngle to grain	formu	la. Building	design	er shoule	d verify		11	10	" at	2.5
capacity of b	pearing su	urface.											11. A.	GILD	11.
Dura dala a							100 11 110				• •				

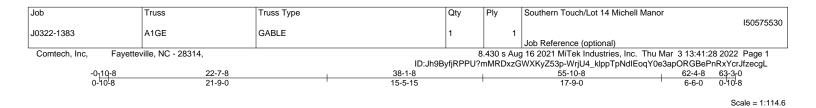
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 17=238, 12=203.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

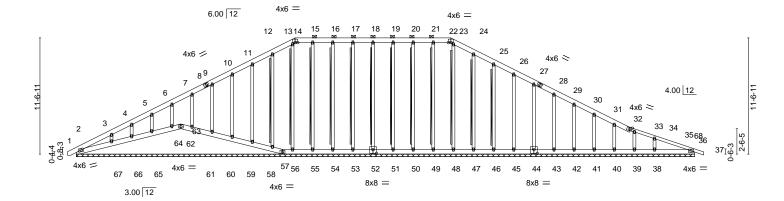
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



mmm

March 4,2022





-0 <u>-10-8</u> 0-10-8	11-3-4	<u>21-4-8</u> 10-1-4				<u>62-4-8</u> 41-0-0			<u>63-3</u> -0 0-10-8
Plate Offsets (X,Y)	[44:0-4-0,0-4-8], [52:0-4-0								
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES 12014	<b>CSI.</b> TC 0.11 BC 0.05 WB 0.14 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (I 0.00 0.00 0.01	loc) l/defl 37 n/r 37 n/r 36 n/a	L/d 120 120 n/a	MT20 24	<b>RIP</b> 14/190 FT = 20%
LUMBER- TOP CHORD 2x6 SF 33-37: BOT CHORD 2x6 SF OTHERS 2x4 SF	2x4 SP No.1 P No.1			BRACING- TOP CHORI BOT CHORI WEBS	2-1 ) Ri T-	0-0 oc purlins igid ceiling dir Brace: asten (2X) T a	(6-0-0 max.) ectly applied 2 and I braces t	rectly applied or 6-0-0 oc p : 14-22. or 10-0-0 oc bracing. 2x4 SPF No.2 - 18-52, 17-5 13-56, 12-58, 19-51, 20-50 24-47, 25-46 to narrow edge of web with h 3in minimum end distanc	3, 16-54, 15-55 ), 21-49, 23-48, 10d
(Ib) - Max H Max U	67=-142(LC 12), 38= Grav All reactions 250 lb	, 50, 49, 47, 46 117(LC 13) or less at joint(s 5, 66, 51, 50, 49	, 45, 44, 43, 42, 41, 40, ;) 2, 63, 57, 52, 53, 54, 5 9, 48, 47, 46, 45, 44, 43,	39, 36 except 55, 56, 58,	ы	race must cov	ei 90 % 01 we	ip lengur.	
TOP CHORD 2-3= 13-14 18-19 23-24	Comp./Max. Ten All for -308/128, 9-10=-90/270, 1 4=-141/400, 14-15=-138/4 9=-138/425, 19-20=-138/4 4=-150/429, 24-25=-131/3 =-194/267	0-11=-110/́328, 25, 15-16=-138 25, 20-21=-138	11-12=-131/388, 12-13 /425, 16-17=-138/425, 1 /425, 21-22=-138/425, 2	=-150/441, 7-18=-138/425,				WITH CAR	·····
<ol> <li>Wind: ASCE 7-10; V gable end zone and 37-3-0, Corner(3) 33 Lumber DOL=1.60 (</li> <li>Truss designed for v Gable End Details a</li> <li>WARNING: This lon handling and erectic Trusses ("BCSI"), jo qualified registered</li> </ol>	a loads have been conside /ult=130mph Vasd=103mp C-C Corner(3) -0-8-10 to 7-3-0 to 41-6-0, Exterior(2) olate grip DOL=1.60 wind loads in the plane of ti is applicable, or consult qu og span truss requires extro on guidance, see Guide to intly produced by SBCA a design professional for the al truss member restraint/b	bh; TCDL=6.0ps 3-6-0, Exterior( 41-6-0 to 62-4 the truss only. Italified building reme care and of Good Practice nd TPI. The built e design and ins	F; BCDL=6.0psf; h=15ft; 2) 3-6-0 to 21-9-0, Corni- 8 zone;C-C for member For studs exposed to wir designer as per ANSI/T experience for proper ar for Handling, Installing & Iding owner or the owne spection of the temporary	er(3) 21-9-0 to 26-1 is and forces & MW nd (normal to the fa Pl 1. Id safe handling and & Bracing of Metal F n's authorized agen y installation restrail	-13, Exte FRS for r ce), see s d erection late Cont t shall co nt/bracing	erior(2) 26-1-1 reactions show Standard Indu n. For general nected Wood ontract with a g and the	3 to wn; ustry	A. GIL	BERTHUM
5) Provide adequate d	rainage to prevent water p T20 unless otherwise indic							March 4	4,2022
WARNING - Verify de Design valid for use onl a truss system. Before u building design. Bracin is always required for st fabrication, storage, del	sign parameters and READ NOTE y with MiTek® connectors. This d use, the building designer must ve g indicated is to prevent buckling ability and to prevent collapse wit very, erection and bracing of trus ailable from Truss Plate Institute,	S ON THIS AND IN esign is based only erify the applicability of individual truss w h possible personal ses and truss syste	upon parameters shown, and of design parameters and pro reb and/or chord members only injury and property damage. ms, see <b>ANSI/TPI1 C</b>	is for an individual buildir perly incorporate this der /. Additional temporary a For general guidance reg Quality Criteria, DSB-89	ig compone sign into the ind perman arding the	ent, not e overall lent bracing	nent	AM 818 Soundside Road Edenton, NC 27932	BY ICCO ITek Affiliate

Job	Truss	Truss Type	Qty	Ply	Southern Touch/Lot 14 Michell Manor
J0322-1383	A1GE	GABLE	1	1	150575530
30322-1363	AIGE	GABLE	1	'	Job Reference (optional)
Comtech, Inc,	Fayetteville, NC - 2	8314,		3.430 s Au	g 16 2021 MiTek Industries, Inc. Thu Mar 3 13:41:28 2022 Page 2
			ID:Jh9ByfjRPPU?	mMRDxzG	WXKyZ53p-WrjU4_klppTpNdlEoqY0e3apORGBePnRxYcrJfzecgL

### NOTES-

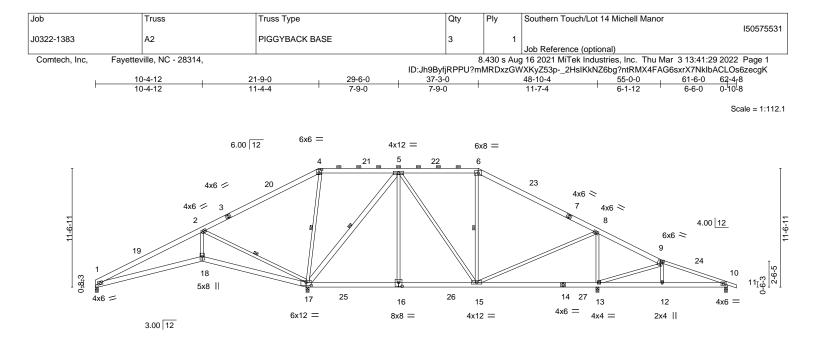
7) Gable requires continuous bottom chord bearing.

8) Gable studs spaced at 2-0-0 oc.

- Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 57, 52, 53, 54, 55, 58, 59, 60, 61, 62, 64, 65, 66, 51, 50, 49, 47, 46, 45, 44, 43, 42, 41, 40, 39, 36 except (jt=lb) 67=142, 38=117.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev, 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

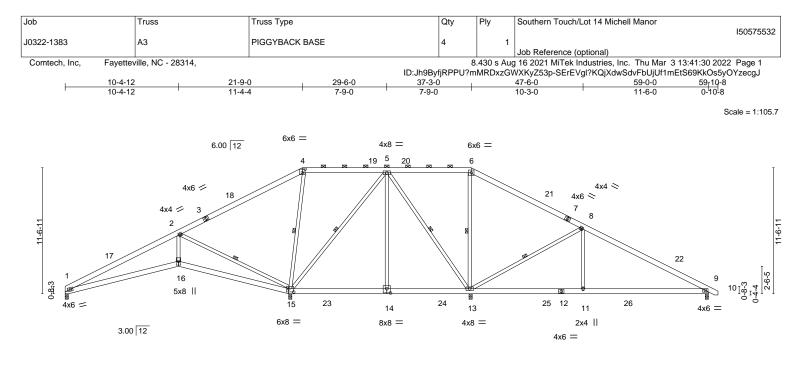




	10-4-12 10-4-12	20-6-0 10-1-4	20-7-12 0-1-12	29-6-0 8-10-4		37-3-0 7-9-0			48-10-4 11-7-4		55-0-0 6-1-12		-6-0 6-0
Plate Offsets (X,Y)	[4:0-3-0,0-4-0], [9:0-2-8,0	-2-8], [16:0-4-0	),0-4-8], [17:(	0-6-0,0-3-8]									
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TI	2-0-0 1.15 1.15 YES Pl2014	<b>CSI.</b> TC BC WB Matrix	0.63 0.33 0.69 <-S		DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.10 -0.21 0.03	(loc) 1-18 1-18 10 1-18	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	M	<b>_ATES</b> T20 eight: 455 lb	<b>GRIP</b> 244/190 FT = 20%
9-11: 2 BOT CHORD 2x6 SF WEBS 2x4 SF	P No.1 *Except* 2x4 SP No.1 P No.1 P No.2 *Except* 2x6 SP No.1					BRACING- TOP CHOR BOT CHOR WEBS	D	2-0-0 o Rigid c	c purlins (	6-0-0 max.) ctly applied	): 4-6. or 6-0-0 o		oc purlins, except
(Ib) - Max H Max U	earings 0-3-8. Horz 1=-149(LC 10) Jplift All uplift 100 lb or le Grav All reactions 250 lb 24)							(LC 24)	, 10=446(L	_C			
TOP CHORD         1-2= 9-10           BOT CHORD         16-1           WEBS         4-17	. Comp./Max. Ten All foi 261/110, 2-4=-105/1120, )=-515/100 7=-88/387, 15-16=-88/387 '=-971/305, 5-17=-1386/26 )=0/485, 6-15=-393/196, 2	4-5=0/774, 5-6 7, 12-13=-13/41 34, 5-15=-92/46	5=-506/333, 6 3, 10-12=-16 51, 8-15=-6/6	6-8=-710/26 0/421 654, 8-13=-1	4, 8-9 220/3								
<ol> <li>Wind: ASCE 7-10; and C-C Exterior(2) 37-3-0 to 41-7-12, l plate grip DOL=1.6( WARNING: This lo handling and erectir Trusses ("BCSI"), jc qualified registered permanent individua bracing.</li> <li>Provide adequate d 5) This truss has been will fit between the l To struss has been will fit between the l Capacity of bearing B Provide mechanical (jt=lb) 17=172, 13=<sup>-1</sup> </li> </ol>	ong span truss requires ext on guidance, see Guide to ointly produced by SBCA a design professional for th al truss member restraint// drainage to prevent water p in designed for a 10.0 psf b en designed for a live load bottom chord and any othe ( considers parallel to grain surface. I connection (by others) of	ph; TCDL=6.0p ) 4-6-9 to 21-9- 8 zone;C-C for reme care and Good Practice and TPI. The bu e design and in pracing. MiTek ponding. ottom chord live of 20.0psf on ti er members, wi n value using A truss to bearin	sf; BCDL=6. 0, Exterior(2 members ar experience + for Handling uilding owner spection of t assumes no e load nonco he bottom ch th BCDL = 1 NSI/TPI 1 ar g plate capa	2) 21-9-0 to and forces & for proper a g, Installing or the own he tempora o responsibi nord in all an 0.0psf. agle to grain ble of withs	26-1- MWF nd sa & Bra er's a ry ins lity fo th any reas v form	13, Interior(1) RS for reactio ife handling ar icing of Metal uthorized age tallation restra r truss manufa y other live loa vhere a rectan ula. Building ng 100 lb uplift	26-1-1: ns sho Plate C nt shall int/brad icture, I ids. igle 3-6 designe	3 to 37-3 wn; Lum tion. For contract contract ing and handling -0 tall b er should t(s) 1, 10	3-0, Exteria ber DOL= general d Wood t with a the t, erection, y 2-0-0 wid d verify D except	or(2) 1.60		in nu	EER RUU
, , , , , , , , , , , , , , , , , , ,				•		<u> </u>							

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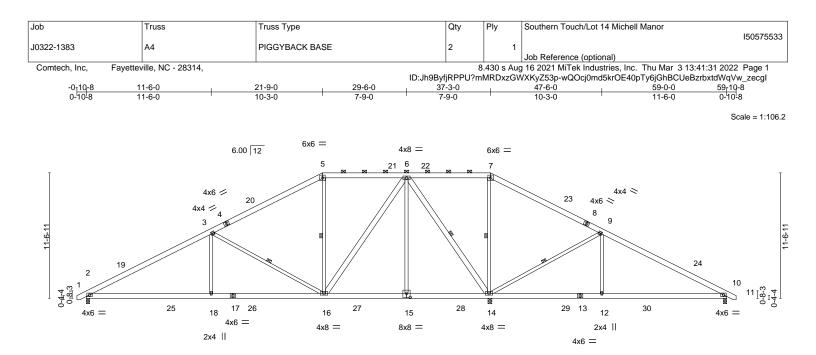
	-4-12	20-6-0	29-6-0	37-4-0			47-6-0		59-0-0	
	-4-12	10-1-4	9-0-0	7-10-0			10-2-0		11-6-0	
Plate Offsets (X,Y)	[4:0-3-0,0-4-0], [14:0	J-4-0,0-4-8 <u>]</u> , [15:0-5	-8,0-3-8]							
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0	SPACING- Plate Grip DC Lumber DOL Rep Stress Ir	1.15 ncr YES	CSI. TC 0.63 BC 0.38 WB 0.77	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.10 -0.23 0.04	(loc) 9-11 9-11 9	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC20	15/TPI2014	Matrix-S	Wind(LL)	0.06	9-11	>999	240	Weight: 426 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SI BOT CHORD 2x6 SI WEBS 2x4 SI REACTIONS. All b	P No.1			BRACING- TOP CHOR BOT CHOR WEBS		2-0-0 o Rigid c	c purlins	(10-0-0 max ectly applied	lirectly applied or 6-0-0 ( ĸ.): 4-6. l or 6-0-0 oc bracing. 2-15, 4-15, 5-15, 5-13, (	
Max L Max C FORCES. (lb) - Max. TOP CHORD 1-2= BOT CHORD 1-16 9-11 WEBS 2-16	All reactions 2: 24) Comp./Max. Ten A -351/92, 2-4=-107/10 =-60/274, 15-16=-59, =-17/558	50 lb or less at joint All forces 250 (lb) or )77, 4-5=0/739, 5-6= /269, 14-15=-392/26 354, 4-15=-948/323	except 15=-219(LC 12), 1 (s) except 1=389(LC 23), eless except when shown. =0/573, 6-8=0/708, 8-9=-7 61, 13-14=-392/261, 11-13 , 5-15=-690/107, 5-14=0/2 356, 8-11=0/523	15=2162(LC 23), 1 40/169 3=-17/558,			S), 9=691	(LC		
and C-C Exterior(2) Exterior(2) 37-3-0 to Lumber DOL=1.60 3) Provide adequate d 4) This truss has been 5) * This truss has bee will fit between the I 6) Bearing at joint(s) 1 capacity of bearing	Vult=130mph Vasd=1 0-1-12 to 4-6-9, Inte o 43-5-10, Interior(1) plate grip DOL=1.60 rainage to prevent with designed for a 10.0 en designed for a live pottom chord and any considers parallel to surface. I connection (by other =102.	03mph; TCDL=6.0 rior(1) 4-6-9 to 21-9 43-5-10 to 59-8-10 ater ponding. psf bottom chord liv load of 20.0psf on t y other members, w grain value using A rs) of truss to bearin	psf; BCDL=6.0psf; h=15ft; -0, Exterior(2) 21-9-0 to 2 zone;C-C for members an re load nonconcurrent with the bottom chord in all are	7-11-11, Interior(1) d forces & MWFR any other live loa as where a rectan ormula. Building c nding 100 lb uplift	) 27-11 S for re ds. gle 3-6 designe at join	6-0 tall by er should t(s) 1 ex	r7-3-0, s shown; y 2-0-0 w d verify ccept (jt=l	ide	ORTH CA SEA 0363	• –

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

SEAL 036322 March 4,2022

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# TRENGINEERING BY A MiTek Affiliate 818 Soundside Road Edenton, NC 27932



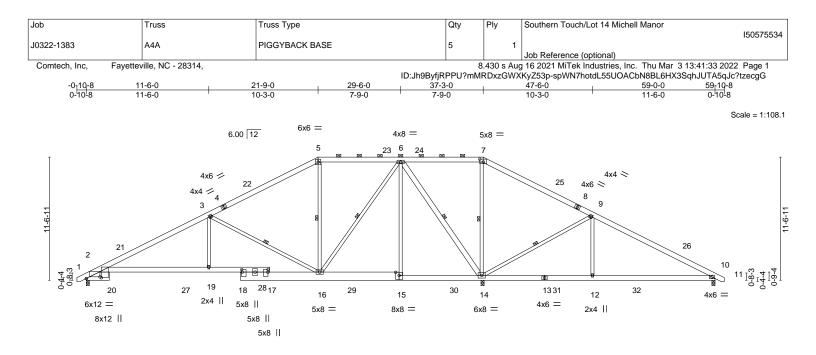
F		11-6-0		21-9-0		29-6-0		37-2-4	4 37	7-3-0	47-6-			59-0-0	
Plate Offsets (		11-6-0 5:0-4-0,0-4-8]		10-3-0		7-9-0		7-8-4	0-	-0 <mark>-</mark> 12	10-3-	-0		11-6-0	
	<u>, , , , ,                          </u>	3.0-4-0,0-4-0]											1		
OADING (ps CLL 20.	· ·	SPACING Plate Grip		2-0-0 1.15	CSI. TC	0.63		DEFL. Vert(LL)		(loc) 2-18	l/defl >999	L/d 360		PLATES MT20	<b>GRIP</b> 244/190
CDL 10.	-	Lumber D		1.15	BC	0.49		Vert(CT)		2-18	>999	240			
CLL 0. CDL 10.	.0 *	Rep Stres Code IRC		YES	WB Matri	0.88 x-S		Horz(CT) Wind(LL)	0.04	14 2-18	n/a >999	n/a 240	,	Weight: 452 lb	FT = 20%
	.0		2010/11	12011	matri	~ 0		Wind(EE)	0.00	2 10	2000	210			11 - 2070
JMBER-								BRACING-		_					
OP CHORD								TOP CHOR	D			sheathing	directly ap	oplied or 4-7-14	oc purlins,
T CHORD	2x6 SP N	lo.1 lo.2 *Except*								except		(6-0-0 ma	v)·57		
-03		10.2 Except 1: 2x6 SP No.1						BOT CHOR	D					oc bracing.	
	0 10,0 14	. 200 01 110.1						WEBS			at midpt			16, 6-14, 7-14, 9	9-14
ACTIONS.	(size)	2=0-3-8, 14=	=0-4-4, 1	0=0-3-8									, -		
	Max Hor	z 2=146(LC 11	1)												
		ft 2=-106(LC 1													
	Max Gra	v 2=1292(LC 2	23), 14=3	3309(LC 2), 10	=543(LC 24	)									
DRCES. (Ib DP CHORD		omp./Max. Ten )33/440, 3-5=-1													
	9-10=-4		1002/300	, 5-0=-750/412	., 0-7 =0/101	5, 7-5=-74/12	. 14,								
OT CHORD		269/1727, 16-18	8=-269/1	727, 12-14=-2	90/246, 10- <sup>-</sup>	12=-290/246									
EBS	3-18=0/	/520, 3-16=-118	82/350, 6	6-16=-228/104	1, 6-15=0/42	24, 6-14=-195	53/431	,							
	7-14=-9	926/295, 9-14=∙	-1223/35	9, 9-12=0/521											
OTES-															
	t roof live la	oads have beer	n considu	ared for this de	sian										
		t=130mph Vasi				0.0psf: h=15ft	: Cat. I	II: Exp C: En	closed	: MWFR	S (envelo	(eac			
		-8-10 to 3-8-3,										. ,			
		3-5-10, Interior													CT.
Lumber DO	L=1.60 pla	te grip DOL=1.	60											11111 00	in the
		nage to preven											1	THUA	ROUL
		signed for a 10												M JEGG	in Init
		designed for a l					eas wh	nere a rectar	gle 3-6	5-0 tall b	y 2-0-0 w	ide	13	+ FEUU	NO NO
		tom chord and onnection (by of					ondina		ot ioin	+(0) 11 0	woont (it		c.		
2=106, 10=			uiers) Of	indes to beam	y plate capa		grinning		at juin	1(5) 14 6	svcehr (li=	-iu)	2 1	<b>`</b>	
		Piggyback Tru	uss Conr	ection Detail fo	or Connectio	on to base tru	iss as	applicable o	r consi	ult qualif	fied buildi	na	z :	SEA	L :
designer.		99,5461 110					55 45 1			an quam			Ξ 🕴	0363	22
														0303	<u> </u>

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

annun HIT The man 036322 C G 1000 minut March 4,2022

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A MITek Atfilla 818 Soundside Road Edenton, NC 27932



⊢	11-6-0	21-9-0		29-6-0	37-2-4	37-3		47-6-0		59-0-0	
	11-6-0			7-9-0	7-8-4	-0-0		10-3-0		11-6-0	
Plate Offsets (X	, 1) [2:0-0-14, 1-5-0], [2:	:1-3-7,0-1-14], [7:0-4	1-0,0-3-4], [15	20-4-0,0-3-8	], [17:0-3-1,0-0-3],	[18:0-2	-14,0-0	-9]			
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip D	OL 1.15	TC	0.69	Vert(LL)	-0.19	2-19	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	_ 1.15	BC	0.61	Vert(CT)	-0.40	2-19	>999	240		
BCLL 0.0	* Rep Stress I	ncr YES	WB	0.94	Horz(CT)	0.10	14	n/a	n/a		
BCDL 10.0	Code IRC20	)15/TPI2014	Matrix	-S	Wind(LL)	0.15	2-19	>999	240	Weight: 474 lb	FT = 20%
LUMBER-	·				BRACING-						
FOP CHORD	2x6 SP No.1				TOP CHOP	RD	Structu	ural wood	sheathing d	directly applied or 4-8-13	oc purlins,
BOT CHORD	2x6 SP 2400F 2.0E *Exce	pt*					except				
	2-20,15-18: 2x10 SP No.1						2-0-0 0	oc purlins	(6-0-0 max.	.): 5-7.	
WEBS	2x4 SP No.2 *Except*				BOT CHOP	RD	Rigid c	ceiling dire	ectly applied	l or 6-0-0 oc bracing.	
	6-14: 2x6 SP No.1				WEBS		1 Row	at midpt		3-16, 5-16, 6-16, 6-14, 7	7-14, 9-14
	(size) 2=0-3-8, 14=0- Max Horz 2=148(LC 11) Max Uplift 2=-90(LC 12), Max Grav 2=1131(LC 23)	14=-57(LC 12), 10=-									
FORCES. (lb)	- Max. Comp./Max. Ten	All forces 250 (lb) o	r less except	when showr	1.						
TOP CHORD	2-3=-1905/437, 3-5=-636	6/300, 5-6=-421/358	, 6-7=-45/149	4, 7-9=-172	/1731,						
	9-10=-173/976										
BOT CHORD	2-19=-281/1609, 16-19=	-263/1609, 15-16=-3	379/255, 14-1	5=-381/254,	12-14=-809/202,						
	10-12=-809/202										
WEBS	3-19=0/635, 3-16=-1395/	/403, 5-16=-294/156	6, 6-16=-250/1	159, 6-15=0	0/444,						
	6-14=-2089/461, 7-14=-1	175/339, 9-14=-123	32/361, 9-12=	0/522							
NOTES-											
NOTES-			sian								

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 21-9-0, Exterior(2) 21-9-0 to 27-11-11, Interior(1) 27-11-11 to 37-3-0, Exterior(2) 37-3-0 to 43-5-10, Interior(1) 43-5-10 to 59-8-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

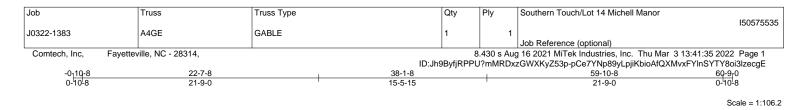
3) Provide adequate drainage to prevent water ponding.

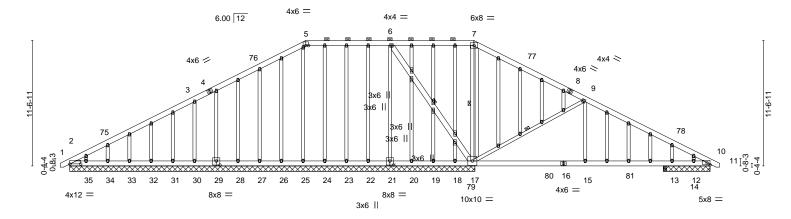
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14 except (jt=lb) 10=226.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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-0 <u><sub>1</sub>10-8</u> 0-10-8	<u>22-7-8</u> 21-9-0		38-1-8 15-5-15			-10-8 I-9-0	60-9 <sub>1</sub> 0 0-10-8
Plate Offsets (X,Y)	[2:1-0-0,0-0-4], [21:0-4-0,0-4-8], [29:0-4	-0,0-4-8]					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.54 BC 0.50 WB 0.65 Matrix-S	Vert(LL) -0.05		9 360 9 240 ′a n/a	PLATES MT20 Weight: 628 lb	<b>GRIP</b> 244/190 FT = 20%
6-17			BRACING- TOP CHORD BOT CHORD WEBS	2-0-0 oc pur	lins (6-0-0 max.): directly applied of	rectly applied or 6-0-0 5-7. or 10-0-0 oc bracing. i-17, 7-17, 9-17	oc purlins, except
(Ib) - Max Max	bearings 37-4-0 except (jt=length) 10=4-3 (Horz 2=228(LC 12) Uplift All uplift 100 lb or less at joint(s) 2 21=-365(LC 19), 17=-416(LC 13), 3 (Grav All reactions 250 lb or less at joint 31, 32, 33, 34, 20, 19, 13 except 2= 10=448(LC 1), 35=521(LC 23), 12=	, 10, 34, 14 except 30=-5: 55=-374(LC 12), 18=-271( (s) 21, 22, 23, 24, 25, 26, -411(LC 1), 30=797(LC 1)	26(LC 12), LC 3), 13=-453(LC 2), 27, 28, , 17=2054(LC 1),				
TOP CHORD         2-3           9-7         9-7           BOT CHORD         2-7           30         24           19         13           WEBS         3-3	ax. Comp./Max. Ten All forces 250 (lb) of 3=-1111/538, 3-5=-1123/904, 5-6=-855/94 10=-1283/682 35=-301/855, 34-35=-301/855, 33-34=-301 -31=-301/855, 28-30=-301/855, 27-28=-30 -25=-301/855, 23-24=-301/855, 22-23=-30 -20=-301/855, 18-19=-301/855, 17-18=-30 -14=-361/1003, 12-13=-361/1003, 10-12=- 30=-728/686, 6-21=-304/468, 6-17=-1142/ 15=0/421	7, 6-7=-304/535, 7-9=-398 /855, 32-33=-301/855, 31 /1855, 26-27=-301/855, 2 1/855, 21-22=-301/855, 2 1/855, 15-17=-361/1003, 361/1003	8/499, -32=-301/855, /5-26=-301/855, /0-21=-301/855, 14-15=-361/1003,			TH CA	Round
<ol> <li>Wind: ASCE 7-10 gable end zone a 37-3-0, Corner(3) shown; Lumber D</li> <li>Truss designed fo Gable End Details</li> <li>Provide adequate</li> <li>All plates are 2x4</li> <li>Gable studs spac</li> <li>This truss has be</li> <li>* This truss has be will fit between the</li> </ol>	en designed for a 10.0 psf bottom chord liv een designed for a live load of 20.0psf on e bottom chord and any other members, w	psf; BCDL=6.0psf; h=15ft; r(2) 3-8-3 to 21-9-0, Corno 9-8-10 zone;C-C for men For studs exposed to win g designer as per ANSI/Ti re load nonconcurrent with the bottom chord in all are	er(3) 21-9-0 to 26-1-13, nbers and forces & MWF nd (normal to the face), s PI 1. n any other live loads.	Exterior(2) 26- RS for reactionsee Standard	1-13 to ns ndustry	CA. C	EEP.K
Design valid for use a truss system. Befo building design. Bra is always required fo fabrication, storage,	v design parameters and READ NOTES ON THIS AND only with MITek® connectors. This design is based on re use, the building designer must verify the applicabil cing indicated is to prevent buckling of individual truss r stability and to prevent collapse with possible persor delivery, erection and bracing of trusses and truss sys available from Truss Plate Institute, 2670 Crain High	ly upon parameters shown, and ity of design parameters and pro web and/or chord members only ial injury and property damage. tems, see <b>ANSI/TPI1 C</b>	is for an individual building con perly incorporate this design in . Additional temporary and pe For general guidance regarding Juality Criteria, DSB-89 and E	nponent, not to the overall rmanent bracing g the	mponent	818 Soundside Edenton, NC 27	

[	Job	Truss	Truss Type	Qty	Ply	Southern Touch/Lot 14 Michell Manor
	10000 4000	1.105	CARLE			150575535
	J0322-1383	A4GE	GABLE	1	1	Job Reference (optional)
- 1						
	Comtech, Inc, Fayette	ville, NC - 28314,		8	.430 s Aug	16 2021 MiTek Industries, Inc. Thu Mar 3 13:41:35 2022 Page 2
			ID:Jh9	ByfjRPPU	?mMRDx	zGWXKyZ53p-pCe7YNp89yLpjiKbioAfQXMvxFYInSYTY8oi3lzecgE

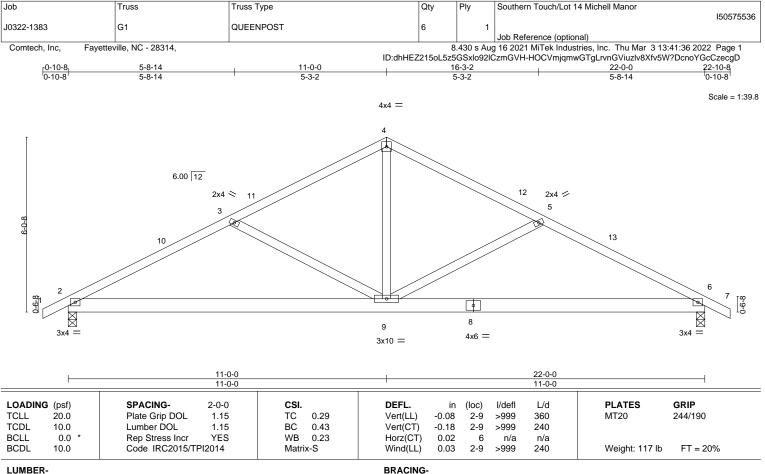
### NOTES-

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 34, 14 except (jt=lb) 30=526, 21=365, 17=416, 35=374, 18=271, 13=453, 12=396.

Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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TOP CHORD

BOT CHORD

### LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WFBS

2x4 SP No.2 (size) 6=0-3-8, 2=0-3-8 Max Horz 2=77(LC 11)

Max Uplift 6=-66(LC 13), 2=-66(LC 12) Max Grav 6=930(LC 1), 2=930(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-1425/377, 3-4=-1076/286, 4-5=-1076/286, 5-6=-1425/377
- BOT CHORD 2-9=-249/1194 6-9=-258/1194

WEBS 3-9=-365/248, 4-9=-76/632, 5-9=-365/248

### NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13, Interior(1) 15-4-13 to 22-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.

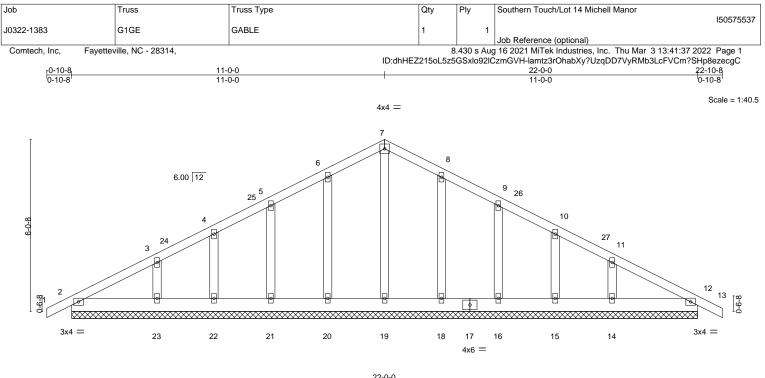


Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.08	Vert(LL)	0.00	12	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT)	0.00	12	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.06	Horz(CT)	0.00	12	n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 133 lb	FT = 20%

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 22-0-0.

Max Horz 2=120(LC 16) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 12, 2, 20, 21, 22, 18, 16, 15 except 23=-108(LC 12), 14=-106(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 12, 2, 19, 20, 21, 22, 23, 18, 16, 15, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 11-0-0, Corner(3) 11-0-0 to 15-4-13, Exterior(2) 15-4-13 to 22-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 2, 20, 21, 22, 18, 16, 15 except (jt=lb) 23=108, 14=106.

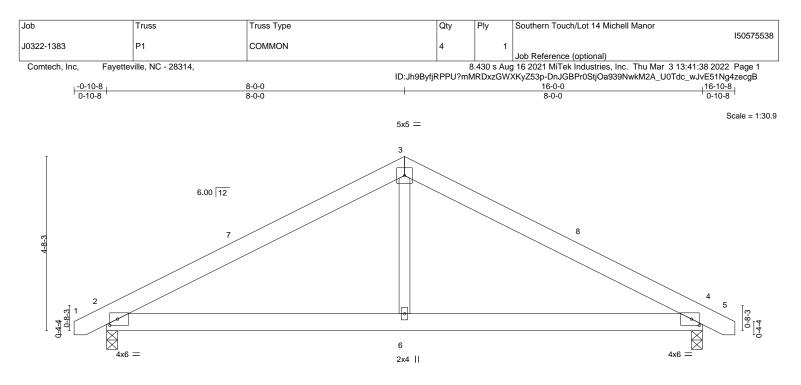


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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<b>⊢</b>	<u>8-0-0</u> 8-0-0						<u>16-0-0</u> 8-0-0		
Plate Offsets (X,Y) [2	2:0-2-8,0-2-0], [4:0-2-8,0-2-0]						000		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.29 BC 0.24 WB 0.13 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.06 -0.05 0.01	(loc) 4-6 4-6 4	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 90 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP 1 BOT CHORD 2x6 SP 1 WEBS 2x4 SP 1	No.1		BRACING- TOP CHOP BOT CHOP	RD				rectly applied or 6-0-0 or 9-5-12 oc bracing.	oc purlins.
Max Up	) 2=0-3-8, 4=0-3-8 yrz 2=-57(LC 10) viift 2=-142(LC 9), 4=-142(LC 8) av 2=680(LC 1), 4=680(LC 1)								

TOP CHORD	2-3=-876/845, 3-4=-876/843
BOT CHORD	2-6=-619/679, 4-6=-619/679

BOT CHORD 2-6=-619/679, 4-6=-619/6	BOT CHORD	2-6=-619/679,	4-6=-619/6
------------------------------------	-----------	---------------	------------

WEBS 3-6=-478/381

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 8-0-0, Exterior(2) 8-0-0 to 12-4-13, Interior(1) 12-4-13 to 16-8-10 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

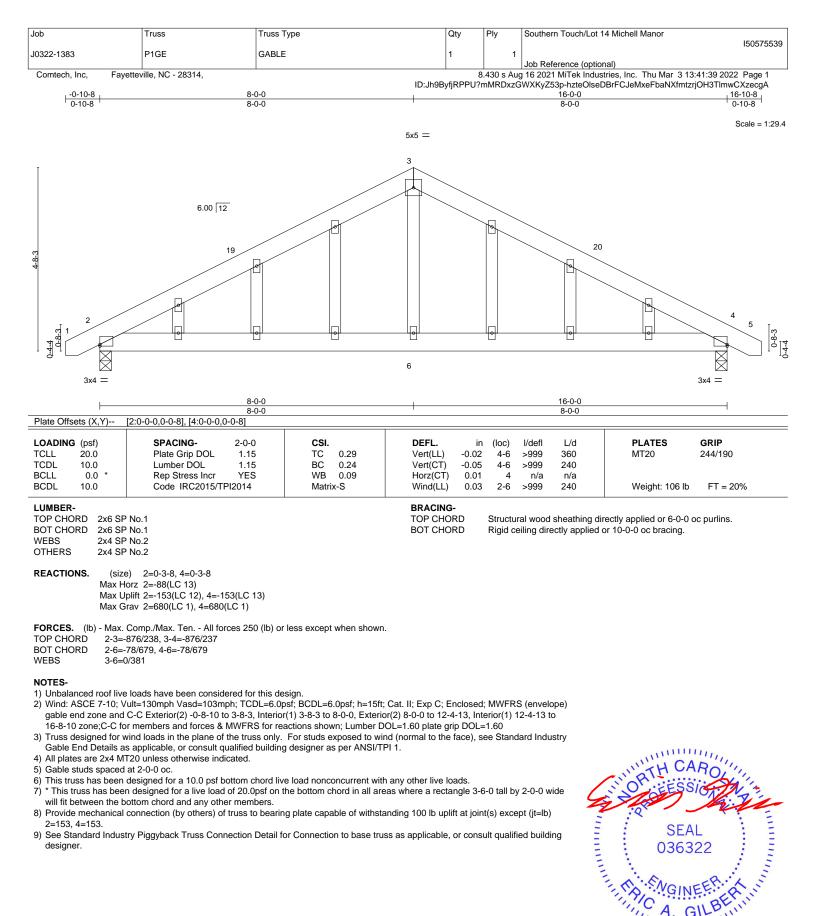
4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=142, 4=142.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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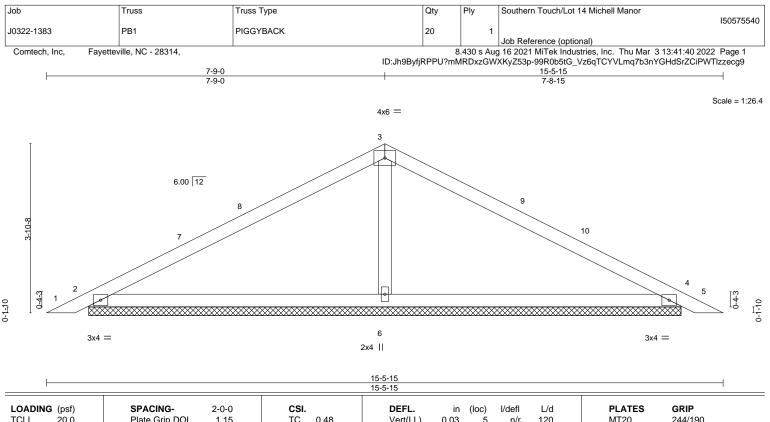


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Edenton, NC 27932

March 4,2022



LOADING(psf)TCLL20.0TCDL10.0BCLL0.0BCDL10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.48 BC 0.33 WB 0.08 Matrix-S	DEFL.         in         (loc)         l/defl         L/d         PLATES         GRIP           Vert(LL)         0.03         5         n/r         120         MT20         244/190           Vert(CT)         0.05         5         n/r         120         MT20         244/190           Horz(CT)         0.00         4         n/a         n/a         M20         244/190
LUMBER- TOP CHORD 2x4 SI	<sup>o</sup> No.1		BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. (size) 2=13-6-13, 4=13-6-13, 6=13-6-13 Max Horz 2=-48(LC 10)

Max Uplift 2=-42(LC 12), 4=-51(LC 13)

Max Grav 2=291(LC 23), 4=291(LC 24), 6=586(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 3-6=-379/187

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-15 to 4-8-11, Interior(1) 4-8-11 to 7-9-0, Exterior(2) 7-9-0 to 12-1-12, Interior(1) 12-1-12 to 15-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

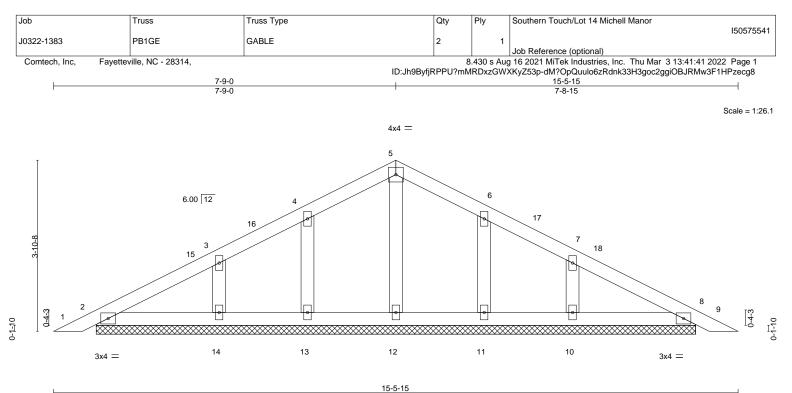
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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# 15-5-15

TCDL	20.0 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI. TC BC	0.07 0.04	DEFL. Vert(LL) Vert(CT)	in 0.00 0.00	(loc) 8 9	l/defl n/r n/r	L/d 120 120	PLATES MT20	<b>GRIP</b> 244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2015/T	YES PI2014	WB Matri	0.04 x-S	Horz(CT)	0.00	8	n/a	n/a	Weight: 61 lb	FT = 20%
LUMBER- TOP CHOR BOT CHOR OTHERS		No.1				BRACING- TOP CHOR BOT CHOR	D				ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins.

REACTIONS. All bearings 13-6-13.

(lb) -Max Horz 2=75(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10 Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-3-15 to 4-8-11, Exterior(2) 4-8-11 to 7-9-0, Corner(3) 7-9-0 to 12-1-12, Exterior(2) 12-1-12 to 15-2-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

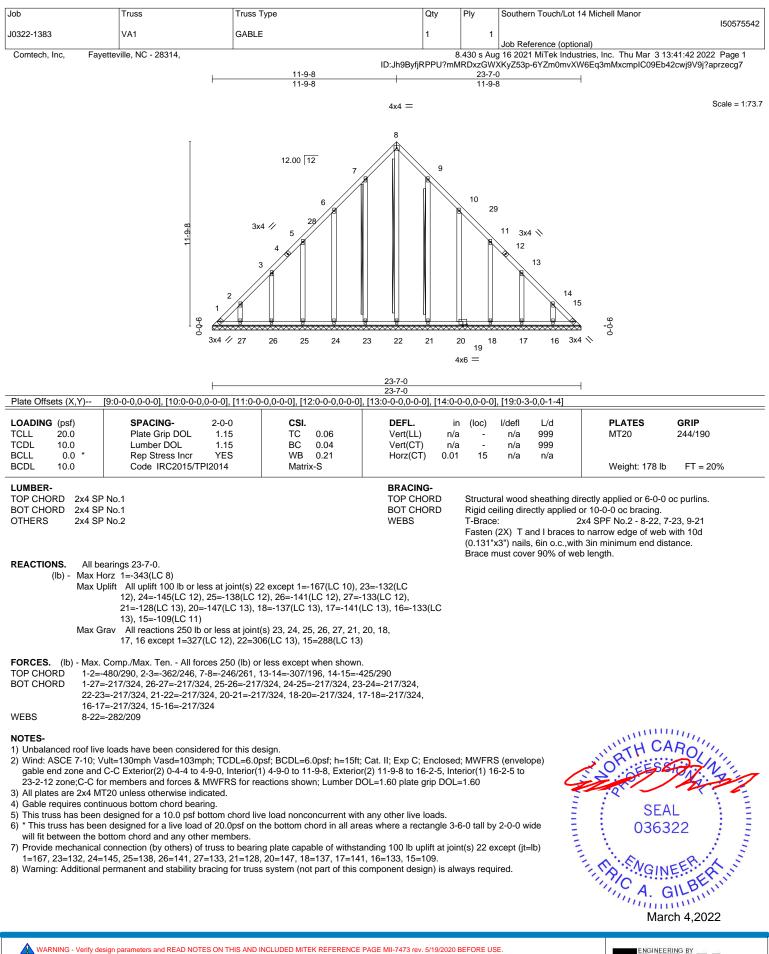
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 14, 11, 10

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

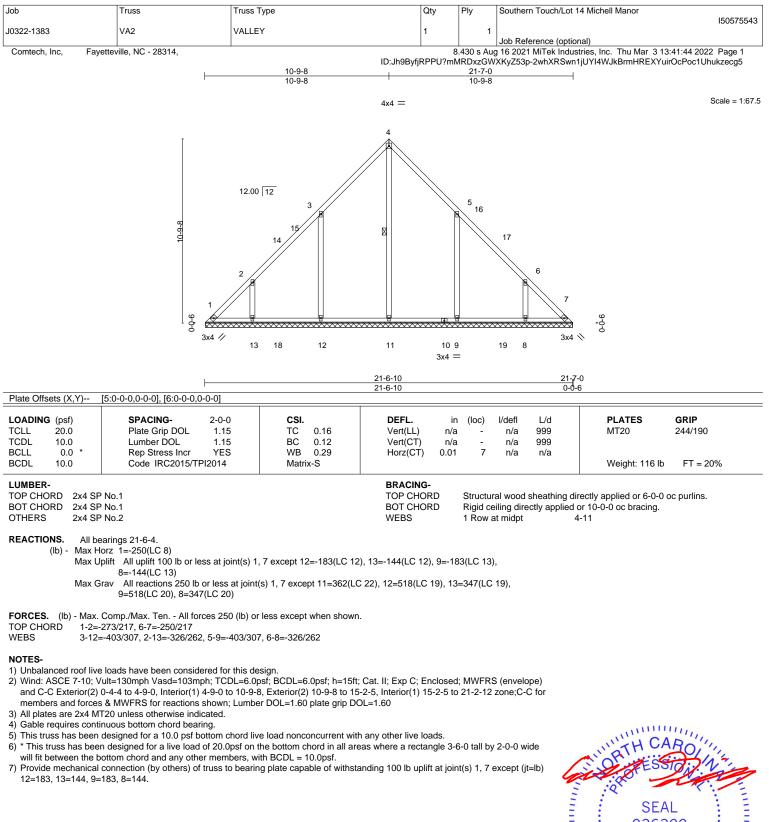


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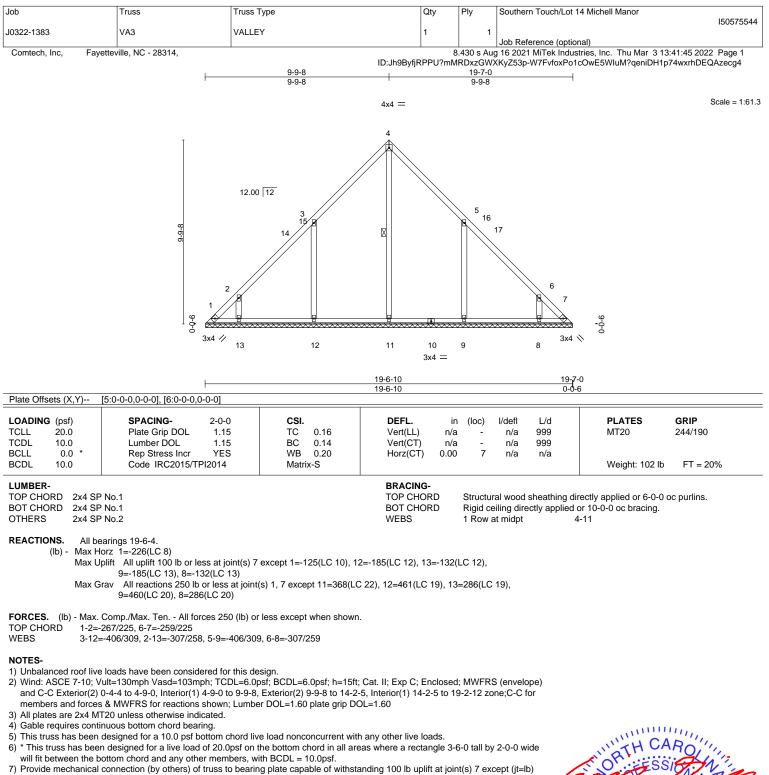


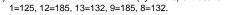


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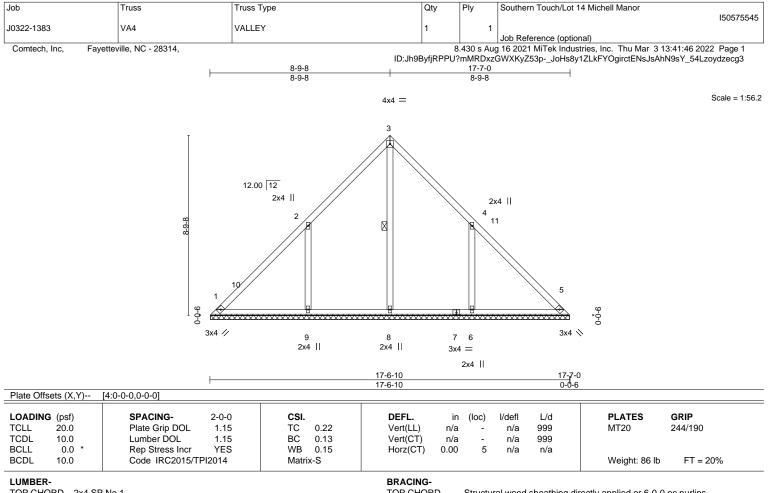






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LUMBER-		BRACING-		
TOP CHORD	2x4 SP No.1	TOP CHORD	Structural wood sheathing	directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.1	BOT CHORD	Rigid ceiling directly applied	d or 10-0-0 oc bracing.
OTHERS	2x4 SP No.2	WEBS	1 Row at midpt	3-8

REACTIONS. All bearings 17-6-4.

(lb) - Max Horz 1=-202(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-213(LC 12), 6=-213(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=345(LC 22), 9=523(LC 19), 6=522(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-9=-457/338, 4-6=-457/338

### NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-8, Interior(1) 4-9-8 to 8-9-8, Exterior(2) 8-9-8 to 13-2-5, Interior(1) 13-2-5 to 17-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

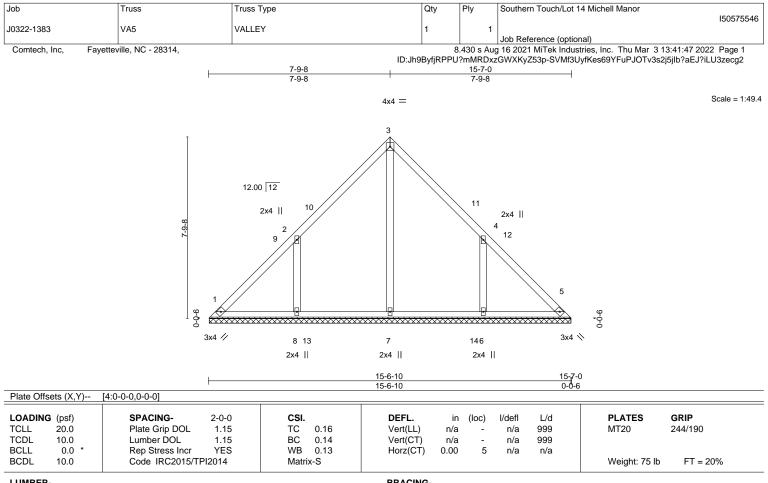
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=213, 6=213.



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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



# LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-6-4.

(lb) - Max Horz 1=-178(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-186(LC 12), 6=-186(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=348(LC 22), 8=449(LC 19), 6=449(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-403/309, 4-6=-403/309

### NOTES-

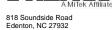
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 7-9-8, Exterior(2) 7-9-8 to 12-2-5, Interior(1) 12-2-5 to 15-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

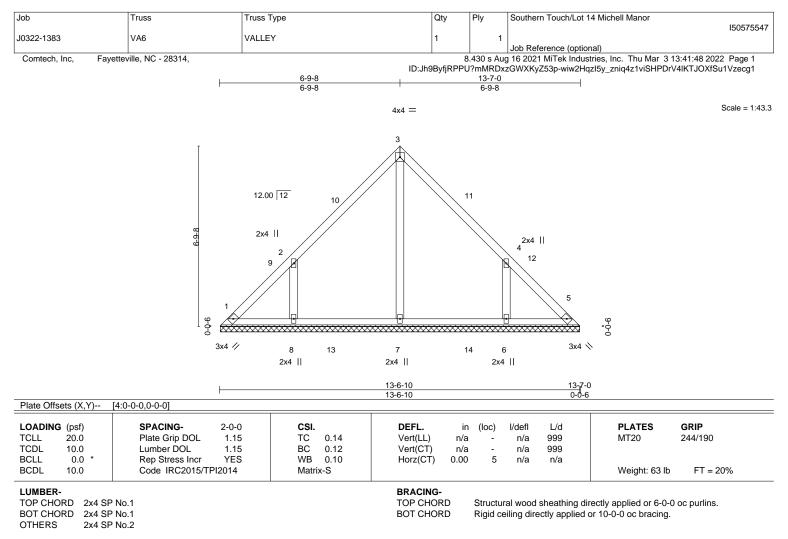
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=186, 6=186.



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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



REACTIONS. All bearings 13-6-4.

(lb) - Max Horz 1=-154(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-166(LC 12), 6=-166(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=335(LC 19), 8=379(LC 19), 6=379(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-364/291, 4-6=-364/292

### NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-9-8, Exterior(2) 6-9-8 to 11-2-5, Interior(1) 11-2-5 to 13-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

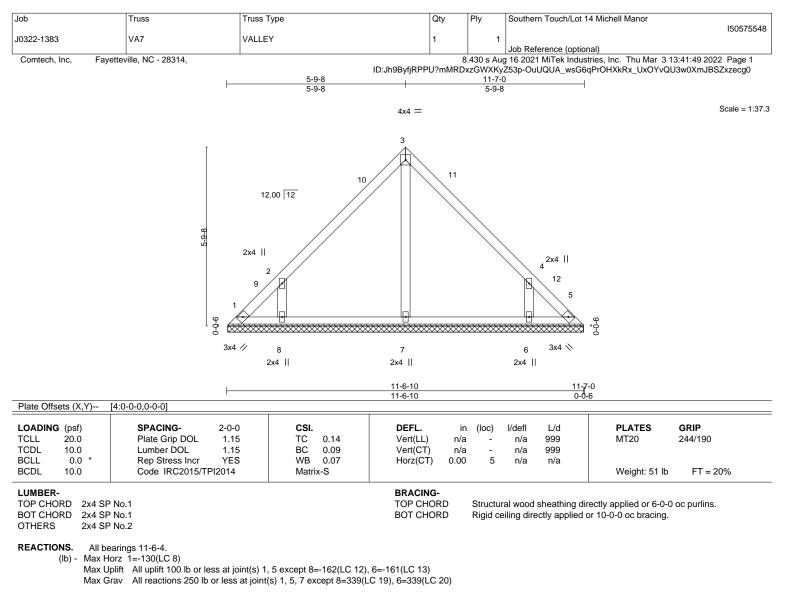
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=166, 6=166.



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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-361/303, 4-6=-361/303

### NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-9-8, Exterior(2) 5-9-8 to 10-2-5, Interior(1) 10-2-5 to 11-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

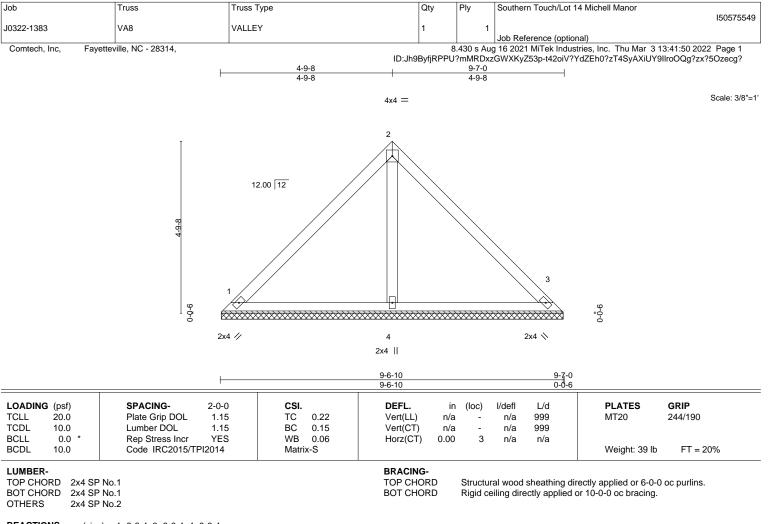
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=162, 6=161.



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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



REACTIONS. (size) 1=9-6-4, 3=9-6-4, 4=9-6-4

Max Horz 1=-106(LC 8)

Max Uplift 1=-26(LC 13), 3=-26(LC 13)

Max Grav 1=201(LC 1), 3=201(LC 1), 4=308(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

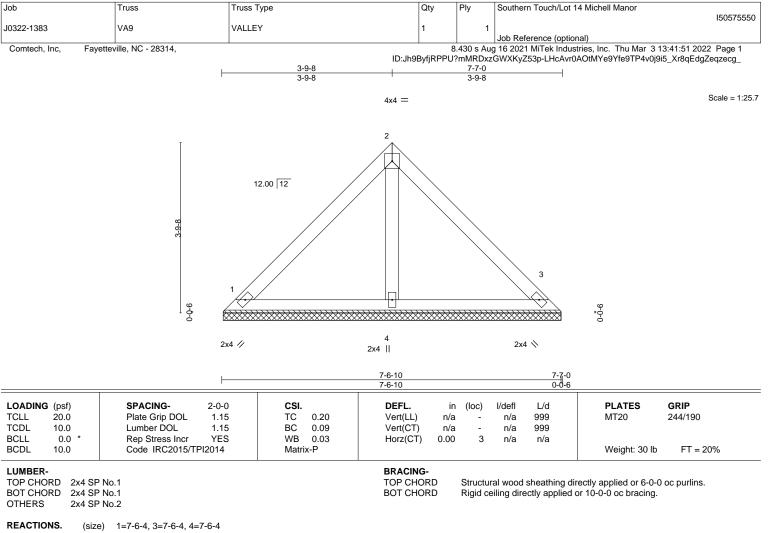
### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)
- and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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Max Horz 1=82(LC 9)

Max Uplift 1=-30(LC 13), 3=-30(LC 13)

Max Grav 1=168(LC 1), 3=168(LC 1), 4=215(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

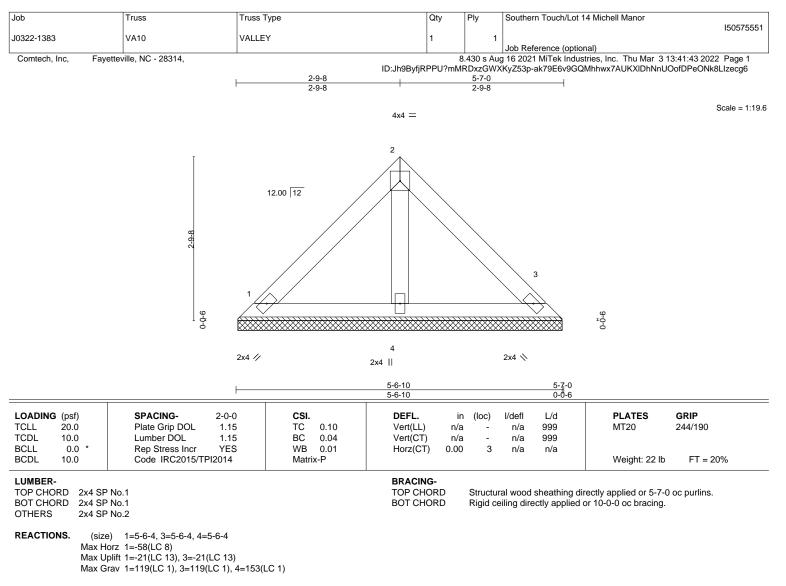
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

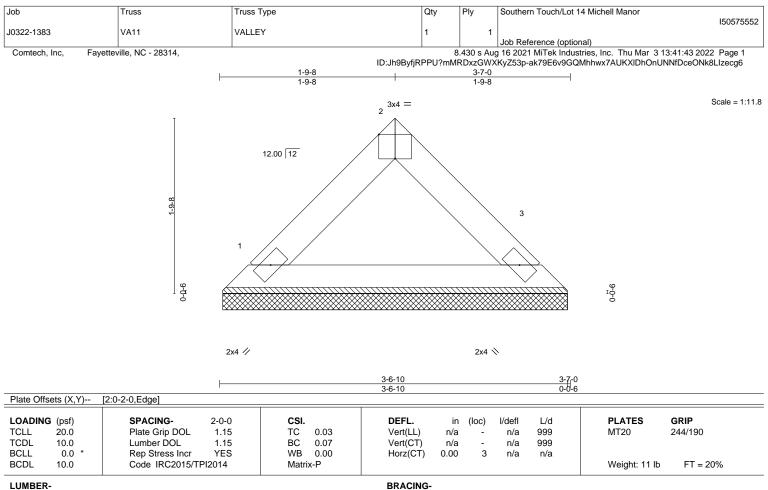
will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS.

(size) 1=3-6-4, 3=3-6-4 Max Horz 1=35(LC 11) Max Uplift 1=-4(LC 12), 3=-4(LC 12)

Max Grav 1=115(LC 1), 3=115(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

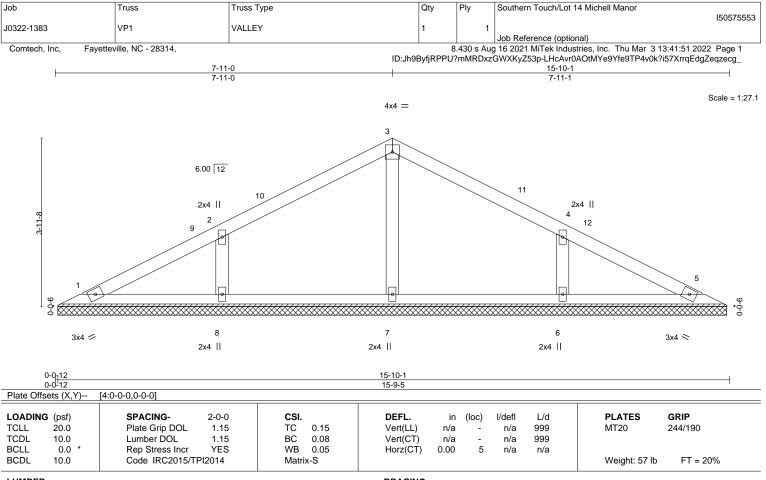
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 3-7-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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LUMBER

LUWBER-	
TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-8-9.

(lb) -Max Horz 1=48(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=272(LC 1), 8=344(LC 23), 6=344(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-260/202, 4-6=-260/202

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

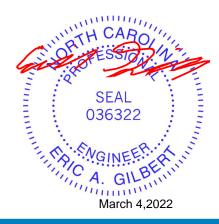
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-7-13 to 5-0-10, Interior(1) 5-0-10 to 7-11-0, Exterior(2) 7-11-0 to 12-3-13, Interior(1) 12-3-13 to 15-2-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

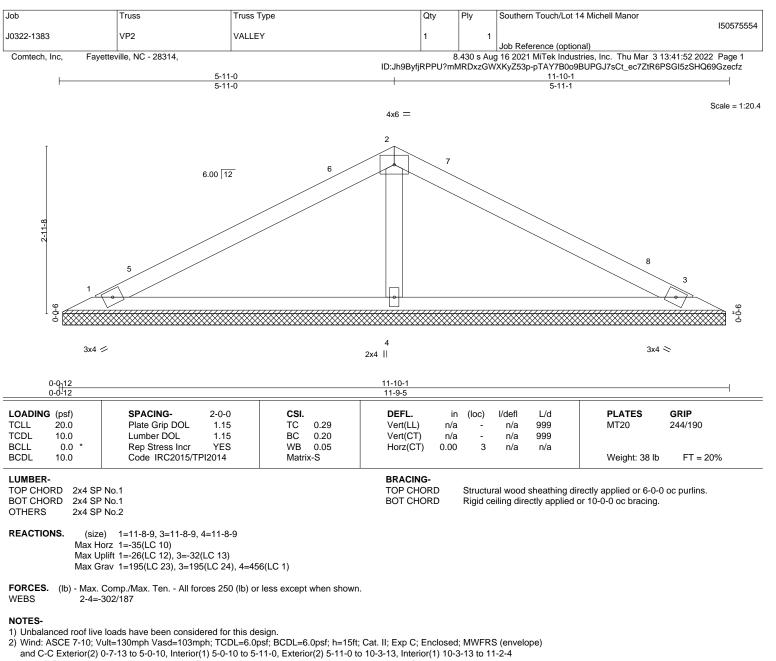
4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8, 6.

6) Non Standard bearing condition. Review required.



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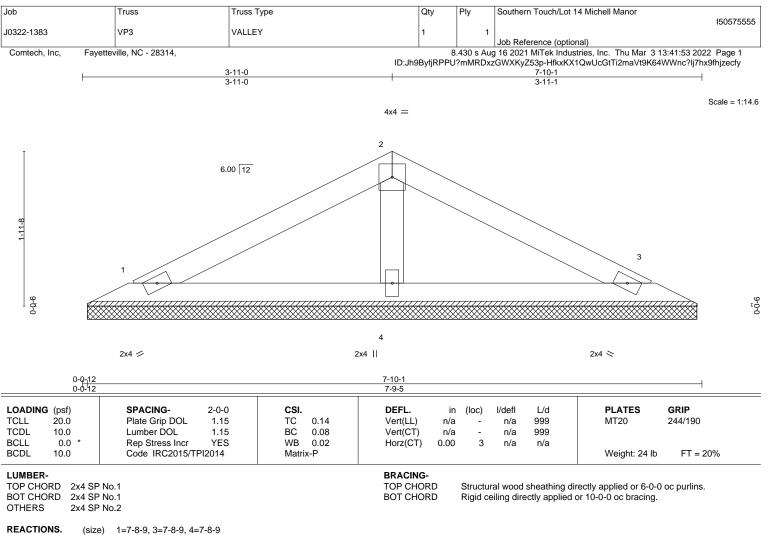
- zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
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Max Horz 1=-21(LC 8)

Max Uplift 1=-21(LC 12), 3=-25(LC 13)

Max Grav 1=133(LC 1), 3=133(LC 1), 4=256(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

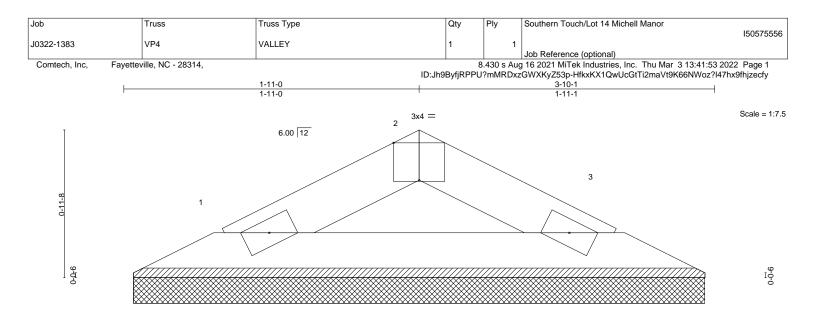
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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2x4 ⋍

2x4 📚

OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.03	Vert(LL) n/a - n/a 999	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.06	Vert(CT) n/a - n/a 999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 10 lb FT = 20%

REACTIONS. (size) 1=3-8-9, 3=3-8-9

Max Horz 1=8(LC 9) Max Uplift 1=-6(LC 12), 3=-6(LC 13)

Max Grav 1=101(LC 1), 3=101(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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