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ELECTRICAL LAYOUT NOTES:

1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.

2.) VANITY LIGHTS TO BE SET @ 90" AFF. (TYP.)

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.

4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- ➡ IIØ ∨ OUTLET
- 😑 110 v GFI OUTLET
- IIØ V SWITCHED OUTLET
- + 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110V GFI

- Ø 10 V DEDICATED CIRCUIT
- 120 Y DEDICATED CIRCUIT
- SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- PENDANT LIGHT

- FLUORESCENT LIGHT
- \$ SWITCH
- \$_D DIMMER SWITCH
- \triangle data
- TELEPHONE AND DATA
- TY- TY CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- 110 V SMOKE/ CM DETECTOR
- 5D 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- ALARM ALARM PANEL
- ~ <u>`</u>
 - CEILING FAN
- CEILING FAN W/ LIGHT





1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.

2.) VANITY LIGHTS TO BE SET @ 90" AFF. (TYP.)

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.

4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- 😑 IIØ V GFI OUTLET
- BB 🕂 IIØ V BASEBOARD OUTLET
- -∰H 4-PLEX
- COUNTER OR FLOOR MOUNTED

COUNTER OR FLOOR MOUNTED 110V GFI

- UEATHERPROOF
- Ø 10 V DEDICATED CIRCUIT
- 120 Y DEDICATED CIRCUIT

● SPECIAL PURPOSE (240 V, ETC.)

- WALL MOUNT LIGHT

- CEILING MOUNT LIGHT
- PENDANT LIGHT
- MINI CAN LIGHT

\$ SWITCH

- \$_D DIMMER SWITCH

- TELEPHONE AND DATA
- TV- TV CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- 110 V SMOKE/ CO DETECTOR
- SD 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL

ALARM ALARM PANEL

OPTIONAL OWNER'S BANK



CEILING: FAN W/ LIGHT



		MACON									
		MASON									
	WALL HEIGHT (FEET)			4" BRICK AND							
PRINT LASIGN LASIGN <thlasign< th=""> <thlasign< th=""> <thlasign< td="" th<=""><td></td><td>8" CMU</td><td>4" CMU</td><td>8" CMU</td><td>12" CMU</td><td></td><td>1</td><td></td><td></td><td>1</td><td></td></thlasign<></thlasign<></thlasign<>		8" CMU	4" CMU	8" CMU	12" CMU		1			1	
	2' OR LESS	UNGROUTED	SOLID	UNGROUTED	UNGROUTED		-		— 14' <i>-Ø</i> " ————		
	3'	UNGROUTED	GROUT SOLID	UNGROUTED	UNGROUTED	-+					
	4'	GROUT SOLID	GROUT SOLID w/ #4 REBAR @ 48" O.C.	GROUT SOLID	GROUT SOLID w/ #4 REBAR @ 64" O.C.					r	
Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control Image: State Control <t< td=""><td>5'</td><td>GROUT SOLID w/ #4 REBAR @ 36" O.C</td><td>n/A</td><td>GROUT SOLID w/ #4 REBAR @ 36" O.C</td><td>GROUT SOLID w/ #4 REBAR @ 64" O.C.</td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	5'	GROUT SOLID w/ #4 REBAR @ 36" O.C	n/A	GROUT SOLID w/ #4 REBAR @ 36" O.C	GROUT SOLID w/ #4 REBAR @ 64" O.C.						
	6'	GROUT SOLID w/ #4 REBAR @ 24" O.C	N/A	GROUT SOLID w/ #4 REBAR @ 24" O.C	GROUT SOLID w/ #4 REBAR @ 64" O.C.						
	7' OR MORE		ENGINEERED BASED	ON SITE CONDITIONS		_					4" CC
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 All - ALXA - RUNCE TO HALF OF RUNCE TO HALF OF RUNCE AT MULTICAL LICENCY TO HALF OF RUNCE AT MULTICAL RUNCE AT	STR	UCTURAL NOTE	<u>S:</u>				-				
	1. T. T 2. T 3. W 4. Pl 5. M 6. B, 7. B, G C N 8. L 9. W G	ABLE ABOVE APPLIE O GARAGE FOUNDA IE MULTIPLE WYTHE /ALL HGT. IS MEASU REP SLAB PER <u>R506.</u> INIMUM 24" LAP SPI ACKFILL OF CLEAN <i>#</i> ACKFILL OF WELL DI RADE) CLASSIFIED <i>#</i> LASSIFICATION SYS [*] CRC ARE ALLOWABL DCATE REBAR IN CE /HERE REQUIRED, F [*] ROUT. USE OF "LOW	S TO HOUSE FOUNDA TION NOT COMMON T ES TOGETHER WITH L RED FROM TOP OF FO <u>2.1</u> AND <u>R505.2.2</u> BAS LICE LENGTH. \$57/ #67 WASHED ST RAINED SAND-GRAVE AS GROUP 1 ACCORDI TEM IN ACCORDANCE E. NTER OF FOUNDATIO ILL BLOCK SOLID WIT V LIFT GROUTING" ME	ATION ONLY. TABLE I FO HOUSE. ADDER WIRE @ 16" DOTING TO TOP OF N SE AND EXCEPTION O TONE IS PERMITTED. L MIXTURE SOILS (4! ING TO UNIFIED SOII E WITH <u>TABLE R405.1</u> IN WALL. TH TYPE "S" MORTAR ETHOD REQUIRED WI	OOES NOT APPLY O.C. VERTICALLY. VALL. OF THE 2018 NCRC S PSF/FT BELOW S OF THE 2018 OF THE 2018 OR 3000 PSI HEN FILLING					26'-5" ll 	
1. FTRUCTURAL DESIDE OF NOT CALOURA Seminative Constraints of the Caloura of the Caloura Provide of the Caloura of the Caloura of the Caloura Provide of the Caloura of the Caloura of the Caloura Provide of the Caloura of the Caloura of the Caloura Provide of the Caloura of the Caloura of the Caloura Provide of the Caloura of the Caloura of the Caloura Provide of the Caloura of the Calo	W <u>ULTIMATE D</u> <u>TH</u>	ALLS WITH GROUT ESIGN WIND SPEED HAN 30' MEAN ROOF	AT HEIGHTS OF 5' AN NOTES FOR LESS HEIGHT:	ID GREATER.							
INSURTON VALUES OF THE BUILDING TO EEN MADE ON CONSTRUCTION OF MERCONSE SUBJECTION INVECTOR SPACING AND ENBEDMENT INTE: HONIZONTAL FOOTING REAR REQUIRED INTE: HONIZON FOR DEAR REQUIRED INTE: HONIZON FOR DEAR REQUIRED INTE: HONIZON FOR DEAR REQUIRED INTE: HONIZON FOR DEAR REQUIRED INTE: HONIZON FOR DEAR REQUIRED INTE: HONIZON FOR DEAR REQUIRED INTE: HONIZON FOR DEAR REQUIRED INTER ADDIFICIENT DEAR REQUIRED FOR DEAR REQUIRED INTER ADDIFICH DEAR REAR REQUIRED FOR DEAR REQUIRED FOR DE	 STRUCTUR/ RESIDENTI/ BOLTS 6'-0' EACH CORN MINIMUM C MASONRY. OF PLATE V FOR 130 MI BOLTS 4'-0' EACH CORN MINIMUM C MASONRY. OF PLATE V MEAN ROOI EXTERIOR V WINDS. INSTALL 7/ WALLS OF / SECTION R ENERGY EF 	AL DESIGN PER NOR AL CODE, 2018 EDIT PH WIND ZONES INS OC. AND WITHIN ER. ANCHOR BOLT OF 7" INTO CONCRET LOCATE BOLT WITH VIDTH. PH WIND ZONES INS OF 7" INTO CONCRET LOCATE BOLT WITH VIDTH. F HEIGHT IS LESS TH WALLS DESIGNED FO ALL STORIES IN ACC 602.10.3 OF THE NC FICIENCY COMPLIAN	TH CAROLINA TON. STALL 1/2" ANCHOR 1'-0" FROM END OF S MUST EXTEND A TE OR 15" INTO IIN MIDDLE THIRD STALL 1/2" ANCHOR 1'-0" FROM END OF S MUST EXTEND A TE OR 15" INTO IN MIDDLE THIRD HAN 30 FEET. OR 120 OR 130 MPH S ON ALL EXTERIOR CORDANCE WITH RC, 2018 EDITION.	8" FDN. ON 16" W	IDE BY 8" DEEP CONT CONC. FTG. (TYP.	- 40'-0" - 26'-0" 			- 13'-5" 16" WIDE BY 9" DEEF THICKENED SLAB (TYP.		- 4" CON SLAB
SHCTING 0-01 OLC 4-01	ACCORDAN 2018 EDITI ANCHO WIND ZONE	DR SPACING AND EN	MBEDMENT	ODTIO						 	[™] [™] [™] [™] [™] [™] [™] [™]
INTE: HORZONTAL POOTING REBAR REQUIRED INTE: HORZONTAL POOTING REBAR REQUIRED INTE: HORZONTAL POOTING REBAR REQUIRED 2 × 10 R2 × 5 STUD FRAMING 2 × 10 R2 × 5 STUD 2 × 10 R2 × 10 × 10 × 10 × 10 × 10 × 10 × 10 × 1	SPACING	6'-0" O.C.	4'-0" O.C.	OPTIC VENEER (NAL CULIURED STON SEE ELEVATION PAGE						
NOTE: HORIZONTAL FOOTING REBAR REQUIRED IN HIGH WIND ZONES ONLY (140-150 MPH) 2 x 1 RTD. BOTTOM PATTES) SECURED AND DEBODED PER CONST FOR NOOR OPENINGS AND WITHIN 12' OF THE RNDS OF EACH PARTE SEE IZIA MEHO RI JON OF EACH PARTE SEE JAM HO RI JON OF HALTES WIND DESIGN NOTES FOR DETAILS WIND DESIGN NOTES FOR DETAILS UNDOCUMENT DO LOUGH STEM WALL REINFORCEMENT OF WOLES FOR DETAILS UNDOCUMENT DO LOUGH STEM WALL REINFORCEMENT STEM WALL REINFORCEMENT OF WOLE STOL DU LOUGH FILLES SOLD.	EMBEDMENT	7"	7" INTO CONCRETE 15" INTO MASONRY	TOREOCATE					- 4" CONC		
2 x 4 0R 2 x 6 STUD FRAMING 2 x 1 RTD, ROTICIP ILATE(S) SECUED AND FRAMING 2 x 1 RTD, ROTICIP ILATE(S) SECUED AND FRAMING 4 'CONCRETE SLAP 4 'CONCRETE SLAP 5 HATHING 4 'CONCRETE SLAP 4 'CONCRETE SLAP 5 HATHING 5 H	NOTE: HORI		EBAR REQUIRED							<u> </u>	
PRAMING 2. X TRTD. BOTTOM PLATE(S) SECURED AND EDDED PER CODOR OPENING AND WITHIN 12' OF THE ENDS OF EACH PLATE SECIEND MEN CODOR OPENING SECIEND MEN CODOR OPENING SECIEND MEN CODOR OPENING WIND DESIGN NOTES FOR DETAILS WIND DESIGN NOTES FOR DETAILS WIND DESIGN NOTES FOR DETAILS WIND DESIGN NOTES FOR DETAILS SIDING AS SPEC. EXPANSION JOINT 4" CONCRETE SLAB- WI FIBER REINFORCING G MIL VAPOR CONCRETE SLAB- WI FIBER REINFORCING WI FIBER REINFORCING UNDISTURBED EARTH- TOP TWO COURSES OF STEM WALL REINFORCEMENT STEM WALL RE	intrion	2 x 4 C	$R 2 \times 6 \text{ STUD}$				<mark>_L</mark>	_'		<u> </u>	
6 MIL. VAPOR BARRIER UNDISTURBED EARTH, COMPACTED FILL OR WASHED STONE TOP TWO COURSES OF STEM WALL AND ALL CELLS W/ REINFORCEMENT TO BE FILLED SOLID. CONC. FTG.	2 X TRTD BEDDED PER CC AND WITHIN S	9. BOTTOM PLATE(S) S DDE (ADJUST FOR DOO N 12" OF THE ENDS OF EE 120 MPH OR 130 M WIND DESIGN NOTES EXPAN 4" CONCF W/ FIBER REIN	FRAMING SECURED AND OR OPENINGS) EACH PLATE. PH ULTIMATE FOR DETAILS NSION JOINT RETE SLAB		SIDING AS SPEC. SHEATHING		- -]'	4" 5'-Ø" 16" x 16" x (W/ VENEE 24" x 24" x	 1'-4" 36" CONC. PIER R) ON 12" CONC. FTG. 	.4"	
OR WASHED STONE TOP TWO COURSES OF STEM WALL AND ALL CELLS W/ REINFORCEMENT TO BE FILLED SOLID. STEM WALL FDN. WALL REINFORCEMENT 16" WIDE BY 8" DEEP CONT. CONC. FTG.		6 MI UNDISTURE COMP/	IL. VAPOR BARRIER BED EARTH,		LADDER AND UN 8" CMU	WIRE EVERY OTHER COURSE DER TOP COURSE GOLID FILLED	-		— 4' <i>-Ø</i> '' ————	2'	-10 1/2"
FILLED SOLID.		OR WASH	HED STONE TOP TWO COURSES OF STEM WALL AND ALL CELLS W/		STEM W WALL REINFORC	ALL FDN. MENT CONT.	-				
		R	FILLED SOLID.								

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TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEET)	MAXIMUM STUD SPACING (INCHES) (PER TABLE R602,3(5)				
(* == :)	16	24			
UP TO 3'	1	1			
4'	2	1			
8'	3	2			
12'	5	3			
16'	6	4			

STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET



S



DATE: JUNE 22, 2021

REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

ATTIC FLOOR FRAMING PLAN

S-3



roof plan S-4





	Bearing deemed requiren attached requiren attached requiren attached reaction Signatur Signatur Not Not Signatur 1700 3400 5100 68500 10200	ROC RUS Reilly R Fayett Phon Fax: reactions to comply ents. The Tables (number of s greater A register to design to design to design to design to design to design that exce A register to design to des	Definition of the second state of the second			
Legend dded HVAC ay Ceiling d Floor Walls op Beam). Angier / Harnett	Mitchell Manor Drive	Roof	1. 12/30/21	Y David Landry	P. Lenny Norris
formation Truss 16d/3-1/2" 10d/3" Net Qty 3	Co. Inc. CITY / CO	Sec. II ADDRESS	C MODEL	DATE REV	DRAWN B	SALES REI
3 2 2 2	R Weaver Development (ME Lot 18 Mitchell Manor	Brinkley "A" / 3GRF, C	ATE N/A	#	J1221-7067
	THIS IS These t comport design german for the	A TRUSSE A TRUSSE A TRUSSE at the spelividual de don the er is responent braci- overell	NUT CONTRACT NOT C	VG IVG IVHUUUUUUUUUUUUU	GRAM ON vidual bui uiding de ich truss g. The bui ary and floor syst	# BOC LLY. ilding ding ssigner. design ilding term and russ

4 All autorian wall to wall dimensions are to
 All exterior wall to wall dimensions are to face of sheathing unless noted otherwise All interior wall dimensions are to face of frame wall unless noted otherwise All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

	All V Consid	Valls Shown Are dered Load Bea	e ring	
			Ha	atch Legend
oof Area = 22 dge Line = 83	11.85 sq.ft. .75 ft.			Padded H
p Line = 0 ft. priz. OH = 189.58 ft.				Tray Ceilin
aked OH = 23 ecking = 76	3.07 ft. sheets			2nd Floor
				Drop Bean
C	onnector I	nformation	Na	il Informatio

	Conne	Nail Info	ormation			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	19	NA	16d/3-1/2"	16d/3-1/2"
	THD26-2	USP	5	NA	16d/3-1/2"	10d/3"

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	22' 0"	1-3/4"x 16" LVL Kerto-S	3	3
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	3	3
BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH	22' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
GDH2	12' 0"	2x12 SPF No.2	2	2

Truss Placement Plan Scale: 1/4"=1

____= Indicates Left End of Truss (Reference Engineered Truss Drawing) **Do NOT Erect Truss Backwards**

for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



A = Indicates Left End of Truss (Reference Engineered Truss Drawing) **Do NOT Erect Truss Backwards**

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



RE: J1221-7067

Lot 18 Mitchell Manor Sec. II

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Weaver Development Co. Inc. Lot/Block: 18	Proje Mode
Address: Mitchell Manor Drive	Subc
City: Angier	State

ct Name: J1221-7067 el: Brinkley division: Mitchell Manor Sec. II : NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 150 mph Floor Load: N/A psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E16001299	A1	8/3/2021	21	E16001319	M1GE	8/3/2021
2	E16001300	A1GE	8/3/2021	22	E16001320	M2	8/3/2021
3	E16001301	A2	8/3/2021	23	E16001321	M2A	8/3/2021
4	E16001302	A3	8/3/2021	24	E16001322	V1	8/3/2021
5	E16001303	A3A	8/3/2021	25	E16001323	V2	8/3/2021
6	E16001304	A4	8/3/2021	26	E16001324	V3	8/3/2021
7	E16001305	A4SG	8/3/2021	27	E16001325	V4	8/3/2021
8	E16001306	B1	8/3/2021	28	E16001326	V5	8/3/2021
9	E16001307	B1A	8/3/2021	29	E16001327	V6	8/3/2021
10	E16001308	B1GE	8/3/2021				
11	E16001309	C1-GR	8/3/2021				
12	E16001310	C1SG	8/3/2021				
13	E16001311	D1	8/3/2021				
14	E16001312	D1GE	8/3/2021				
15	E16001313	G1	8/3/2021				
16	E16001314	G1GE	8/3/2021				
17	E16001315	H1GE	8/3/2021				
18	E16001316	J1	8/3/2021				
19	E16001317	J1GE	8/3/2021				
20	E16001318	M1	8/3/2021				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

August 03, 2021

Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Mano	r Sec. II	E10001000
J1221-7067	A1	COMMON	1	1			E16001299
					Job Reference (optio	inal)	7.54.00.0004 D (
Comtech, Inc, F	ayetteville, NC - 28314,		ID:I4HRAT3eIT9	8.430 s Ju aoRidAoE:	n 22021 Milek Indus s 5z0Axv-pWGVSa9V	stries, Inc. Tue Aug 30 Vmr6nthb4c3R2ZWvIUv	vFIIhR4O_vzMvvrd61
F	6-8-8	15-8-8	24-8-8	3		33-8-0	34-7-0
	6-8-8	9-0-0	9-0-0			8-11-8	0-11-0
21-22-8 3x4 8-6-1 14 6-1 14	4x6 = 4x6 = 2 15 13 $x6 = 1$	6.00 12 16 16 16 12 12 11 19 4x6 =	5x8 =	7 1 0 9 3x4 =	2x4 // 4x6 = 5 6	18	Scale: $3/16'=1'$
Plate Offsets (X,Y)	<u>9-8-8</u> <u>9-8-8</u> [7:0-1-4,0-0-7], [13:0-1-8,0-4-0		4x 1-8-8 2-0-0	• — —	33	3-8-0 -11-8	—
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0 Plate Grip DOL 1.1 Lumber DOL 1.4 Rep Stress Incr YE	0 CSI. 5 TC 0.36 5 BC 0.66 S WB 0.57	DEFL. in Vert(LL) -0.35 Vert(CT) -0.48 Horz(CT) 0.05	(loc) 9-12 9-12 7	l/defl L/d >999 360 >833 240 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.07	7-9	>999 240	Weight: 230 lb	FT = 20%
LUMBER- TOP CHORD 2x6 3 BOT CHORD 2x6 4 WEBS 2x4 3 1-13 REACTIONS. (s Max Max	SP No.1 SP No.2 *Except* : 2x6 SP No.1 :Ex6 SP No.1 :Horz 13=-193(LC 13) :Uplift 13=-222(LC 12), 7=-263(L :Grav 13=1333(LC 1), 7=1379(L	C 13) S 1)	BRACING- TOP CHORD BOT CHORD WEBS	Structur except e Rigid ce 1 Row a	al wood sheathing di end verticals. iling directly applied t midpt	rectly applied or 4-9-1 or 9-9-4 oc bracing. 2-13	0 oc purlins,
FORCES.(lb) - MaTOP CHORD1-2BOT CHORD12WEBS2-1	ux. Comp./Max. Ten All forces 2 2=-300/179, 2-4=-1841/805, 4-6=- -13=-482/1658, 9-12=-230/1276, 12=-242/311, 4-12=-140/593, 4-9=	50 (lb) or less except when shown. 2084/871, 6-7=-2336/875, 1-13=-2 *-9=-635/1990 -273/970, 6-9=-522/454, 2-13=-18	54/214 06/660				
NOTES- 1) Unbalanced roof I 2) Wind: ASCE 7-10 and C-C Exterior(members and for 3) This truss has bee 4) * This truss has be will fit between the 5) Refer to girder(s) 6) Provide mechanic 13=222 7=263	ive loads have been considered f ; Vult=150mph Vasd=119mph; TC 2) 0-4-4 to 4-9-1, Interior(1) 4-9-1 ses & MWFRS for reactions show en designed for a 10.0 psf bottom een designed for a live load of 30 e bottom chord and any other mei for truss to truss connections. al connection (by others) of truss	or this design. DL=6.0psf; BCDL=6.0psf; h=15ft; to 15-8-8, Exterior(2) 15-8-8 to 20- 1; Lumber DOL=1.60 plate grip DC chord live load nonconcurrent with Opsf on the bottom chord in all are nbers, with BCDL = 10.0psf. to bearing plate capable of withsta	Cat. II; Exp C; Enclosed :1-5, Interior(1) 20-1-5 to \L=1.60 any other live loads. as where a rectangle 3-6 nding 100 lb uplift at joir	l; MWFRS 534-4-10 6-0 tall by ht(s) excep	6 (envelope) zone;C-C for 2-0-0 wide pt (jt=lb)		2.

7) This trues is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



August 3,2021

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Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II	
J1221-7067	A1GE	COMMON SUPPORTED GAB	1	1	E16001300	
					Job Reference (optional)	
Comtech, Inc, Fayette	mtech, Inc, Fayetteville, NC - 28314, 8.430 s Jun 2 2021 MiTek Industries, Inc. Tue Aug 3 07:54:22 2021 Pr					
		ID:I4HRAT3eIT9qoRIdAoEs_5z0Axy-lvOGtGBnITMV6?ITjTTXex1Adk56mhhNslR4Royrd6?				

NOTES-

11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2401/917, 2-4=-1846/799, 4-5=-1715/885, 5-7=-1992/885, 7-8=-2282/897 1-14=-633/2050, 13-14=-633/2050, 4-13=-270/273, 10-12=-64/251, 8-10=-652/1953 BOT CHORD 2-14=0/303, 2-13=-561/329, 10-13=-188/1073, 5-13=-355/764, 5-10=-253/676, WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-8-8, Exterior(2) 15-8-8 to 20-1-5, Interior(1) 20-1-5 to 34-4-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.

7-10=-522/457

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=232.8=271.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



August 3,2021

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



	-							
Job	Truss	Truss Type	Qt	y	Ply	Lot 18 Mitchell Man	or Sec. II	E16001302
J1221-7067	A3	COMMON	3		1			
Comtech Inc					 B/130 e lu	Job Reference (opti p. 2 2021 MiTek Indu	onal) Istries Inc. Tue Aug. 3 (17:54:24 2021 Page 1
Connech, inc,	ayettevine, 140 - 20314,		ID:I4HRA	3elT90	oRidAoE	s_5z0Axy-hHW0HxD	1q4cDLJurruV?kM6RTX	atEVQgJbwBUgyrd5z
F	6-8-8	15-8-8	1	24-8-8	3		33-8-0	34-7-0
	0-0-0	3-0-0		3-0-0			0-11-0	0-11-0
								Scale: 3/16"=1'
		E	5x8 =					
		6.00 12	4					
I								
			\sim / f^{*}					
		16	$\parallel \sim$	17	7			
	4x6 📁			\sim		2x4 //		
	4x6 ==				\sim	4x6 5		
12	2	/ //				6		
9-7-	15					19		
		、 //	\backslash	$\langle \rangle$			\sim	
3x4		\sim //					18	
				//	\setminus /			
8	e			```	$\backslash //$			78
				þ				
	13 19 2	0 12 11 21		22 1	09	23	24	
14	4 13	4x6 =			3x6 =			4,00 —
6	5x6 =	3x4 =		5x	8 =			
F	9-8-8	12	-8-8 2-0-0			1	1-11-8	
Plate Offsets (X,Y)	- [7:0-1-4,0-0-7], [13:0-1-8,0-4-4	1					1	
LOADING (psf)	SPACING- 2-0	-0 CSI .	DEFL.	in	(loc)	l/defl l/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.	5 TC 0.36	Vert(LL)	-0.30	9-12	>999 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.	5 BC 0.78	Vert(CT)	-0.42	9-12	>946 240		
BCDL 10.0	Code IRC2015/TPI201	4 Matrix-S	Wind(LL)	0.06	7-9	>999 240	Weight: 230 lt	o FT = 20%
	SP No 1		BRACING-	П	Structur	al wood sheathing (lirectly applied or 4-5-3	
BOT CHORD 2x6	SP No.1			D	except e	and verticals.		o oc parinis,
WEBS 2x4	SP No.2 *Except*		BOT CHOR	D	Rigid ce	iling directly applied	l or 9-9-4 oc bracing.	
1-13	3: 2x6 SP No.1		WEBS		1 Row a	t midpt	2-13	
REACTIONS.	size) 13=Mechanical, 7=0-3-8							
Ma	x Horz 13=-193(LC 13)	2.40						
Ma	x Opiint 13=-222(LC 12), 7=-263(L x Grav 13=1525(LC 2), 7=1551(L	C 13) C 2)						
		,						
FORCES. (lb) - Ma	ax. Comp./Max. Ten All forces 2	50 (lb) or less except when shown.	55/214					
BOT CHORD 12	2=-323/179, 2-4=-2090/805, 4-6=- 2-13=-482/1866, 9-12=-230/1449.	2413/871, 0-7=-2040/873, 1-13=-2 7-9=-635/2287	55/214					
WEBS 2-	12=-242/311, 4-12=-140/683, 4-9	-273/1190, 6-9=-522/454, 2-13=-1	940/660					
NOTES-								
1) Unbalanced roof	live loads have been considered f	or this design.						
2) Wind: ASCE 7-10); Vult=150mph Vasd=119mph; T	CDL=6.0psf; BCDL=6.0psf; h=15ft;	Cat. II; Exp C; En	closed	; MWFRS	6 (envelope)		
and C-C Exterior	(2) 0-4-4 to 4-9-1, Interior(1) 4-9-1	to 15-8-8, Exterior(2) 15-8-8 to 20-	1-5, Interior(1) 20	-1-5 to	34-4-10	zone;C-C for		
3) This truss has be	en designed for a 10.0 psf bottom	chord live load nonconcurrent with	any other live loa	ds.				
4) * This truss has b	been designed for a live load of 30	Opsf on the bottom chord in all are	as where a rectan	gle 2-6	5-0 tall by	2-0-0 wide	- And Web	NU J Simo
5) Refer to girder(c)	e bottom chord and any other me	nbers, with BCDL = 10.0psf.					ANS C	ABAR
6) Provide mechani	cal connection (by others) of truss	to bearing plate capable of withsta	nding 100 lb uplift	at join	it(s) excei	ot (jt=lb)		
13=222, 7=263.			0		. ,	- /	U CESS	17 V -

7) This trues is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



August 3,2021

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Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor	Sec. II	
J1221-7067	A3A	COMMON	1	1			E16001303
Comtach Inc. Equation	villo NC - 28314			420 c lu	Job Reference (option	nal) rios Inc. Tuo Aug. 3.07	54:25 2021 Page 1
Connech, Inc, Tayene	o o o	15.0.0	ID:I4HRAT3eIT	9qoRldA	DEs_5z0Axy-AT4OVHE	ofbOk4zST2Oc1EGZfZ5	xt?zvapYFfk06yrd5y
	6-8-8	9-0-0	9-0-0			8-11-8	0-11-0
							Scale: 3/16"=1'
		5x8					
		6.00 12 4					
Ī							
			18				
	4x6 🗐	17			3x4 📚		
	4x8 📂			\sim	4x6 📚		
5	2		M		6		
-7-6	16	٣/ /					
3x4						10	
1						19	
a P							70
					g		
م م 14	20 21	13 12 22	23 11 10	24	9	25	4x8 ≈
15 6x8 =		4x6 =	8x8 =		2x4		
		3x4 =	6x6	=			
—	9-8-8	19-5-8		24-8-8		33-8-0	———————————————————————————————————————
Plate Offsets (X,Y) [7:0	-1-0,0-1-13], [11:0-4-0,0-4-12	2], [14:0-2-4,0-4-4]		000		0110	
LOADING (psf)	SPACING- 2-0-0	CSI. DEI	FL. in	(loc)	l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.56 Ver BC 0.97 Ver	t(LL) -0.19	11-13 11-13	>999 360	MT20	244/190
BCLL 0.0 *	Rep Stress Incr NO	WB 0.77 Hor	z(CT) 0.09	7	n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S Win	nd(LL) 0.21	9-11	>999 240	Weight: 237 lb	FT = 20%
	.1	BRA	ACING-	Structur	al wood choothing dir	actly applied or 2.4.0	
BOT CHORD 2x6 SP No	.1		CHORD	except e	and verticals.	ectly applied of 3-4-9 (je pullins,
WEBS 2x4 SP No 1-14 ⁻ 2x6 S	.2 *Except* SP No 1	BOT	T CHORD BS	Rigid ce	iling directly applied of the	or 5-9-3 oc bracing. -13 4-11 2-14	
						, ,	
Max Horz	14=-193(LC 13)						
Max Uplift Max Grav	14=-373(LC 12), 7=-491(LC 14=2046(I C 19) 7=2357(I C	13) 2 20)					
TOP CHORD 1-2=-435	np./Max. Ten All forces 250 /227, 2-4=-2926/1389, 4-6=-3) (lb) or less except when shown. 3695/1822, 6-7=-4439/2001, 1-14=-315/247	1				
BOT CHORD 13-14=-9	48/2587, 11-13=-817/2396, 9	-11=-1625/3860, 7-9=-1625/3860	1				
6-9=-78/4	489	00/2010, 0 11- 000/020, 2 14- 200 //110	.,				
NOTES-							
1) Unbalanced roof live loa	ds have been considered for	this design.			(
and C-C Exterior(2) 0-4-	4 to 4-9-1, Interior(1) 4-9-1 to	512-6.0psr; BCDL=6.0psr; n=15n; Cat. II; Exp $15-8-8$, Exterior(2) 15-8-8 to 20-1-5, Interio	or(1) 20-1-5 to	34-4-10	zone;C-C for		
members and forces & I	MWFRS for reactions shown;	Lumber DOL=1.60 plate grip DOL=1.60	r live loads				
4) * This truss has been de	signed for a live load of 30.0	psf on the bottom chord in all areas where a	a rectangle 2-6	-0 tall by	2-0-0 wide		
5) Refer to girder(s) for true	m chord and any other meml	pers, with $BCDL = 10.0psf$.				ATROP	POL MA
6) Provide mechanical con	nection (by others) of truss to	bearing plate capable of withstanding 100	Ib uplift at joint	t(s) exce	pt (jt=lb)	O FSS	TUN
7) This truss is designed in	accordance with the 2015 Ir	ternational Residential Code sections R502	2.11.1 and R80	2.10.2 a	nd 🧣		
referenced standard AN	SI/TPI 1.	vided sufficient to support concentrated load	d(c) 985 lb dow	n and 55	2 lb up at	SEA	
19-7-12, and 575 lb dow	n and 322 lb up at 21-9-4 or	bottom chord. The design/selection of su	ch connection of	device(s)	is the	0363	22 8
responsibility of others.9) In the LOAD CASE(S) s	ection, loads applied to the fa	ace of the truss are noted as front (F) or bac	ck (B).			ц.	
			. /				
1) Dead + Roof Live (balar	nced): Lumber Increase=1.15	, Plate Increase=1.15				2216	- CON
Uniform Loads (plf)	l-8=-60 7-15=-20					4 C	IL COM
von. 1- 4 –-00, 4						Augu	st 3,2021
Continued on page 2						g~	·

t 818 Soundside Road Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II	
11221 7067	A2A	COMMON	1	1	E16001303	
51221-7067	ASA		'	'	Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 07:54:25 2021 Page 2	
		ID:I4HRAT3eIT9qoRIdAoEs_5z0Axy-AT4OVHDfbOk4zST2Oc1EGZfZ5xt?zvapYFfk06yrd5y				

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 11=-985(F) 24=-575(F)

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August 3,2021

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	7-11-8		<u>17-11-8</u> 10-0-0	25-	-11-0 11-8
Plate Offsets (X,Y)	[2:0-2-6,0-2-0], [5:0-3-0,Edge], [8:0-2-6,	0-2-0]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.85 BC 0.43 WB 0.60 Matrix-S	DEFL. in (lk Vert(LL) -0.28 10- Vert(CT) -0.46 10- Horz(CT) 0.04 Wind(LL) 0.23	loc) I/defl L/d -12 >999 360 -12 >663 240 8 n/a n/a -12 >999 240	PLATES GRIP MT20 244/190 Weight: 174 lb FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF	2 No.1 2 No.1		BRACING- TOP CHORD Str BOT CHORD Rig	ructural wood sheathing o gid ceiling directly applied	directly applied or 4-4-14 oc purlins. d or 10-0-0 oc bracing.

WEBS 2x4 SP No.2 **REACTIONS.** (size) 2=0-3-8, 8=0-3-8 Max Horz 2=119(LC 11) Max Uniff 2=-203(LC 12) 8=-2

Max Uplift 2=-203(LC 12), 8=-203(LC 13) Max Grav 2=1140(LC 2), 8=1140(LC 2)

- FORCES. (ib) Max. Comp./Max. Ten. All forces 250 (ib) or less except when shown. TOP CHORD 2-3=-1772/588, 3-4=-1401/651, 4-5=-286/978, 5-6=-286/978, 6-7=-1401/651, 7-8=-1772/588
- BOT CHORD 2-12=-347/1438, 10-12=-350/1438, 8-10=-347/1438
- WEBS 3-12=0/497, 7-10=0/497, 4-6=-2532/1014

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 12-11-8, Exterior(2) 12-11-8 to 17-4-5, Interior(1) 17-4-5 to 26-7-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 203 lb uplift at joint 2 and 203 lb uplift at joint 8.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

Scale = 1:53.3

August 3,2021

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Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II
J1221-7067	B1A	COMMON	1	1	E16001307
					Job Reference (optional)
Comtech, Inc, Fa	vetteville, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 07:54:30 2021 Page 2
		ID:I4HR	AT3elT9q	RIdAoEs_	5z0Axy-WRtHY?HoPwNM3EM?B9cPzdMTaygQeEEYiXNViKyrd5t

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 17=-985(B) 18=-575(B)

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Max Uplift All uplift 100 lb or less at joint(s) 2, 22, 25, 20, 17, 14 except 23=-115(LC 12), 24=-110(LC 12), 26=-171(LC 12), 19=-118(LC 13), 18=-109(LC 13), 16=-167(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 21, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 7-8=-120/304, 8-9=-120/303

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members. 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 22, 25, 20, 17, 14 except (jt=lb) 23=115, 24=110, 26=171, 19=118, 18=109, 16=167.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

August 3,2021

🗥 WARNING - Verify design p eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Plate Offsets (X,Y)	[1:Edge,0-4-10], [2:0-2-14,0-2-4], [7:Edg	ge,0-4-10], [9:0-4-0,0-4-12	2], [12:0-2-8,0-2-8]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.86 BC 0.73 WB 0.91 Matrix-S	DEFL. in (l Vert(LL) -0.15 12 Vert(CT) -0.31 12 Horz(CT) 0.05 Wind(LL) 0.15 12	loc) l/defl L/c -13 >999 360 -13 >843 240 7 n/a n/a -13 >999 240	H PLATES GRIP MT20 244/190 Weight: 399 lb FT = 20%	
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF WEBS 2x4 SF WEDGE Left: 2x4 SP No.2 , Rig	2 No.1 2 2400F 2.0E 2 No.2 ht: 2x4 SP No.2		BRACING- TOP CHORD St BOT CHORD Ri	ructural wood sheat gid ceiling directly a	hing directly applied or 4-11-9 oc purlins. pplied or 10-0-0 oc bracing.	
REACTIONS. (siz Max H Max U Max G	e) 1=0-3-8, 7=0-3-8 lorz 1=-275(LC 25) lplift 1=-1263(LC 8), 7=-1390(LC 9) rav 1=7583(LC 1), 7=7705(LC 2)					
FORCES. (Ib) - Max. TOP CHORD 1-2= 6-7= BOT CHORD 12-12 9-10 WEBS 13-1 4-12 6-8= NOTES- 1) 2-ply truss to be con Top chords connect Bottom chords conn Webs connected as 2) All loads are considd ply connections hav 3) Unbalanced roof live	Comp./Max. Ten All forces 250 (lb) or 10129/1691, 2-3=-9954/1689, 3-4=-637 10911/1947 3=-199/1348, 1-15=-1112/6124, 14-15=- =-1203/6789, 8-9=-1363/7919, 7-8=-136 4=-158/1269, 3-13=-496/3482, 3-12=-33 =-1276/7071, 9-12=-232/708, 5-12=-336 -446/2474 Intected together with 10d (0.131*x3") na ed as follows: 2x6 - 2 rows staggered at ected as follows: 2x6 - 2 rows staggered at ected as follows: 2x6 - 2 rows staggered follows: 2x4 - 1 row at 0-9-0 oc. ered equally applied to all plies, except if e been provided to distribute only loads a loads have been considered for this de	less except when shown 2/1197, 4-5=-6163/1169, 1112/6124, 10-14=-1192 3/7919, 2-13=-266/1793 57/714, 5-9=-681/4032, 1 3/728, 2-15=-386/1995, 6 ils as follows: 0-9-0 oc. I at 0-5-0 oc. i noted as front (F) or bac noted as (F) or (B), unles sign.	5-6=-8950/1599, /6650, 0-12=-123/1345, i-9=-955/322, k (B) face in the LOAD CASI s otherwise indicated.	E(S) section. Ply to		
 4) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 						
WARNING - Verify de Design valid for use onh a truss system. Before building design. Bracin, is always required for st fabrication, storage, del Safety Information av	sign parameters and READ NOTES ON THIS AND I y with MITek® connectors. This design is based on use, the building designer must verify the applicability indicated is to prevent buckling of individual truss ability and to prevent collapse with possible person very, erection and bracing of trusses and truss sy aliable from Truss Plate Institute, 2670 Crain Highw	NCLUDED MITEK REFERENCE y upon parameters shown, and ty of design parameters and pro web and/or chord members only al injury and property damage. Iems, see <u>ANSUTPI1</u> vay, Suite 203 Waldorf, MD 2066	PAGE MII-7473 rev. 5/19/2020 BEFC is for an individual building compone perty incorporate this design into the v. Additional temporary and perman For general guidance regarding the <i>buality Criteria</i> , DSB-89 and BCS/ 31	DRE USE. ent, not e overall ent bracing Building Component	B18 Soundside Road Edenton, NC 27932	

Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II
					E16001309
J1221-7067	C1-GR	Roof Special Girder	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fa	yetteville, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 07:54:33 2021 Page 2
		IC	D:14HRAT3	elT9qoRld	AoEs_5z0Axy-x0ZQA0Kgirlxxh5asHA6bF_rK9gerTG_0Vb9Jfyrd5q

NOTES-

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1313 lb down and 242 lb up at 2-0-12, 1318 lb down and 252 lb up at 4-0-12, 1318 lb down and 252 lb up at 6-0-12, 1318 lb down and 252 lb up at 6-0-12, 1318 lb down and 252 lb up at 10-0-12, 1505 lb down and 242 lb up at 12-0-12, 1505 lb down and 242 lb up at 12-0-12, and 2003 lb down and 393 lb up at 18-0-12, and 808 lb down and 202 lb up at 20-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 12-29=-20, 1-7=-20, 2-29=-20

Concentrated Loads (lb)

Vert: 16=-1318(B) 17=-1318(B) 18=-1318(B) 19=-1313(B) 20=-1318(B) 23=-1313(B) 24=-1313(B) 25=-1313(B) 26=-1934(B) 27=-739(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidual truss evel and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

TOP CHORD

BOT CHORD

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

REACTIONS. (size) 4=0-3-8, 2=0-3-0 Max Horz 2=-71(LC 17) Max Uplift 4=-163(LC 13), 2=-162(LC 12) Max Grav 4=836(LC 1), 2=835(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1239/498, 3-4=-1240/498

2x4 SP No 2

BOT CHORD 2-7=-293/1030, 4-7=-293/1030 WEBS 3-7=0/477

NOTES-

WFBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-6 to 3-8-7, Interior(1) 3-8-7 to 9-11-8, Exterior(2) 9-11-8 to 14-4-5, Interior(1) 14-4-5 to 20-7-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 163 lb uplift at joint 4 and 162 lb uplift at joint 2.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

Structural wood sheathing directly applied or 5-10-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

August 3,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

			19-11-0 19-11-0						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.03 BC 0.01 WB 0.03 Matrix-S	DEFL. Vert(LL) 0. Vert(CT) 0. Horz(CT) 0.	in (loc) 00 12 00 12 00 12	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 130 lb	GRIP 244/190 FT = 20%	
LUMBER-	SP No 1	·	BRACING-	Structu	ral wood	sheathing di	irectly applied or 6-0-0 or	- purlins	

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x6 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 19-11-0.

Max Horz 2=-120(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 12, 2, 20, 22, 17, 15 except 21=-102(LC 12), 23=-116(LC 12), 16=-103(LC 13), 14=-112(LC 13) Max Grav All reactions 250 lb or less at joint(s) 12, 2, 19, 20, 21, 22, 23, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 2, 20, 22, 17, 15 except (it=lb) 21=102, 23=116, 16=103, 14=112.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This does not have a seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **AVSUTPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

August 3,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

	↓ 4-1 ⁻ 4-1 ⁻	I-8		9-11-0			
Plate Offsets (X,Y)	[2:0-0-0,0-2-11], [6:Edge,0-2-11]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.42 WB 0.06 Matrix-S	DEFL. i Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) -0.0 Wind(LL) 0.0	n (loc) l/defl L/d 8 >999 360 2 8 >999 240 1 6 n/a n/a 2 8 >999 240	PLATES GRIP MT20 244/190 Weight: 49 lb FT = 20%		
LUMBER-BRACING-TOP CHORD2x4 SP No.1TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins.BOT CHORD2x6 SP No.1BOT CHORDBOT CHORDRigid ceiling directly applied or 9-1-14 oc bracing.WEBS2x4 SP No.22x4 SP No.2CHORDStructural wood sheathing directly applied or 9-1-14 oc bracing.							
REACTIONS. (size) 2=0-3-0, 6=0-3-0 Max Horz 2=-66(LC 13) Max Uplift 2=-297(LC 8), 6=-297(LC 9) Max Grav 2=449(LC 1), 6=449(LC 1)							
FORCES. (lb) - Max. TOP CHORD 2-3= BOT CHORD 2-10: WEBS 4-9=	Comp./Max. Ten All forces 250 (lb) o -541/873, 3-4=-494/920, 4-5=-494/920, =-688/437, 9-10=-688/437, 8-9=-688/43 -534/232	r less except when shown. 5-6=-541/873 7, 6-8=-688/437					
NOTES- 1) Unbalanced roof liv. 2) Wind: ASCE 7-10; V gable end zone and Lumber DOL=1.60 p 3) Truss designed for Gable End Details a 4) Gable studs spaced 5) This truss has been 6) * This truss has been will fit between the t 2) Provide mechanical	e loads have been considered for this de /ult=150mph Vasd=119mph; TCDL=6.0 C-C Exterior(2) zone; porch left and rig olate grip DOL=1.60 wind loads in the plane of the truss only is applicable, or consult qualified buildin at 2-0-0 oc. designed for a 10.0 psf bottom chord liv in designed for a live load of 30.0psf on pottom chord and any other members.	esign. psf; BCDL=6.0psf; h=15ft; ht exposed;C-C for membe . For studs exposed to wir g designer as per ANSI/TP re load nonconcurrent with the bottom chord in all area	Cat. II; Exp C; Enclose ers and forces & MWFF nd (normal to the face), 'I 1. any other live loads. as where a rectangle 3 price 100 lb uplift at joi	d; MWFRS (envelope) S for reactions shown; see Standard Industry -6-0 tall by 2-0-0 wide int(s) except (it-lb)			

on (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) () 2=297, 6=297.

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

August 3,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidual truss evel and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Plate Offsets (X,Y)	[2:0-2-14,0-0-6]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code UPC2015/JE7/2014	CSI. TC 0.45 BC 0.18 WB 0.00 Matrix P	DEFL. ir Vert(LL) 0.04 Vert(CT) -0.03 Horz(CT) 0.00	n (loc) l/defl L/d 4 2-4 >999 240 3 2-4 >999 240 0 n/a n/a	PLATES GRIP MT20 244/190 Weight: 27 lb ET = 20%
LUMBER- TOP CHORD 2x4 SF	2 No.1		BRACING- TOP CHORD	Structural wood sheathing di	rectly applied or 6-0-0 oc purlins.
BOT CHORD 2x6 SF WEBS 2x6 SF	P No.1 P No.1		BOT CHORD	except end verticals. Rigid ceiling directly applied	or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=75(LC 8) Max Uplift 2=-188(LC 8), 4=-143(LC 8) Max Grav 2=294(LC 1), 4=220(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=188, 4=143.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Plate Offsets (X,Y) [2:0-2-14,0-0-6], [6:Edge,0-2-0]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.19 BC 0.18 WB 0.02 Matrix-S	DEFL. ir Vert(LL) 0.04 Vert(CT) -0.02 Horz(CT) -0.00	n (loc) l/defl L/d 4 8 >999 240 2 8 >999 240 0 6 n/a n/a	PLATES GRIP MT20 244/190 Weight: 29 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x6 SP OTHERS 2x4 SP	No.1 No.1 No.1 No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	rectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-1-8 Max Horz 2=106(LC 8) Max Uplift 2=-259(LC 8), 6=-199(LC 8) Max Grav 2=294(LC 1), 6=220(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-8=-275/133, 7-8=-275/133, 6-7=-275/133 BOT CHORD

NOTES-

- 1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=259. 6=199.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

August 3,2021

🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.21	Vert(CT)	-0.00	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TPI	2014	Matri	x-P	Wind(LL)	0.00	2	****	240	Weight: 20 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 2x6 SP No.1 WFBS

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=84(LC 12)

Max Uplift 2=-48(LC 8), 4=-52(LC 12) Max Grav 2=218(LC 1), 4=136(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 4-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.







LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.10 WB 0.02 Matrix-S	DEFL. in (loc) I/defl L/d Vert(LL) 0.00 8 >999 240 Vert(CT) -0.00 8 >999 240 Horz(CT) -0.00 6 n/a n/a	PLATES GRIP MT20 244/190 Weight: 23 lb FT = 20%
LUMBER-			BRACING-	

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD 2x6 SP No.1 *Except* WEBS 3-8: 2x4 SP No.2 OTHERS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 6=0-1-8 Max Horz 2=121(LC 12) Max Uplift 2=-90(LC 12), 6=-93(LC 12) Max Grav 2=218(LC 1), 6=136(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6. 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 4-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

August 3,2021





Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II
					E16001320
J1221-7067	M2	HALF HIP	6	1	
					Job Reference (optional)
Comtech, Inc,	Fayetteville, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 07:54:43 2021 Page 2

ID:I4HRAT3eIT9qoRIdAoEs_5z0Axy-ex9CHRRyMv?W7DrVSOLS?MPgQBCBBCQTh30hf4yrd5g

2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 3-8=-100, 5-8=-130, 2-6=-20
	Concentrated Loads (lb)
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
	Uniform Loads (plf)
	Concentrated Loads (lb)
1	Vert: 8=-375
4	Uniform Loads (plf)
	Vert: 1-2=98, 2-3=82, 3-4=207, 3-5=67, 2-6=-12 Horz: 1-2=-110, 2-3=-94, 3-4=-219
	Concentrated Loads (lb)
5	Vert: 8=467) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60. Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=/3, 2-3=82, 3-4=/3, 3-5=67, 2-6=-12 Horz: 1-2=-85, 2-3=-94, 3-4=-85
	Concentrated Loads (lb)
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert. 1-2=9, 2-3=-34, 3-4=-50 Horz: 1-2=-25, 2-3=-34, 3-4=-50
	Concentrated Loads (lb)
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Horz: 1-2=25, 2-3=34, 3-4=25
	Concentrated Loads (lb) Vert: 8=-462
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf) Vert: 1-2=40, 2-3=20, 3-4=11, 3-5=11, 2-6=-12
	Horz: 1-2=-52, 2-3=-32, 3-4=-23
	Vert: 8=121
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Vert: 1-2=11, 2-3=20, 3-4=41, 3-5=11, 2-6=-12
	Horz: 1-2=-23, 2-3=-32, 3-4=-53
	Vert: 8=121
1	 Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
	Vert: 1-2=3, 2-3=-6, 3-4=3, 3-5=-15, 2-6=-20
	Horz: 1-2=-23, 2-3=-14, 3-4=-23 Concentrated Loads (lb)
	Vert: 8=-306
1	 Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
	Vert: 1-2=3, 2-3=-6, 3-4=3, 3-5=-15, 2-6=-20
	Concentrated Loads (lb)
4	Vert: 8=-306
1	Uniform Loads (plf)
	Vert: 1-2=22, 2-3=31, 3-4=22, 3-5=-5, 2-6=-12 Horz: 1-2=-34, 2-3=-43, 3-4=-34
	Concentrated Loads (lb)
1	Vert: 8=121 3) Dead + 0.6 MWERS Wind (Pos. Internal) 2nd Parallel: Lumber Increase-1.60. Plate Increase-1.60
	Uniform Loads (plf)
	Vert: 1-2=6, 2-3=15, 3-4=6, 3-5=-5, 2-6=-12 Horz: 1-2=-18, 2-3=-27, 3-4=-18
	Concentrated Loads (Ib)
1	Vert: 8=21 4) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60. Plate Increase=1.60
'	Uniform Loads (plf)
	vert: 1-2=22, 2-3=31, 3-4=22, 3-5=-5, 2-6=-12 Horz: 1-2=-34, 2-3=-43, 3-4=-34
	Concentrated Loads (lb)
	vert: 8=121

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II
14004 7007	MO		C		E16001320
J1221-7067	MZ		ю		Job Reference (optional)
Comtech, Inc, Fayette	rille, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 07:54:43 2021 Page 3

ID:I4HRAT3eIT9qoRIdAoEs_5z0Axy-ex9CHRRyMv?W7DrVSOLS?MPgQBCBBCQTh30hf4yrd5g

LOAD CASE(S) Standard 15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=15, 3-4=6, 3-5=-5, 2-6=-12 Horz: 1-2=-18, 2-3=-27, 3-4=-18 Concentrated Loads (lb) Vert: 8=21 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=5, 3-4=14, 3-5=-31, 2-6=-20 Horz: 1-2=-34, 2-3=-25, 3-4=-34 Concentrated Loads (lb) Vert: 8=-306 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-11, 3-4=-2, 3-5=-31, 2-6=-20 Horz: 1-2=-18, 2-3=-9, 3-4=-18 Concentrated Loads (lb) Vert: 8=-306 18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 3-5=-120, 2-6=-20 Concentrated Loads (lb) Vert: 8=-250 19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-33, 2-3=-40, 3-4=-33, 3-8=-81, 5-8=-111, 2-6=-20 Horz: 1-2=-17, 2-3=-10, 3-4=-17 Concentrated Loads (lb) Vert: 8=-480 20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-33, 2-3=-39, 3-4=-33, 3-8=-81, 5-8=-111, 2-6=-20 Horz: 1-2=-17, 2-3=-11, 3-4=-17 Concentrated Loads (lb) Vert: 8=-480 21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-24, 2-3=-31, 3-4=-24, 3-8=-93, 5-8=-123, 2-6=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26 Concentrated Loads (lb) Vert: 8=-480 22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-36, 2-3=-43, 3-4=-36, 3-8=-93, 5-8=-123, 2-6=-20 Horz: 1-2=-14, 2-3=-7, 3-4=-14 Concentrated Loads (lb) Vert: 8=-480 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 3-8=-40, 5-8=-80, 2-6=-20 Concentrated Loads (lb) Vert: 8=-500 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 3-8=-40, 5-8=-80, 2-6=-20 Concentrated Loads (lb) Vert: 8=-500 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 3-8=-100, 5-8=-130, 2-6=-20 Concentrated Loads (lb) Vert: 8=-438 26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 3-8=-100, 5-8=-130, 2-6=-20 Concentrated Loads (lb) Vert: 8=-438





Beign valid for use only with MTTek® connectors. This and include the applicability of design parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

BIB Soundside Road Edenton, NC 27932

	Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II	
						E	16001321
	J1221-7067	M2A	HALFHIP	1	2		
					-	Job Reference (optional)	
ĺ	Comtech, Inc, Fayettev	rille, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 07:54:44 2021 F	Page 2

ID:I4HRAT3eIT9qoRIdAoEs_5z0Axy-67jaUnSa6D8NINQh?5shXaxt?baLwgIcwjmFBWyrd5f

Concentrated Loads (lb) Vert: 8=-500 2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 3-8=-220, 5-8=-250, 2-6=-20 Concentrated Loads (lb) Vert: 8=-438 3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 3-5=-160, 2-6=-40 Concentrated Loads (lb) Vert: 8=-375 4) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=40, 2-3=20, 3-4=11, 3-5=-109, 2-6=-12 Horz: 1-2=-52, 2-3=-32, 3-4=-23 Concentrated Loads (lb) Vert: 8=121 5) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=11, 2-3=20, 3-4=41, 3-5=-109, 2-6=-12 Horz: 1-2=-23, 2-3=-32, 3-4=-53 Concentrated Loads (lb) Vert: 8=121 6) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=3, 2-3=-6, 3-4=3, 3-5=-135, 2-6=-20 Horz: 1-2=-23, 2-3=-14, 3-4=-23 Concentrated Loads (lb) Vert: 8=-306 7) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=3, 2-3=-6, 3-4=3, 3-5=-135, 2-6=-20 Horz: 1-2=-23, 2-3=-14, 3-4=-23 Concentrated Loads (lb) Vert: 8=-306 8) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=22, 2-3=31, 3-4=22, 3-5=-125, 2-6=-12 Horz: 1-2=-34, 2-3=-43, 3-4=-34 Concentrated Loads (lb) Vert: 8=121 9) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=15, 3-4=6, 3-5=-125, 2-6=-12 Horz: 1-2=-18, 2-3=-27, 3-4=-18 Concentrated Loads (lb) Vert: 8=21 10) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60. Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=22, 2-3=31, 3-4=22, 3-5=-125, 2-6=-12 Horz: 1-2=-34, 2-3=-43, 3-4=-34 Concentrated Loads (lb) Vert: 8=121 11) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=15, 3-4=6, 3-5=-125, 2-6=-12 Horz: 1-2=-18, 2-3=-27, 3-4=-18 Concentrated Loads (lb) Vert: 8=21 12) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=5, 3-4=14, 3-5=-151, 2-6=-20 Horz: 1-2=-34, 2-3=-25, 3-4=-34 Concentrated Loads (lb) Vert: 8=-306 13) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-11, 3-4=-2, 3-5=-151, 2-6=-20 Horz: 1-2=-18, 2-3=-9, 3-4=-18 Concentrated Loads (lb)

Vert: 8=-306

LOAD CASE(S) Standard

Uniform Loads (plf)

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Vert: 1-3=-60, 3-4=-60, 3-8=-160, 5-8=-200, 2-6=-20



Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II
					E16001321
J1221-7067	M2A	HALF HIP	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayette	/ille, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 07:54:44 2021 Page 3

8.430 s Jun 2 2021 MiTek Industries, Inc. Tue Aug 3 07:54:44 2021 Page 3 ID:I4HRAT3eIT9qoRIdAoEs_5z0Axy-67jaUnSa6D8NINQh?5shXaxt?baLwglcwjmFBWyrd5f

LOAD CASE(S) Standard	
14) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90	
Uniform Loads (plf)	
Vert: 1-3=-20, 3-4=-20, 3-5=-240, 2-6=-20	
Concentrated Loads (Ib)	
Vert: 8=-250	
15) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase	e=1.60, Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=-33, 2-3=-40, 3-4=-33, 3-8=-201, 5-8=-231, 2-6=-20	
Horz: 1-2=-17, 2-3=-10, 3-4=-17	
Concentrated Loads (b)	
Vert: 8=-480	
16) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWERS Wind (Neg. Int) Right): Lumber Increa	ase=1.60. Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=-33, 2-3=-39, 3-4=-33, 3-8=-201, 5-8=-231, 2-6=-20	
Horz: 1-2=-17, 2-3=-11, 3-4=-17	
Concentrated Loads (Ib)	
Vert: 8=-480	
17) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber	Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=-24, 2-3=-31, 3-4=-24, 3-8=-213, 5-8=-243, 2-6=-20	
Horz: 1-2=-26, 2-3=-19, 3-4=-26	
Concentrated Loads (lb)	
Vert: 8=-480	
18) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumbe	Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=-36, 2-3=-43, 3-4=-36, 3-8=-213, 5-8=-243, 2-6=-20	
Horz: 1-2=-14, 2-3=-7, 3-4=-14	
Concentrated Loads (Ib)	
Vert: 8=-480	
19) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (plf)	
Vert: 1-3=-60, 3-4=-60, 3-8=-160, 5-8=-200, 2-6=-20	
Concentrated Loads (Ib)	
Vert: 8=-500	
20) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (plf)	
Vert: 1-3=-20, 3-4=-20, 3-8=-160, 5-8=-200, 2-6=-20	
Concentrated Loads (Ib)	
Vert: 8=-500	
21) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (plf)	
Vert: 1-3=-50, 3-4=-50, 3-8=-220, 5-8=-250, 2-6=-20	
Concentrated Loads (lb)	
Vert: 8=-438	
22) 4th Dead + 0.75 Koot Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15	
vert: 1-3=-20, 3-4=-20, 3-8=-220, 5-8=-250, 2-6=-20	
VEIL 0=-430	





BOT CHORD

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 17-1-0.

(lb) - Max Horz 1=195(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-218(LC 12), 6=-218(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=418(LC 19), 9=496(LC 19), 6=496(LC 20)

```
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-9=-455/344, 4-6=-455/345
```

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-5 to 4-7-0, Interior(1) 4-7-0 to 8-7-0, Exterior(2) 8-7-0 to 12-11-13, Interior(1) 12-11-13 to 16-8-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=218, 6=218.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



a waternations

Rigid ceiling directly applied or 10-0-0 oc bracing.





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. All bearings 14-5-0.

(lb) - Max Horz 1=163(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-184(LC 12), 6=-184(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=375(LC 19), 6=375(LC 20)

WEBS 2-8=-388/310, 4-6=-388/310

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0ps; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-5 to 4-10-1, Interior(1) 4-10-1 to 7-3-0, Exterior(2) 7-3-0 to 11-7-13, Interior(1) 11-7-13 to 14-0-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=184, 6=184,

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

August 3,2021

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters show, and is for an individual building designer must verify the applicability of design parameters and property incorporate this design in the overall building designer must verify the applicability of design parameters and property incorporate this design in the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, deliver, erection and bracing of trusses and fruss systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



(lb) - Max Horz 1=-131(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-172(LC 12), 6=-171(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=253(LC 1), 8=343(LC 19), 6=342(LC 20)

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-5 to 4-10-1, Interior(1) 4-10-1 to 5-11-0, Exterior(2) 5-11-0 to 10-3-13, Interior(1) 10-3-13 to 11-4-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=172.6=171.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



August 3,2021

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-8=-372/316, 4-6=-372/316



REACTIONS. (size) 1=9-1-0, 3=9-1-0, 4=9-1-0

Max Horz 1=99(LC 11)

Max Uplift 1=-42(LC 12), 3=-52(LC 13), 4=-24(LC 12) Max Grav 1=171(LC 1), 3=172(LC 20), 4=321(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4. 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.



🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=6-5-0, 3=6-5-0, 4=6-5-0

Max Horz 1=-67(LC 8)

Max Uplift 1=-37(LC 12), 3=-44(LC 13)

Max Grav 1=126(LC 1), 3=126(LC 1), 4=197(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3x4 🥢

2x4 ||

3x4 📎

		3-9-8		0-0-8
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. DEFL. TC 0.03 Vert(LL) BC 0.02 Vert(CT) WB 0.01 Horz(CT) Matrix-P Horz(CT)	in (loc) l/defl L/d n/a - n/a 999 n/a - n/a 999 0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 12 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=3-9-0, 3=3-9-0, 4=3-9-0

Max Horz 1=-35(LC 8)

Max Uplift 1=-20(LC 12), 3=-23(LC 13)

Max Grav 1=66(LC 1), 3=66(LC 1), 4=104(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 3-10-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.





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J1221-7068	N/A	Brinkley "A" /	Lot 18 Mitche	Weaver Deve		-		cement Plan 1 ⁻ 1	Truss Pla Scale: 1/4" -	-(1	
QUOTE # JOB #	SEAL DATE	PLAN	JOB NAME	BUILDER							
RAM ONLY. idual building the building diding designer. h truss design The building y and oor system and of the truss	IENT DIA ed as indi prated intr of the b ets for ea the drawing r tempora roof and 'he design	S PLACEN e designe e incorpo ecification esign she placemen onsible fo ng of the ructure. T	A TRUSS russes ar lents to b at the spo ividual de d on the r is respo ent bracin overall st	THIS IS These to comport design See ind identified designed permant for the support	Truss	eft End o	dicates Lo	∧ = 1			

(Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards Do NOT Erect Truss Backwards



Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



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CSD 🚟

-	Client: Weaver Developm	ient Co. Inc. Di	ate: 12/30/2021	Page 2 of 11
	Project: Brinkley	In	put by: David Landry	
isDesign	Address: Mitchell Manor	Drive Jo	bb Name: Lot 18 Mitchell Manor Sec. II	
	Angier, NC 275	01 Pr	roject #: J1221-7068	
BM1 Kerto-S LVL	_ 1.750" X 16.000"	3-Plv - PASSE	D Level: Level	
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Continued from page 1				
ID Load Type	Location Trib Width Sid	e Dead 0.9 Live	1 Snow 1.15 Wind 1.6 Const	1.25 Comments
6 Part. Uniform	12-7-0 to 22-0-0 Nea	r Face 137 PLF 0 PLF	F 137 PLF 0 PLF 0	PLF M2
Self Weight		10 PLF		
Cell Weight				
			Manufersteinen lief.	Comtach Inc
Notes	chemicals	For flat roofs provide proper drainage to ponding	prevent Manufacturer Info	1001 S. Reilly Road, Suite #639
calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria, and loadings phases in the structural labels of the structural structura structural	1 LVL beams must not be cut or drilled		301 Merritt 7 Building, 2nd Floor	USA 28314
responsibility of the customer and/or the contractor the ensure the component suitability of the intender	2 Refer to manufacturer's product information to regarding installation requirements, multi-ply additional production and the statement of		Norwalk, CT 06851 (800) 622-5850	910-864-TRUS
application, and to verify the dimensions and loads.	 rastening details, beam strength values, and code approvals Damaged Beams must not be used 		www.metsawood.com/us	
Dry service conditions, unless noted otherwise Dry service conditions, unless noted otherwise	 Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid 		100-ED: EDR-3033	Comtecul
2. LVL not to be treated with fire retardant or corrosiv	/e lateral displacement and rotation	This design is valid until 4/24/202	3	Connech
Version 20.40.075 Powered by iStruct™				CSD

	Client:	Weaver Development Co. Inc.	Date: 12/30/2021	Page 3 of 11
	Project:	Brinkley	Input by: David Landry	
isDesig	Address:	Mitchell Manor Drive	Job Name: Lot 18 Mitchell Manor Sec. II	
		Angier, NC 27501	Project #: J1221-7068	
BM1 Kerto	-S LVL 1.750"	X 16.000" 3-Ply - PAS	SED Level: Level	
1 SPF End Grain	· · · · · · ·	1 ↓		Image: Constraint of the second se
Multi-Ply Analysis				
Fasten all plies using	g 3 rows of 10d Box nails	(.128x3") at 12" o.c except for reg	ions covered by concentrated load faste	ning.
Nail from both sides	s. Maximum end distance	e not to exceed 6"	÷	-
Capacity	64.7 %			
Load	182.7 PLF			
Yield Limit per Foot	282.4 PLF			

Luau	IOZ./ FLF	
Yield Limit per Foot	282.4 PLF	
Yield Limit per Fastener	94.1 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination	D+S	
Duration Factor	1.15	

Concentrated Load

Fasten at concentrated side load at 12-5-8 with a minimum of (6) – 10d Box nails (.128x3") in the

pattern shown. Repeat fasteners on both sides.

pattern shown. Repet	te lasteriers off boar slate
Capacity	83.6 %
Load	472.0lb.
Total Yield Limit	564.7 lb.
Cg	0.9998
Yield Limit per Fastener	94.1 lb.
Yield Mode	IV
Load Combination	D+S
Duration Factor	1.15

Min/Max fastener distances for Concentrated Side Loads



Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is responsible only of the structural advances of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2, LVL not to be treated with fire retardant or corrosive	Handling & Installation 1. UL beams must not be cut or dniled 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	ponding This design is valid until 4/24/2023	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	Fayetteville, ŃC USA 28314 910-864-TRUS

Version 20.40.075 Powered by iStruct™



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CSD

inDesign	Client: Weaver Developm Project: Brinkley	ent Co. Inc. Date: Input	: 12/30/2021 by: David Landry	Page 5 of 11
TSDesign	Address: Mitchell Manor Angier, NC 275	Drive Job N D1 Proje	Name: Lot 18 Mitchell Manor Sec. II ect #: J1221-7068	
BM2 Kerto-S LVL	1.750" X 16.000"	3-Ply - PASSED	Level: Level	
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<i>†</i>		15'		ł
Multi-Ply Analysis				
Fasten all plies using 4 rows of 1	0d Box nails (.128x3") at 12"	o.c Nail from both sides. N	Maximum end distance not to	exceed
Capacity 100.0) %			
Load 327.3 Yield Limit per Foot 327.4	3 PLF I PLF			
Yield Limit per Fastener 81.9 Yield Mode IV	lb.			
Edge Distance 1 1/2'	"			
Load Combination D+L				
Duration Factor 1.00				
Notes Calculated Structured Decides is representials only of the Use	chemicals	6. For flat roofs provide proper drainage to prev ponding	Vent Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Favetteville, NC
structural adequacy of this component based on the 1 design criteria and loadings shown. It is the 2 responsibility of the customer and/or the contracts to	LVL beams must not be cut or drilled Refer to manufacturer's product information		301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
esponsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.	regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals		(800) 622-5850 www.metsawood.com/us	
Lumber 3. 1. Dry service conditions, unless noted otherwise 4. 2. LVL not to be treated with fire retardant or corrosive 5.	Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral direction		ICC-ES: ESR-3633	соттесн
Version 20.40.075 Powered by iStruct™	assist displacement and lotation	i his design is valid until 4/24/2023	1	CODIEN

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Null: Ply Analysis Image: Start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maximum end distance not to exceed 6° Creating and the start all ples using 2 rows of 10d Box nalls (128x3') at 12° oc. Maxim	BM3	Kerto-S L\	/L 1.750'	' X 9.250"	2-Ply	- PASSE	D	evel: Level	
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Image: Set End Gram 2 SPE End Gram Image: Set End Gram 1 Set End Gram 2 SPE End Gram Image: Set End Gram 1 Set End Gram 2 SPE End Gram Image: Set End Gram 1 Set End Gram 10 ¹¹ 10 ¹¹ Image: Set End Gram Multic H2 Analysis Exact and place using 2 rows of 10d Box nails (128x3 ¹) at 12 ¹² o.c. Maximum end distance not to exceed 6 ¹² Image: Set End Gram Mode Unit per Food 00 Pr F 10 Pr F Image: Set End Gram Image: Set End Gram Mode Unit per Food 00 Pr F 10 Pr F Image: Set End Gram Image: Set End Gram Mode Unit per Food 00 Pr F 10 Pr F Image: Set End Gram Image: Set End Gram Mode Unit per Food 00 Pr F 10 Pr F Image: Set End Gram Image: Set End Gram Mode Unit per Food 10 Pr F Image: Set End Gram Image: Set End Gram Image: Set End Gram Mode Unit per Food 10 Pr F Image: Set End Gram Mode Unit per Food 10 Pr F Image: Set End Gram				$\Lambda \Lambda /$					
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Image: Set End Gaan 2 SPF End Gaan Image: Set End Gaan 910° Store 910° Multi- Pty Analysis Eastern all plies using 2 rooms of 10d Box nails (128x3°) at 12° o.c. Maximum end distance not to exceed 6° Construction 900° Vide Limit per Seatone 19.0°									
Image: Set End Grain 3 10° Set End Grain 3 10° Will Ump and per setseer 5 10° Will Ump are Foot 10.3 20° Multi Ump are Foot 10.2 20° Multi Ump are Foot 10.0 20° Multi Multi Are Foot 10.0 20° Multi Multi Are Foot 10.0 20° Multi Multi Are	•	•	•	•	•	• •		Ξ.	
Image: Stripping of the st								1 1/2	9 1/4
1 SPF End Grain 2 SPF End Grain A 4 0100 910°	•	•	•	٠	•	• •		<u> </u>	
Protect 010° Faster all plies using 2 rows of 10d Box nalls (128x3°) at 12° o.c. Maximum end distance not to exceed 6° Capeady Load 0.0 PF Well imper Faster Bill alls 813 B. Well imper Faster Bill alls 10° Well imper Faster Bill al		SPF End Grain			2 5	SPF End Grain		Λ	
Image: State and price using 2 rows of 10d Box nails (128x3") at 12" o.c. Maximum end distance not to exceed 6" Grapady Load 0.0 PUF Using performed for the state of the state o			5'	10"					3 1/2"
Multi-Ply Analysis Fasten all plies using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance not to exceed 6" Capacity 0.0% Lod 0.0% Medi Imper Fox 100.1% State all plies using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance not to exceed 6" Medi Imper Fox 100.1% State all prices using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance not to exceed 6" Medi Imper Fox 100.1% State all prices using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance not to exceed 6" Minimum end distance not to exceed 6" State all prices using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance not to exceed 6" State all prices using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance not to exceed 6" State all prices using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance State all prices using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance State all prices using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance State all prices using 2 rows of 10d 8ox nails (128x3") at 12" o.c., Maximum end distance Databaal backed bac	1		5''	10"			1		
Note: Capacity 0.0% Capacity 0.0% Capacity 0.0% Valid imit por Foot 103.7 PLF Valid imit por Factor 1.00	Multi-P	ly Analysis							
Capacity I 0.0 % Load 0.0 PUF Wide Limit per Food 153.7 PLF Wide Limit per Fastener 8.19 bb. Wide Limit per Fastener 8.19 bb. Duration Fastener 9.10 bb. Based Combination Duration Fastener 8.10 bb. Based States and Based States 1.20 Based State	Fasten a	Il plies using 2 rows	of 10d Box nails	(.128x3") at 12" c	.c Maximu	m end distar	nce no	t to exceed 6"	
Netel Lumit pper Ford 103.7 PLPF Well Lumit pper Forders N B Bb. Well Mode N Segle Distance 3" Lud Combination 3" Dardon Factor 1.02	Capacity		0.0 %						
Mode V Edge Distance 11/2* Mode V Edge Distance 11/2* Duration Factor 1.00	Yield Limit	per Foot	163.7 PLF						
Edge Distance 1 12" Min. End Distance 3" Load Combination 1.00	Yield Limit Yield Mode	per Fastener	81.9 lb. IV						
Meth. End Distance 3° Quardion Factor 1.00	Edge Dista	nce	1 1/2"						
Duration Factor 1.00 Netse 1.00 Nation Factor 1.00	Min. End D	istance	3"						
Notes Cataland Statutud Grages in wegenality of the statutud Grages in wegenality of the statutur of statutur and statutur of and statutur of the stat	Duration Fa	actor	1.00						
Notes chemicals chemicals 6. For flat roofs provide proper drainage to provent Manufacturer Info Contrach. Inc. Calculated Structural adequacy of this component based on the design of the contractor to responsibility of the customer and/or the contractor to responsibility of the customer and/or the contractor to regarding installation requirements, multiply fastering details, beam strength values, and code application, and to verify the dimensions and loads. Manufacturer info Contlech. Inc. 1. Dyservice conditions, unless noted otherwise 1. by service conditions, unless noted otherwise 3. Damaged Beams must note bus end Albergin gravitation 4. Design assumes to p edue is laterally negating installation requirements, multiply assumes to pedue is laterally negating installation (equirements, multiply assumes to pedue is laterally negating installation (equirements, multiply assumes to pedue is laterally negating installation (equirements, multiply assumes to pedue is laterally negating installation (equirements, multiply assumes to pedue is laterally negating installation (equirements, multiply assumes to pedue is laterally negating installation (equirements, multiply assumes to pedue is laterally negating installation (equirements, multiply assumes to pedue is laterally negating installation (equirements, multiply assumes to pedue is laterally negating installation (equirements, multiply assumes) Provide lateral equirements, multiply assumes) Contlech. Inc. 1. Dyservice conditions, unless noted otherwise 1. Dyservice conditions, unless noted otherwise 2. Uk not to be treated with fire retardant or corrowise This design is valid until 4/24/2023 <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th>									
Lumber 3. Damaged Beams must not be used 3. Damaged Beams must not be used 1. Dry service conditions, unless noted otherwise 1. Dry service conditing notherwise 1. Dry service conditio	Notes Calculated Str structural ade design criter responsibility ensure the application, an	uctured Designs is responsible only of quacy of this component based on ia and loadings shown. It is of the customer and/or the contracto component suitability of the intem- d to verify the dimensions and loads.	chemicals the Handling & Installati the 1. LVL beams must not be of the 2. Refer to manufactur regarding installation fastening details, beam approvale etails, beam	6 ON ut or drilled er's product information requirements, multi-ply strength values, and code	. For flat roofs provid ponding	Je proper drainage to p	prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
Lateral displacement and rotation This design is valid until 4/24/2023 Version 20.40.075 Powered by iStruct™	Lumber 1. Dry service 2. UVL part to	e conditions, unless noted otherwise	3. Damaged Beams must n 4. Design assumes top edg 5. Provide lateral support	ot be used e is laterally restrained at bearing points to avoid				ICC-ES: ESR-3633	Comtecul
	Version 20.4	0.075 Powered by iStruct [™]	Iateral displacement and	rotation	This design is v	alid until 4/24/2023			CSDIRM



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1	isDesign		Client: Project: Address:	Weaver Developm Brinkley Mitchell Manor I Angier, NC 2750	ent Co. Inc. Drive D1] 	Date: nput by: Job Name: Proiect #:	12/30/2021 David Landry Lot 18 Mitchell Manor Sec. II J1221-7068	Page 9 of 11
GDH	Kerto-S	LVL	1.750"	X 11.875"	2-Ply	- PASSE	D	evel: Level	
					-				
	• •	• •	•	• •	• •	• •	•	• • •	
	• •	•	•	• •			•		····
	End Grain							2 SPF End	Grain //
					16'10"				13 1/2"
					16'10"				I
Multi-Ply	Analysis								
Fasten all	plies using 2 r	rows of 10	d Box nails	(.128x3") at 12"	o.c Maxim	num end dist	ance not	t to exceed 6"	
Load		0.0 % 0.0 PL	F						
Yield Limit per Yield Limit per	r Foot r Fastener	163.7 81.9 lb	PLF).						
Yield Mode Edge Distance	<u>م</u>	IV 1 1/2"							
Min. End Dista	ance	3"							
Load Combina Duration Fact	ation or	1.00							
Notes	and Decision 1	(hemicals	tion	6. For flat roofs pr ponding	ovide proper drainage 1	to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Eavetheville, NC
design criteria responsibility of the	red Designs is responsible cy of this component ba and loadings shown. ne customer and/or the c	e only of the nal sed on the 1 L It is the 2 F contractor to r	VL beams must not be Refer to manufactu egarding installation	cut or drilled irer's product information requirements. multi-nuv	-			Netsa Wood 801 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the com application, and to Lumber 1. Dry service cor	ponent suitability of th verify the dimensions and nditions, unless noted othe	e intended loads. 3.[4.[rwise	astening details, bean approvals Damaged Beams must Design assumes top ed	n strength values, and code not be used ge is laterally restrained t at hearing points to				800) 622-5850 www.metsawood.com/us CC-ES: ESR-3633	
2. LVL not to be	treated with fire retardant	or corrosive	ateral displacement and	d rotation	This design is	s valid until 4/24/20	23		соттесн

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Version 20.40.075 Powered by iStruct™



isl	Design	Client: Project: Address:	Weaver Development Co. Inc. Brinkley Mitchell Manor Drive	Date: Input by: Job Name	12/30/2021 David Landry : Lot 18 Mitchell Manor Sec. II	Page 11 of 11
	• - ·		Angier, NC 27501	Project #:	J1221-7068	
GDH2	S-P-F #2	2.000"	X 12.000" 2-Ply	- PASSED		
	•	• •	• •	• •	۲. ۲.	M
						11 1/4"
	•	• •	• •	• •	· · · · · · · · · · · · · · · · · · ·	
1 SPF Er	id Grain				2 SPF End Grain	
			8'10"]	1 13"
			8'10"		1	
Multi-Dly Ar	alvsis					
Fasten all plie	es usina 2 rows a	of 10d Box nails	(128x3") at 12" o.c. Maxim	um end distance no	ot to exceed 6"	
Capacity	C).0 %				
Load Yield Limit per Fo	C ot 1	0.0 PLF 157.4 PLF				
Yield Limit per Fa Xield Mode	stener 7	78.7 lb.				
Edge Distance	1	v 1/2"				
Min. End Distance	e 3	3"				
Duration Factor	1	.00				
				[Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
				Ī		Fayetteville, NC USA 28314
						910-864-TRUS
			This design is	valid until 4/24/2023		соттесн



RE: J1221-7068

Lot 18 Mitchell Manor Sec. II

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Customer: Weaver Development Co. Inc. Lot/Block: 18 Address: Mitchell Manor Drive City: Angier

Project Name: J1221-7068 Model: Brinkley Subdivision: Mitchell Manor Sec. II State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E16477192	ET1	12/9/2021
2	E16477193	ET2	12/9/2021
3	E16477194	ET3	12/9/2021
4	E16477195	ET4	12/9/2021
5	E16477196	ET5	12/9/2021
6	E16477197	F1	12/9/2021
7	E16477198	F1A	12/9/2021
8	E16477199	F2	12/9/2021
9	E16477200	F2A	12/9/2021
10	E16477201	F3	12/9/2021
11	E16477202	F4	12/9/2021
12	E16477203	F5	12/9/2021
13	E16477204	F6	12/9/2021
14	E16477205	FG1	12/9/2021
15	F16477206	FG2	12/9/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



December 09, 2021

Job		Truss		Truss Type					Qty	Ply	Lot 18	3 Mitchell	Manor Sec	. 11			= 10.1=	
11221-7068		FT1		GABLE					1	1							E1647	//192
01221 7000		2.1.		ONDEL							Job R	eference	(optional)					
Comtech, Inc,	Fayettev	ville, NC - 28314	ł,						8	.430 s Au	g 16 20	21 MiTek	Industries,	Inc. Th	u Dec 9	07:37:42 2	021 Page	1
								ID:14HRA	136119doi	RIDAOES_	SZUAXY	-2Кј26рС	zciaeicDji	Ceqrz	9QWFyQ	VOINIBAINI2	Ракуатн	N
0- <u>11</u> 8																	0- <u>1</u> -8	8
																	Scale -	1.28.8
																	Ocale -	1.20.0
								3x4 —	3x6 FP -	_								
1	2	2	4	5	6		7	0,4	0 1	_	11		10	12		14	15	
T LA	2	3	4	5	0		1						12	13		14	15	T
31	P	P	Ĕ	Ŭ.	9		P	Æ		Ľ	Ĕ		Ĕ	۲ ۲		Ľ	ĥ	32
4																	•	4-0
	Ц		Ц								Н		Ц	Ц		H		-
																		1
30	29	28	27	26	25	24	23	22	2	21	20		19	18		17	16	
3x4 =						3x6 FF	>=										3x4 =	
							3x4 =											

	1-4-0	2-8-0	4-0-0	5-4-0	6-8-0	8-0-0	9-4-0 10	-8-0	12-0-	-0	13-4-0	14-8-0	16-0-0	17-4-12
	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0 1	-4-0	1-4-	0 /	1-4-0	1-4-0	1-4-0	1-4-12
Plate	Offsets (X,Y)	[8:0-1-8,E	Edge], [23:0-1-	8,Edge]										
LOAD TCLL	ING (psf) 40.0	SF Pla	PACING- ate Grip DOL	2-0-0 1.00	CSI. TC BC	0.06	DEFL. Vert(LL)	in n/a	(loc) -	l/defl n/a	L/d 999	PL MT	ATES 120	GRIP 244/190
BCLL BCDL	0.0 5.0	Re	ep Stress Incr ode IRC2015/	YES 1.00 YES TPI2014	WB Matr	0.03 ix-S	Horz(CT)	0.00	16	n/a	n/a	We	eight: 79 lb	FT = 20%F, 11%E
LUMB	ER- HORD 2x4	SP No.1(flat	:)				BRACING TOP CHOI	RD	Structu	ral woo	d sheathing	directly appl	ied or 6-0-0	oc purlins,

BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat) except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3(flat) OTHERS

REACTIONS.

All bearings 17-4-12.
 (lb) - Max Grav All reactions 250 lb or less at joint(s) 30, 16, 29, 28, 27, 26, 25, 23, 22, 21, 20, 19, 18, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

- 3) Gable requires continuous bottom chord bearing. 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 6) Gable studs spaced at 1-4-0 oc.
 6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.







Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II
					E16477193
J1221-7068	E12	GABLE	1	1	Job Reference (ontional)
Comtech, Inc.	Favetteville, NC - 28314.		8	3.430 s Au	16 2021 MiTek Industries, Inc. Thu Dec 9 07:37:43 2021 Page 1

ID:I4HRAT3eIT9qoRIdAoEs_5z0Axy-1dHxK9Rbz0iVwMovow935AibGeHgE4cLO0ry9myAmHM

Scale = 1:30.1



	1-4-0	2-8-0	4-0-0	5-4-0	6-8-0	8-0-0	9-4-0	10-8-0	12-0-0	13-4-0	14-8-0	16-0-0	17-4-0	18-1-0	
1	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	0-9-0	
Plate C	Plate Offsets (X,Y) [1:Edge,0-1-8], [8:0-1-8,Edge], [25:0-1-8,Edge], [32:Edge,0-1-8]														

LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0))))	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.00 1.00 YES Pl2014	CSI. TC BC WB Matri	0.06 0.01 0.03 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 17	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 83 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS	2x4 SP 2x4 SP 2x4 SP 2x4 SP 2x4 SP	No.1(flat) No.1(flat) No.3(flat) No.3(flat)				BRACING- TOP CHOR BOT CHOR	D D	Structu except Rigid ce	ral wood end verti eiling dire	sheathing di cals. ectly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,

REACTIONS. All bearings 18-1-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 32, 17, 31, 30, 29, 28, 27, 25, 24, 23, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center. 3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

- 6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

8) CAUTION, Do not erect truss backwards.





^{0-&}lt;u>1</u>-8

Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II
					E16477194
J1221-7068	ET3	GABLE	1	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Thu Dec 9 07:37:44 2021 Page 1

 $ID:I4HRAT3eIT9qoRIdAoEs_5z0Axy-VqqJXVRDjJqMYWN5LdgIeOFm?2dvzXsUdgaVhDyAmHL$

0<u>-1-</u>8

Scale = 1:13.9



	1-4-0	2-8-0	4-0-0	5-4-0	6-8-0	7-5-12			
	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	0-9-12			
Plate Offsets (X,Y) [1:Edge,0-1-8], [3:0-1-8,Edge], [11:0-1-8,Edge], [14:Edge,0-1-8], [15:0-1-8,0-1-8]									

LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-P	DEFL. ii Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n (loc) l/defl L/d a - n/a 999 a - n/a 999 D 8 n/a n/a	PLATES MT20 Weight: 39 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x BOT CHORD 2x WEBS 2x OTHERS 2x	SP No.1(flat) SP No.1(flat) SP No.3(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,

REACTIONS. All bearings 7-5-12.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 14, 8, 13, 12, 11, 10, 9

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center. 3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

- 6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

8) CAUTION, Do not erect truss backwards.





Job	Truss	Truss Type	Qty	Ply	Lot 18 Mitchell Manor Sec. II
					E16477195
J1221-7068	ET4	GABLE	1	1	
					Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,		8	.430 s Aug	16 2021 MiTek Industries, Inc. Thu Dec 9 07:37:44 2021 Page 1

ID:I4HRAT3eIT9qoRIdAoEs_5z0Axy-VqqJXVRDjJqMYWN5LdgleOFm?2dvzXsUdgaVhDyAmHL

0<u>11</u>8

Scale = 1:16.9



<u> </u>	2-8-0	4-0-0	5-4-0	6-8-0	0 0	8-0- 1-4-	-0	<u>9-4-0</u> 1-4-0	10-3-8 0-11-8
Plate Offsets (X,Y)-	[1:Edge,0-1-8], [4:0-1-8,Edg	ge], [14:0-1-8,Edge], [1	8:Edge,0-1-8]						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 CSI 1.00 TC 1.00 BC YES WB	0.06 0.01 0.03	DEFL. Vert(LL) Vert(CT) Horz(CT) 0.	in (loc) n/a - n/a - 00 10	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2	.014 Mat	rix-S					Weight: 50 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4	SP No.1(flat)	I	BRACING- TOP CHORD	Struc	ural wood st	neathing di	rectly applied or 6-0-0	oc purlins.	

 BOT CHORD
 2x4 SP No.1(flat)
 FOP CHORD
 Structural wood shearing directly applied or 6-0-0 oc punil

 BOT CHORD
 2x4 SP No.1(flat)
 except end verticals.

 WEBS
 2x4 SP No.3(flat)
 BOT CHORD
 Rigid ceiling directly applied or 10-0 oc bracing.

 OTHERS
 2x4 SP No.3(flat)
 BOT CHORD
 Rigid ceiling directly applied or 10-0 oc bracing.

REACTIONS. All bearings 10-3-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 18, 10, 17, 16, 15, 14, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

- 6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

8) CAUTION, Do not erect truss backwards.





Job	Truss	Truss Type		Qty	Ply	Lot 18 Mitchell Manor	Sec. II	F40477400
J1221-7068	ET5	GABLE		1	1			E 16477 196
Comtach Inc. Fourt					0.420 a Aus	Job Reference (option	nal) rice las Thu Des 0.07	1-27:45 2024 Dama 4
Comiech, Inc, Fayer	teville, NC - 28314,			ID:I4HRAT	8.430 S Aug BelT9qoRldAc	Es_5z0Axy-z0OhkrSr	UdyD9gylvLBXAbnwsS	zCi_rdrJK3EfyAmHK
0 ₁₁ 8								0 ₁₁₇ 8
								Scale = 1:23.3
			3x4 =					
1 2	27 3	4	5 28 6	7	8	29 9	10	11 12
					•			
	0			0		le le		
								-9-
24 23	22	21	20 19	18	17	16	15	14 13
3x6 = 2x6	2x6	2x6	2x6 3x6	2x6	2x6	2x6	2x6	3x6 =
								2x6
1-4-0	2-8-0 4	-0-0 5-4-0	6-8-0	8-0-0	9-4-0	10-8-0	<u>12-0-0 13-4-0</u> 1-4-0 1-4-0	14-0-0
Plate Offsets (X,Y) [5	i:0-1-8,Edge]		I-+-0	1-4-0	1	1-4-0	1-4-0	
				555				

LOADING (P TCLL 40 TCDL 10 BCLL 0 BCDL 5	osf) 0.0 0.0 0.0 5.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.00 1.00 YES Pl2014	CSI. TC BC WB Matri:	0.12 0.00 0.05 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 13	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 84 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS	 2x4 SP 2x4 SP 2x4 SP 2x4 SP 2x4 SP 	No.1(flat) No.1(flat) No.3(flat) No.3(flat)				BRACING- TOP CHOR BOT CHOR	.D .D	Structur except e Rigid ce	al wood end verti eiling dire	sheathing dir cals. ectly applied o	rectly applied or 6-0-0 or 6-0-0 oc bracing.	oc purlins,

REACTIONS. All bearings 14-0-0.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 24, 23, 22, 21, 20, 19, 18, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center. 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
- Uniform Loads (plf)
 - Vert: 13-24=-10, 1-12=-100

Concentrated Loads (lb) Vert: 4=-91 7=-91 10=-91 27=-91 28=-91 29=-91



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3x4 =

3x6 =

4x4 =

3x6 =

3x4 =

			17-4-12 17-4-12					
Plate Offsets (X,Y)	[17:0-1-8,Edge], [18:0-1-8,Edge]							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.48 BC 0.69 WB 0.46 Matrix-S	DEFL. Vert(LL) -(Vert(CT) -(Horz(CT) (in (loc) 0.19 17-18 0.26 17-18 0.06 14	l/defl >999 >777 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 93 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP	No.1(flat) No.1(flat) No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structur except Rigid ce	ral wood end verti eiling dire	sheathing dire cals. ctly applied or	ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins,	
REACTIONS. (size Max G	REACTIONS. (size) 22=0-3-8, 14=Mechanical Max Grav 22=937(LC 1), 14=943(LC 1)							
FORCES. (lb) - Max. TOP CHORD 2-3=- 9-10= BOT CHORD 21-22	Comp./Max. Ten All forces 250 (lb) or 1705/0, 3-4=-2823/0, 4-5=-2823/0, 5-6=- -2823/0, 10-11=-2823/0, 11-12=-1705/0 2=0/1015, 19-21=0/2365, 18-19=0/3144,	less except when shown. 3312/0, 6-7=-3312/0, 7-9= 17-18=0/3312, 16-17=0/3	=-3312/0, 144, 15-16=0/2365,	,				

BOT CHORD	21-22=0/1015, 19-21=0/2365, 18-19=0/3144, 17-18=0/3312, 16-17=0/3144, 15-16=0/236
	14-15=0/1016
WEBS	2-22=-1349/0, 2-21=0/960, 3-21=-918/0, 3-19=0/622, 5-19=-436/0, 12-14=-1352/0,

12-15=0/959, 11-15=-918/0, 11-16=0/623, 9-16=-436/0, 9-17=-86/552, 7-17=-313/5, 5-18=-86/552, 6-18=-313/5

NOTES-

1) Unbalanced floor live loads have been considered for this design.

4x4 =

3x6 =

3x6 FP = 3x6 =

- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.









22

6x6 =

21

2x6 ||

20

6x6 =

19

2x6 ||

18

6x8 =

17 16

2x6 ||

23

2x6 ||

X ¥

3x6 =

28

3x4 =

27

5x8 ||

3x4 =

26 25 24

3x8 M18AHS FP =

TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	TC 0.67 BC 0.74 WB 0.69 Matrix-S	Vert(LL) Vert(CT) Horz(CT)	-0.20 22-23 -0.28 22-23 0.05 15	>999 >739 n/a	480 360 n/a	MT20 M18AHS Weight: 128 lb	244/190 186/179 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP REACTIONS. (size Max G	No.1(flat) No.1(flat) No.3(flat) 29 29=0-3-8, 15=Mechanical rav 29=1112(LC 1), 15=1169(LC 1)		BRACING- TOP CHOR BOT CHOR	D Struct excep D Rigid	ural wood t end verti ceiling dire	sheathing dire cals. ectly applied o	ectly applied or 6-0-0 c r 10-0-0 oc bracing.	oc purlins,
FORCES. (lb) - Max. TOP CHORD 2-3=- 10-11 BOT CHORD 27-25 20-2 WEBS 2-29= 13-15 7-22=	Comp./Max. Ten All forces 250 (lb) or 2171/0, 3-5=-3758/0, 5-6=-4691/0, 6-7= =-4088/0, 11-12=-2410/0, 12-13=-2410)=0/1244, 26-27=0/3037, 24-26=0/4691, 1=0/4965, 19-20=0/3348, 18-19=0/334 =-1654/0, 2-27=0/1258, 3-27=-1174/0, 3 ;=-1725/0, 13-18=0/1458, 11-18=-1237/ :-541/0, 6-22=0/978, 6-23=-458/0	less except when shown -5203/0, 7-8=-5203/0, 8-1 /0 23-24=0/4691, 22-23=0// , 17-18=0/1304, 15-17=0, -26=0/970, 5-26=-1275/0, 0, 11-20=0/976, 8-20=-11	10=-4093/0, 4691, 21-22=0/496 /1304 , 5-24=0/452, 30/0, 8-22=0/394,	55,				
NOTES- 1) Unbalanced floor live 2) All plates are MT20 3) Plates checked for a 4) Refer to girder(s) for 5) This truss is designer referenced standard 6) Recommend 2x6 str Strongbacks to be a 7) CAUTION, Do not el 8) Hanger(s) or other or chord. The design/s 9) In the LOAD CASE(e loads have been considered for this de plates unless otherwise indicated. I plus or minus 1 degree rotation about i truss to truss connections. ed in accordance with the 2015 Internation ANSI/TPI 1. ongbacks, on edge, spaced at 10-0-0 o tached to walls at their outer ends or re rect truss backwards. onnection device(s) shall be provided su election of such connection device(s) is S) section, loads applied to the face of th	esign. Is center. onal Residential Code sec c and fastened to each trr strained by other means. ufficient to support concer the responsibility of other he truss are noted as fron	ctions R502.11.1 a uss with 3-10d (0.1 ntrated load(s) 481 rs. rs. (F) or back (B).	nd R802.10.2 131" X 3") nail Ib down at 9	and s. 9-4 on top			Ale and a second s
LOAD CASE(S) Stand 1) Dead + Floor Live (b Uniform Loads (plf) Vert: 15-29: Concentrated Loads Vert: 31=-40	dard lalanced): Lumber Increase=1.00, Plate =-10, 1-14=-100 (lb) 01(F)	Increase=1.00						



0-1-8

15

3x6 =






			18-1-0				
Plate Offsets (X.)) [1:Edge.0-1-8], [17:0-1-8.Edge], [18:0-1	-8.Edgel	18-1-0				
	/ [-,g-,					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.56	Vert(LL) -0.22	17-18 >956	480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.77	Vert(CT) -0.31	17-18 >695	360		
BCLL 0.0	Rep Stress Incr YES	WB 0.48	Horz(CT) 0.06	14 n/a	n/a		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S				Weight: 96 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) REACTIONS. (size) 22=Mechanical, 14=0-3-8		BRACING- TOP CHORD BOT CHORD	Structural wood except end verti Rigid ceiling dire	sheathing directicals. ectly applied or	ctly applied or 6-0-0 10-0-0 oc bracing.	oc purlins,	
FORCES (Ib) -	Max Comp /Max Ten - All forces 250 (lb) of	less excent when shown					
	2-3=-1787/0 3-4=-2985/0 4-5=-2985/0 5-6=	-3581/0 6-7=-3581/0 7-9	=-3581/0				
	9-10=-2985/0. 10-11=-2985/0. 11-12=-1787/	0	- 000 170,				
BOT CHORD 21-22=0/1058, 19-21=0/2486, 18-19=0/3347, 17-18=0/3581, 16-17=0/3347, 15-16=0/2486,							

14-15=0/1058 WEBS 2-22=-1409/0, 2-21=0/1013, 3-21=-972/0, 3-19=0/678, 5-19=-492/0, 5-18=-55/627, 6-18=-316/0, 12-14=-1406/0, 12-15=0/1014, 11-15=-973/0, 11-16=0/678, 9-16=-492/0, 9-17=-55/627, 7-17=-316/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.







			<u>18-1-0</u> 18-1-0						
Plate Offsets (X,Y)	[9:0-3-0,Edge], [10:0-3-0,Edge], [21:0-1-	-8,Edge], [22:0-1-8,Edge]	10 1 0						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.53 BC 0.94 WB 0.92 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.25 -0.35 0.08	(loc) 22 22 18	l/defl >845 >610 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 109 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	2 No.1(flat) 2 No.1(flat) 2 No.3(flat)		BRACING- TOP CHORE BOT CHORE		Structur except Rigid ce	ral wood end verti eiling dire	sheathing dir cals. ectly applied c	rectly applied or 5-10-1 or 10-0-0 oc bracing.	oc purlins,
REACTIONS. (size Max G	e) 26=Mechanical, 18=0-3-8 irav 26=1498(LC 1), 18=1066(LC 1)								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-3150/0, 3-4=-3150/0, 4-6=-3973/0, 6-7=-3970/0, 7-9=-4445/0, 9-10=-4442/0, 10-12=-4454/0, 12-13=-3379/0, 13-14=-3379/0, 14-16=-1986/0 BOT CHORD 25-26=0/1698, 24-25=0/3684, 22-24=0/4215, 21-22=0/4442, 20-21=0/3824, 19-20=0/2777, 18-19=0/1162 WEBS 2-26=-2210/0, 2-25=0/1927, 3-25=-776/0, 4-25=-709/0, 4-24=0/380, 16-18=-1545/0, 16-19=0/1145, 14-19=-1101/0, 14-20=0/818, 12-20=-605/0, 12-21=0/1075, 10-21=-658/0, 7-24=-333/0, 7-22=-126/529, 9-22=-328/61									
 NOTES- Unbalanced floor live loads have been considered for this design. All plates are MT20 plates unless otherwise indicated. Plates checked for a plus or minus 1 degree rotation about its center. Refer to grider(s) for truss to truss connections. This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1 1. Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. CAUTION, Do not erect truss backwards. Hanger(s) or other connection device(s) is the responsibility of others. In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). LOAD CASE(S) Standard Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 18-26=-10, 1-17=-100 Concentrated Loads (lb) Vert: 3=-609(F) 									

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L		9-7-0)		ę ę	-11-0
I.		9-7-0)		'()-4-0 ¹
Plate Offsets (X,Y)	[3:0-1-8,Edge], [4:0-1-8,Edge], [14:0-1-8	,0-1-8]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.47 WB 0.31 Matrix-S	DEFL. in Vert(LL) -0.07 Vert(CT) -0.09 Horz(CT) 0.02	(loc) l/defl L/d 11 >999 480 11 >999 360 7 n/a n/a	PLATES MT20 Weight: 54 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
REACTIONS. (siz	e) 13=0-3-8, 7=0-3-8 Grav 13=511(LC 1), 7=517(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-781/0, 3-4=-965/0, 4-6=-499/0, 6-7=-499/0

BOT CHORD 12-13=0/541, 11-12=0/965, 10-11=0/965, 9-10=0/965

WEBS 7-9=0/649, 2-13=-718/0, 2-12=0/334, 3-12=-307/0, 4-9=-640/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

6) CAUTION, Do not erect truss backwards



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2-0-0

0-1-8 Scale = 1:26.5



0-4-0 0-4-0 Plate Offsets (X.Y)-	[1:0-3-0.Edge]. [5:0-1-8.Edge]. [15:0-1-	8.Edgel	15-8-8 15-4-8		I	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.66 BC 0.94 WB 0.56 Matrix-S	DEFL. ir Vert(LL) -0.21 Vert(CT) -0.28 Horz(CT) 0.02	n (loc) l/defl L/d 14-15 >856 480 14-15 >640 360 12 n/a n/a	PLATES GRIP MT20 244/190 Weight: 84 lb FT = 20%F, 11%E	
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied 2-2-0 oc bracing: 15-16.	rectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing, Except:	
REACTIONS. (size) 12=0-3-8, 1=0-3-8 Max Grav 12=829(LC 1), 1=835(LC 1)						
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-900/0, 2-4=-903/0, 4-5=-1988/0, 5-6=-2524/0, 6-7=-2524/0, 7-8=-2371/0, 8-9=-2371/0, 9-10=-1469/0 BOT CHORD 17-18=0/1531, 16-17=0/2524, 15-16=0/2524, 14-15=0/2568, 13-14=0/2027, 12-13=0/891 WEBS 1-18=0/1172, 4-18=.857/0, 4-17=0/794/0, 10-12=-1184/0, 10-13=0/804, 9-13=-776/0, 9-14=0/468, 7-14=-278/0, 7-15=-258/302						
NOTES- 1) Unbalanced floor	live loads have been considered for this d	esign.				

All plates are 3x4 MT20 unless otherwise indicated.

- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and

referenced standard ANSI/TPI 1.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

- Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

7) CAUTION, Do not erect truss backwards.









 	21-9-4	L			35	5-11-0	
Plate Offsets (X,Y)	[7:0-3-0,Edge], [8:0-3-0,0-0-0], [21:0-3-	0,Edge], [31:0-1-8,Edge],	[39:0-1-8,Edge]			4-1-1Z	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.85 BC 0.79 WB 0.74 Matrix-S	DEFL. in Vert(LL) -0.32 Vert(CT) -0.44 Horz(CT) 0.07	(loc) l/defl 39-40 >803 39-40 >596 34 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 195 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 5-7-6 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.			
REACTIONS. (si Max Max	REACTIONS. (size) 44=0-3-8, 34=0-3-8, 27=0-3-8 Max Uplift 27=-31(LC 3) Max Grav 44=1028(LC 3), 34=2416(LC 1), 27=654(LC 4)						
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1903/0, 3-4=-3213/0, 4-5=-3213/0, 5-7=-3892/0, 7-8=-4134/0, 8-11=-4134/0, 11-12=-3071/0, 12-13=-1761/0, 13-14=-1761/0, 14-15=0/653, 15-16=0/3181, 16-17=0/3181, 17-19=-271/1913, 19-21=-1588/905, 21-22=-1572/905, 22-24=-1570/502, 15-16=0/3181, 16-17=0/3181, 17-19=-271/1913, 19-21=-1588/905, 21-22=-1572/905, 22-24=-1570/502, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 15-16=0/3181, 1							
BOT CHORD 43- 36-	44=0/1119, 42-43=0/2658, 41-42=0/3642 -37=0/2554, 35-36=-198/873, 34-35=-170	, 40-41=0/4134, 39-40=0/ 5/0, 32-34=-2289/0, 31-32	'4134, 37-39=0/3568, 2=-1519/881,				
30-3 WEBS 2-4 14-3 5-4 19-3 24-3	30-31=-905/1572, 29-30=-905/1572, 28-29=-233/1489, 27-28=-69/685 2-44=-1487/0, 2-43=0/1091, 3-43=-1050/0, 3-42=-0755, 15-34=-1965/0, 15-35=0/1563, 14-35=-1537/0, 14-36=0/1242, 12-36=-1112/0, 12-37=0/751, 11-37=-728/0, 5-42=-583/0, 5-41=0/446, 7-41=-489/83, 11-39=0/1122, 8-39=-665/0, 17-34=-1477/0, 17-32=0/1061, 19-32=-1107/0, 19-31=0/1493, 25-27=-909/93, 25-28=-99/575, 24-28=-543/128, 24-29=-368/110, 22-29=-6/544, 21-31=-845/0						
NOTES- 1) Unbalanced floor li 2) All plates are MT21 3) All plates are 3x4 I 4) Plates checked for 5) Provide mechanicc 6) This truss is design referenced standau 7) Recommend 2x6 s Strongbacks to be	ive loads have been considered for this de 0 plates unless otherwise indicated. MT20 unless otherwise indicated. • a plus or minus 1 degree rotation about i al connection (by others) of truss to bearin ned in accordance with the 2015 Internation of ANS//TPI 1. strongbacks, on edge, spaced at 10-0-0 constrained to walls at their outer ends or re	esign. ts center. Ig plate capable of withsta onal Residential Code sec or and fastened to each tr strained by other means.	anding 31 lb uplift at joint ctions R502.11.1 and R8(uss with 3-10d (0.131" X	27. 02.10.2 and 3") nails.			

8) CAUTION, Do not erect truss backwards.

-December 9,2021





1		7-10-8	14-	0-8	2	1-11-0	
г		7-10-8	6-2	2-0	1 7	7-10-8	
Plate Of	sets (X,Y)	[8:0-3-0,0-0-0], [22:0-3-0,Edge]					
LOADIN TCLL TCDL BCLL BCDL	G (psf) 40.0 10.0 0.0 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.37 BC 0.60 WB 0.64 Matrix-S	DEFL. in Vert(LL) -0.33 Vert(CT) -0.45 Horz(CT) 0.06	(loc) l/defl L/d 21 >797 480 21 >579 360 16 n/a n/a	PLATES GRIP MT20 244/190 M18AHS 186/179 Weight: 158 lb FT = 20%F, 11%E	
LUMBEI TOP CH BOT CH WEBS	R- ORD 2x4 SF ORD 2x4 SF 2x4 SF	P No.1 (flat) P No.1 (flat) P No.3 (flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.	
REACTI	ONS. (siz Max G	e) 27=0-3-8, 16=0-3-8 Grav 27=1185(LC 1), 16=1185(LC 1)					
FORCES	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.						

- TOP CHORD
 2-3=-2352/0, 3-4=-4056/0, 4-6=-4064/0, 6-7=-5383/0, 7-8=-5847/0, 8-9=-5847/0, 9-10=-5402/0, 10-12=-4085/0, 12-13=-4085/0, 13-14=-2350/0

 BOT CHORD
 26-27=0/1358, 25-26=0/3320, 23-25=0/4944, 22-23=0/5847, 21-22=0/5847, 20-21=0/5744, 18-20=0/5011, 17-18=0/3314, 16-17=0/1360

 WEBS
 2-27=-1765/0, 2-26=0/1348, 3-26=-1313/0, 3-25=0/978, 14-16=-1768/0, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=0/1344, 14-17=
- WEBS 2-27=-1765/0, 2-26=0/1348, 3-26=-1313/0, 3-25=0/978, 14-16=-1768/0, 14-17=0/1344, 13-17=-1307/0, 13-18=0/1024, 10-18=-1200/0, 10-20=0/518, 9-20=-508/0, 6-25=-1151/0, 6-23=0/707, 7-23=-889/0, 9-21=-301/635, 8-21=-272/53

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x6 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.







December 9,2021



Edenton, NC 27932

ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE MARNING - Verify design pa Design valid for use only with MTek® connectors. This does not have a seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **AVSUTPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



December 9,2021



