VINYL SIDING

# **FRONT ELEVATION** WITH SIDE LOAD GARAGE

SCALE 1/8" = 1'-0"

# **PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE** RESIDENTIAL BUILDING CODE

MEAN ROOF HEIGHT 25'-8		HEIGHT TO	RIDGE 30'-0"
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A
FENESTRATION U-FACTOR	0.35	0.35	0.35
SKYLIGHT U-FACTOR	0.55	0.55	0.55
GLAZED FENESTRATION SHGC	0.30	0.30	0.30
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci
WALL R=VALUE	15	15	19
FLOOR R-VALUE	19	19	30
* BASEMENT WALL R=VALUE	5/13	10/15	10/15
** SLAB R-VALUE	0	10	10
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19

F 10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION

NISULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING: INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL IGNED FOR WIND SPEED OF 120 MPH, 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "B"

COMPONENT	& CLA	DDING	DESIG	NED FO	R THE	FOLLO	WING	LOADS
MEAN ROOF	UP T	O 30,	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 451
ZONE 1	14.2	-15.0	14,9	15.8	15,5	16.4	15.9	16.8
ZONE 2	14,2	-18.0	14,9	18.9				20.2
ZONE 3	14.2	18.0		18.9			15.9	20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	17.4	17.4	17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	21.8	17.4	-22.4
DESIGNED FOR WIN	D SPEED	OF 130 MR	H, 3 SEO	OND GUST	(101 FAS	TEST MIL	EXP09.	RE "B"
COMPONENT	& CLA	DDINĞ	DESIG	NED FO	OR THE	FOLLO	WING	LOADS
MEAN ROOF	UP T	O 30,	30'-1"	TO 35'	35 1"	TO 40'	40'-1"	TO 45'
ZONE 1	16.7	-18.0	17.5	-18.9	18,2	-19.6	18.7	-20.2
ZONE 2	16.7	-21,0	17.5	-22.1	18,2			-23.5
ZONE 3	16.7	-21.0	17.5	-22-1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	20.7	20.4	21.3
ZONE 5	18.2	-24.0	19.1	-25-2	19.8	-26.2	20.4	-26.9

# AIR LEAKAGE

Section N1102.4
N1102.4.1 Building thermal envelope. The building thermal nazuza-i buuging thermal envelope, in e buunng themal envelope and be durably sealed with an air barrier system to limit infligitation. The earling method between dissimilar materials shall allow for different expansion and contraction, for all allow for different expansion and contraction, for all when the properties of the called gesteet, we will will be a supported by the called gesteet, we will be a support of the called gesteet and the called the called gesteet the called the called gesteet and the called the called gesteet the called gesteet the called the called gesteet the

open to unconditioned or exterior space.
2. Capping and sealing shafts or chases, including flue shafts.

3. Capping and sealing soffit or dropped ceiling area

## **ROOF VENTILATION**

### SECTION R806

R806.1 Ventilation required. Enclosed attics and enclosed rafter spaces rough: I vertication required, inclosed artics and enclosed ratter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1,6 mm) minimum and 1/4 inch (6,4 mm) maximum. Ventilation openings having a least dimension larger than mm) maximum, verdisplano openings having a least amerision larger than 1/4 inch (6.4 mm) shell be provided with corrosion-resistant wire doth screening, hardware doth, or similar material with openings having a least dimension of 1/5 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum, Openings in roof framing members shall conform to the requirements of Section R802.7.

R806.2 Minimum area, The total rise free vertilating area shall not be less

than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross ventilation area may be reduced to 1/300 when a Class I or II

net rice cross-reministion area may be reduced to 1750 which is used for it vapor retarder is installed on the warm-in-winter side of the celling. Exceptions:

1. Endosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 1558 SQ.FT.

NET FREE CROSS VENTILATION NEEDED: WITHOUT 50% TO 80% OF VENTING 3"0" ABOVE EAVE = 10,39 SQ FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE: OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 5.16 SO.FT.

### **GUARD RAIL NOTES**

### SECTION R312

SECTION R312

R312.1 Where required, Cuards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 min) measured vertically to the floor or grade below at any point within 36 inches (9014 min) horizontally to the dego of the open side. Insect screening shall not be considered as a quard. Mining syrfaces, including states, proches, becomes or landings, shall be not less than 36 inches (914 min) high measured vertically above the adjacent vealing surface, adjacent foed seating or the he connection the leading oction of the treads.

Exceptions

1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

Where the top of the guard also serves as a handrail on the open sides of A writer life top on the *guard* has seven as a final and on the open studies of stairs, the top of the *guard* has lost leven stains, the top of the *guard* has lost leven than 34 inches (965 mm) measured vertically from a line connecting the leading adopted the treads.

R312.3 Opening limitations. Required *guards* shall not have openings from the

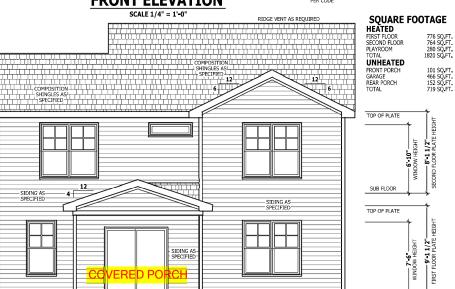
walking surface to the required quard height which allow passage of a sphere 4 inches (102 mm)in diameter, Excentions:

1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 mm) in diameter.

 Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.



# FRONT ELEVATION



101 SQ.FT. 466 SQ.FT. 152 SQ.FT.

SUB FLOOR

SQUARE FOOTAGE HEATED FIRST FLOOR 776 SQ.FT. PLAYROOM TOTAL UNHEATED FRONT PORCH

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PROCEDURES.

CODES MID CONDITIONS MAY WARY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR INSIGNER SHOULD BE CONSULTE BEFORE CONSTRUCTION. THESE DRAWING ARE INSTRUMENTS OF SERVICE AND AS SUCH SMALL REPMAIN PROPERTY OF THE DESIGNER.

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**REAR ELEVATIONS** 

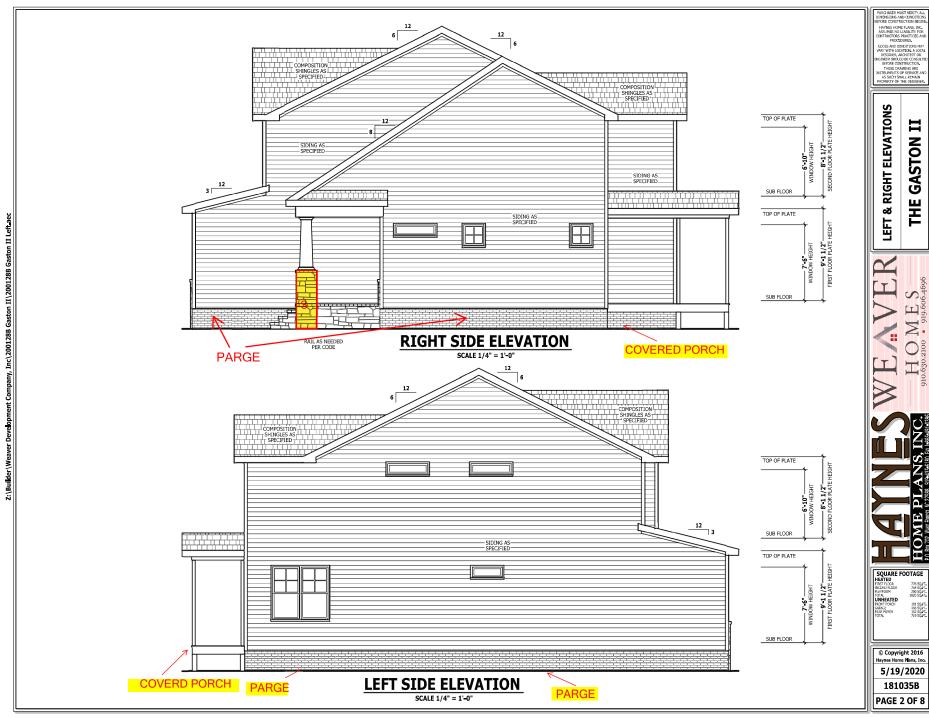
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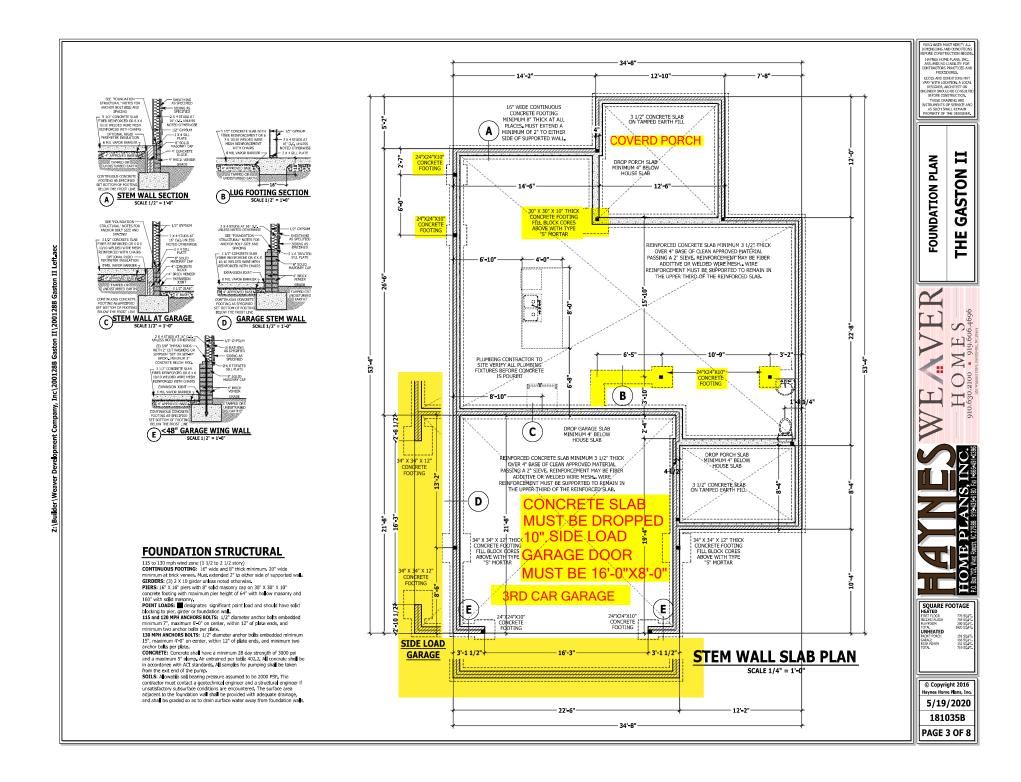
FRONT

PAGE 1 OF 8

**REAR ELEVATION** 

SCALE 1/4" = 1'-0"





### STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code, JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc, assumes no

liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor a failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	_	_
Guardrail in-fill components	50	_	_
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	_	L/360
Cnow	1 20	_	_

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss installed according to the manufacture's specifications. Any orange in orang oots at 2-4 or or center or spans up to 18-9 unless noted observings.

FLOOR SHEATHING: OSB or COX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19,2" on center joist spacing, spacing, and minimum 3/4" thick for 24" on center joist spacing, ROOF SHEATHING: OSB or COX roof sheathing minimum 3/8" thick. CONCRETE AND SOILS: See foundation notes,

# **BRACE WALL PANEL NOTES**

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

GYPSUM: All interior sides of exterior walls and both sides

interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length.

HD: 800 lbs hold down hold down device fastened to the edge

of the brace wall panel closets to the corner.

### Methods Per Table R602 10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0,113" diameter), CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

GB: Interior walls show as GB are to have minimum 1/2' gypsum board on both sides of the wall fastened at 7" of center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. PF: Portal fame per figure R602\_10\_1

# **ROOF TRUSS REQUIREMENTS**

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or installation, in the all yields in the class in the class in inhallation and in a lineat or exceed designated heal helghts, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, cattention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics.

BEARING, All trusses shall be designed for bearing on SPF #2 plates or edgers unless noted otherwise.

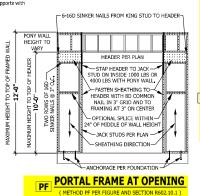
Plate Heights & Floor Systems. See elevation page(s) for plate heights

# **EXTERIOR HEADERS**

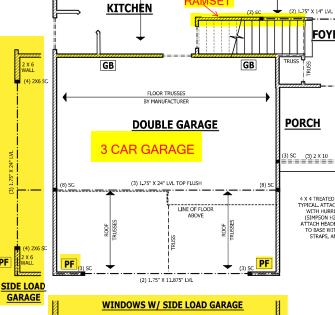
- (2) 2 X 6 WITH 1 JACK STUD EACH END - KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

# INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE NON LOAD BEARING HEADERS TO BE



SCALE 1/4" = 1'-0"



4 X 4 TREATED POST OR FOUTVALENT TYPICAL.

ATTACH RAFTERS TO HEADER WITH HURRICANE CONNECTORS (SIMPSON H2.5 OR EQUIVALENT). ATTACH HEADER TO POST AND POST TO BASE WITH

POST CAP, METAL STRAPS, AND/OR POST BASE,

FLOOR TRUSSES

**DINING ROOM** 

(2) 1-75" X 14" LVL

(4) SC

(3) SC

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(2) 2 X 10

PORCH

ROOF TRUSSES

BY MANUFACTURER

(2) 1.75" X 9.25" LVL

3 TACKS FACH END

FAMILY ROOM

(2) 2 X 8

(3) SC

FOYER

4 X 4 TREATED POST OR EQUIVALENT TYPICAL, ATTACH RAFTERS TO HEADER

WITH HURRICANE CONNECTORS (SIMPSON H2,5 OR EQUIVALENT), ATTACH HEADER TO POST AND POST

TO BASE WITH POST CAP, METAL

STRAPS, AND/OR POST BASE

FIRST FLOOR STRUCTURAL SCALE 1/4" = 1'=0"

PROCEDURES.

COES AND CONDITIONS MAY VARY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR NOTHER SHOULD BE CONSULTE BEFORE CONSTRUCTION.

THESE DRAWING ARE INSTRUMENTS OF SERVICE AND AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER.

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**FIRST FLOOR STRUCTURAL** 

SQUARE FOOTAGE HEATED FIRST FLOOR 776 SQLFI UNHEATED

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# STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall

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DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with Imited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		_
Guardral in fill components	50	_	_
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20	_	_

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise,

### ENGINEERED WOOD REAMS

Eminateak waneer lumber (LVL) = Fb=2800 PSI, Fv=285 PSI, E=1,9x106 PSI
Paralel strand lumber (PSJ.) = Fb=2800 PSI, Fv=290 PSI, E=2,0x106 PSI
Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1,55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND INIOIST MEMBERS: All mof truss and Inioist layouts shall be prepared in accordance with this document.

Trusses and I-joists shall be installed according to the
manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel LINTELS: Brick lincles shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6"0" span, 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9"0" unless noted otherwise, 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" boths at 2"0" on center for spans up to 18"0" unless noted otherwise.

FLOOR SHEATHING: OSB or CDX floor sheathling minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4"

thick for 24" on center joist spacing, ROOF SHEATHING: OSB or CDX roof sheathing minimum

# CONCRETE AND SOILS: See foundation notes. **ATTIC ACCESS**

### SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net deer opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other mm oy 762 mm) and shall be located in a naiway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located

### Exceptions:

1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.

2. Pull down stair treads, stringers, handrails, and hardware may

### **EXTERIOR HEADERS**

- (2) 2 X 6 WITH 1 JACK STUD FACH END UNLESS NOTED OTHERWISE

- KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16'

### INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED

### **ROOF TRUSS REQUIREMENTS**

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and Refer which and Celland includes a minimizer which reights and celling heights are shown furred down 10° from roof decking not insulation. If for any reason the truss manufacturer falls to meet or exceed designated heel heights, finished knee wall heights, or finished celling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Havnes Home Plans. Inc. valy. Any unsupparty must be prought to traying a more main, and attention, so a suitable solution can be reached before construction begins, Any variation due to these conditions not being met is the reasonability of the truss manufacturer. ANCHORAGE. All required anchors for trusses due to uplift or bearing.

shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

# **WALL THICKNESSES**

Exterior walls and walls adjacent to a garage area include 1/2" sheathing or gypsum. Subtract 1/2" for

stud face,

Interior walls are drawn as 3 1/2" or as noted 2 X 6

are drawn as 5 1/2", and do not include gypsum.

# **EXTERIOR WINDOWS AND DOORS**

protrude into the net dear opening.

R612 General. This section prescribes performance and construction requirements for not say deleted. In second prescribed per letter of the say that the control of the say that the control of the say that the say the say that the say that the say that the say that the say the say that the say the sa

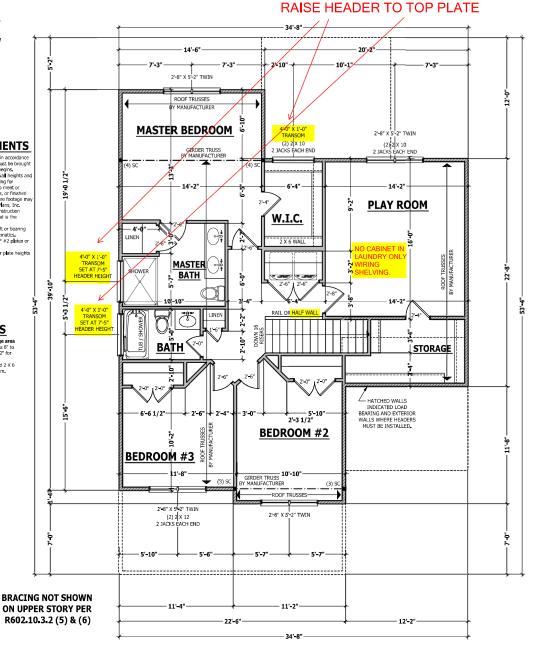
or door.

R612.2 Window sills. In dwelling units, where the opening of an operable window is NO.1.24 window sins. I in overlify units, where the opening of an operace window sins. I in a vowering units, which is the finished grade of surface bodie, the breast part of the dair opening of the window shall be a minimum of 24 inches (GIO min) above the finished grade of surface window shall not be a minimum of 24 inches (GIO min) above the finished floor of the room in which the windows is kacked. Operate sections of windows shall not permit postings that allow passage of a 4 linch (102 min) diameter sphere where such openings are located within 24 inches (GIO min) of the finished floor. Exceptions:

In Windows whose openings will not allow a 4-inch diameter (102 mm) sphere to pass through the opening when the opening is in its largest opened position.

2. Openings that are provided with window fall prevention devices that comply with Section

 Openings that are provided with fall prevention devices that comply with ASTM F 2090. 4. Windows that are provided with opening limiting devices that comply with Section R612.4. R612.3 Window fall prevention devices. Window fall prevention devices and window guards, where provided, shall comply with the requirements of ASTM F 2090.



**SECOND FLOOR PLAN** 

SCALE 1/4" = 1'-0"

EFORE CONSTRUCTION BEGIN HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES.

PROCEDIRES.
COSES AND CURRENTS MAY
VARY WITH LOCATIONS ANY
VARY WITH LOCATIONS A LOCAL
DESIGNER, ARCHITECT OR
ENGINEER SHOULD BE CONSULTE
BEFORE CONSTRUCTION.
THESE DRAWNING ARE
INSTRUMENTS OF SERVICE ANI
AS SUCH SHALL REMAIN
PROPERTY OF THE DESIGNER.

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PLAN **SECOND FLOOR** 

SQUARE FOOTAGE UNHEATED

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**ROOF PLAN** 

Η **THE GASTON** 

| SQUARE FOOTAGE | HEATED | FROM 1995 | FR

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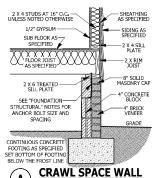
PAGE 7 OF 8

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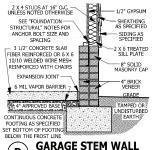
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**Development** 

Builder\Weaver



SCALE 3/4" = 1'-0"



# **DECK STAIR NOTES**

SECTION AM110
AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have minimum 3 1/2 inches between step cut and back of stringer. If used, suspended headers shall shall be attached with 3/8 inch galvanized bolts with nuts and washers to securely support stringers at the top.

SCALE 3/4" = 1'-0"

### **DECK BRACING**

SECTION AM109

AM109\_1 Deck bracing\_ Decks shall be braced to provide lateral stability. The following are acceptable means to provide lateral stability.

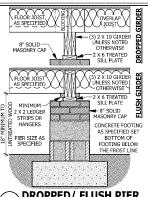
AM109-1-1. When the deck floor height is less than 4'-0' AM109 11 while the deck foot flegint is less traft + a above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

AM109\_1\_2\_ 4 x 4 wood knee braces may be provided on each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be bolted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the brace per Figure AM109.1

AM109-1-3. For freestanding decks without knee braces or diagonal bracing, lateral stability may be provided by

and the following:									
POST SIZE	MAX TRIBUTARY AREA	MAX, POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER					
4 X 4	48 SF	4'-0"	2'-6"	1'-0"					
6 X 6	120 SF	6'-0"	3'-6"	1'-8"					

AM109.1.4. 2 x 6 diagonal vertical cross bracing may be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 x 6's shall exterior column line for attached decks, The 2 x 6's sibe attached to the posts with one 5/8 inch hot dipped galvanized bolt with nut and washer at each end of each bracing member per Figure AM109.3. AM109.1.5. For embedment of piles in Coastal Regions,



2 X 4 STUDS AT 16" O.C. — UNLESS NOTED OTHERWISE

SUB FLOOR AS-

2 X 6 TREATED SILL PLATE

SEE "FOLINDATION:

STRUCTURAL" NOTES FOR ANCHOR BOLT SIZE AND

SPACING

CONTINUOUS CONCRETE

FOOTING AS SPECIFIED

SET BOTTOM OF FOOTING

2 x 6 TREATEI SILL PLATE

8" SOLID -

8" CONCRETI

C

FLOOR JOIST AS SPECIFIED

-1/2" GYPSUM

-8" SOLID MASONRY CAP

4" CONCRETE BLOCK

-4" BRICK VENEER

- EXPANSION JOINT

6 MIL VAPOR BARRIER

3 1/2" SLAR

CRAWL SPACE AT GARAGE

SCALE 3/4" = 1'-0"

-2 X 4 STUDS AT 16" O.C.

2 X 4 SOLE PLATE

FILLED PORCH SECTION WITH VENT

BOLT POST TO GIRDER
WITH (2) 1/2" HOT DIPPED
GALVANIZED BOLTS
5/4 X 6 OR 2 X 4 TREATED
DECKING MINIMUM 1/4"
GAP BETWEEN DECKING

CONTRACTOR PLAN

ACH JOIST WITH HANGE

G DECK ATTACHMENT

SMOKE ALARMS

e detection and notification. All smoke alarms shall be

Isted in accordance with UL 217 and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72. R314.2 Smoke detection systems. Household fire alarm systems

installed in accordance with NFPA 72 that include smoke alarms, or a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be permitted. The household fire alarm system shall provide the same.

Exception: Where smoke alarms are provided meeting the

exception: where sinuse agains are provided meeting the requirements of Section R314.4.

R314.3 Location. Smoke alarms shall be installed in the following

Outside each separate sleeping area in the immediate vicinity of

SECTION R314

cations: . In each sleeping room

SCALE 1/2" = 1'-0

4" BASE

NDISTURBED

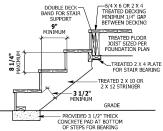
" CONCRET BLOCK

4" BRICK

### DROPPED/ FLUSH PIER В SCALE 3/4" = 1'-0"



### <48" GARAGE WING WALL SCALE 3/4" = 1'-0'



# FIGURE AM110 TYPICAL DECK STAIR DETAIL

SCALE 3/4" = 1'-0"

# WEEP SCREEDS

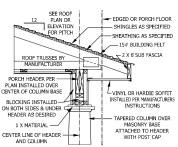
All weep screeds and stone veneer to be installed per manufactures instructions and per the 2012 North Carolina Residential

mm) (No. 26 galvanized sheet gage), corrosion-resistant weep screed or plastic weep screed, with a minimum vertical attachment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep screed shall be placed a minimum of 4 inches (102 mm) above the earth or 2 inches (51 mm) above paved areas and shall be of a type that will allow trapped water to drain to the exterior of the building. The weather-resistant barrier shall lap the attachment flange. The exterior lath

the bedrooms. R703-6-2-1 = A minimum 0-019-inch (0-5 the begrooms.

3. On each additional story of the dwelling, including basements.

3. On each adulturial skory of the avenue, including care-writers and habitable statics (initished) but not indiciding carely spaces, uninhabitable (unfinished) attics and uninhabitable (unfinished) attics star durinhabitable (unfinished) attics staries. In overlings or develing unit's with split kevels and without an intervening door between the adjacent levels, a moke alerm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level. below the upper level. When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of alarms in the individual unit. P314 4 Power source Smoke alarms shall receive their primary R314.4 Power source. Smoke alarms shall receive their primary power from the building wiring when such wiring is served from a commercial source, and when primary power is interrupted, shall receive power from a battery. Wiring shall be permanent and without a disconnecting switch other than those required for overcurrent protection. Smoke alarms shall be interconnected. attachment flange of the weep screed.



# **PORCH HEADER WITH TAPERED COLUMN**

SCALE 3/4" = 1'-0"

# CARBON MONOXIDE ALARMS

SECTION R315

R315.1 Carbon monoxide alarms. In new construction, dwelling units shall be K313.1 Carbon monoxide adams, in new construction, divided junits shall be provided with an approved carbon monoxide adam installed outside of each separate sleeping area in the immediate vicinity of the bedroom(s) as directed by the adam manufacturer. R313.2 Where required modelings, In existing dividings, when the constructions, repairs, full-fired appliance replacements, or additions

requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section

R315.3 Alarm requirements. The required carbon monoxide alarms shall be K313.3 Again requirements, I he required carbon monoxide agains and be audible in all bedrooms over background noise levels with all intervening doors closed, Single station carbon monoxide alarms shall be listed as complying with UL 2034 and shall be installed in accordance with this code and the manufacturer's installation instructions,

R311.7.2 Headroom. The minimum headroom in all parts of the stainvay shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the

R311.7.4 Stair treads and risers. Stair treads and risers shall meet the KS11./ A Stair treads and riseds, Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges of

permitted, before the detection and after a system shall provide the alone level of smoke detection and should be a simple should be a simple should be a simple should be a simple should be a R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1/4 inches (32 mm) shall be provided on stairways with solid approved supervising station and be maintained in accordance with

continuous run of treads or flight with four or more risers.

R311.7.1. Height, Handrall neight, measured vertically from the sloped plane adjoining the tread nosing, or finish surfaces of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm).

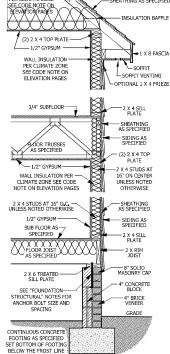
1. The use of a volute, turnout or starting easing shall be allowed over the

full length of the flight, from a point directly above the top rise of the flight to a point directly above the top rise of the flight to a point directly above the lowest riser of the flight, thandrail ends shall be returned or shall terminate in newl posts or safety terminals, Handrail adjacent to a well shall have a space of not less than 11/2 inch (38 mm) between the well and the handrails.

2. The use of a volute, turnout, starting easing or starting newel shall be allowed over the lowest tread

allowed over the lowest tread.

3. Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrai /handrail, the wall-mounted rail must return into the wall.



ITCH PER ROOF PLAN

SHINGLES AS SPECIFIED

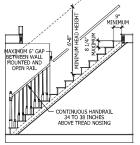
-15# BUILDING FELT

SHEATHING AS SPECIFIED

OR FLEVATIONS

ROOF INSULATION

PER CLIMATE ZONE



TYPICAL WALL DETAIL

SCALE 3/4" = 1'-0"

TYPICAL STAIR DETAIL SCALE 1/4" = 1'-0'

SQUARE FOOTAGE UNHEATER

O

CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR WINEER SHOULD BE CONSULTE BEFORE CONSTRUCTION.

THESE DRAWING ARE STRUMENTS OF SERVICE .

AS SUCH SHALL REMAIN ROPERTY OF THE DESIGNE

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DETAIL

TYPICAL

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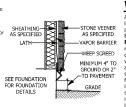
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181035B PAGE 8 OF 8



**WEEP SCREED** SCALE 3/4" = 1'-0"

# **STAIRWAY NOTES**

landing or platform on that portion of the stairway.

the adiacent treads. R311.7.4.2 Tread denth. The minimum tread denth shall be 9 inches (229 R311.7-4.2 Tread depth. The minimum tread depth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

minimum tread depth of 4 inches (102 mm) at any point.

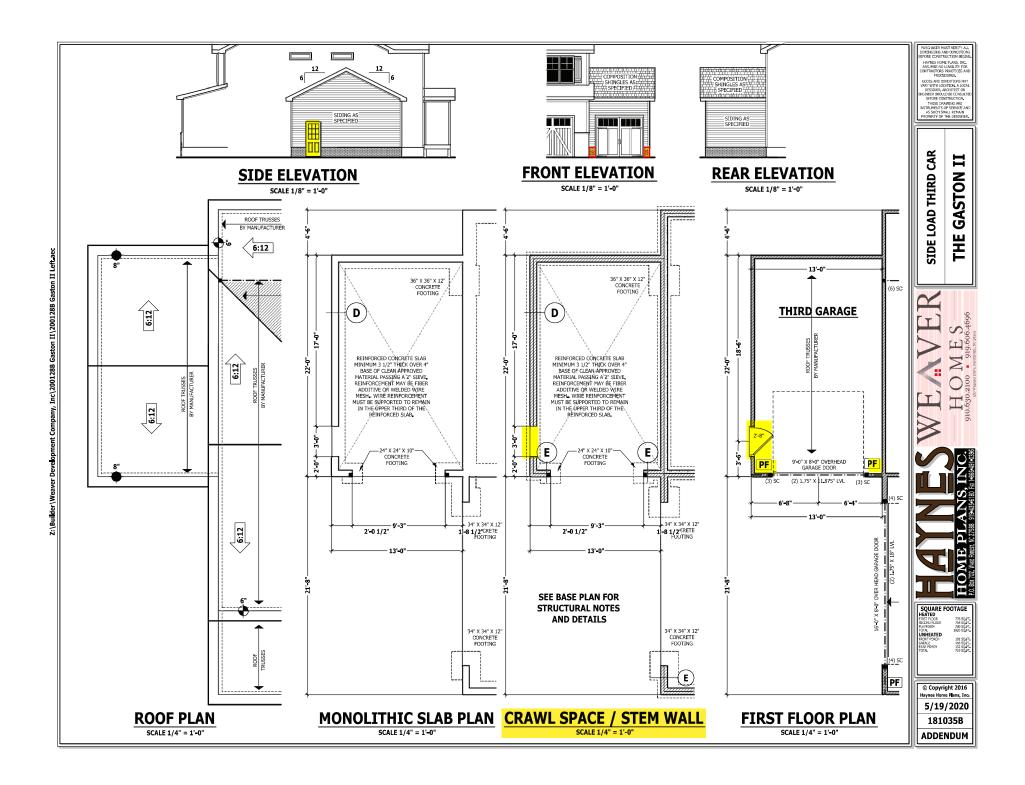
R311.7.7 Handrails, Handrails shall be provided on at least one side of each

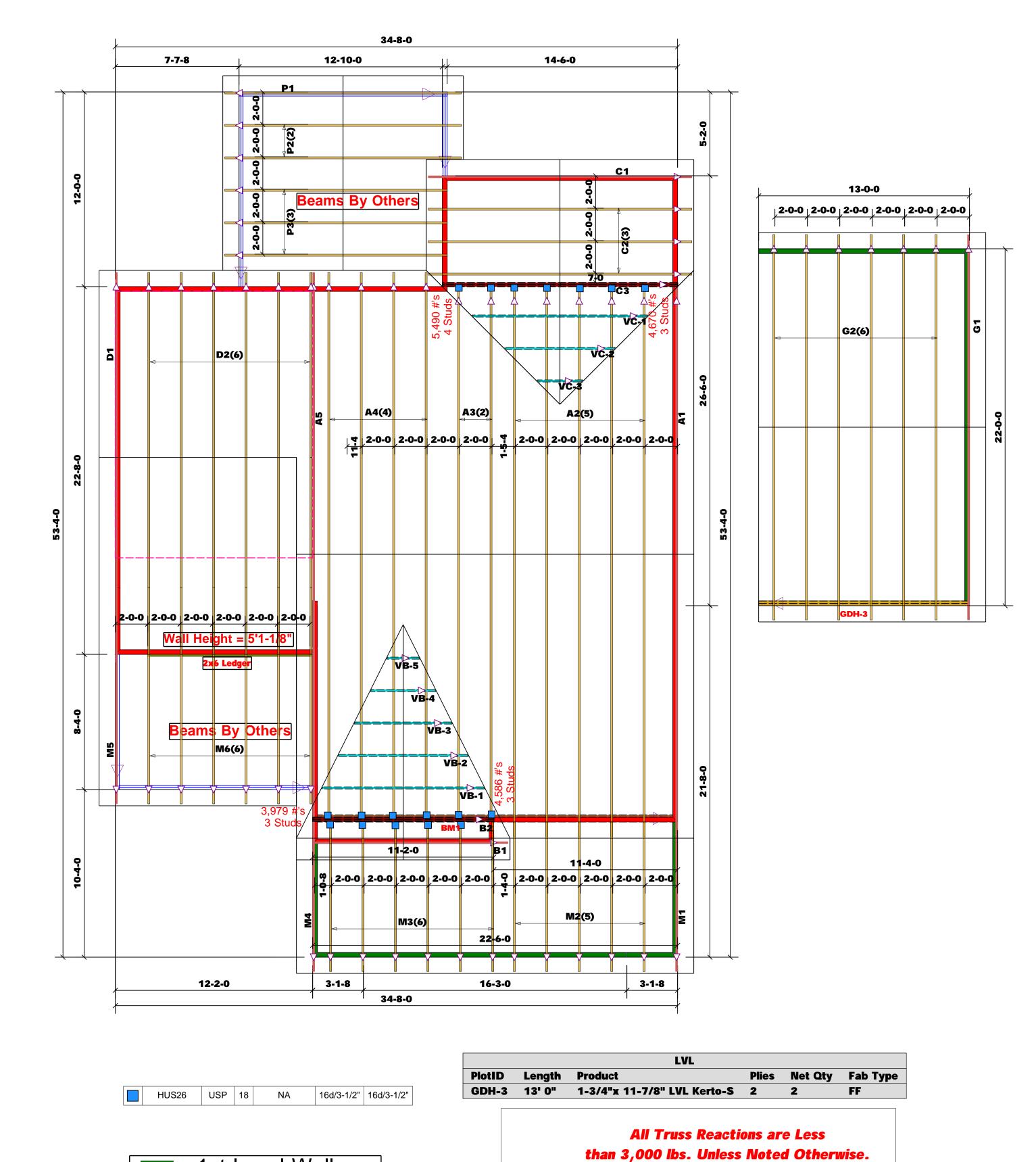
lowest tread

lowest trees. When handral fittings or bendings are used to provide continuous transition between flights, the transition from handral to guardral, or used at the start of a flight, the handral height at the fittings or bendings shall be permitting to exceed the maximum height. \$11,17,12 Continuity, Handrals for stativays shall be continuous for the

Exceptions:

1. Handrails shall be permitted to be interrupted by a newel post.





= 1st Level Wall

= 2nd Level Wall

-- Denotes Reaction Greater than 3,000 lbs.

Reaction / # of Studs

Truss Placement Plan SCALE: 1/4"=1' = Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

LOAD CHART FOR JACK STUDS  (BASE ON LABOR SECSIO) 4-001  MARIE OF JACK STUDS SECURITION (ALTHOUGH										
		PEAGER/1	61R068							
END REACTION (OT FU)	SEC DISTUDS FOR COMPANY HEADER	SND PENCTION (LP TO)	MRQ IS STUDS FOR CIPAN - PARKIN	END REACTION (0.º 170)	REQUESTLES FOR (4) MY HEADER					
1700	1	2550	1	3400	1					
3400	2	5100	2	6600	2					
5100	3	7650	3	10200	3					
6800	4	10200	4	13600	4					
8500	5	12750	5	17000	5					
10200	6	15300	6							
11900	7									
13600	8									
15300	9									

			SCALE: 1/4"=1"	
BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A These trus the building
JOB NAME	Lot 4R Mitchell Manor	ADDRESS	159 Mitchell Manor Dr.	is responsi the overall walls, and regarding b
PLAN	Gaston II (181035B) 3 Car/SL	MODEL	Roof	Bearing represcriptive
SEAL DATE	N/A	DATE REV.	11	( derived to foundation than 3000 be retaine
QUOTE #		DRAWN BY Marshall Naylor		specified retained to
JOB#	J0522-2437	SALESMAN	Lenny Norris	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

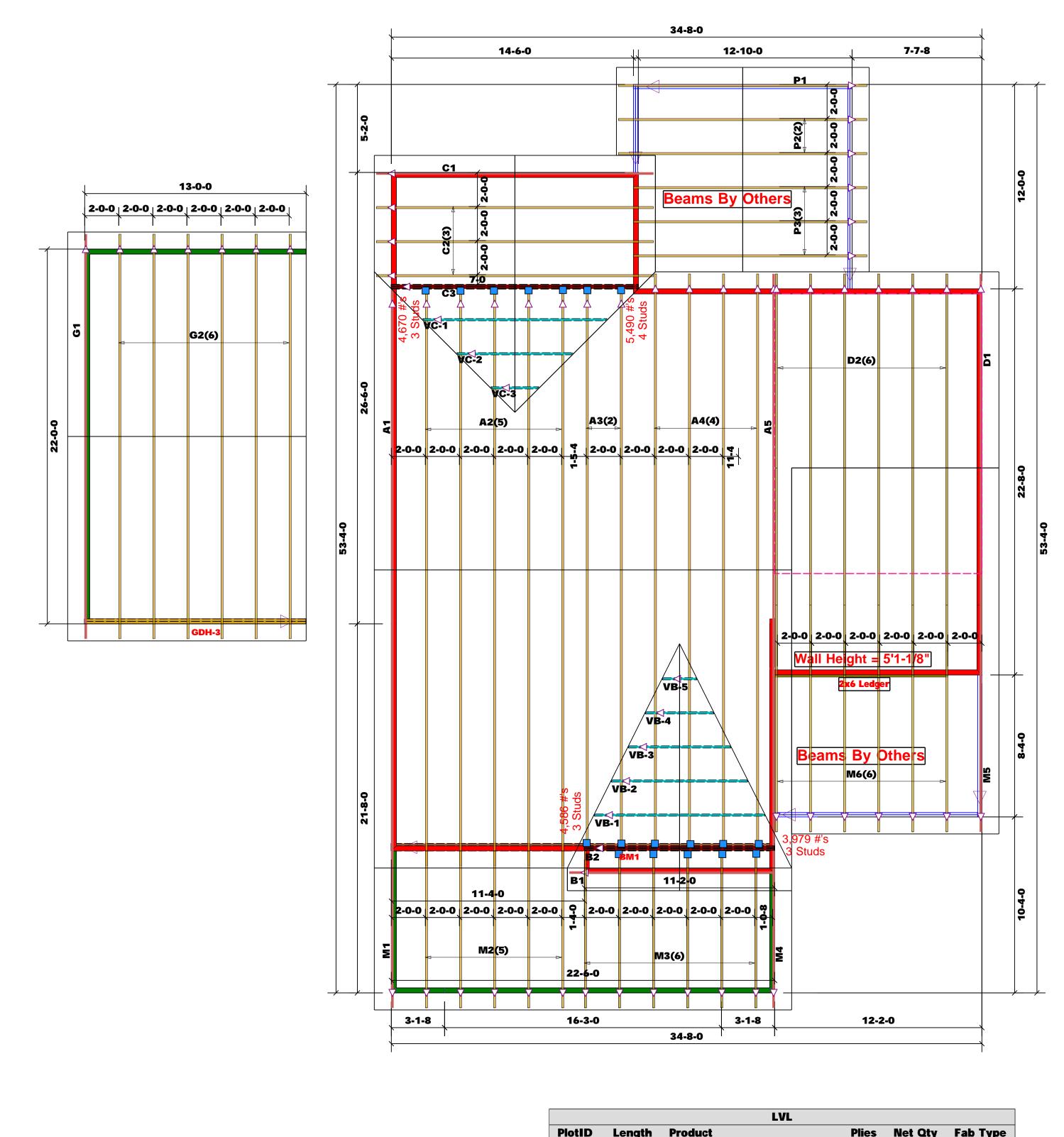
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Marshall Naylor



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444



USP 18 HUS26 16d/3-1/2" | 16d/3-1/2" **PlotID** Length **Product Plies** Net Qty Fab Type GDH-3 1-3/4"x 11-7/8" LVL Kerto-S FF 13-0-0

= 1st Level Wall

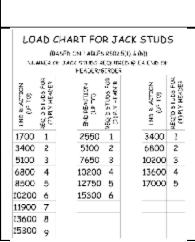
= 2nd Level Wall

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

Truss Placement Plan SCALE: 1/4"=1'

= Indicates Left End of Truss (Reference Engineered Truss Drawing) **Do NOT Erect Truss Backwards** 



			SCALE: 1/4 =1	
BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS These tre the buildi sheets fo
JOB NAME	Lot 4R Mitchell Manor	ADDRESS	159 Mitchell Manor Dr.	is respon the overa walls, and regarding
PLAN	Gaston II (181035B) 3 Car/SL	MODEL	Roof	Bearing prescrip
SEAL DATE	N/A	DATE REV.	11	( derived foundation than 300 be retain
QUOTE #		DRAWN BY	Marshall Naylor	specified retained
JOB#	J0522-2437	SALESMAN	Lenny Norris	
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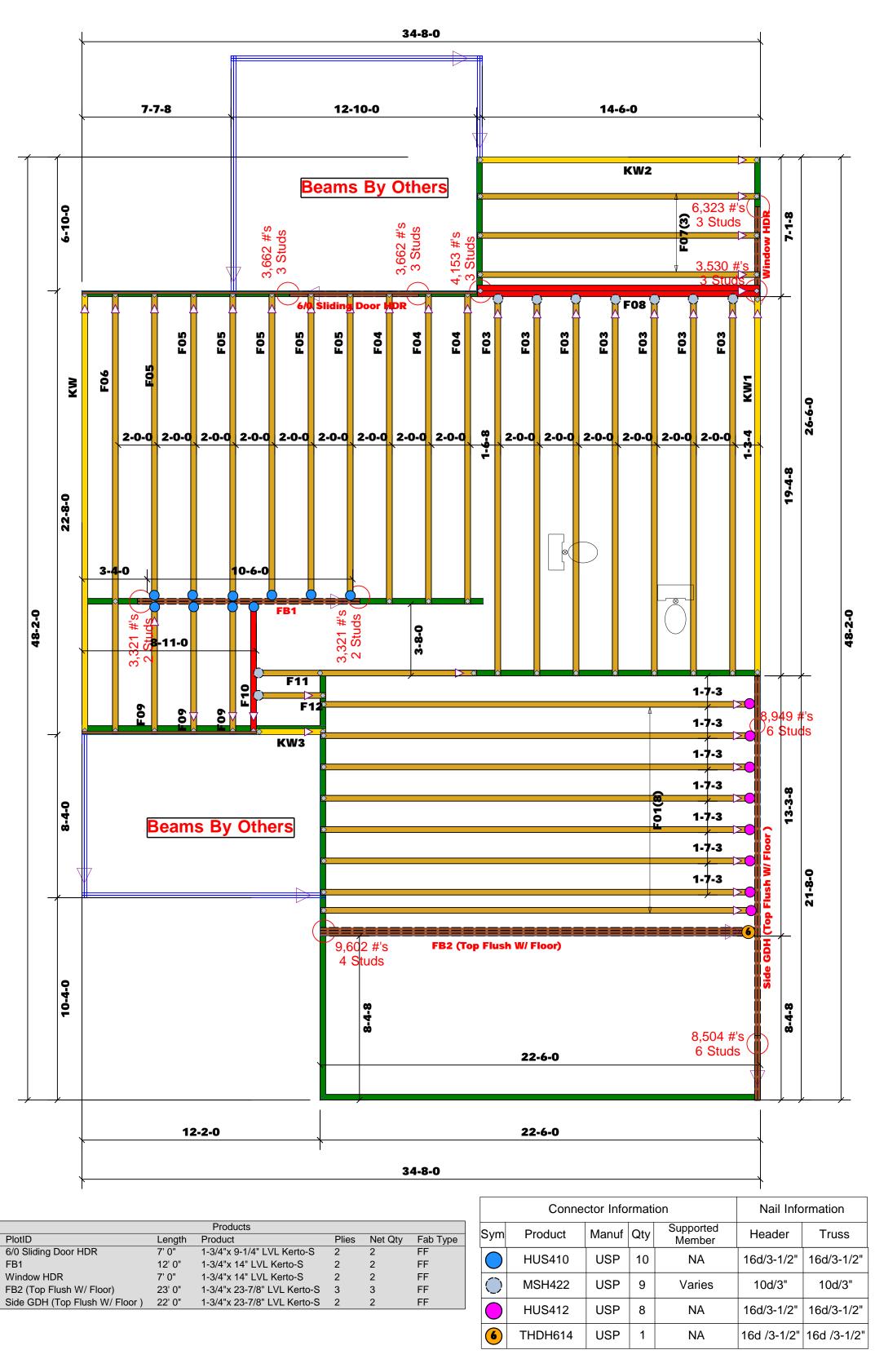
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**Marshall Naylor** 



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Truss Placement Plan

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

LO	LOAD CHART FOR JACK STUDS											
N.A		ASENION LABLES El Jack Sturg R		5(1) & (6() (Co (6) CA CND OF								
		FEADER/6										
END REACTION (UF 10)	SEC DISTURBINGS CONTRACTOR	SND PENCTION (LP TL)	REQUESTABLE FOR CORN - FARER	END REACTION (01°10)	REQ'D STUDS FOR (4) MY HEADER							
1700	1	2550	1	3400	1							
3400	2	5100	2	6600	2							
5100	3	7650	3	10200	3							
6800	4	10200	4	13600	4							
8500	5	12750	5	17000	5							
10200	á	15300	6									
11900	7											
13600	8											
15300	9											

			SCALE: NTS	
BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A These trus the building sheets for e
JOB NAME	Lot 4R Mitchell Manor	ADDRESS	159 Mitchell Manor Dr.	is responsi the overall walls, and o regarding b
PLAN	Gaston II (181035B) 3 Car/SL	MODEL	Floor	or online @  Bearing re prescriptive
SEAL DATE	N/A	DATE REV.	11	( derived f foundation than 3000 be retained
QUOTE #	B0520-1988	DRAWN BY	Marshall Naylor	specified i retained to
JOB#	J0522-2438	SALESMAN	Lenny Norris	

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**Marshall Naylor** 

ROOF & FLOOR
TRUSSES & BEAMS

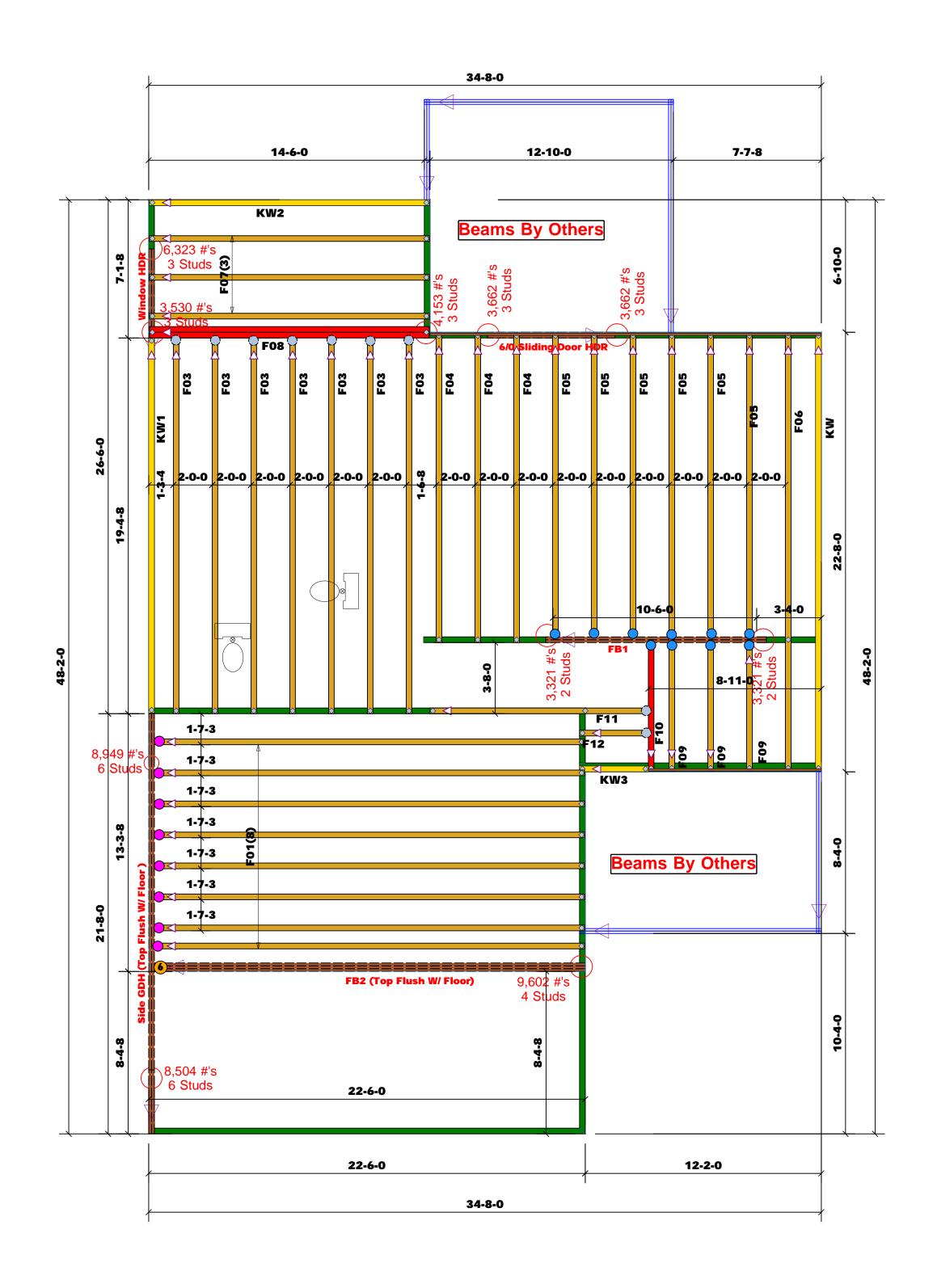
TRUSSES & BEAMS

TRUSSES & BEAMS

Reilly Road Industrial Park

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

соттесн



		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
6/0 Sliding Door HDR	7-0-0	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
FB1	12-0-0	1-3/4"x 14" LVL Kerto-S	2	2	FF
Window HDR	7-0-0	1-3/4"x 14" LVL Kerto-S	2	2	FF
FB2 (Top Flush W/ Floor)	23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3	FF
Side GDH (Top Flush W/ Floor )	22-0-0	1-3/4"x 23-7/8" LVL Kerto-S	2	2	FF

	Conne	Nail Info	rmation			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS410	USP	10	NA	16d/3-1/2"	16d/3-1/2"
$\bigcirc$	MSH422	USP	9	Varies	10d/3"	10d/3"
	HUS412	USP	8	NA	16d/3-1/2"	16d/3-1/2"
6	THDH614	USP	1	NA	16d /3-1/2"	16d /3-1/2"

# Truss Placement Plan

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

LO	AD C	HAL	RT FO	R J	ACK STUD	5				
NLA					5(1) & (6() ((5 @ CA CM5 5))					
PEADER/BERDER										
OT 50)	AEC DISTURS FOR CONTY HEADER		SND PENCTION OF ALC	REQUESTADS FOR CORN FABER	END NACTION (01°10)	REQ'D STUDS FOR				
1700	1		2550	1	3400	1				
3400	2		5100	2	6800	2				
5100	3		7650	3	10200	3				
0086	4		10200	4	13600	4				
8500	5		12750	5	17000	- 5				
10200	á		15300	6						
11900	7									
13600	8									
15300	9									

			SCALE: NIS	
BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUSS PLACEM These trusses are designed as in the building design at the specific sheets for each truss design iden
JOB NAME	Lot 4R Mitchell Manor	ADDRESS	159 Mitchell Manor Dr.	is responsible for temporary and the overall structure. The design walls, and columns is the respon regarding bracing, consult BCSI-
PLAN	Gaston II (181035B) 3 Car/SL	MODEL	Floor	or online @ sbcindustry.com  Bearing reactions less than or prescriptive Code requiremen
SEAL DATE	N/A	DATE REV.	11	( derived from the prescriptive foundation size and number of than 3000# but not greater that be retained to design the supplemental to the sign that it is not seen that the supplemental to the supplemental
QUOTE #	B0520-1988	DRAWN BY	Marshall Naylor	specified in the attached Table retained to design the suppor
JOB#	J0522-2438	SALESMAN	Lenny Norris	Signature

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sign the support system for all reactions that exceed 15000

Signature

**Marshall Naylor** 

ROOF & FLOOR TRUSSES & BEAMS Reilly Road Industrial Park Fayetteville, N.C. 28309

> Phone: (910) 864-8787 Fax: (910) 864-4444

соттесн



Client: Weaver Homes

Project:

Address: Gaston II (181035B) Date: 5/12/2022

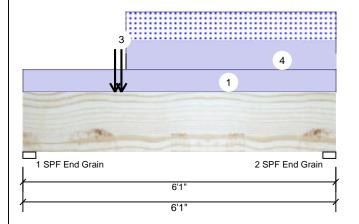
Input by: Marshall Naylor Job Name: Gaston II

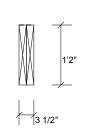
Project #:

**Kerto-S LVL** 1.750" X 14.000" 2-Ply - PASSED Window HDR

Level: Level

Describes HRIDATTEDRIED IL (Helifa)





Page 1 of 1

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TI:	360

Member Information

Importance: Normal - II

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Rea	ctions dive	ALIEKNEL	, ip (Obiiti	)		
Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	723	4403	1838	0	0
2	Vertical	282	2388	1142	0	0

# **Analysis Results**

Temperature:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10224 ft-lb	1'11"	31049 ft-lb	0.329 (33%)	D+0.75(L+S)	L
Unbraced	10224 ft-lb	1'11"	17620 ft-lb	0.580 (58%)	D+0.75(L+S)	L
Shear	6150 lb	1'5"	12021 lb	0.512 (51%)	D+0.75(L+S)	L
LL Defl inch	0.016 (L/4193)	2'3 15/16"	0.143 (L/480)	0.114 (11%)	0.75(L+S)	L
TL Defl inch	0.052 (L/1315)	2'4"	0.190 (L/360)	0.274 (27%)	D+0.75(L+S)	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

Temp <= 100°F

- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

Bearings	Bearings													
Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.							
1 - SPF End Grain	3.000"	Vert	72%	4403 / 1920	6323	L	D+0.75(L+S)							
2 - SPF	3.000"	Vert	40%	2388 / 1142	3530	L	D+S							

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
2	Point	1-9-8		Тор	3014 lb	1005 lb	0 lb	0 lb	0 lb	F8
	Bearing Length	0-3-8								
3	Point	1-11-0		Тор	2335 lb	0 lb	2335 lb	0 lb	0 lb	C3
	Bearing Length	0-3-8								
4	Part. Uniform	2-0-0 to 6-1-0		Тор	158 PLF	0 PLF	158 PLF	0 PLF	0 PLF	C2
	Self Weight				11 PLF					

Grain

# Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

  1. UVI beams must not be out or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info







Project: Address:

Weaver Homes

Date: 5/12/2022

Input by: Marshall Naylor Job Name: Gaston II

Project #:

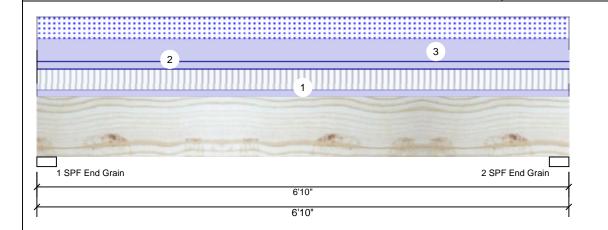
6/0 Sliding Door HDR **Kerto-S LVL** 

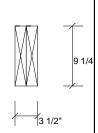
1.750" X 9.250"

Gaston II (181035B)

2-Ply - PASSED

Level: Level





Page 1 of 1

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П٦	7	•		u			к	w	•			a		u	•	

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)												
Brg	Direction	Live	Dead	Snow	Wind	Const						
1	Vertical	1100	1965	1162	0	0						
2	Vertical	1100	1965	1162	0	0						

# **Analysis Results**

Temperature:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5588 ft-lb	3'5"	14423 ft-lb	0.387 (39%)	D+0.75(L+S)	L
Unbraced	5588 ft-lb	3'5"	10130 ft-lb	0.552 (55%)	D+0.75(L+S)	L
Shear	2573 lb	1' 1/4"	7943 lb	0.324 (32%)	D+0.75(L+S)	L
LL Defl inch	0.051 (L/1511)	3'5"	0.161 (L/480)	0.318 (32%)	0.75(L+S)	L
TL Defl inch	0.111 (L/700)	3'5"	0.215 (L/360)	0.514 (51%)	D+0.75(L+S)	L

# **Bearings**

Grain

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.000"	Vert	42%	1965 / 1696	3662	L	D+0.75(L+S)
2 - SPF End	3.000"	Vert	42%	1965 / 1696	3662	L	D+0.75(L+S)

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.

Temp <= 100°F

- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width

. Laterart	ordinadiriodd ratio badda dii	omigio pij maan									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	108 PLF	322 PLF	0 PLF	0 PLF	0 PLF	F4	
2	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	

3 Uniform Top 340 PLF 0 PLF

Self Weight 7 PLF

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code
  approvals

- approvals
  Damaged Beams must not be used
  Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

0 PLF

340 PLF

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





0 PLF



Project: Address:

Weaver Homes

Gaston II (181035B)

Date: Input by: 5/12/2022

Page 1 of 2

Marshall Naylor Job Name: Gaston II

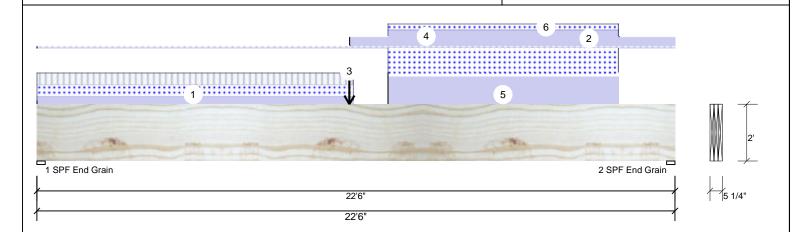
Project #:

**Kerto-S LVL** FB<sub>2</sub>

1.750" X 24.000"

3-Ply - PASSED

Level: Level



### Member Information R Туре: Girder Application: Floor Plies: 3 Design Method: ASD Moisture Condition: Dry **Building Code:** IBC 2012 Deflection LL: 480 Load Sharing: Yes Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F Bearings

### **Analysis Results**

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	59233 ft-lb	11' 1/4"	131295 ft-lb	0.451 (45%)	D+S	L
Unbraced	59233 ft-lb	11' 1/4"	59272 ft-lb	0.999 (100%)	D+S	L
Shear	9560 lb	20'2 1/2"	30912 lb	0.309 (31%)	D+S	L
LL Defl inch	0.209 (L/1267)	11'4"	0.552 (L/480)	0.379 (38%)	S	L
TL Defl inch	0.454 (L/583)	11'5 7/16"	0.735 (L/360)	0.618 (62%)	D+S	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 4'5 3/4" o.c.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width

Reaction	is UNF	PATTERNEI	D lb (Uplif	t)	
			·		

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	1493	3705	3383	0	0
2	Vertical	632	5449	4153	0	0

Dear mig.	•						
Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	48%	3705 / 3657	7362	L	D+0.75(L+S)
2 - SPF End Grain	3.500"	Vert	62%	5449 / 4153	9602	L	D+S

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 11-2-0		Near Face	100 PLF	150 PLF	150 PLF	0 PLF	0 PLF	M3
2	Tie-In	0-0-0 to 22-6-0	0-6-0	Far Face	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	1' Floor
3	Point	11-0-4		Тор	2294 lb	0 lb	2294 lb	0 lb	0 lb	B2
	Bearing Length	0-3-8								
4	Part. Uniform	11-0-4 to 22-6-0		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
5	Part. Uniform	12-4-8 to 20-6-0		Тор	360 PLF	0 PLF	360 PLF	0 PLF	0 PLF	A2

Continued on page 2...

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Indicating & Installation

  I. VIL beams must not be cut or drilled

  Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  1. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info







Weaver Homes

Project: Address:

Gaston II (181035B)

Date: 5/12/2022

Input by: Marshall Naylor Job Name: Gaston II

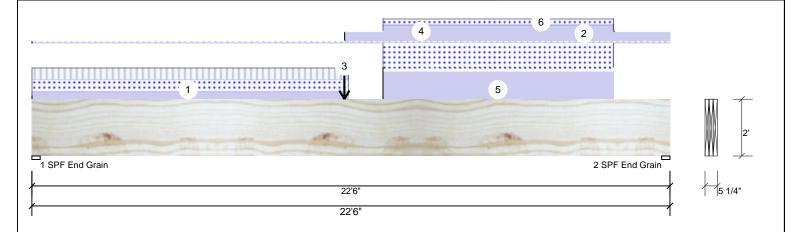
Page 2 of 2

Project #:

1.750" X 24.000" **Kerto-S LVL** FB<sub>2</sub>

3-Ply - PASSED

Level: Level



.Continued from page 1

ID Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments Load Type 6 Part. Uniform 12-4-8 to 20-6-0 Near Face 79 PLF 0 PLF 79 PLF 0 PLF 0 PLF M2

> Self Weight 28 PLF

# Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. IVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- This design is valid until 11/3/2024

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info







Client: Weaver Homes

Project:

Address: Gaston II (181035B) Date: 5/12/2022

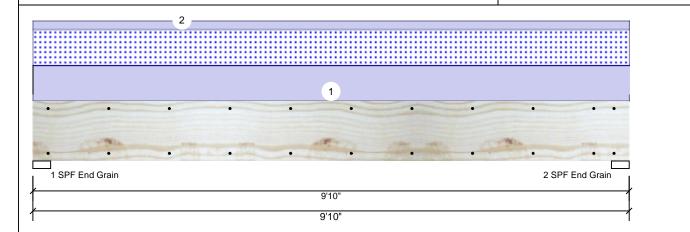
Input by: Marshall Naylor Job Name: Gaston II

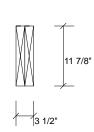
Project #:

**Kerto-S LVL** GDH-2

1.750" X 11.875" 2-Ply - PASSED

Level: Level





Page 1 of 1

### Member Information Type Girder

Typo.	Ondo
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal -

Ш Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

# **Reactions UNPATTERNED Ib (Uplift)**

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1540	1200	0	0
2	Vertical	0	1540	1200	0	0

•	Maiysis Res	uits					
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	6122 ft-lb	4'11"	22897 ft-lb	0.267 (27%)	D+S	L
	Unbraced	6122 ft-lb	4'11"	9857 ft-lb	0.621 (62%)	D+S	L
	Shear	2035 lb	1'3 3/8"	10197 lb	0.200 (20%)	D+S	L
	LL Defl inch	0.051 (L/2213)	4'11"	0.234 (L/480)	0.217 (22%)	S	L

# Analysis Results

Moment	6122 ft-lb	4'11"	22897 ft-lb	0.267 (27%) D+S	L	
Unbraced	6122 ft-lb	4'11"	9857 ft-lb	0.621 (62%) D+S	L	
Shear	2035 lb	1'3 3/8"	10197 lb	0.200 (20%) D+S	L	
LL Defl inch	0.051 (L/2213)	4'11"	0.234 (L/480)	0.217 (22%) S	L	
TL Defl inch	0.116 (L/969)	4'11"	0.312 (L/360)	0.372 (37%) D+S	L	

# **Bearings**

Grain

l	Bearing	Length	Dir.	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
ı	_	3.500"	Vert	27%	1540 / 1200	2740	L	D+S
1	End Grain							
	2 - SPF End	3.500"	Vert	27%	1540 / 1200	2740	L	D+S

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	244 PLF	0 PLF	244 PLF	0 PLF	0 PLF	G2
2	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Load
	Self Weight				9 PLF					

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- L. UVL beams must not be cut or drilled
   Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
   Damaged Beams must not be used

- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info







Project: Address:

Weaver Homes

Gaston II (181035B)

Date: 5/12/2022

Input by: Marshall Naylor Job Name: Gaston II

Page 1 of 1

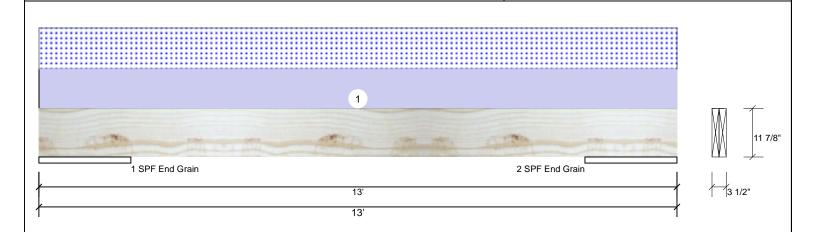
Project #:

Kerto-S LVL FB1

1.750" X 11.875"

2-Ply - PASSED

Level: Level



Member Info	Member Information				ctions UNP	ATTERNE	) lb (Uplift	)		
Type:	Girder	Application:	Floor	Brg	Direction	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	Vertical	0	1685	1625	0	0
Moisture Conditi	on: Dry	Building Code:	IBC 2012	2	Vertical	0	1685	1625	0	0
Deflection LL:	480	Load Sharing:	No							
Deflection TL:	360	Deck:	Not Checked							
Importance:	Normal - II									
Temperature:	Temp <= 100°F									
				Doo	rimae					

# **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5595 ft-lb	6'6"	22897 ft-lb	0.244 (24%)	D+S	L
Unbraced	5595 ft-lb	6'6"	9857 ft-lb	0.568 (57%)	D+S	L
Shear	1860 lb	2'10 3/8"	10197 lb	0.182 (18%)	D+S	L
LL Defl inch	0.052 (L/2160)	6'6"	0.234 (L/480)	0.222 (22%)	S	L
TL Defl inch	0.106 (L/1060)	6'6"	0.312 (L/360)	0.340 (34%)	D+S	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

Posringe				
Bearings				
Bearing Length	Dir.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	22.500"	Vert	5%	1685 / 1625	3310	L	D+S
2 - SPF End Grain	22.500"	Vert	5%	1685 / 1625	3310	L	D+S

### ID Load Type Live 1 Snow 1.15 Trib Width Side Dead 0.9 Wind 1.6 Const. 1.25 Comments Location

1	Uniform	Тор	250 PLF	0 PLF	250 PLF	0 PLF	0 PLF	G2
	Self Weight		9 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Notes

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

  1. IVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info







Client: Weaver Homes

Project:

Address: Gaston II (181035B) Date: 5/12/2022

Marshall Naylor Input by: Job Name: Gaston II

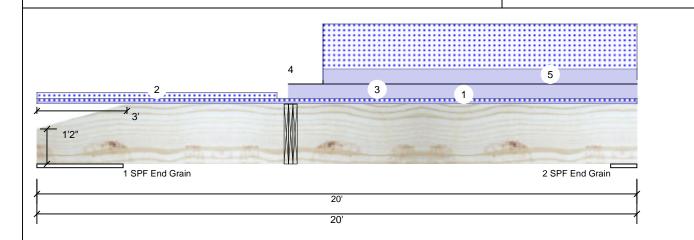
Project #:

**Kerto-S LVL** Side GDH

1.750" X 24.000"

2-Ply - PASSED

Level: Level



### Member Information Girder Type Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)													
Brg	Direction	Live	Dead	Snow	Wind	Const							
1	Vertical	414	4766	4182	0	0							
2	Vertical	218	4109	4395	0	0							

Page 1 of 2

### Analysis Results Comb. Case Analysis Actual Location Allowed Capacity Moment 0.570 (57%) D+S 47951 ft-lb 8'5 1/2" 84163 ft-lb L Unbraced 47951 ft-lb 8'5 1/2" 48066 ft-lb 0.998 L Shear 9556 lb 4'10 1/2" 20608 lb 0.464 (46%) D+S 0.427 (43%) D+S 157 psi, 8784 368 psi Lt. Scarf L lb LL Defl inch 0.153 (L/1286) 10'6 3/16" 0.410 (L/480) 0.373 (37%) S L TL Defl inch 0.320 (L/615) 10'4 7/16" 0.547 (L/360) 0.585 (59%) D+S

# Bearings

Grain

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 34.500" Vert 4766 / 4182 8949 L D+S End Grain 4109 / 4395 D+S 2 - SPF 10.500" Vert 8504 L Fnd

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Notches in LVL are in accordance with APA Form No. EWS G535, Figure 1.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 3'7 7/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Tie-In	0-0-0 to 20-0-0	1-0-0	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	2' Roof
2	Part. Uniform	8-0-0 to 0-0-0		Тор	0 PLF	0 PLF	50 PLF	0 PLF	0 PLF	Gable
3	Part. Uniform	8-4-4 to 20-0-0		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall

Continued on page 2...

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVI beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code
  approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us





isDesign

Client:

Address:

Weaver Homes Project:

Gaston II (181035B)

Date: 5/12/2022

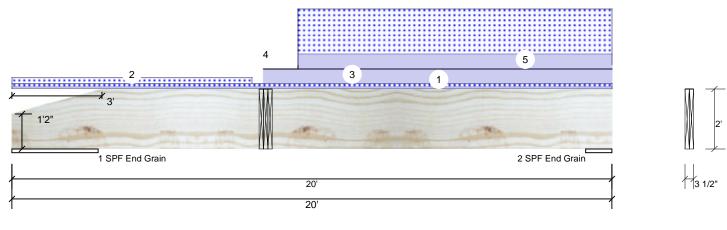
Input by: Marshall Naylor Job Name: Gaston II

Project #:

**Kerto-S LVL** Side GDH

1.750" X 24.000" 2-Ply - PASSED

Level: Level





Page 2 of 2

Continued	from	page	1
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ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
4	Point	8-5-8		Far Face	5449 lb	632 lb	4153 lb	0 lb	0 lb	FB2 Brg 2	
5	Part. Uniform	9-6-8 to 20-0-0		Far Face	120 PLF	0 PLF	360 PLF	0 PLF	0 PLF	F01	
	Self Weight				19 PLF						

# Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. IVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- This design is valid until 11/3/2024

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info



