

WITH SIDE LOAD GARAGE

SCALE 1/8" = 1'-0"

PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE**

AN ROOF HEIGHT 25'-8	•	HEIGHT TO	RIDGE 30'-0"
IMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A
ESTRATION U-FACTOR	0.35	0.35	0.35
LIGHT U-FACTOR	0.55	0.55	0.55
ZED FENESTRATION SHGC	0.30	0.30	0.30
LING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci
LL R=VALUE	15	15	19
OR R-VALUE	19	19	30
ASEMENT WALL R-VALUE	5/13	10/15	10/15
SLAB R-VALUE	0	10	10
RAWL SPACE WALL R-VALUE	5/13	10/15	10/19
0/13" MEANS R-10 SHEATHING INS	JLATION OR R-13 C	AVITY INSULATION	
NOT RATION DEDTU WITH MONOUT	UTC CLUD 241 OD D	OM INCONTION C	AD TO POTTOM OF

O BOTTOM OF FOOTING: INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL

 COMPC IBLAITIN GPTW MITLS 48 24 K1 DI DOTTO IF COULAIOW

 PODIBLI BLAITIN GPTW MITLS 48 24 K1 DI DOTTO IF COULAIOW

 DEGRED KWIND GPT DI ZIWH 332000 CAL (9 KATEST MILLS DESCRIFE ")

 COMPONENT & CLADDING DESCRIFE POR "THE FOLLOWING LOADS 1

 MEAN ROOF IL UP TO 301 391-170 41 MILLS 153 393-170 41 MILL 154

 ZONF TI 14,2 1-55,0 14,91 1-54,1 55,5 1-6,4 15,9 2-70,2

 ZONF 1
 14,2 1-85,0 14,91 1-54,1 55,5 1-6,4 15,9 2-70,2

 ZONF 2
 14,2 1-80,0 14,91 1-8,81 155, 1-6,4 15,9 2-70,2

 ZONF 3
 14,2 1-80,0 14,91 1-8,81 155, 1-6,4 15,9 2-70,2

 ZONF 4
 15,5 1-6,1 14,31 1-5,81 15,9 1-71,4 1-7,9

 ZONF 5
 15,5 1-6,0 11,5,1 1-7,81 15,9 1-7,41 1-7,9

 ZONF 5
 15,5 1-6,0 11,5,1 1-7,81 15,9 1-7,41 1-7,4 1-7,9

 ZONF 5
 15,5 1-6,0 11,5,1 1-7,81 15,9 1-7,41 1-7,4 1-7,9

 ZONF 5
 15,5 1-6,0 11,5,1 1-7,61 1,61 1-2,1,8 11,7,4 1-7,9

 ZONF 5
 15,5 1-6,0 11,5,1 1-7,61 1,61 1-2,1,8 11,7,4 1-7,9

 ZONF 5
 15,5 1-6,0 11,5,1 1-7,61 1,61 1-2,1,8 11,7,4 1-7,9

 ZONF 6
 15,5 1-7,0 11,5,1 1-7,61 1,61 1-2,1,8 11,7,4 1-7,9

 ZONF 7
 15,5 1-7,0 11,5,1 1-7,0 1,7,61 1-2,1,7,4 1-7,9
 IGNED FOR WIND SPEED OF 130 MPH, 3 SECOND GUST (101 FASTEST MILE) EXPOSURE "B" MPONENT & CLADDING DESIGNED FOR THE FOLLOWING LOADS

CONFLICTENT & CLADAINAS DESIGNED FORK THE FOLLOWING LOADS (MANN ROOF UP TO 301 30-11 TO 331 33-11 TO 401 40-11 TO 451 ZONE 1 16,7 -186,0 17,5 -186,1 18,2 -19,6 18,7 -20,2 ZONE 2 16,7 -21,0 17,5 -22,1 18,2 -22,2 18,7 -23,5 ZONE 3 16,7 -21,0 17,5 -22,1 18,2 -22,3 18,7 -23,5 ZONE 3 16,7 -21,0 17,5 -22,1 18,2 -22,1 18,7 -23,5 ZONE 4 18,7 -21,0 17,5 -22,1 18,2 -22,1 18,7 -23,5 ZONE 4 18,7 -21,0 17,5 -22,1 18,2 -22,1 18,7 -23,5 ZONE 4 18,7 -21,0 19,1 -20,0 19,8 -20,7 20,4 -21,3

AIR LEAKAGE

Section N1102.4 N1102.4.1 Building thermal envelope. The building thermal

LILLOCAL building thermal envelope. In Eduancy thermal envelope and building thermal envelope and building thermal infiltration. The avaiing methods between dismitter materials shall allow for different expansion and contraction, for all nomes, where present, the following shall be called, gasketd, weather entroped or otherwise earlied with an air barrier material or call and the share of the share L. Bloking and skaling theoreting systems and carlor knee was need to use of the share bene to use on other of the share of the sha

open to unconditioned or exterior space. 2. Capping and sealing shafts or chases, including flue shafts.

3. Capping and sealing soffit or dropped ceiling area

ROOF VENTILATION SECTION R806

R806.1 Ventilation required. Enclosed attics and enclosed rafter spaces Rouse. I vertuation requires indices attics and encoded rater spaces formed where cellings are applied directly to the underside of nof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or encow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than mm maximum, verindenon openings having a least dimension signer than 1/4 inch (6.4 mm shall be provided with corrisoline resistant wire doth dimension of 1/16 inch (1.4 mm) minimum and 1/4 nch (6.4 mm) maximum, Openings in roof framing members shall conform to the requirements of Section R802.7 R806.2 Minimum area. The total net free vertilating area shall not be less

than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or comice vents with the balance of the required ventilation provided by eave or comice vents. As an alternative, the net free cross ventilation area may be reduced to 1/300 when a Class I or II

The three cross-vertice to only and a may be reduced to 1,300 when a class 1 of 11 vapor retarder is installed on the warm-in-winter side of the celling. Exceptions: 1. Endosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only. SQUARE FOOTAGE OF ROOF TO BE VENTED = 1558 SQ FT.

NET FREE CROSS VENTILATION NEEDED: WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 10.39 SQ FT.

WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE: OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 5.16 SO.FT.



SECTION R312

SECTION R312 R312.1 Where required, *Guards* shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (707, mm) measured vertically to the loss or grade-belw at any point within 35 inches (914 nm) honzonrally to the adge of the open aids. Insect screening shall not be considered as a guard. R312.2 Height, Required guards it open-sided walking surfaces, including stairs, porches, bolconse or landings, while to not be shall to inches (914 nm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the locannecting the locating screen of the treads.

Exceptions

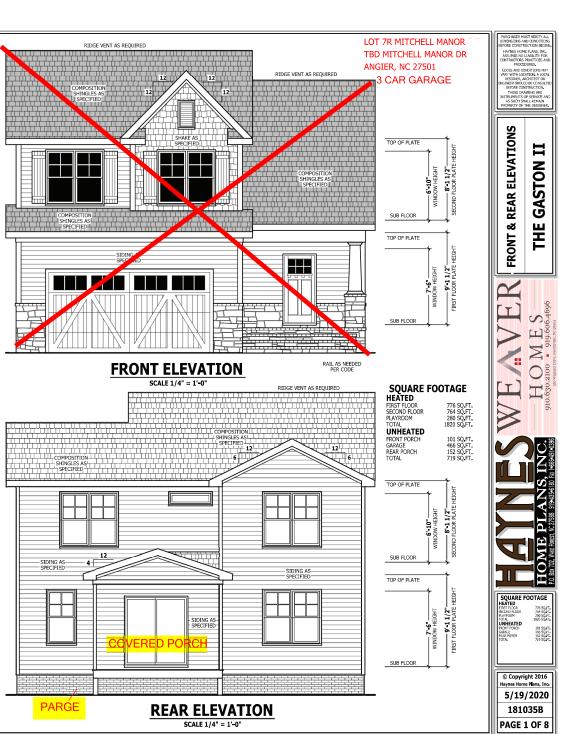
1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

treads. Where the top of the guard also serves as a handrail on the open sides of 2. Where the cup of the guard also serves as a halination of the open subs of stars, the top of the guard shall not be not less than 34 inches (864 mm) and not more than 33 inches (965 mm) measured vertically from a line connecting the leading address of the trads. R312.3 Opening limitations. Required guards shall not have openings from the

walking surface to the required *quard* height which allow passage of a sphere 4 inches (102 mm)in diameter,

Excentions: The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 mm) in diameter. Guards on the open sides of stairs shall not have openings which allow

passage of a sphere 4 3/8 inches (111 mm) in diameter.

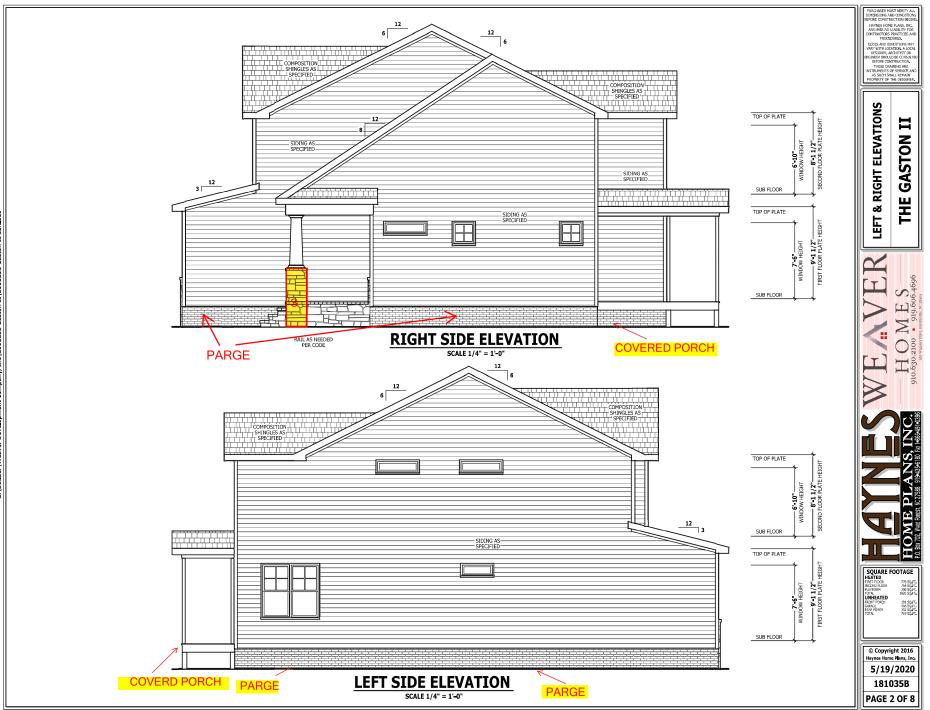


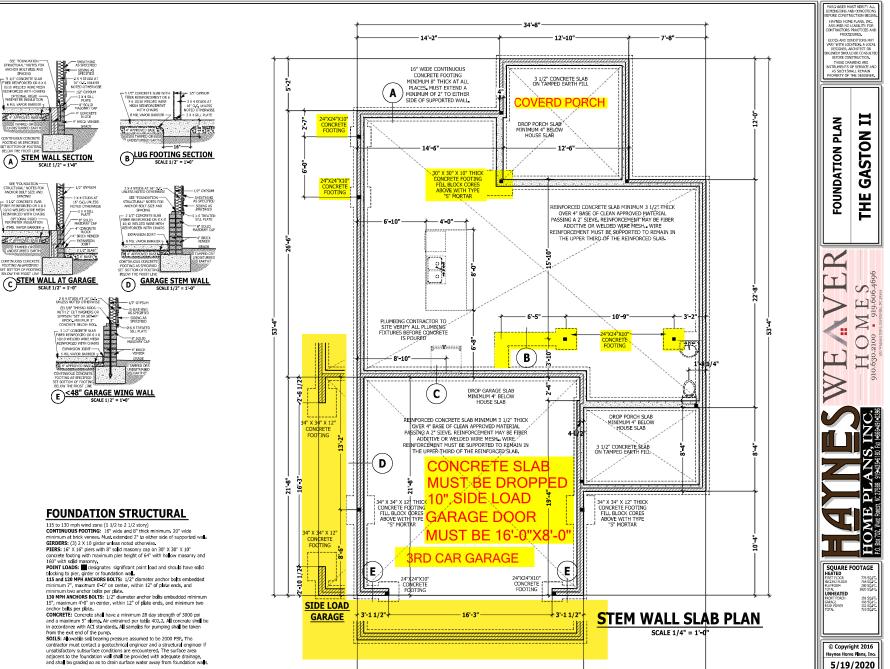
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22'-6"

34'-8'

12'-2"

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PAGE 3 OF 8



STRUCTURAL" NOTES FOR ANCHOR BOLT SIZE AND SPACING

- 3 1/2" CONCRETE SLAB FIBER REINFORCED OR 6 X 6

WELDED WIRE NESH

6 MIL VAPOR BAR

CONTINUOUS CONCRET FOOTING AS SPECIFIED SET BOTTOM OF FOOTIN

STRUCTURAL' NOTES FOR ANCHOR BOLT SIZE AND SPACING

SPACING = 3 1/2" CONCRETE SLAB F.BER.REINFORCED OR 6 X 6 10/10 WELDED WIRE MESH REINFORCED WITH CHAIRS OPTIONAL DION

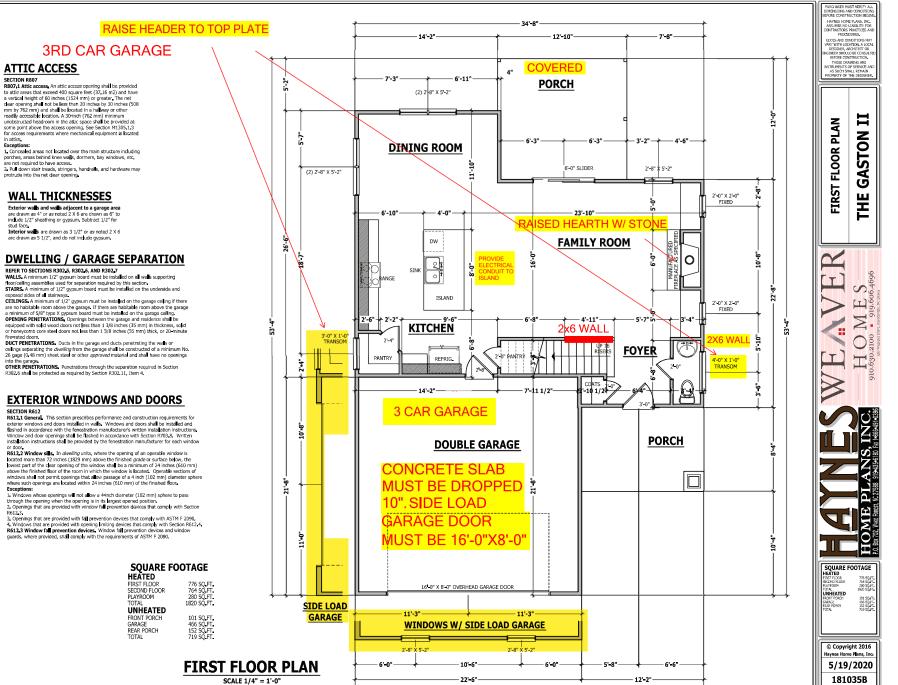
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UNDISTUREE

CONTINUOUS CONCRE ROOTING AS SPECIFIE SET BOTTOM OF FOOTI

1.1

contractor must contact a geotechnical engineer and a structural engineer if unsatisfactory subsurface conditions are encountered. The surface area adjacent to the foundation wall shall be provided with adequate drainage,



34'-8"

PAGE 4 OF 8

SECTION R807

a version regit of to unkness (124 mm) or greater in het dear opening shall not be less than 20 inches (503 mm by 762 mm) and shall be located in a hallway or other readily accessible location, A 30-inch (762 mm) inimium unotstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

 Pull down stair treads, stringers, handralls, and hardware may protrude into the net clear opening.

WALL THICKNESSES

Exterior walls and walls adjacent to a garage area are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum, Subtract 1/2" for stud face. Interior was are drawn as 3 1/2" or as noted 2 X 6

a minimum of 5/8" type x gypsum board must be installed on the galage damage OPENING PENETRATIONS, Openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute element doors not less than 1 3/8 inches (35 mm) thick, or 20-minute

SECTION R612

2. Openings that are provided with window fail prevention devices that comply with Section

STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code. JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Havnes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

(PSF) 10 10 10 10 10 	(LL) L/240 L/360 L/360 L/360 L/360
10	L/360 L/360 L/360
10	L/360 L/360
10	L/360
10	L/360
-	-
-	-
10	L/360
10	L/360
10	L/360
-	L/360
_	-
	10 - -

875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise. ENGINEERED WOOD BEAMS :

ENGLIFECTED WOUD BEAMS : Laminated verse (Linber (1KL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10* PSI ParaBill strand Linber (FSI.) = Fb=260 PSI, Fv=280 PSI, E=2520,046 PSI Laminated strand Linber (SI.) PSI-220 PSI, Fv=200 PSI, E=1.25x104 PSI Instal al connections per manufactures instructions, TRUSS AND DIOIST MEMBERS: All roof truss and Joint Layout, shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss Instance according to the manufacture's specifications. Any orange in true of Hoak Specifications in the manufacture's specifications. In the other of the specification of the Pote at 2-2 of on center for spars up to 15-0 unless noted observate. FLOOR SHEATHING: OSB or COX floor sheathing minimum 1/2' thick for 16' on center joist spacing, minimum 3/6' thick for 19.2'' on center joist spacing, and minimum 3/4' thick for 24'' on center joist spacing. ROOF SHEATHING: OSB or COX roof sheathing minimum 3/6' thick. CONCRETE AND SOILS: See foundation notes.

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BRACE WALL PANEL NOTES EXTERIOR WALLS: All exterior walls to be sheathed with

CS-WSP or CS-SFB in accordance with section R602.10.3 unless GYPSUM: All interior sides of exterior walls and both sides

interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1. REQUIRED LENGTH OF BRACING: Required brace wall enoth for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length. HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner. Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter), CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing nais.

GB: Interior walls show as GB are to have minimum 1/2' gypsum board on both sides of the wall fastened at 7" of center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. PF: Portal fame per figure R602_10_1

PONY WALL

HEIGHT TO

VAF

20

HEIGHT T - 10'

AXIMUM

50

17VO

PF

PORTAL FRAME AT OPENING

(METHOD PF PER FIGURE AND SECTION R602.10.1)

SCALE 1/4" = 1'-0"

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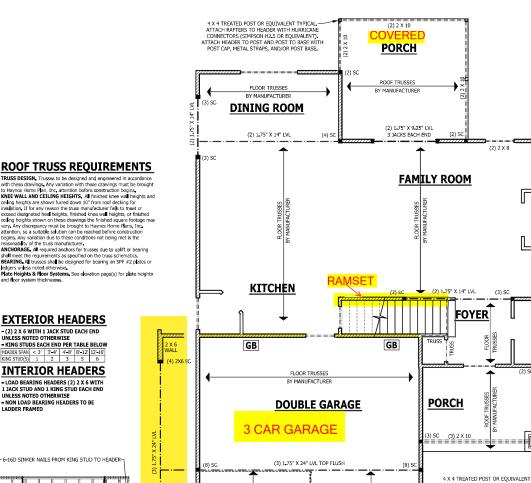
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2 2

HEIGHT

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LINE OF FLOOR ABOVE

FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'=0"

PF

(3) 50

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(2) 2 X 8

(3) SC

TYPICAL, ATTACH RAFTERS TO HEADER

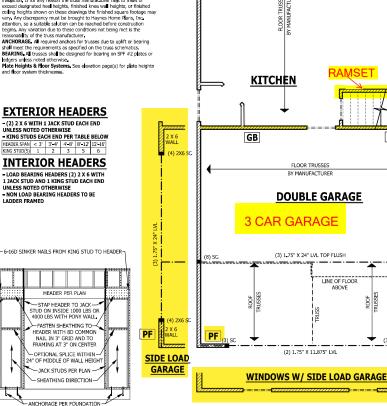
WITH HURRICANE CONNECTORS (SIMPSON H2.5 OR EQUIVALENT), ATTACH HEADER TO POST AND POST

TO BASE WITH POST CAP, METAL

STRAPS, AND/OR POST BASE

F

URCHASER MUST VERIFY IMENSIONS AND CONDIT



ceiling heights are shown furred down 10" from roof decking for

insulation. If for any reason the truss manufacturer fails to meet or

STRUCTURAL NOTES

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JOB SITE FRACILLES AND SAFETY: Haynes Home Hans Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with imited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Baconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	-	-
Guardrai in fill components	50	-	-
Passenger vehicle garages	50	10	L/360
Rooms other than seeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	-	L/360
Snow	20	-	-

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless unbedeated by the state of the state noted other wise,

ENGINEERED WOOD REAMS

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Laminatez volue activity in the second secon Instal al connections per manufacturers instructions.

TRUSS AND INIOIST MEMBERS: All mod truss and L joist ayouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufactures specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick Intels shall be 3 1/2" x 3 1/2" x 1/4" steel LINTELS: Brick lincts shall be 3 $1/2^{*} \times 3 J/2^{*} \times 1/4^{*}$ steel angle for up to 6⁻⁰' span, 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9⁻⁰' unless noted otherwise, 3 $1/2^{*} \times 3 J/2^{*} \times 1/4$ " steel angle with $1/2^{*}$ bolts at 2-0" on center for spans up to 18⁻⁰" unless noted otherwise, FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing, ROOF SHEATHING: OSB or CDX roof sheathing minimum

CONCRETE AND SOILS: See foundation notes.

ATTIC ACCESS SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net deer opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other mm dy roz mm) and shall be located in a narway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

 Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.
 Pull down stair treads, stringers, handrails, and hardware may protrude into the net dear opening.

EXTERIOR WINDOWS AND DOORS

SECTION R612

R612 General This section prescribes performance and construction requirements for Notace detection instructions shall be provided by Windows and doors shall be installed and flashed in accordance with the fenestration manufacturer's written installation instructions. Window and door openings shall be fisshed in accordance with Section R703.8. Written installation instructions shall be provided by the fenestration manufacturer for each window or door. R612,2 Window sills. In dwelling units, where the opening of an operable window is

No.1.42 wrote wrote wills, in overlarg units, where the opening of an operace writow is a black more than 0 peracet wrote wrote the opening of the window shall be a minimum of 24 inches (616 mm) above the finished beaution of the window shall be a minimum of 24 inches (616 mm) above the finished beaution of the window shall be a minimum of 24 inches (616 mm) above the finished beaution of the window shall be a minimum of 24 inches (616 mm) above the finished beaution of the window shall be a minimum of 24 inches (616 mm) above the finished beaution of the window shall be a minimum of 24 inches (616 mm) above the finished beaution of the window shall be a minimum of 24 inches (616 mm) above the finished beaution of the minished beauti Exceptions:

I. Windows whose openings will not allow a 4-inch diameter (102 mm) sphere to pass through the opening when the opening is in its largest opened position. 2. Openings that are provided with window fall prevention devices that comply with Section R612.3

Openings that are provided with fall prevention devices that comply with ASTM F 2090. 4. Windows that are provided with opening limiting devices that comply with Section R612.4. R612.3 Window fall prevention devices. Window fall prevention devices and window guards, where provided, shall comply with the requirements of ASTM F 2090.

EXTERIOR HEADERS - (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE
 KING STUDS EACH END PER TABLE BELOW

HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12-16' KING STUD(S) 1 2 3 5 6 INTERIOR HEADERS - LOAD BEARING HEADERS (2) 2 X 6 WITH

1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE

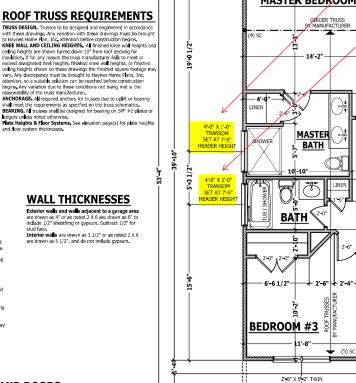
ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and After which and cellure networks, an initiate the wait heights and celling heights are shown furred down 10⁴ from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designeted head heights, finished knew wall heights, or finished celling heights shown on these drawings the finished square footage may

vary. Any discrepancy must be brought to Havnes Home Plans. Inc.

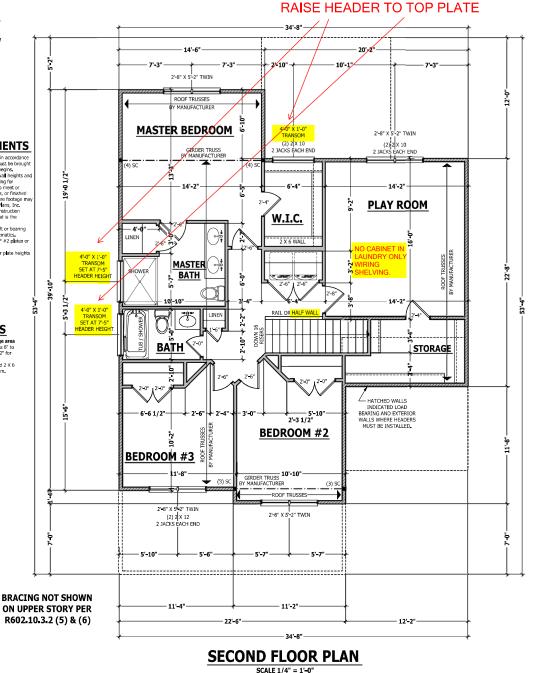
shall meet the requirements as specified on the truss schematics. BEARING A trusses shall be designed for bearing on SPF #2 plates or

and floor system thickne



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EFORE CONSTRUCTION BEGIN HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES. FROCEDURES. CODES NIC DIMENTIONS MAY VIRY WITH LOCATIONS ANY USEN WITH LOCATIONS AND DESIGNER, ARCHITECT OR ENGINEER SHOLL DE CONSULTE BEFORE CONSTITUTION, THESE DERAVITION ARE INSTRUMENTS OF SERVICE AND AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER. PLAN Ħ GASTON SECOND FLOOR THE S EL a ≥. \frown C SQUARE FOOTAGE 776 SQ FI 764 SQ FI 280 SQ FI 1820 SQ FI UNHEATED 101 SQ FI 466 SQ FI 152 SQ FI 719 SO FI © Copyright 2016 aynes Home Plans, In 5/19/2020 181035B PAGE 6 OF 8

URCHASER MUST VERIFY A

PURCHARD MUST VERDY ALL DIVERSIONS AND CONDITIONS REPORE CONSTITUTION REGISLS, HAYNES HOME PLANS, INC. ASSIME'S NOL BURLITY FOR CONTRACTORS PURCTURE MUST PURCTURE PU Η THE GASTON **ROOF PLAN**

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 SQUARE FOOTAGE

 HEATED
 7850 77

 Past Floor
 789 307

 Past Floor
 289 307

 Past Floor
 289 307

 TOTAL
 1800 507

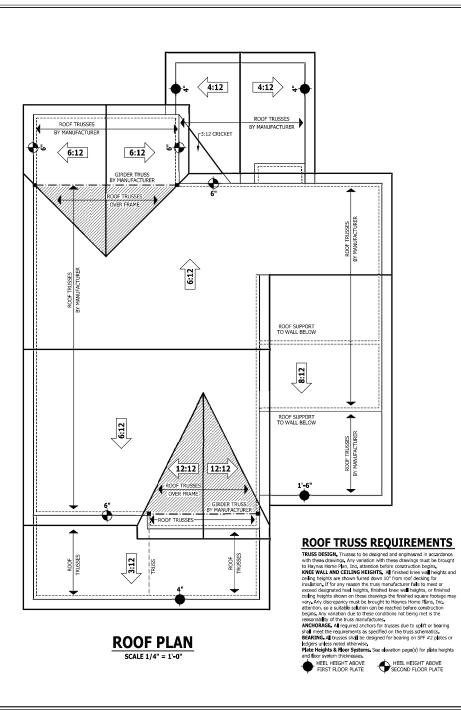
 UNHEATED
 180 307

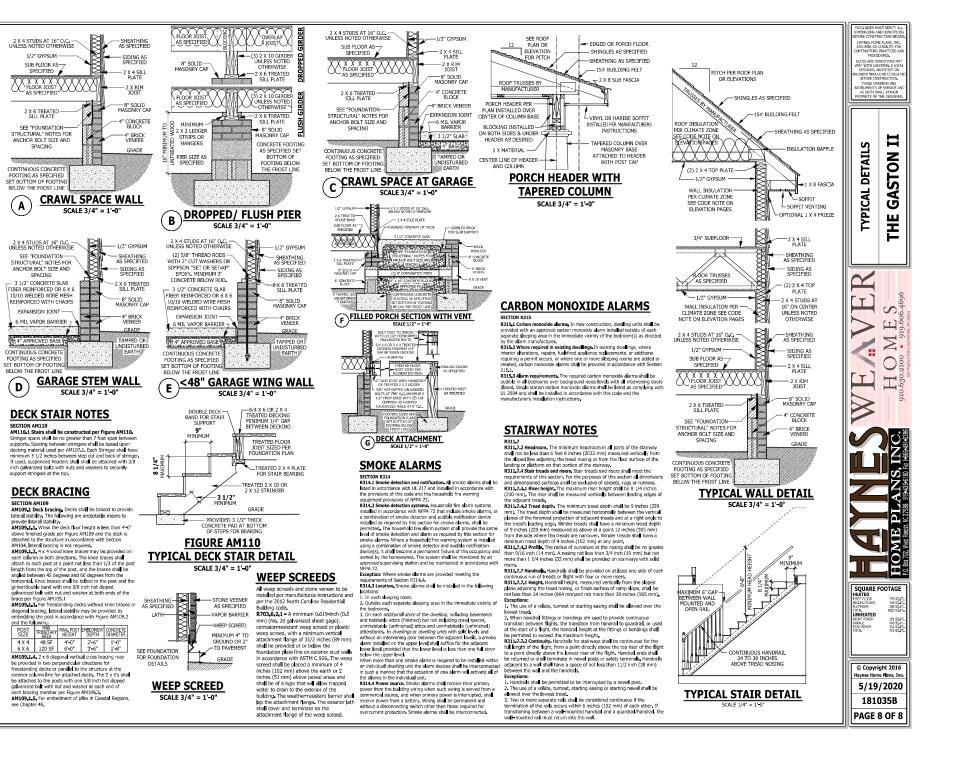
 ROM POSCH
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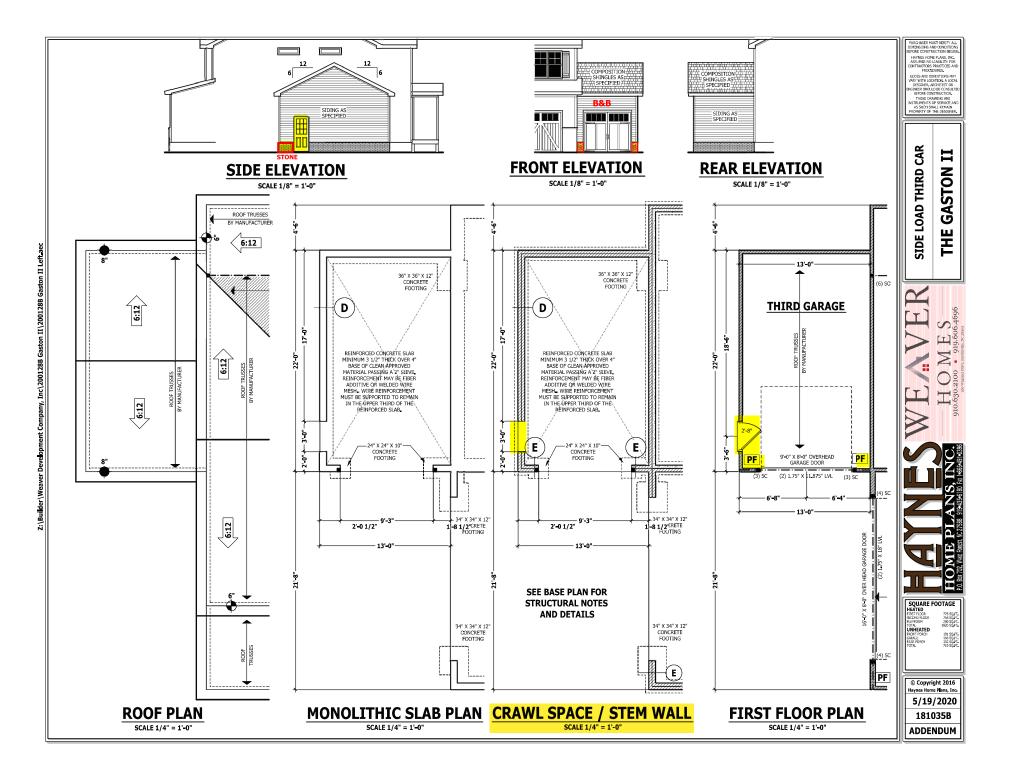
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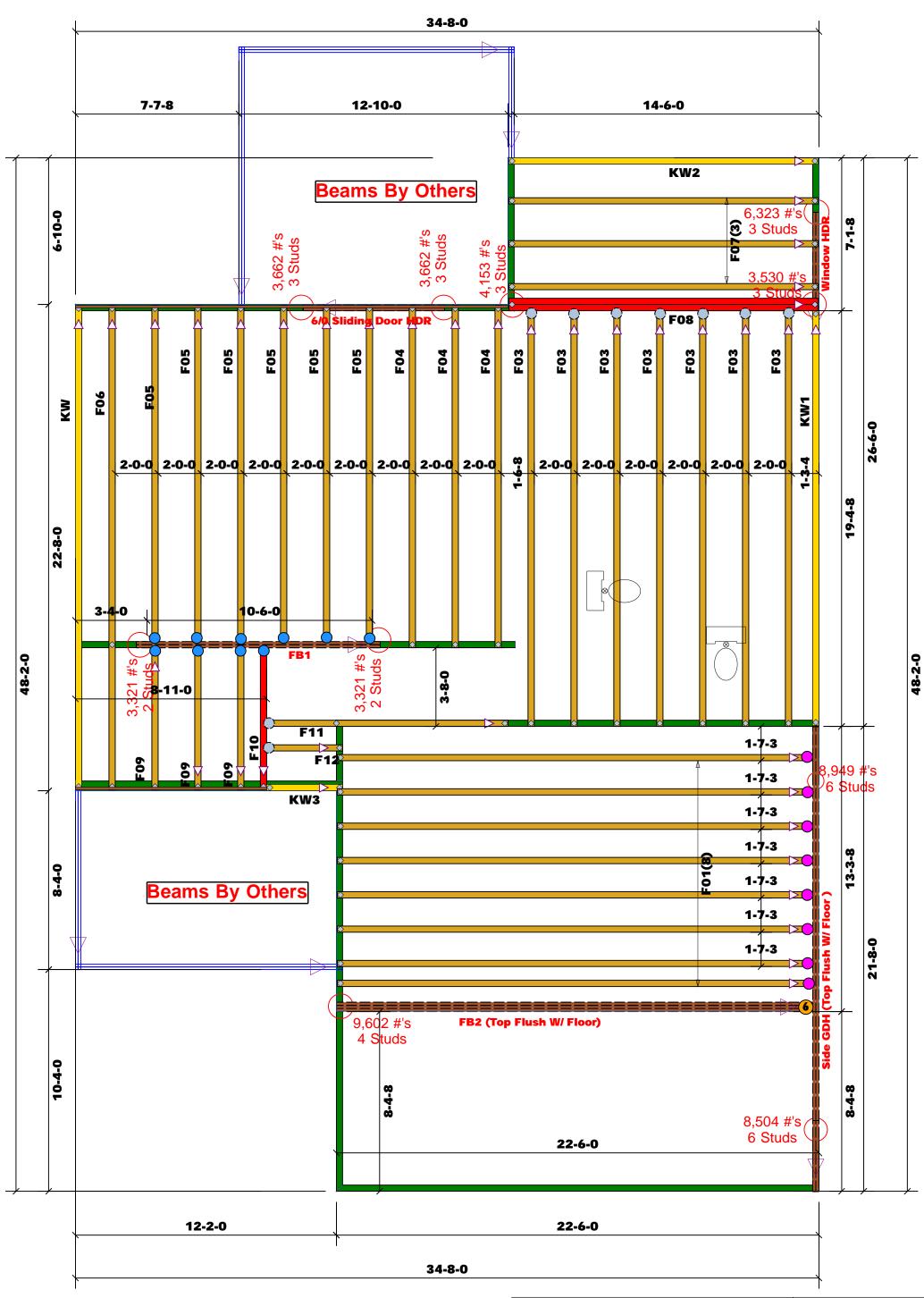
 TOTAL
 101 3037

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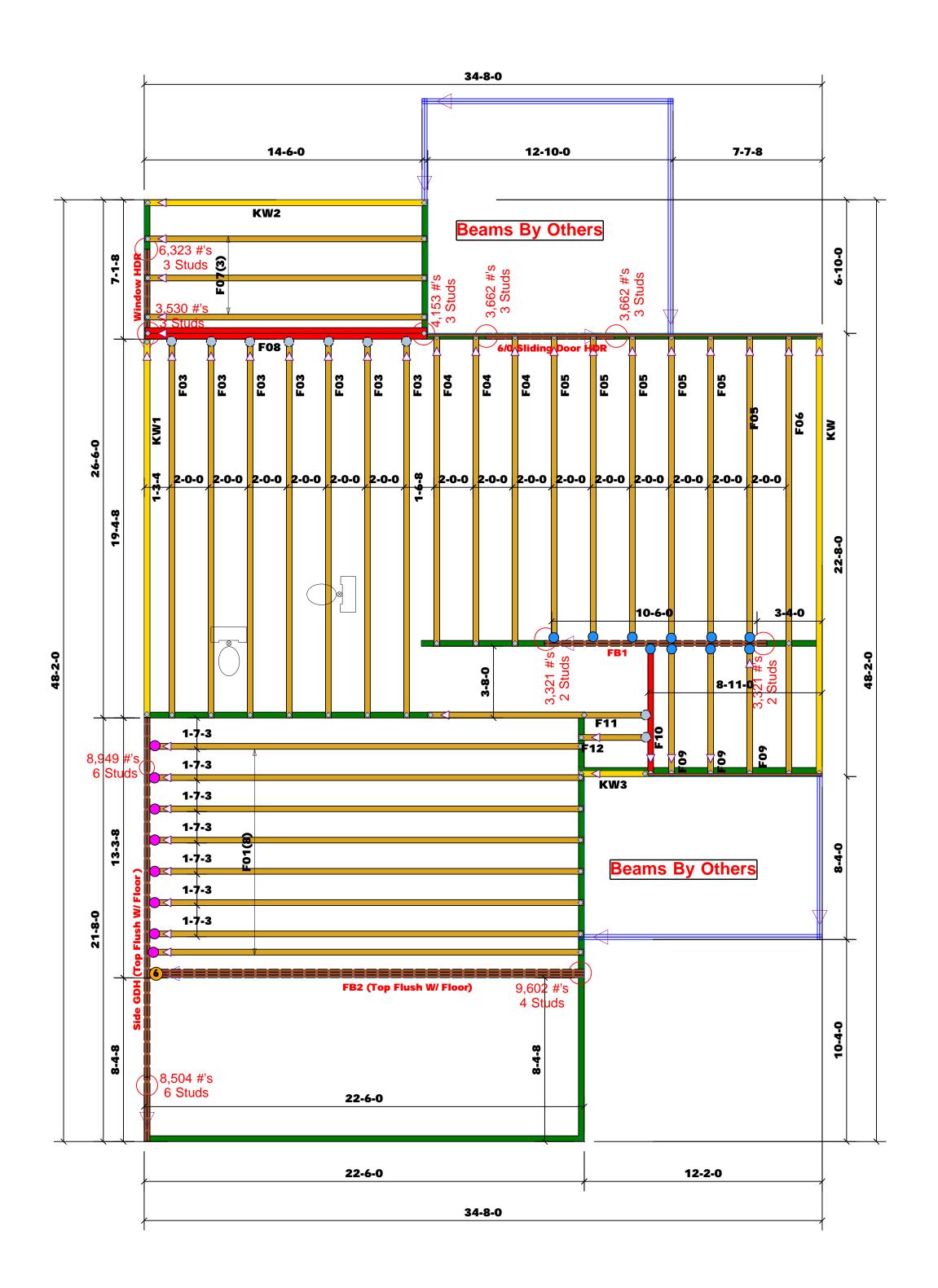
	Products			
Length	Product	Plies	Net Qty	Fab Type
7-0-0	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
12-0-0	1-3/4"x 14" LVL Kerto-S	2	2	FF
7-0-0	1-3/4"x 14" LVL Kerto-S	2	2	FF
23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3	FF
22-0-0	1-3/4"x 23-7/8" LVL Kerto-S	2	2	FF
	7-0-0 12-0-0 7-0-0 23-0-0	Length Product 7-0-0 1-3/4"x 9-1/4" LVL Kerto-S 12-0-0 1-3/4"x 14" LVL Kerto-S 7-0-0 1-3/4"x 14" LVL Kerto-S 23-0-0 1-3/4"x 23-7/8" LVL Kerto-S	Length Product Plies 7-0-0 1-3/4"x 9-1/4" LVL Kerto-S 2 12-0-0 1-3/4"x 14" LVL Kerto-S 2 7-0-0 1-3/4"x 14" LVL Kerto-S 2 23-0-0 1-3/4"x 23-7/8" LVL Kerto-S 3	Length Product Plies Net Qty 7-0-0 1-3/4"x 9-1/4" LVL Kerto-S 2 2 12-0-0 1-3/4"x 14" LVL Kerto-S 2 2 7-0-0 1-3/4"x 14" LVL Kerto-S 2 2 23-0-0 1-3/4"x 23-7/8" LVL Kerto-S 3 3

	Conne	ctor Info	on	Nail Information			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss	
\bigcirc	HUS410	USP	10	NA	16d/3-1/2"	16d/3-1/2"	
\bigcirc	MSH422	USP	9	Varies	10d/3"	10d/3"	
\bigcirc	HUS412	USP	8	NA	16d/3-1/2"	16d/3-1/2"	
6	THDH614	USP	1	NA	16d /3-1/2"	16d /3-1/2"	

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

(045Fb	NRT FOR JAC ON 1 ABLES (\$502.5)) SK STUDG (COURACT)	4.000	BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
	FEADEWEIRDER	LEADER 100 145ADER	JOB NAME	Lot 7 Mitchell Manor	ADDRESS	Mitchell Manor Dr.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
END REAC	n and due C 410 C	LINE RIA UN REQUEST	PLAN	Gaston II (181035B) 3 Car/SL	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	N/A	DATE REV.	11	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 1500#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	B0520-1988	DRAWN BY	Marshall Naylor	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9			JOB #	J0522-2440	SALESMAN	Lenny Norris	Signature Marshall Naylor	Fax: (910) 864-4444

<u>Truss</u> <u>Placement</u> <u>Plan</u> SCALE: NTS



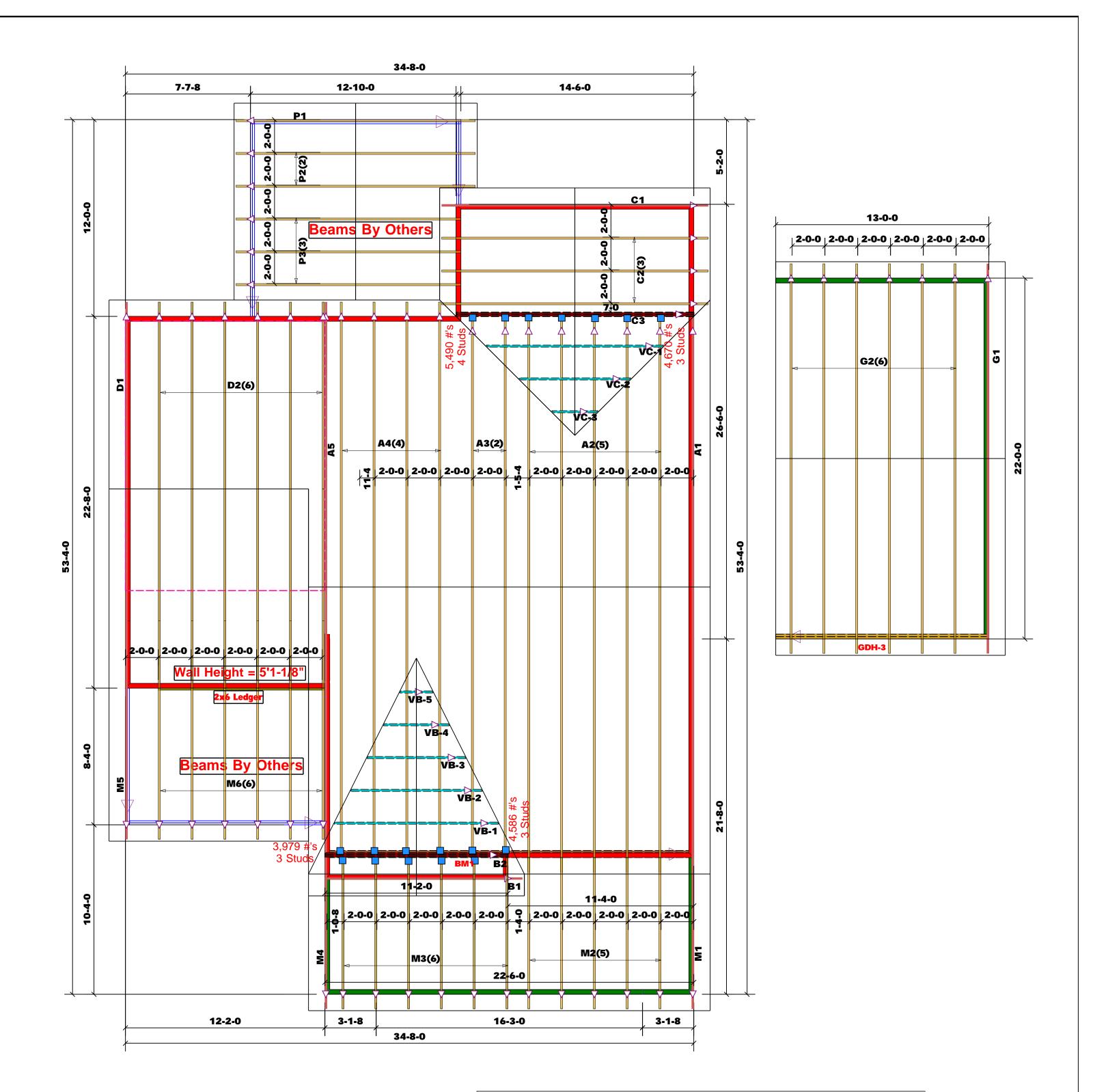
		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
6/0 Sliding Door HDR	7-0-0	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
FB1	12-0-0	1-3/4"x 14" LVL Kerto-S	2	2	FF
Window HDR	7-0-0	1-3/4"x 14" LVL Kerto-S	2	2	FF
FB2 (Top Flush W/ Floor)	23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3	FF
Side GDH (Top Flush W/ Floor)	22-0-0	1-3/4"x 23-7/8" LVL Kerto-S	2	2	FF

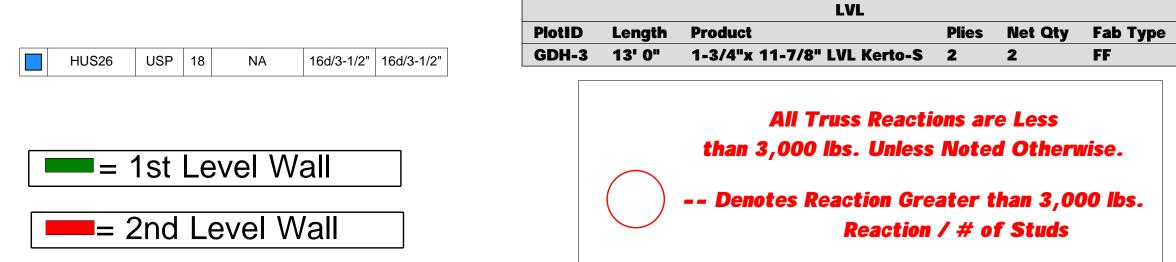
	Conne	ctor Info	rmati	on	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS410	USP	10	NA	16d/3-1/2"	16d/3-1/2"
\bigcirc	MSH422	USP	9	Varies	10d/3"	10d/3"
\bigcirc	HUS412	USP	8	NA	16d/3-1/2"	16d/3-1/2"
6	THDH614	USP	1	NA	16d /3-1/2"	16d /3-1/2"

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

	(04sEb	IRT FOR JA ON 1 ABLES (502.5) N STUDS (COUTA)	0.4.000	BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
NOL		PEADERVETROER	chack 00 EABER	JOB NAME	Lot 7 Mitchell Manor	ADDRESS	Mitchell Manor Dr.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
Ino silve	MAN HANG	an) Belgya	UN SING (N)	PLAN	Gaston II (181035B) 3 Car/SL	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
170 340 510	02	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	N/A	DATE REV.	11	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park
680 850 1020	05	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	B0520-1988	DRAWN BY	Marshall Naylor	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
1190 1360 1530	8 00			JOB #	J0522-2440	SALESMAN	Lenny Norris	Signature Marshall Naylor	Fax: (910) 864-4444

<u>Truss</u> <u>Placement</u> <u>Plan</u> SCALE: NTS

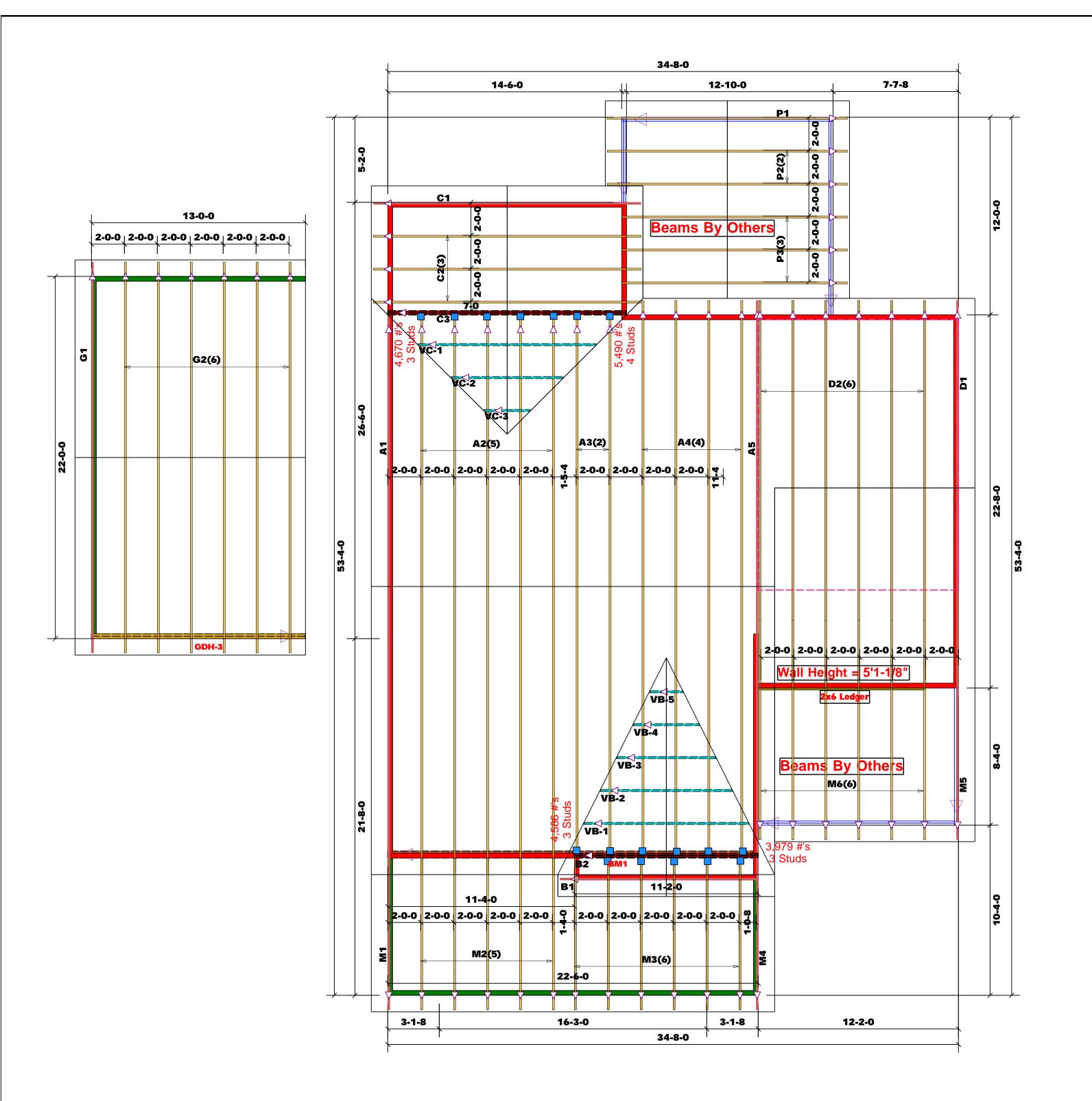


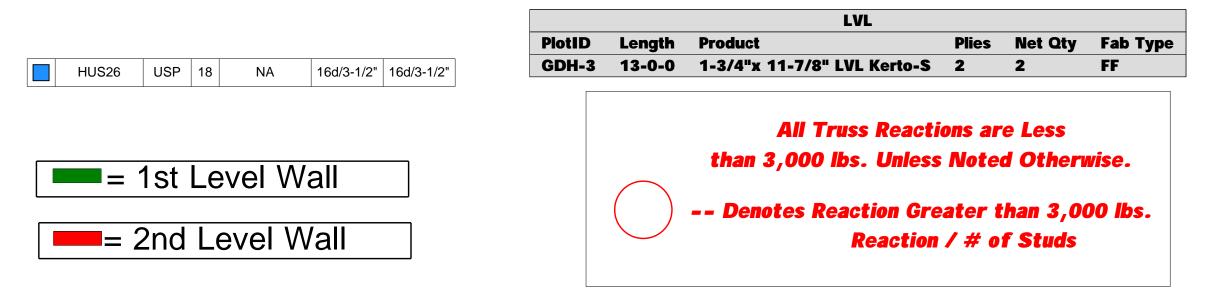


 \triangle = Indicates Left End of Truss (Reference Engineered Truss Drawing) **Do NOT Erect Truss Backwards**

	DAD CHART FOR ((045Pb on 1404Ps R5) LANCE OF JACK STUDS 4CO)	502 5(1) Å (b())		BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
NOLD	FEADEWERK		ubs Fux EXDER	JOB NAME	Lot 7 Mitchell Manor	ADDRESS	209 Mitchell Manor Dr.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
END REAL		Citary -	HEUDSI.	PLAN	Gaston II (181035B) 3 Car/SL	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
170 340 510	0 2 5100 2 0 3 7650 3	3 10200	2	SEAL DATE	N/A	DATE REV.	11	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park
680 850 1020	0 5 12750 5 0 6 15300 6	5 17000		QUOTE #		DRAWN BY	Marshall Naylor	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
1190 1360 1530				JOB #	J0522-2439	SALESMAN	Lenny Norris	Signature Marshall Naylor	Fax: (910) 864-4444

Truss Placement Plan SCALE: 1/4"=1'





= Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
•	Lot 7 Mitchell Manor	ADDRESS	209 Mitchell Manor Dr.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
	Gaston II (181035B) 3 Car/SL	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
E	N/A	DATE REV.	//	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	Reilly Road Industrial Park
		DRAWN BY	Marshall Naylor	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
	J0522-2439	SALESMAN	Lenny Norris	Signature Marshall Naylor	Fax: (910) 864-4444

<u>Truss</u> <u>Placement</u> <u>Plan</u> SCALE: 1/4"=1'

LOAD CHART FOR JACK STUDS

(DANFE ON 1 ABLES 2502 5(1) & (6)) NUMBER OF DACK STUDIE REQUIRE(DIE) CA CND OF FEADER/FERDER

ADARA CUT ALC ADA SOUTS CLOSE ADARA - VIALO

2550 1 5100 2

7650 3

10200 4 12750 5 15300 6 LINE RIACTION (J¹ TO) (J¹ TO) (A) RY HEADER

3400 ! 6600 2

10200 3

13600 4 17000 5

(ND REACTION (OF TO) SEC D STUDS FOR SEC D STUDS FOR

BUILDER

PLAN

JOB NAME

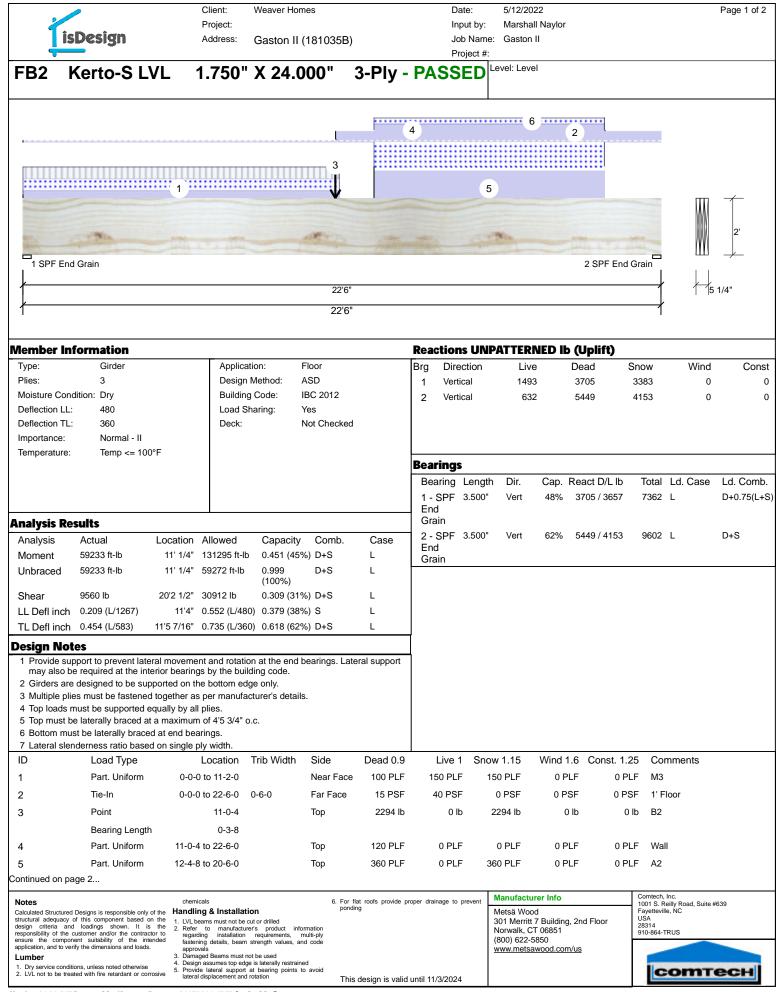
SEAL DATE

QUOTE #

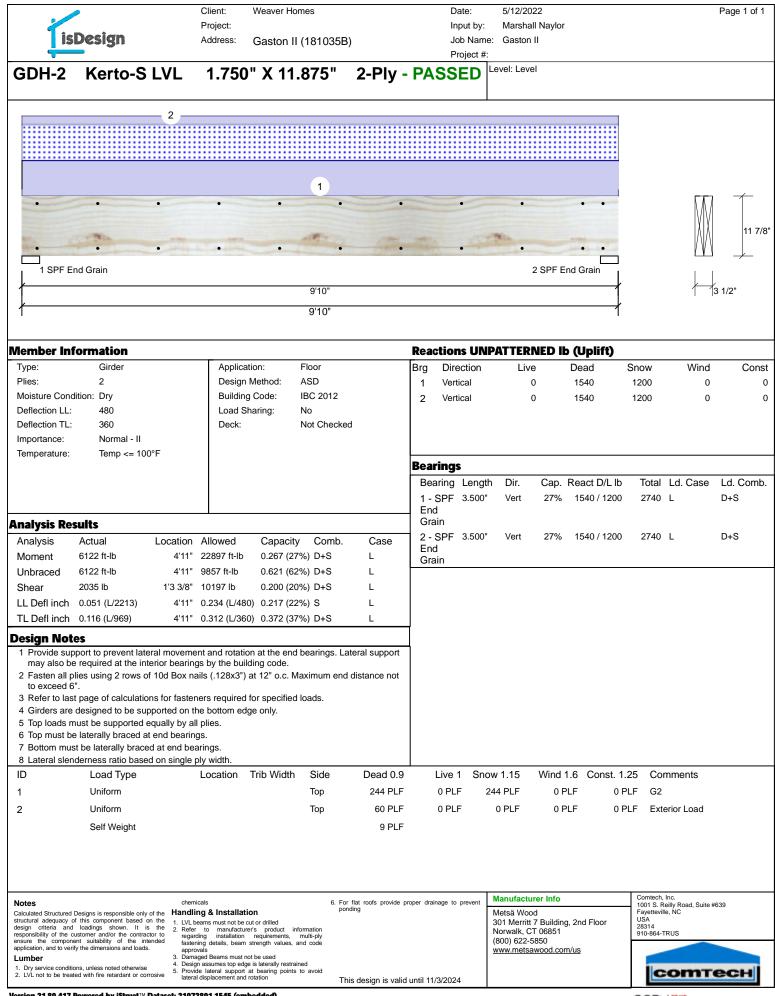
JOB#

2	•	Client:	Weaver H	omes		Date		5/12/2022				Page 1 o
Tie	Design	Project:	0			-	-	Marshall Nayl	or			
	-	Address:	Gaston	II (181035B)			Name: ect #:	Gaston II				
Vindow		o-S LVL	1 750")	(1/ 000"	2-Plv	- PASSE		el: Level				
WIIIGOW	IIDIX Kerti	0-0 202	1.750 7	(14.000	2-1 Iy	-14001						
, 1 SPF End	1 Grain	1	4 2 SPF En	d Grain								1'2"
		014.	2 0 2.									, 0.4.(0)
		6'1"									I I	3 1/2"
1		6'1"		1								
lember inf	ormation					Reactions	UNPA	TTERNED	lb (Uplift))		
Туре:	Girder		cation:	Floor		Brg Direct		Live	Dead	Snow	Wind	Co
Plies:	2		gn Method:	ASD		1 Vertica	al	723	4403	1838	0	
Moisture Cond	lition: Dry		ing Code:	IBC 2012		2 Vertica	al	282	2388	1142	0	
Deflection LL:	480	Load	Sharing:	No								
Deflection TL:	360	Deck		Not Checked								
Importance:	Normal - II											
Temperature:	Temp <= 100°F											
						Bearings						
						Bearing L	.ength	Dir. Cap	. React D/L	.lb Tota	Ld. Case	Ld. Cor
						1 - SPF 3	.000"	Vert 72	6 4403 / 19	6323	3 L	D+0.75(
						End						
nalysis Res	sults					Grain						
Analysis	Actual L	ocation Allowed	Capaci	ty Comb.	Case	2 - SPF 3. End	.000"	Vert 40	6 2388 / 11	42 3530) L	D+S
Moment	10224 ft-lb	1'11" 31049 ft-	b 0.329 (3	33%) D+0.75(L+	S) L	Grain						
Unbraced	10224 ft-lb	1'11" 17620 ft-	b 0.580 (5	58%) D+0.75(L+	S) L							
Shear	6150 lb	1'5" 12021 lb	0.512 (5	51%) D+0.75(L+	S) L							
LL Defl inch	0.016 (L/4193) 2'	3 15/16" 0.143 (L/										
	0.052 (L/1315)		, ,	27%) D+0.75(L+								
		21 01100 (2)			0, =	1						
esign Not	es port to prevent lateral	movement and ret	tion at the on	d boorings ato	ral cupport	1						
	e required at the interio			u bearings. Late	aa support							
	designed to be suppo		• •									
	s must be fastened to		ifacturer's det	ails.								
	iust be supported eque laterally braced at er											
•	t be laterally braced a	•										
	derness ratio based o											
ID	Load Type	Location	Trib Width	n Side	Dead 0.9	Live 1	Snow 1	1.15 Win	d 1.6 Cons	t. 1.25 C	omments	
1	Uniform			Тор	120 PLF	0 PLF	0	PLF (PLF	0 PLF W	all	
2	Point	1-9-8		Тор	3014 lb	1005 lb		0 lb	0 lb	0 lb F8	3	
	Bearing Length	0-3-8										
3	Point	1-11-0		Тор	2335 lb	0 lb	233	35 lb	0 lb	0 lb C:	3	
	Bearing Length	0-3-8										
4	Part. Uniform	2-0-0 to 6-1-0		Тор	158 PLF	0 PLF	158	PLF () PLF	0 PLF C	2	
	Self Weight			-6	11 PLF							
latas		chemicals		6 Er- 41	at roofe provide ar	oper drainage to pre	Ma	nufacturer Info)	Comtec		
lotes Calculated Structured	Designs is responsible only of th	he Handling & Instal	ation	6. For fia	ig ig	oper urainage to pre	Me	etsä Wood		Fayette	Reilly Road, Suite ville, NC	#639
م بمميتهمامم امتناميته	f this component based on the loadings shown. It is the	ne 1. LVL beams must not	be cut or drilled cturer's product	information			30	1 Merritt 7 Build rwalk, CT 0685		USA 28314	TRUE	
lesion criteria and									1	910-864	4-TRUS	
lesign criteria and esponsibility of the component ensure the component	ustomer and/or the contractor t ent suitability of the intende	to regarding installat ed fastening details, be					(80	00) 622-5850	,			
esign criteria and esponsibility of the consure the component	ustomer and/or the contractor t ent suitability of the intende fy the dimensions and loads.	to regarding installat	am strength values st not be used	and code			(80	00) 622-5850 /w.metsawood.o	<u>com/us</u>			

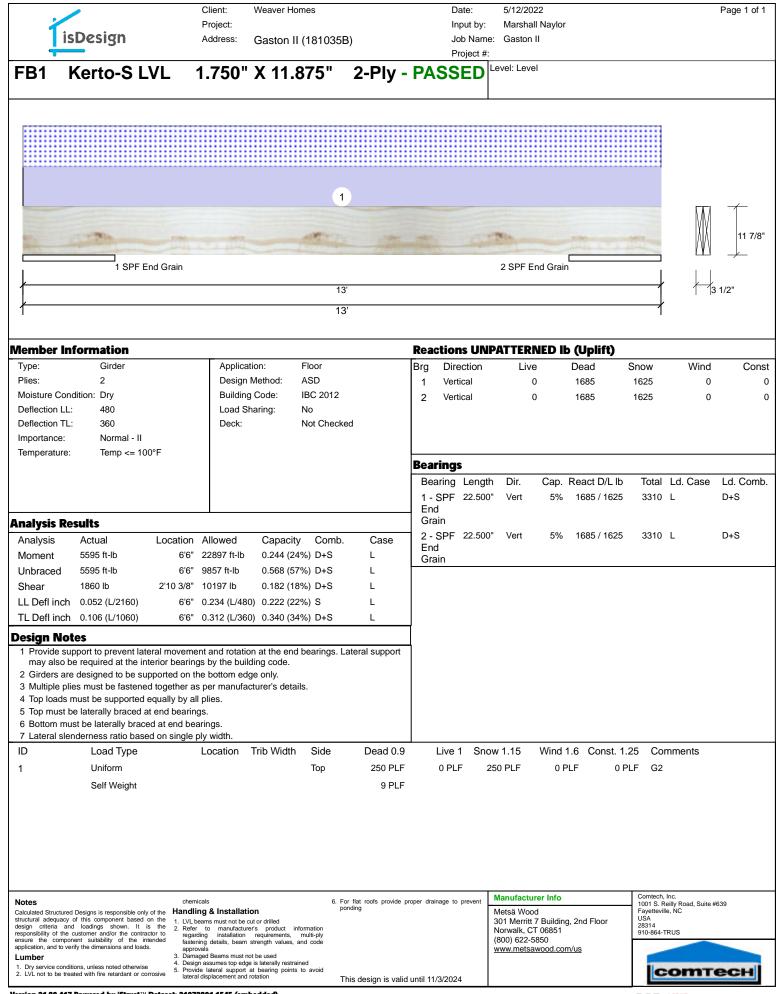
	Client:	Weaver Homes		Date:	5/12	/2022				Page 1 of 1
	Project:			Input	by: Mars	shall Naylor				
isDesign	Address:	Gaston II (181035	B)		lame: Gas	ton II				
6/0 Sliding Door HDP	Kerto-S LVL	1.750" X 9.2	250" 2 DI	Projection Projection		evel				
6/0 Sliding Door HDR	Kento-S LVL	1.750 A 9.2	250 2-61	y - PASSE	.U					
				3						
2				3						
		1								
									ΓÆ.	1 1
									IXIX	9 1/4
and the second second	THE LESS HER	all		The state	C. C. C. C.				///	
1 SPF End Grain				2	SPF End G	rain I				
		6'10"							/	3 1/2"
/		6'10"								
Member Information				Reactions	UNPATTE	RNED Ib	(Uplift)			
Type: Girder	Applica			Brg Direction	on l	_ive [Dead	Snow	Wind	Const
Plies: 2	-	Method: ASD		1 Vertical			1965	1162	0	0
Moisture Condition: Dry	Building			2 Vertical	1	100	1965	1162	0	0
Deflection LL: 480 Deflection TL: 360	Load SI Deck:	Not Check	(ed							
Importance: Normal - II	Deck.	Not Oncer	icu -							
Temperature: Temp <= 100°F										
				Bearings						
				Bearing Le	-	Cap. R	eact D/L lb		Ld. Case	Ld. Comb.
				1 - SPF 3.0 End	000" Vert	42%	1965 / 1696	3662	L	D+0.75(L+S)
Analysis Results	I			Grain						
	ocation Allowed	Capacity Comb	. Case	2 - SPF 3.0 End	000" Vert	42%	1965 / 1696	3662	L	D+0.75(L+S)
Moment 5588 ft-lb	3'5" 14423 ft-lb	0.387 (39%) D+0.75	5(L+S) L	Grain						
Unbraced 5588 ft-lb	3'5" 10130 ft-lb	0.552 (55%) D+0.75								
Shear 2573 lb	1' 1/4" 7943 lb	0.324 (32%) D+0.75								
LL Defl inch 0.051 (L/1511)		0) 0.318 (32%) 0.75(L·								
TL Defl inch 0.111 (L/700)	3'5" 0.215 (L/360	0) 0.514 (51%) D+0.75	5(L+S) L	4						
Design Notes			L = t = m = 1 =							
 Provide support to prevent lateral may also be required at the interior 			Lateral support							
2 Girders are designed to be suppo										
3 Multiple plies must be fastened to4 Top loads must be supported equilation		cturer's details.								
5 Top must be laterally braced at er										
6 Bottom must be laterally braced a7 Lateral slenderness ratio based o	U									
ID Load Type		Trib Width Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.	6 Const. 1.	25 Coi	nments	
1 Uniform		Тор	108 PLF	322 PLF	0 PLF	0 PL	F OF	LF F4		
2 Uniform		Тор	120 PLF	0 PLF	0 PLF	0 PL	F OF	LF WA	LL	
3 Uniform		Тор	340 PLF	0 PLF	340 PLF	0 PL	F 0F	LF A4		
Self Weight			7 PLF							
Notos	chemicals	~ -	For flat racks and the	ronor dreisone to	Manufa	cturer Info		Comtech,		
Notes Calculated Structured Designs is responsible only of the	roper drainage to preve	Metsä V	Vood		Fayettevill	eilly Road, Suite # e, NC	639			
structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor		Norwalk	rritt 7 Building, , CT 06851	2nd Floor	USA 28314 910-864-T	RUS				
ensure the component suitability of the intender application, and to verify the dimensions and loads.	ed fastening details, beam approvals	requirements, multi-ply strength values, and code			(800) 62		us			
Lumber 1. Dry service conditions, unless noted otherwise	 Damaged Beams must no Design assumes top edge Provide lateral support 	e is laterally restrained						1100	OFFE	
2. LVL not to be treated with fire retardant or corrosiv	 Provide lateral support lateral displacement and 		This design is valid	until 11/3/2024				9	OMT	ech
Version 21 90 417 Deword by istruct M Dat	and 24072004 4545 (am)	haddad0								

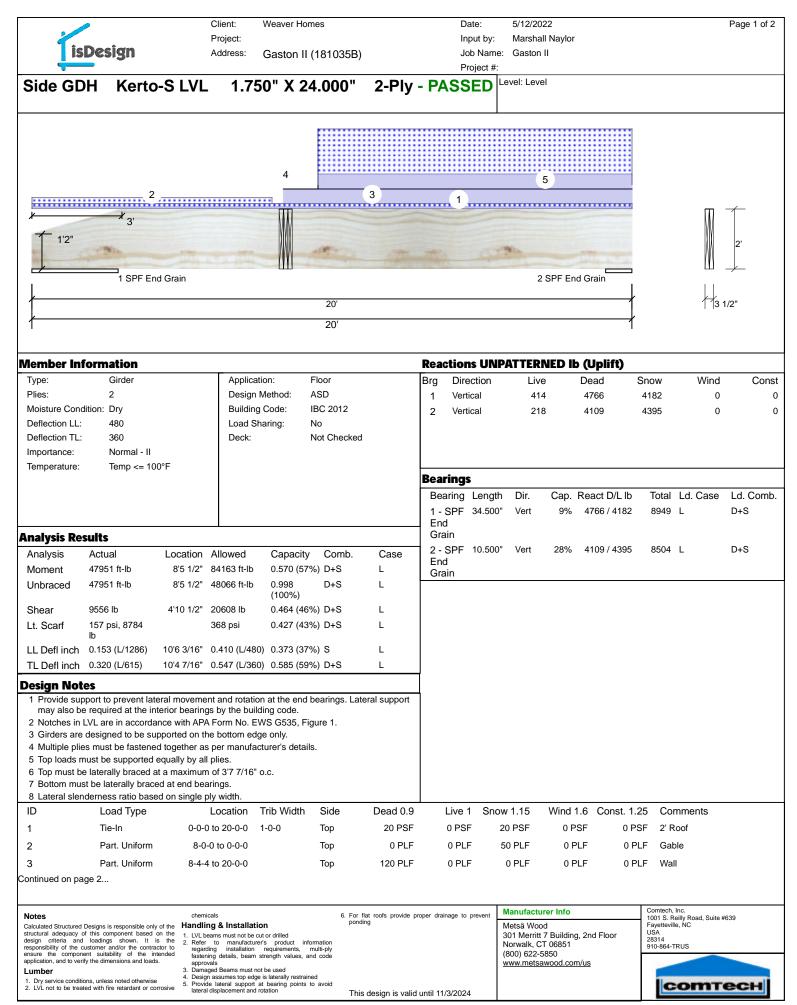






Version 21.80.417 Powered by iStruct™ Dataset: 21072801.1545 (embedded)







Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	1001 S. Reilly Road, Suite #639	
structural adequacy of this component based on the	Handling & Installation 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-phy fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	ponding This design is valid until 11/3/2024	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Fayetteville, NC USA 28314 910-864-TRUS	

