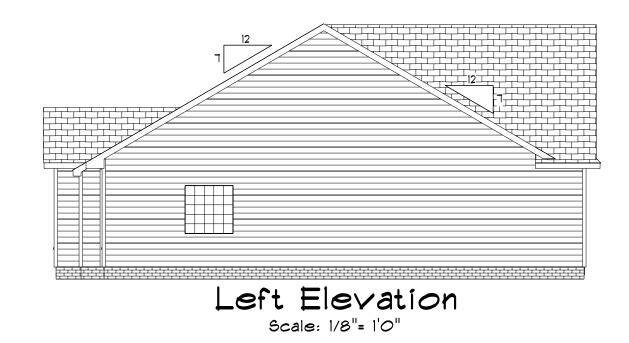


Front Elevation
Scale: 1/4"= 1'0"

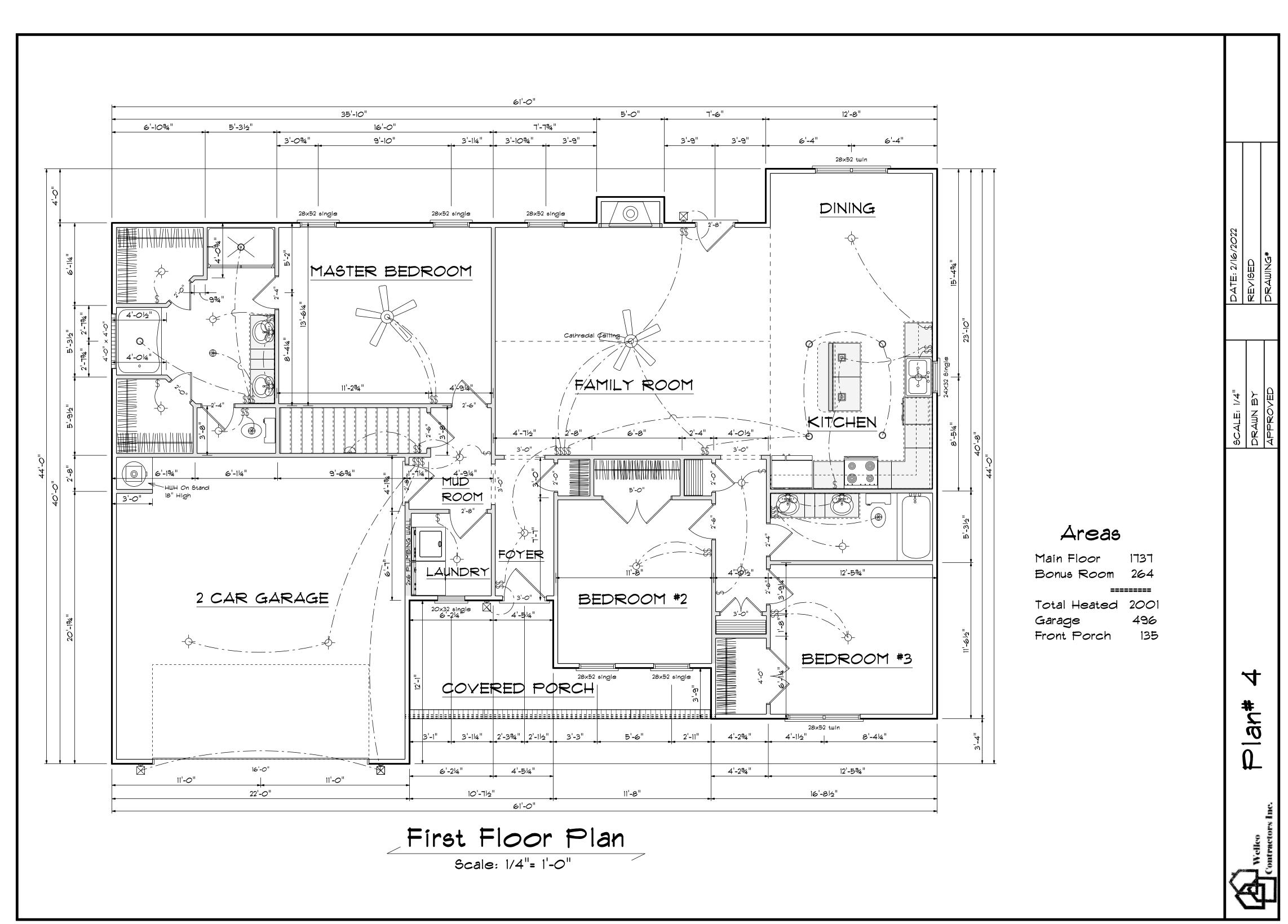


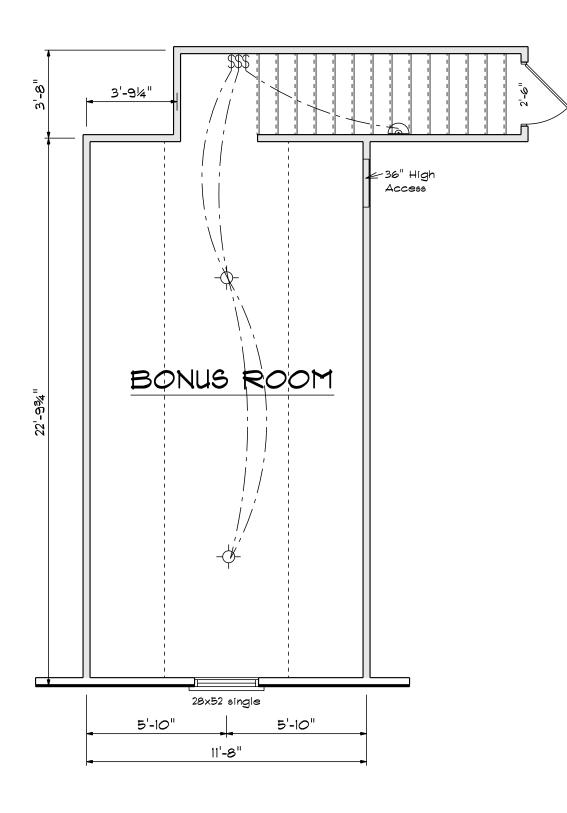




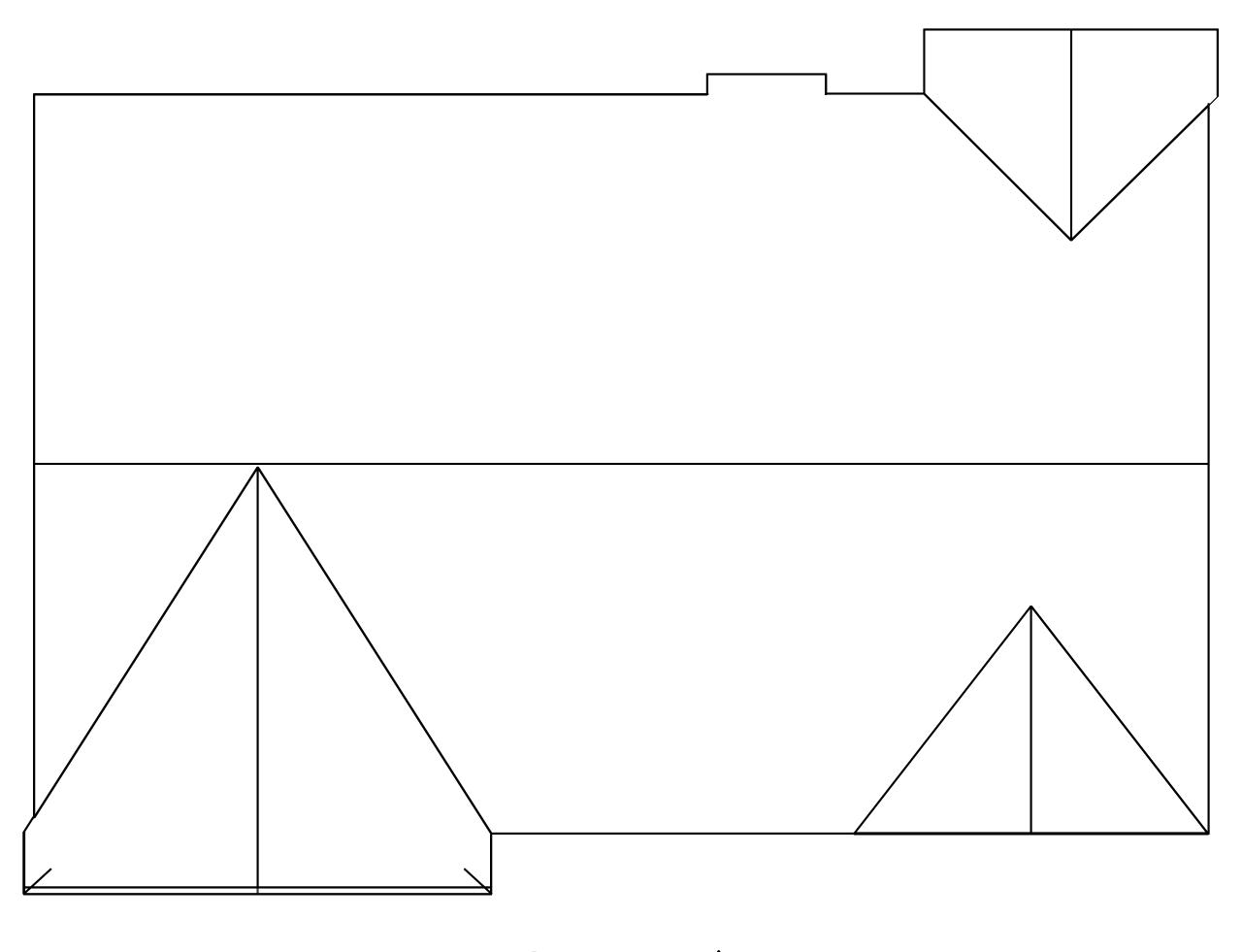
04/26/2022

Harnett C O U N T Y NORTH CAROLINA





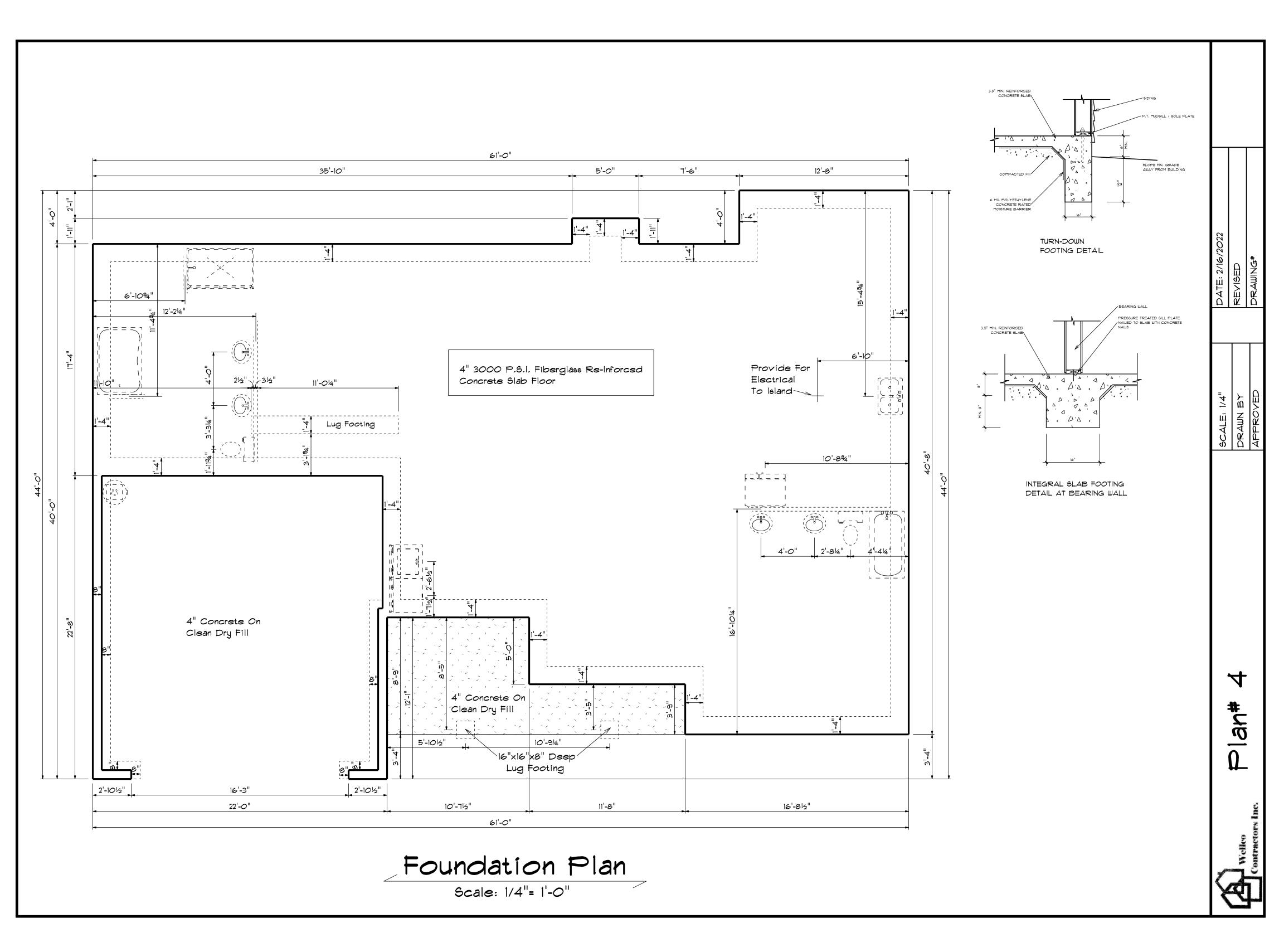
OPENI	NG SCHEDULE			
MAIN FLOOR PRODUCT CODE	SIZE	HINGE	REVERSED	COUNT
36X80 COLONIAL A 1	3'-0"	R	NO	1
32X80 FRENCH A 1	2'-8"	L	NO	1
192X84 - 4 PANEL GARAGE DOOR	16'-0"	U	NO	1
2-0 Door Unit	2'-0"	R	NO	2
2-0 Door Unit	2'-0"	L	NO	2
2-4 Door Unit	2'-4"	R	NO	3
2-6 Door Unit	2'-6"	R	NO	2
2-6 Door Unit	2'-6"	L	NO	2
2-8 Door Unit	2'-8"	L	NO	1
2-8 Door Unit	2'-8"	R	NO	1
3-0 Doublehung Door Unit	3'-0"	LR	NO	1
4-0 Doublehung Door Unit	4'-0"	LR	NO	1
5-0 Doublehung Door Unit	5'-0"	LR	NO	1
20x32 single	2'-0" x 3'-2"	N	NA	1
24X32 Single	2'-4" x 3'-2"	N	NA	1
28x52 single	2'-8" x 5'-2"	N	NA	6
28x52 twin	5'-4" x 5'-2"	NN	NA	2
4X8 GLASS BLOCK	4'-0" x 4'-0"	N	NA	1

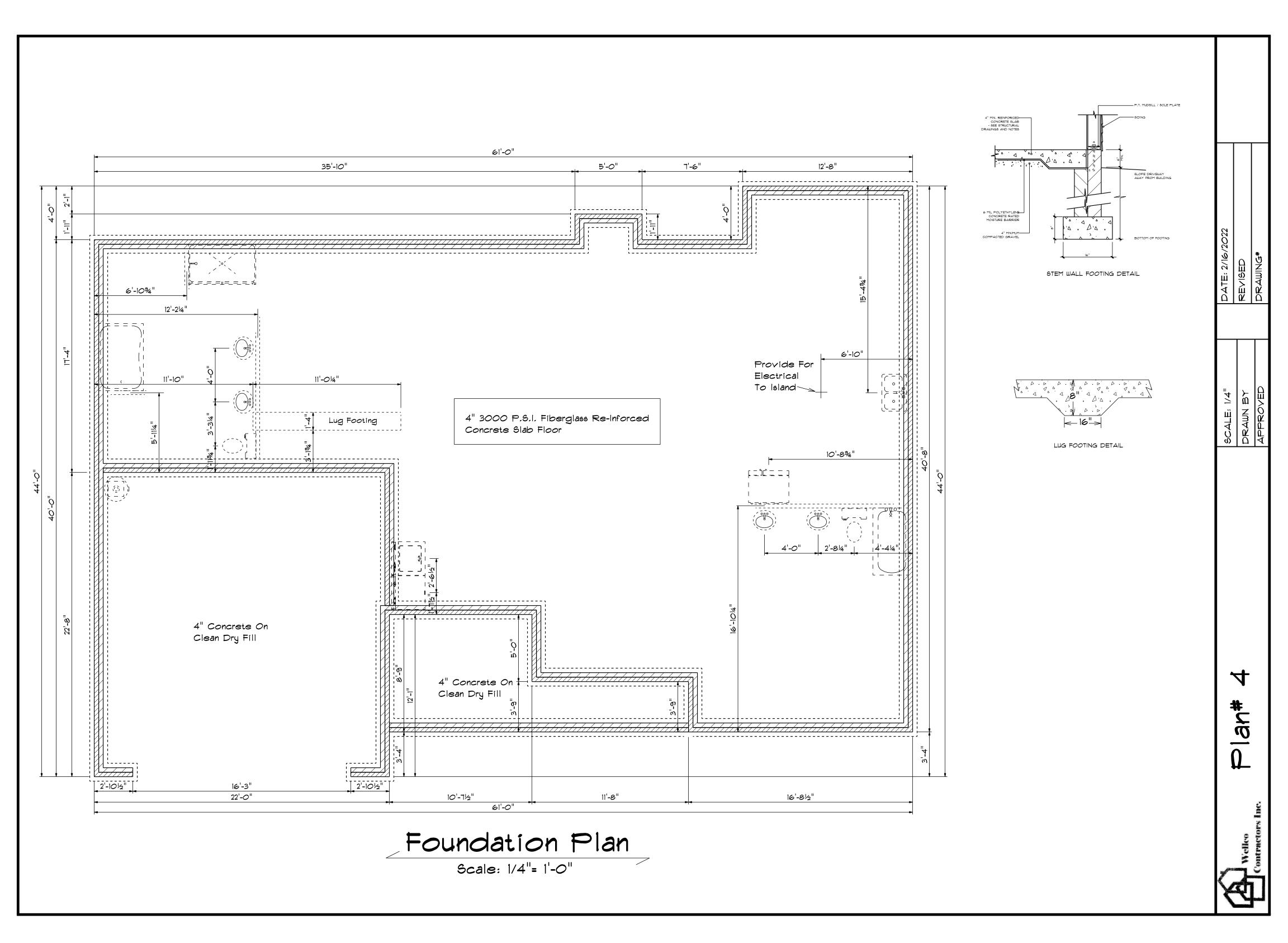


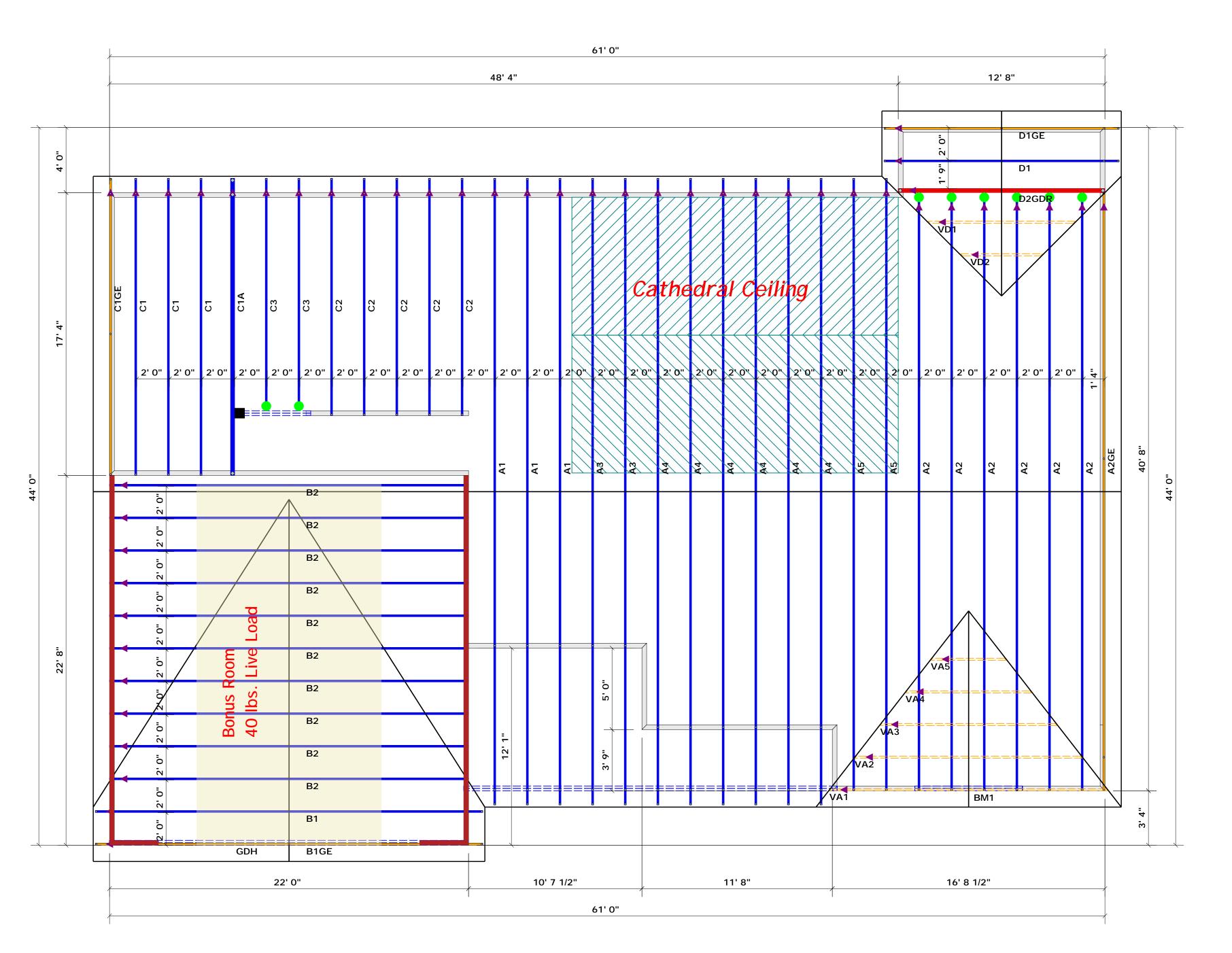
Roof Plan











<u>HANGER</u> <u>LEGEND</u>
= USP HUS410 / Double 2x Hanger
= USP HUS26 / Single 2x Hanger

▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards

Hatch Legend Garage Walls Dropped 1'

Truss Placement Plan SCALE: 1/4" = 1'

		Beam Legend			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH	22' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF

соттесн **ROOF & FLOOR TRUSSES & BEAMS**

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

earing reactions less than or equal to 3000# are seemed to comply with the prescriptive Code equirements. The contractor shall refer to the tached Tables (derived from the prescriptive Cod equirements) to determine the minimum foundatic ze and number of wood studs required to support aactions greater than 3000# but not greater than 5000#. A registered design professional shall be stained to design the support system for any faction that exceeds those specified in the attached ables. A registered design professional shall be stained to design the support system for all factions that exceed 15000#.

Curtis Quick

15300 9

Curtis Quick

LOAD CHART FOR JACK STUDS

END REACTION (UP TO) REQ'D STUDS FOR (3) ALY HEADER

1700 1 3400 1 2550 1 3400 2 6800 2 5100 2 5100 3 7650 3 10200 3 13600 4 6800 4 10200 4 8500 5 12750 5 17000 5 10200 6 15300 6 11900 7 13600 8

Lenny Norris

Spring Lake / Harnett Curtis Quick 15 DRAWN BY SALES REP. CI TY / CO.

34 Hidden Lakes Wellco Contractors J0422-2186 Seal Date Quote Lot JOB NAME **SEAL DATE**

BUILDER QUOTE 7 THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Client: Wellco Contractors

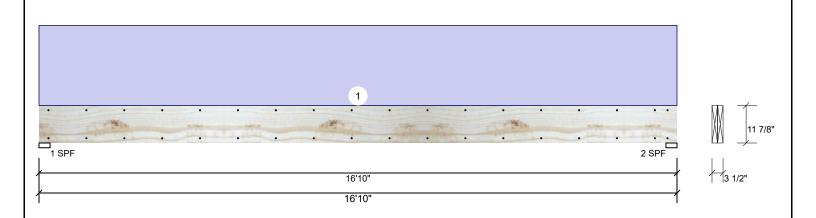
Project: Address: 4/19/2022

Input by: Curtis Quick Job Name: Plan 4 Beams

Project #:

Kerto-S LVL 2-Ply - PASSED 1.750" X 11.875" **GDH**

Level: Level



Member Inform	nation			Read	tions UNP	ATTERN	ED lb (Uplift)			
Type:	Girder	Application:	Floor	Brg	Direction	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	Vertical	0	2182	0	0	0
Moisture Condition:	: Dry	Building Code:	IBC/IRC 2015	2	Vertical	0	2182	0	0	0
Deflection LL:	480	Load Sharing:	No							
Deflection TL:	360	Deck:	Not Checked							
Importance:	Normal - II									
Temperature:	Temp <= 100°F									
				Bear	rings					
				Bea	aring Length	Dir.	Cap. React D/L	b Total	Ld. Case	Ld. Comb.
				1 - 3	SPF 3.500"	Vert	42% 2182 /	0 2182	Uniform	D
A l				2 - 3	SPF 3.500"	Vert	42% 2182 /	0 2182	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8689 ft-lb	8'5"	17919 ft-lb	0.485 (48%)	D	Uniform
Unbraced	8689 ft-lb	8'5"	8702 ft-lb	0.998 (100%)	D	Uniform
Shear	1859 lb	15'6 5/8"	7980 lb	0.233 (23%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.453 (L/433)	8'5 1/16"	0.546 (L/360)	0.831 (83%)	D	Uniform

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 10'8 15/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	250 PLF	0 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				9 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 1 of 4

This design is valid until 11/3/2024 CSD DESIGN

Manufacturer Info

isDesign

Client:

Project: Address: Wellco Contractors

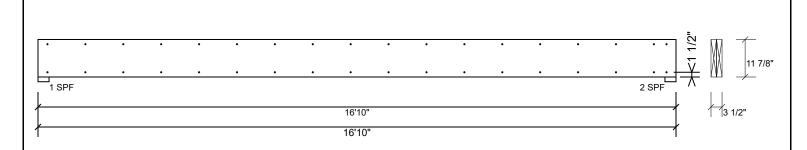
4/19/2022

Input by: Curtis Quick Job Name: Plan 4 Beams

Project #:

Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED **GDH**

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c., Maximum end distance not to exceed 6".

1 3		`	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	163.7 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes

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 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

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Page 2 of 4





Client: Wellco Contractors

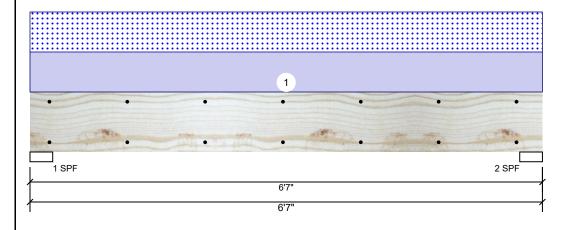
Project: Address: Date: 4/19/2022

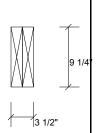
Input by: Curtis Quick Job Name: Plan 4 Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED Kerto-S LVL BM1

Level: Level





Page 3 of 4

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Live Wind Brg Direction Dead Snow Const 0 1423 1399 0 Vertical 0 1 2 Vertical 0 1423 1399 0 0

Bearings

Bearing	Length	Dir.	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	54%	1423 / 1399	2822	L	D+S
2 - SPF	3.500"	Vert	54%	1423 / 1399	2822	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4020 ft-lb	3'3 1/2"	14423 ft-lb	0.279 (28%)	D+S	L
Unbraced	4020 ft-lb	3'3 1/2"	10451 ft-lb	0.385 (38%)	D+S	L
Shear	1916 lb	1' 3/4"	7943 lb	0.241 (24%)	D+S	L
LL Defl inch	0.036 (L/2028)	3'3 1/2"	0.153 (L/480)	0.237 (24%)	S	L
TL Defl inch	0.073 (L/1005)	3'3 1/2"	0.204 (L/360)	0.358 (36%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
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- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	425 PLF	0 PLF	425 PLF	0 PLF	0 PLF	A2
	Self Weight				7 PLF					

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- Handling & Installation
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This design is valid until 11/3/2024

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Client: Wellco Contractors

Project: Address: Date: 4/19/2022

Input by: Curtis Quick Job Name: Plan 4 Beams

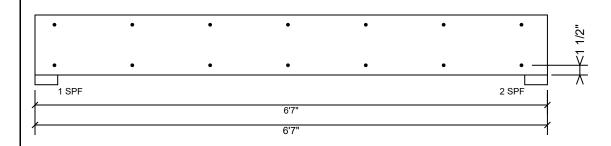
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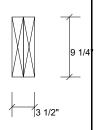
Kerto-S LVL BM1

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 4 of 4

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

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