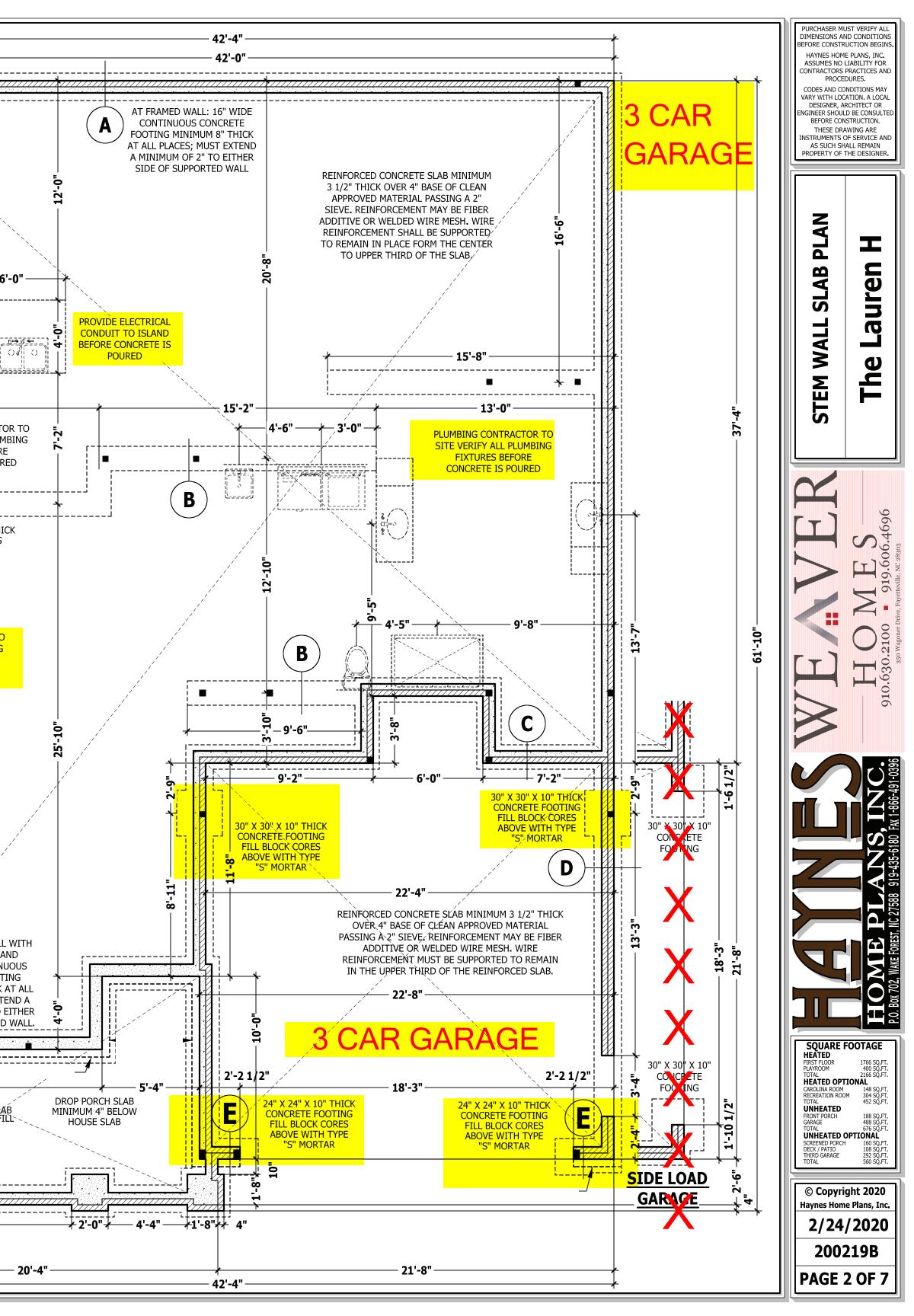


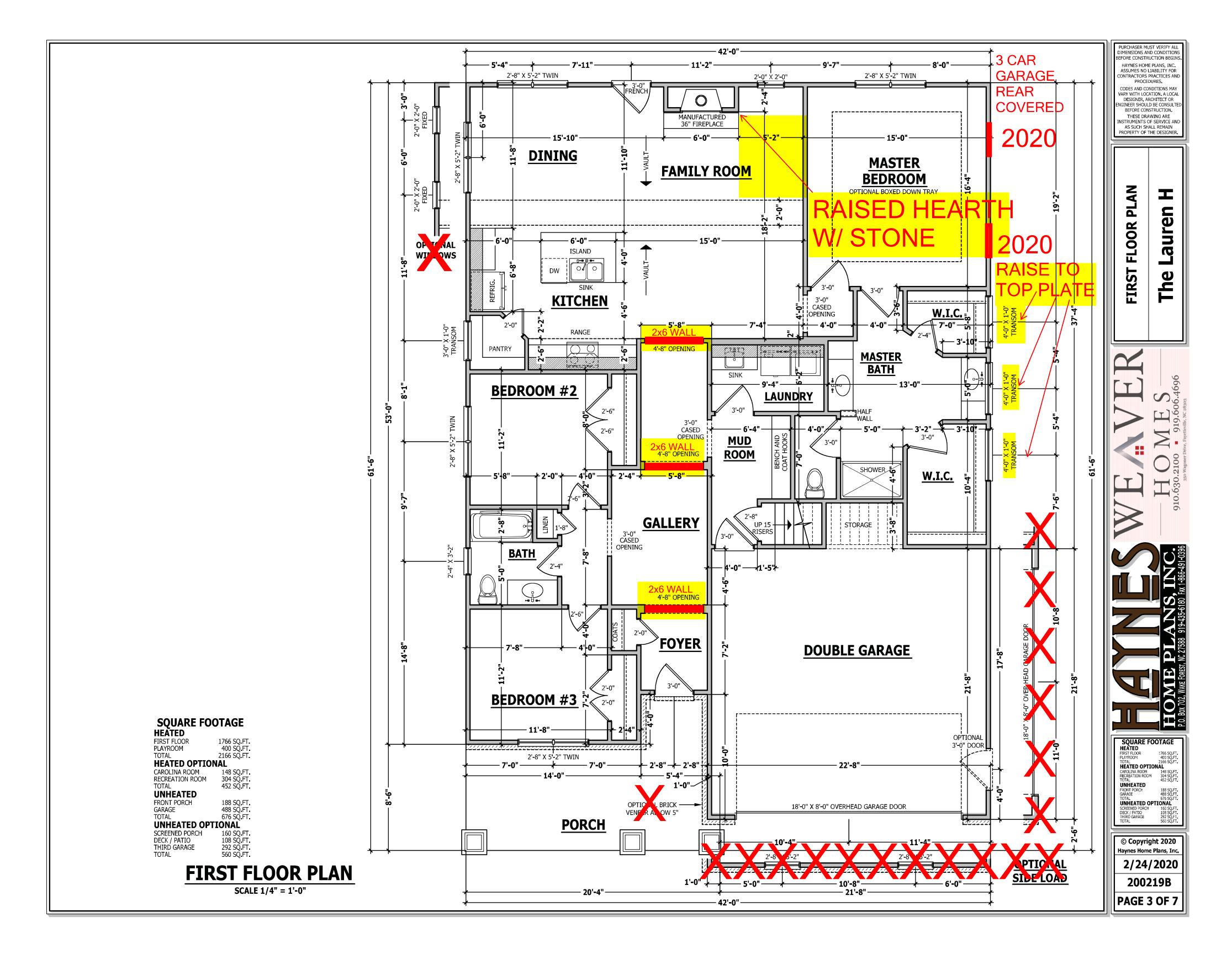
6'-0'

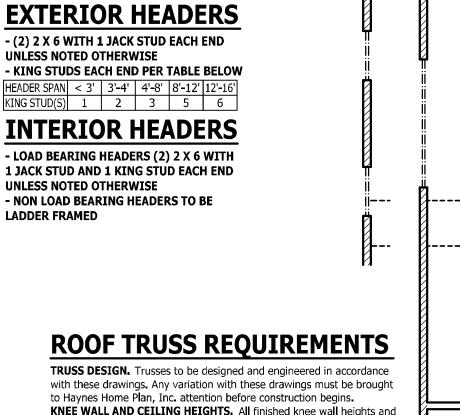
Odii

4"

SCALE 1/4" = 1'-0"







### **STRUCTURAL NOTES**

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		-
Guardrail in-fill components	50		—
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		-

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1 9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum

5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing **ROOF SHEATHING:** OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on

center rafters. **CONCRETE AND SOILS:** See foundation notes.

with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may

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shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

# **BRACE WALL PANEL NOTES**

**EXTERIOR WALLS:** All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

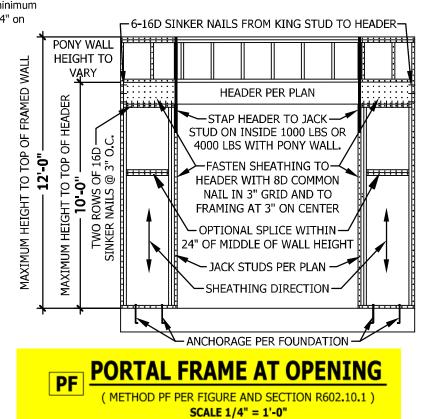
**REOUIRED LENGTH OF BRACING:** Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length.

**HD**: 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

Methods Per Table R602.10.1

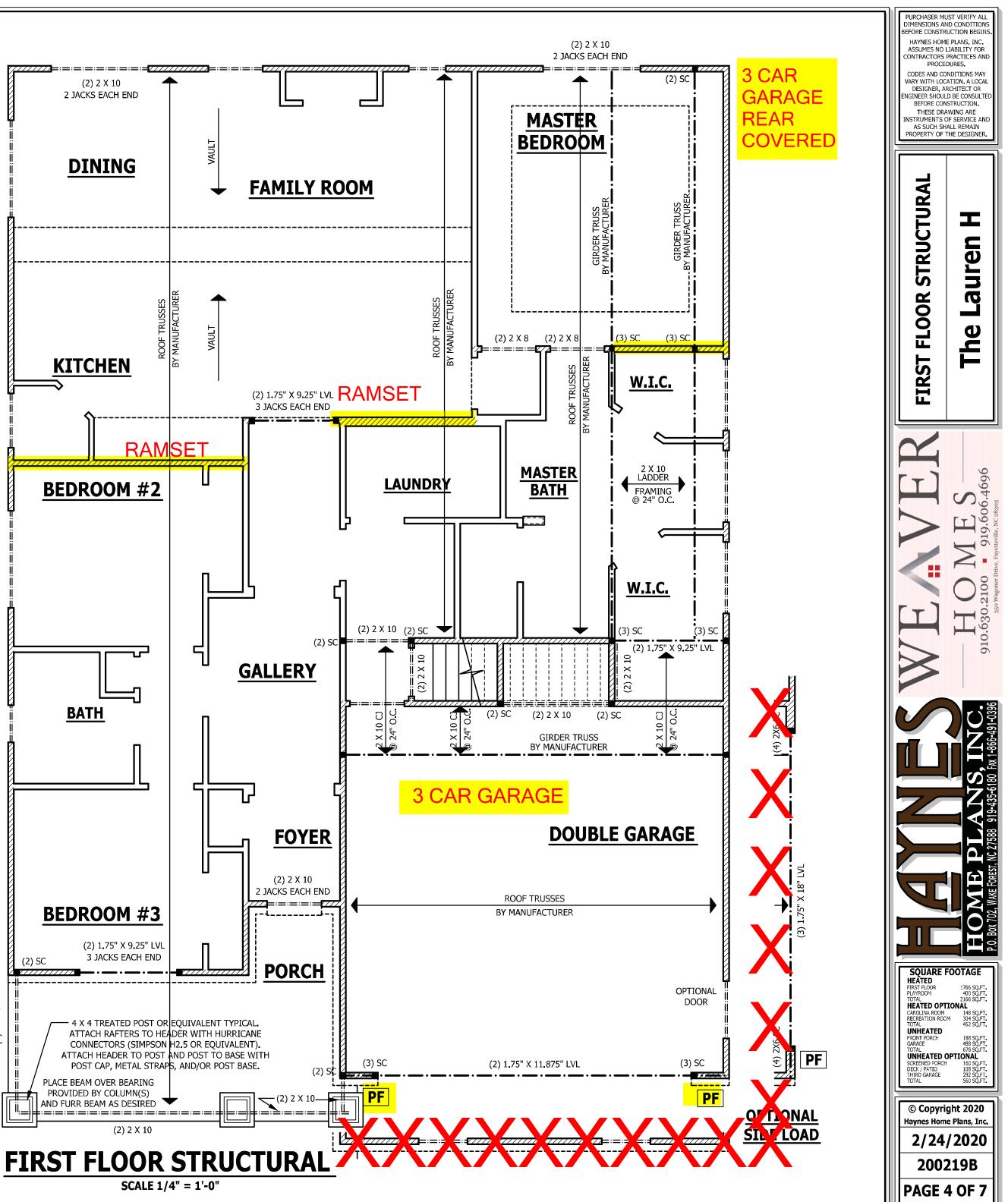
CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or  $8d(2 1/2" \log x 0.113" diameter)$ . **CS-SFB:** Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with  $1 \frac{1}{2}$  long x 0.12" diameter galvanized roofing nails.

**GB:** Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. **PF**: Portal fame per figure R602.10.1



10

(2) 2 X



### **ROOF TRUSS REQUIREMENTS**

**TRUSS DESIGN.** Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

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Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

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DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		—
Guardrail in-fill components	50		—
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		_

**FRAMING LUMBER:** All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS :

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10<sup>6</sup> PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x10<sup>6</sup> PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x10<sup>6</sup> PSI Install all connections per manufacturers instructions.

**TRUSS AND I-JOIST MEMBERS:** All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3  $1/2" \times 3 1/2" \times 1/4"$  steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3  $1/2" \times 3 1/2" \times 1/4"$  steel angle with 1/2"bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. **FLOOR SHEATHING:** OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing. **ROOF SHEATHING:** OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters. **CONCRETE AND SOILS:** See foundation notes.

# ATTIC ACCESS

### SECTION R807

**R807,1 Attic access.** An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.

2. Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.

# **EXTERIOR HEADERS**

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE

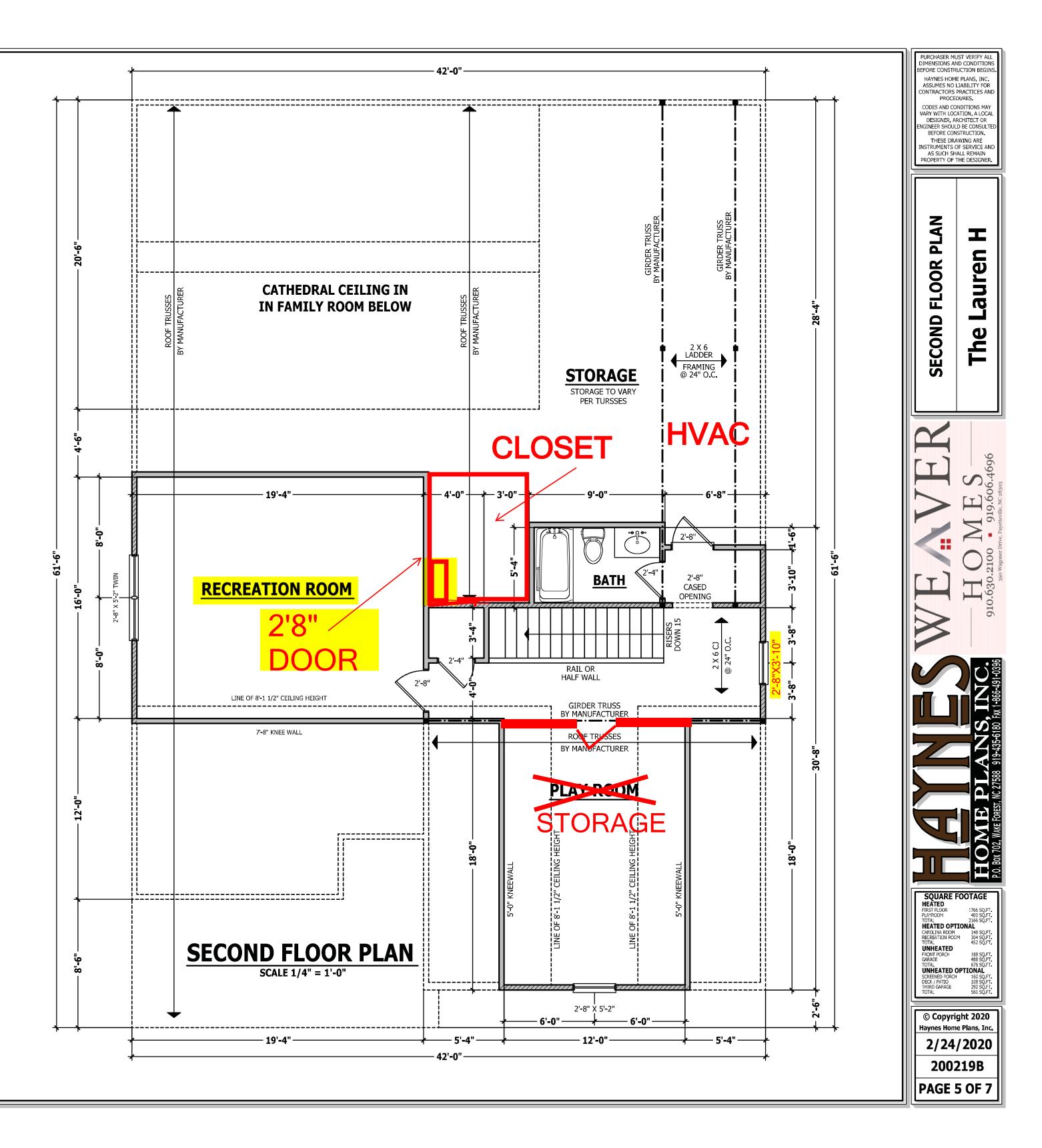
 - KING STUDS EACH END PER TABLE BELOW

 HEADER SPAN
 < 3'</td>
 3'-4'
 4'-8'
 8'-12'
 12'-16'

 KING STUD(S)
 1
 2
 3
 5
 6

## **INTERIOR HEADERS**

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED



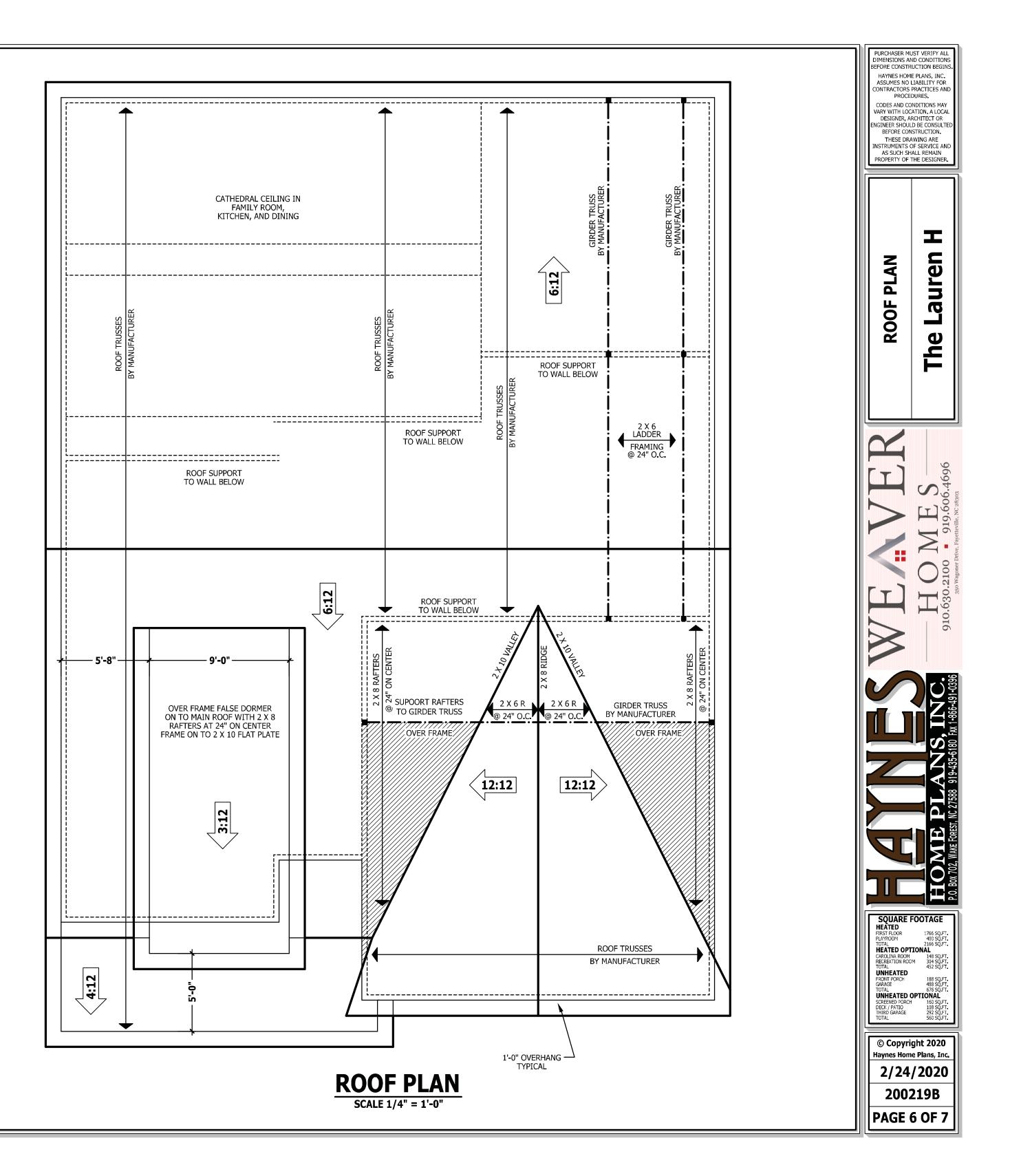
# **ROOF TRUSS REQUIREMENTS**

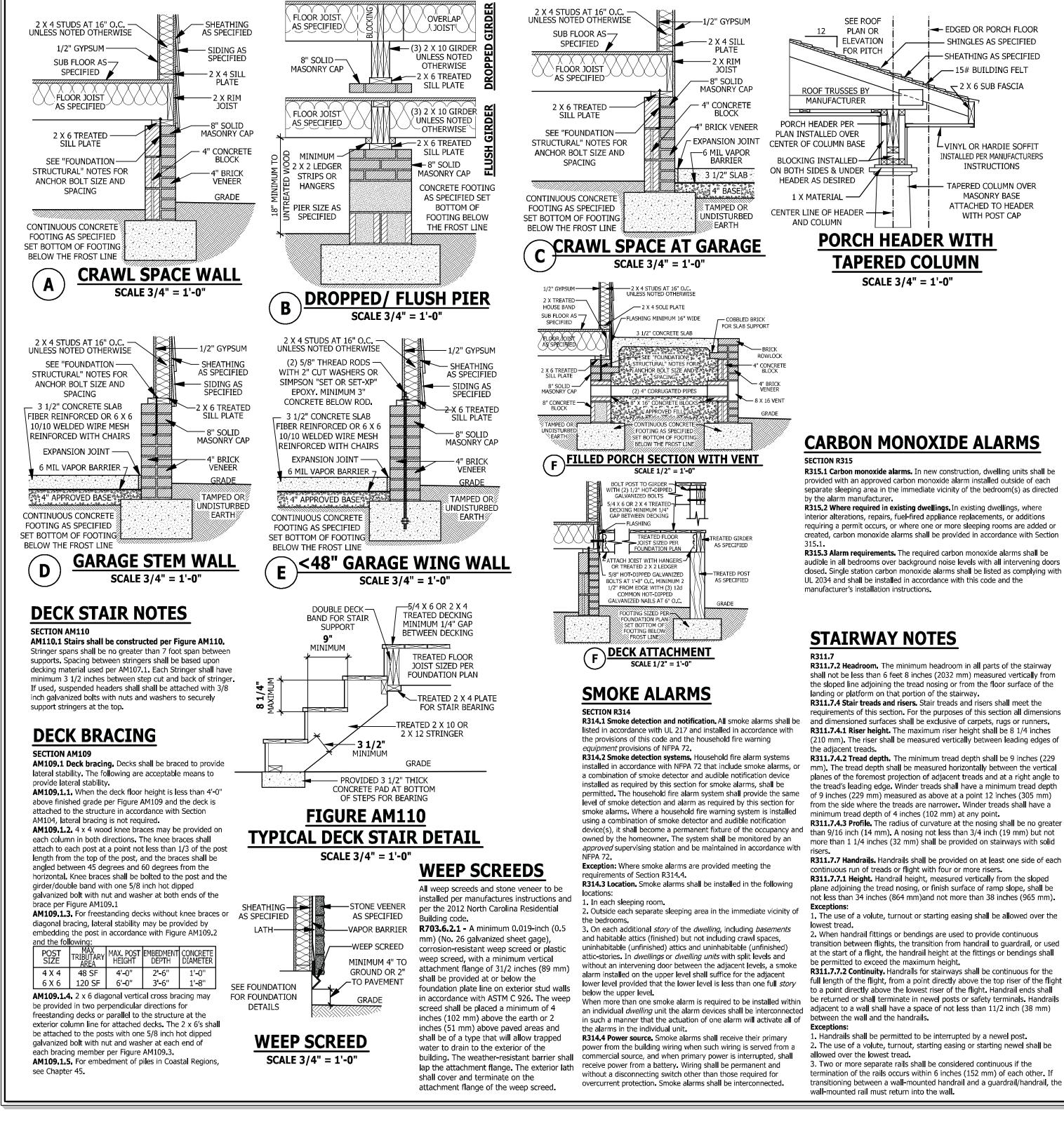
**TRUSS DESIGN.** Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

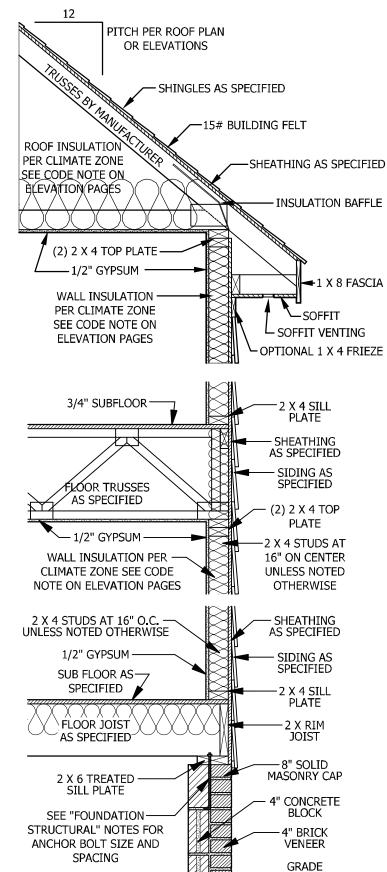
**ANCHORAGE.** All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or

ledgers unless noted otherwise. **Plate Heights & Floor Systems.** See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE FIRST FLOOR PLATE HEEL HEIGHT ABOVE SECOND FLOOR PLATE







separate sleeping area in the immediate vicinity of the bedroom(s) as directed

requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section

audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with

requirements of this section. For the purposes of this section all dimensions (210 mm). The riser shall be measured vertically between leading edges of

R311.7.4.2 Tread depth. The minimum tread depth shall be 9 inches (229 planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread dept

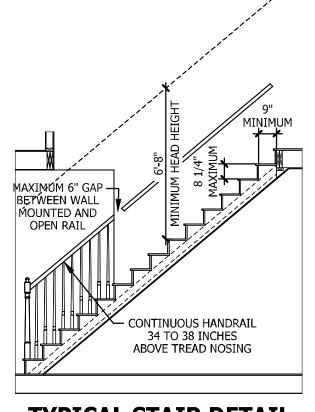
R311.7.7 Handrails. Handrails shall be provided on at least one side of each plane adjoining the tread nosing, or finish surface of ramp slope, shall be

1. The use of a volute, turnout or starting easing shall be allowed over the

at the start of a flight, the handrail height at the fittings or bendings shall

full length of the flight, from a point directly above the top riser of the flight be returned or shall terminate in newel posts or safety terminals. Handrails

transitioning between a wall-mounted handrail and a guardrail/handrail, the



**TYPICAL WALL DETAIL** 

SCALE 3/4" = 1'-0"

CONTINUOUS CONCRETE

FOOTING AS SPECIFIED

SET BOTTOM OF FOOTING

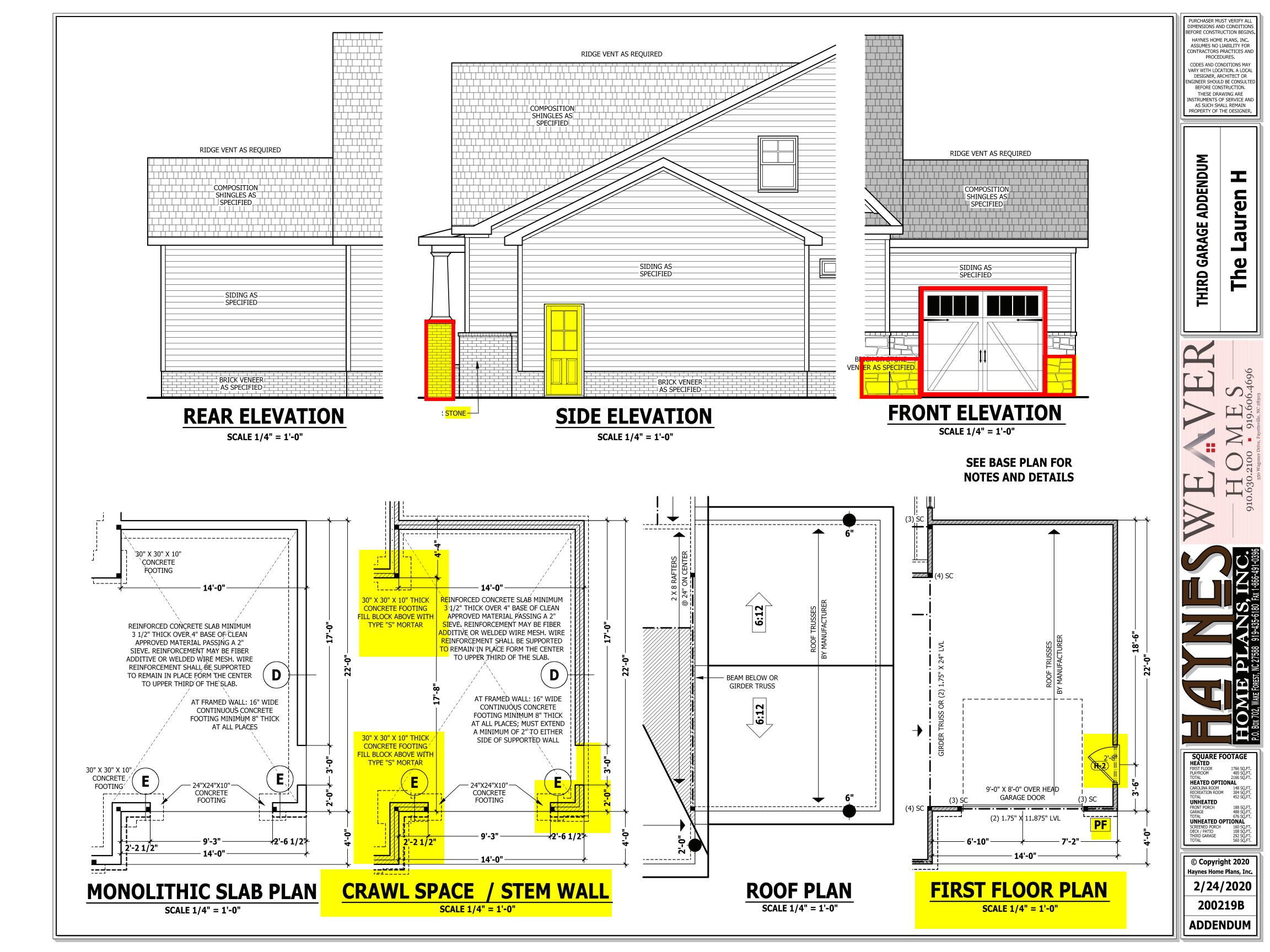
BELOW THE FROST LINE

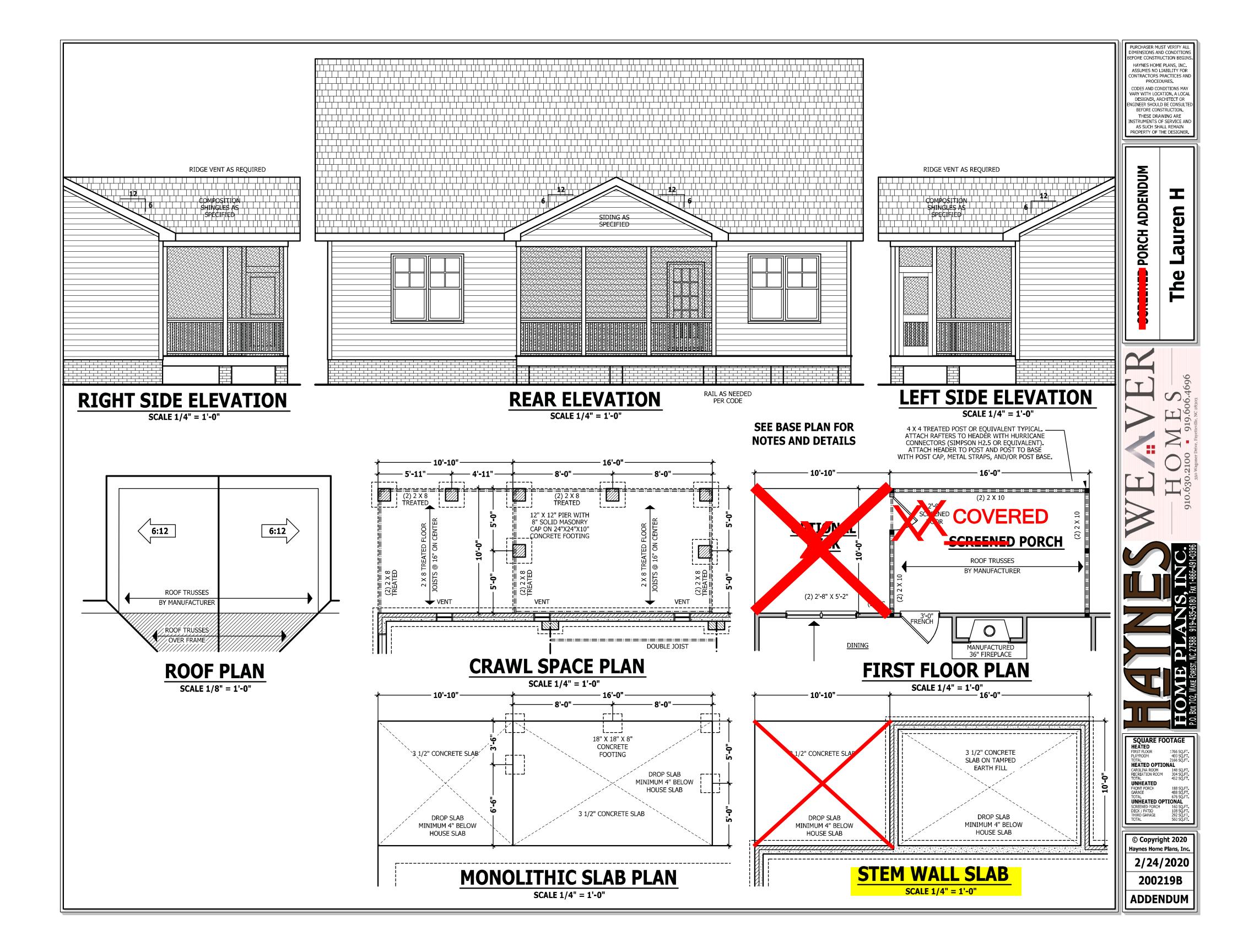
**TYPICAL STAIR DETAIL** SCALE 1/4" = 1'-0"



PURCHASER MUST VERIFY AL

IMENSIONS AND CONDITION

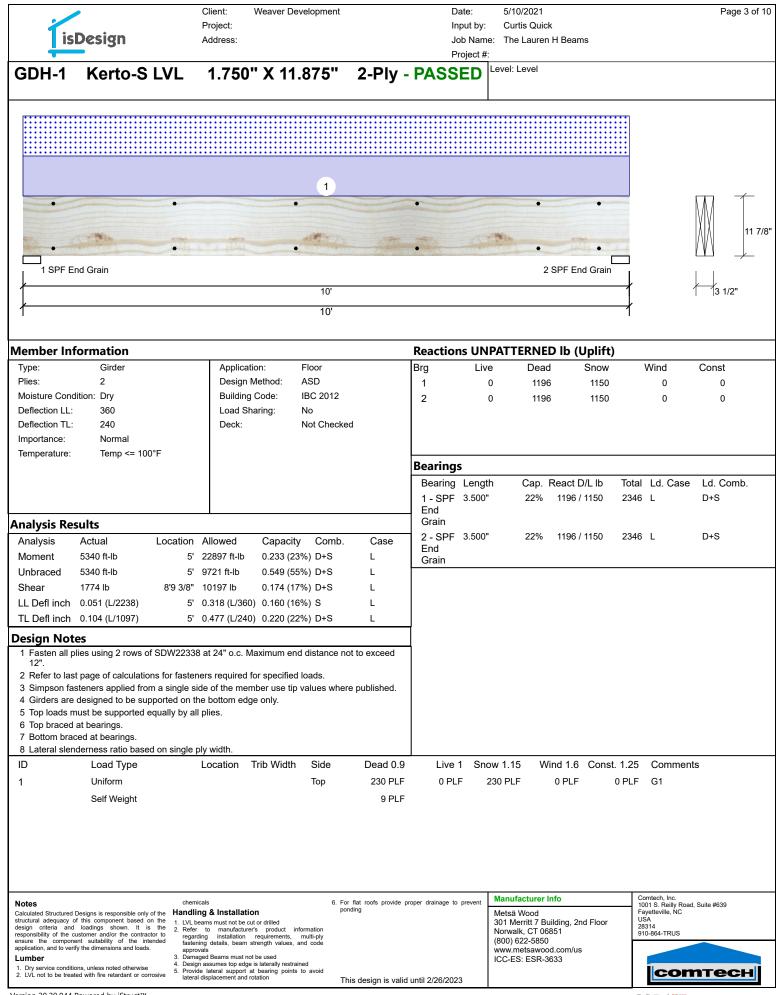




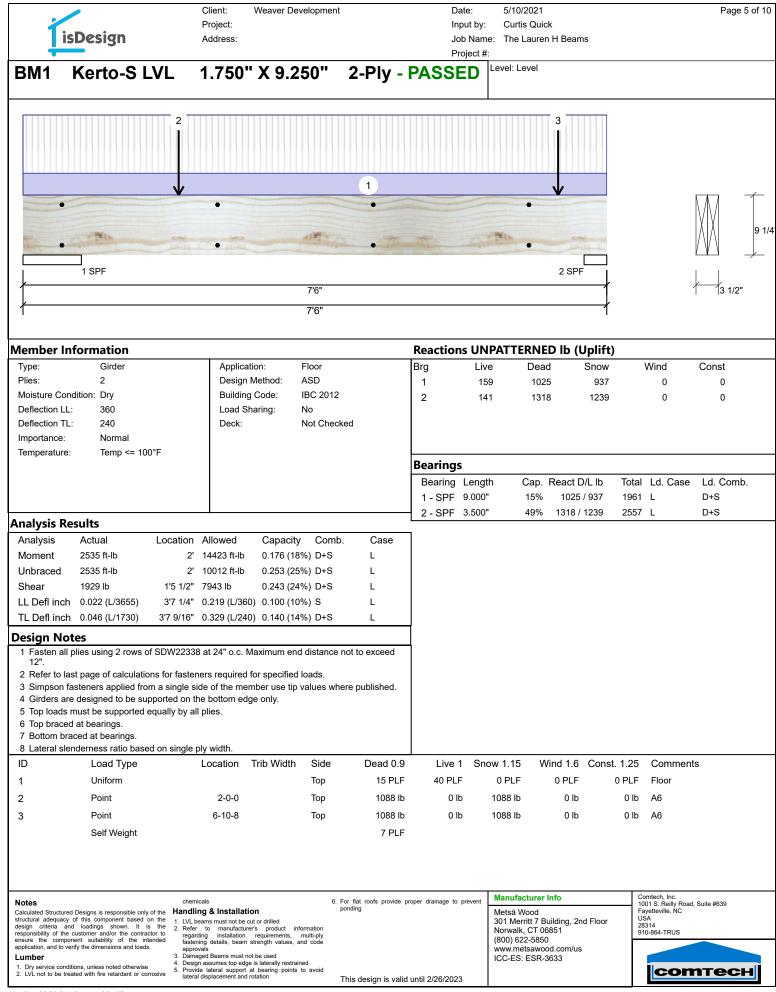
	Design	Address:		P	roject #:	he Lauren H Beams		
BDH	Kerto-S LVI	_ 1.750" X 14.00	00" 2-Ply	- PASSEI	D			
1 SPF End	••••••••••••••••••••••••••••••••••••••	•	1		199	2 SPF	End Grain	1'2"
			18'10"				r	3 1/2"
<u> </u>			18'10"				ł	
	formation					TERNED lb (Uplift		
ype: Plies: Aoisture Cond Deflection LL: Deflection TL: mportance:	360 240 Normal	Application: Design Method: Building Code: Load Sharing: Deck:	Floor ASD IBC 2012 No Not Checked	Brg 1 2	Live 0 0	Dead         Snow           2457         0           2457         0	Wind 0 0	Const 0 0
emperature:	Temp <= 100°F			Bearing	s			
				Bearing 1 - SPF End	Length	Cap. React D/L lb 23% 2457 / 0	Total Ld. Case 2457 Uniform	Ld. Comb. D
<b>halysis Re</b> Analysis		ocation Allowed Capacity	Comb. Cas	Grain 2 - SPF	3.500"	23% 2457 / 0	2457 Uniform	D
Aoment Jnbraced Shear	11011 ft-lb 11011 ft-lb	9'5" 24299 ft-lb 0.453 (45 9'5" 11013 ft-lb 1.000 (100%) 1'4 3/4" 9408 lb 0.222 (22	%) D Unifo D Unifo	orm				
	0.000 (L/999) 0.444 (L/497) 9	0 999.000 (L/0) 0.000 (0%) 5 1/16" 0.919 (L/240) 0.480 (48)	·	orm				
esign Not		(40 (40 (12 to)) 0.400 (40						
12". 2 Refer to las 3 Simpson fa 4 Girders are 5 Top loads r 6 Top must b 7 Bottom bra 3 Lateral sler	st page of calculations is isteners applied from a e designed to be suppo must be supported equi- e laterally braced at a ced at bearings. inderness ratio based of	maximum of 9'7 1/2" o.c. n single ply width.	loads. values where publishe	ed.				
D	Load Type Uniform	Location Trib Width	Side Dead Top 250				t. 1.25 Commen 0 PLF	ts
	Self Weight		•	PLF 0FL	. vr	011	V I EI	
otes	Designs is responsible only of th of this component based on th	chemicals • Handling & Installation 1. LVL beams must not be cut or drilled	6. For flat roofs pro ponding	ovide proper drainage to	Mets	ufacturer Info ä Wood Merritt 7 Building, 2nd Floor	Comtech, Inc. 1001 S. Reilly Roa Fayetteville, NC USA 28314	d, Suite #639

CSD DESIGN

	-		Client:	Weaver Developm	nent		Date:	5/10/2021	Page 2 of 10
			Project:				Input by:	Curtis Quick	
	isDesign		Address:					: The Lauren H Beams	
							Project #:	_evel: Level	
GDH	Kerto-S	LVL	1.750"	' X 14.000"	2-Ply	- PASSE	ED		
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	•	•	•	•	•	•	•	• •	·
•	•		•	• •	•	•		• •—	
1 SPI	F End Grain							2 SPF End	Grain //
					18'10"				3 1/2"
					18'10"				
					18 10				
Multi-P	ly Analysis								
Fasten a	ll plies using 3 r	ows of	<sup>F</sup> SDW22338 a	t 24" o.c Maxin	num end dist	ance not to	exceed	12"	
Capacity			) %						
Load Yield Limit	per Foot		) PLF 2.5 PLF						
Yield Limit	per Fastener		5.0 lb.						
Yield Mode			okup						
Edge Dista Min. End D		6"	1/2"						
Load Comb	pination								
Duration Fa	actor	1.0	00						
									1
Notes			chemicals	-41	<ol><li>For flat roofs pro ponding</li></ol>	vide proper drainage	to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
structural ade	uctured Designs is responsible quacy of this component bas	sed on the	1. LVL beams must not b	e cut or drilled	ponding			Metsä Wood 301 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA 28314
aesign criteri responsibility o ensure the o	a and loadings shown. of the customer and/or the co component suitability of the	n is the ontractor to e intended	<ol> <li>Refer to manufact regarding installation</li> </ol>	turer's product information on requirements, multi-ply				Norwalk, CT 06851 (800) 622-5850	28314 910-864-TRUS
application, an	d to verify the dimensions and	loads.	approvals 3. Damaged Beams mus	am strength values, and code at not be used				www.metsawood.com/us ICC-ES: ESR-3633	
1. Dry service	conditions, unless noted othe be treated with fire retardant	rwise	<ol> <li>Design assumes top e</li> <li>Provide lateral support</li> </ol>	edge is laterally restrained ort at bearing points to avoid				100°LU. LUIX*3033	соттесн
Z. LVL HUL TO	so acalog with me relargant i	. conosive	lateral displacement a	nd rotation		valid until 2/26/2	023		



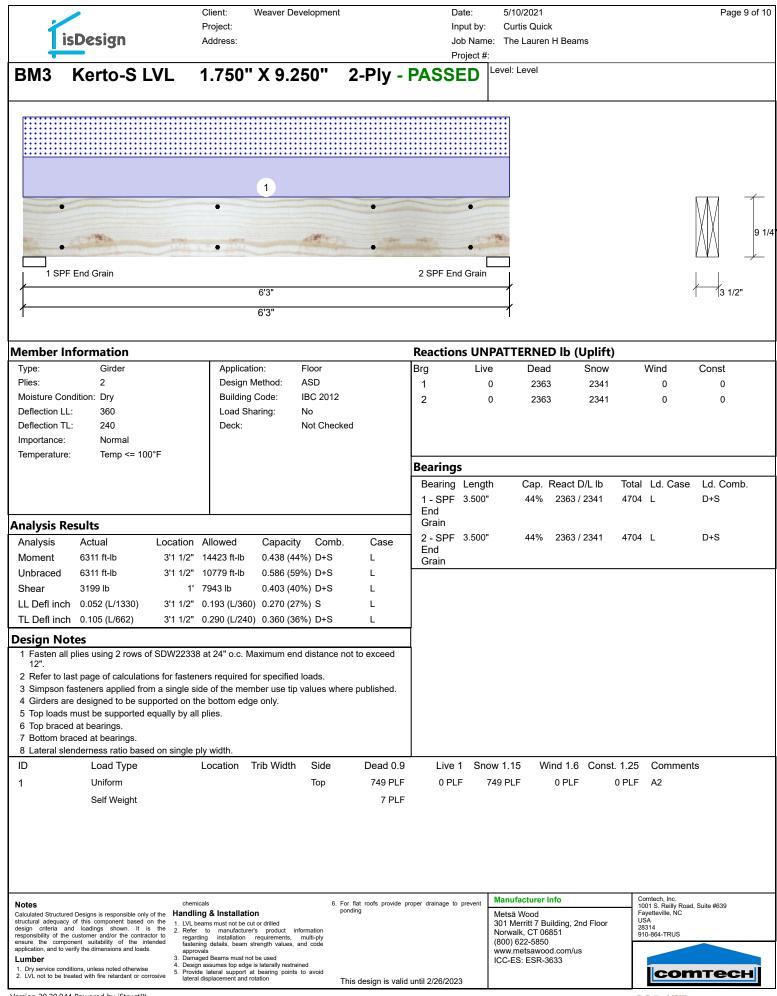
lis	Design	Client: Weaver Project: Address:	Development	Date: Input by:	5/10/2021 Curtis Quick e: The Lauren H Beams	Page 4 of 10
				Project #:		
GDH-1	Kerto-S LV	′L 1.750'' X 1	1.875" 2-Pl	y - PASSED	Level: Level	
•		•	•	•	• •	
	ind Grain	•	•	•	• • •	
			10'			3 1/2"
<del> </del>			10'			ł
Multi-Ply A	nalvsis					
Fasten all pli	ies using 2 rows of	SDW22338 at 24" o.c.	. Maximum end dis	ance not to exceed	l 12"	
Capacity Load		PLF				
Yield Limit per F Yield Limit per F		5.0 PLF 5.0 lb.				
Yield Mode Edge Distance	Loc 1 1/	okup /2"				
Min. End Distan	ce 6"					
Load Combination Duration Factor	on 1.0	0				
					Manufacturer Info	Comtech, Inc.
Notes Calculated Structured	Designs is responsible only of the	chemicals Handling & Installation	<ol> <li>For flat roofs pro ponding</li> </ol>	vide proper drainage to prevent	Metsä Wood	<ul> <li>1001 S. Reilly Road, Suite #639</li> <li>Fayetteville, NC</li> </ul>
design criteria and responsibility of the o	customer and/or the contractor to	1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product regarding installation requirements	information		301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the compon application, and to ver	ient suitability of the intended ify the dimensions and loads.	fastening details, beam strength value approvals 3. Damaged Beams must not be used	s, and code		(800) 622-5850 www.metsawood.com/us	
1. Dry service conditi 2. LVL not to be treat		<ol> <li>Design assumes top edge is laterally res</li> <li>Provide lateral support at bearing po</li> </ol>	ints to avoid		ICC-ES: ESR-3633	соттесн
		lateral displacement and rotation	This design is	valid until 2/26/2023		



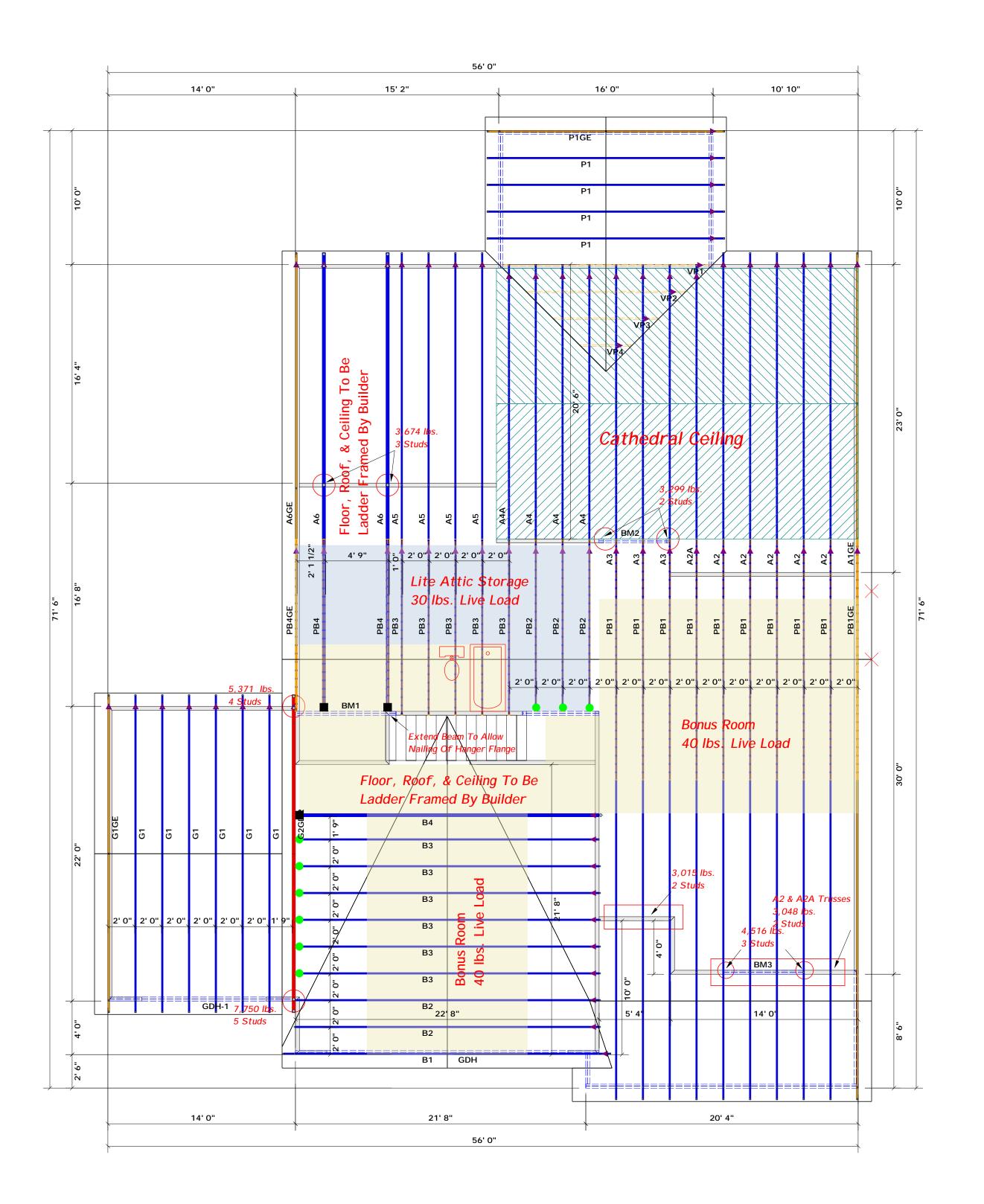
	Client: Weaver Developme	ent Date:	5/10/2021	Page 6 of 10
	Project:	Input by:		
isDesign	Address:	Job Nam	e: The Lauren H Beams	
		Project #		
BM1 Kerto-S LVL	1 750" X 9 250"	2-Ply - PASSED	Level: Level	
	11100 / 01200			
•	•	•	•	
				■
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				$\wedge$ $\rightarrow$
1 SPF			2 SPF	
11	7'6"			3 1/2"
/	7'6"		ł	
			'	
Multi-Ply Analysis				
Fasten all plies using 2 rows of SD	)W22338 at 24" o.c., Maximi	um end distance not to excee	d 12"	
Capacity 0.0 %			~	
Load 0.0 PL	F			
Yield Limit per Foot 255.0 F				
Yield Limit per Fastener 255.0 I				
Yield Mode Lookup Edge Distance 1 1/2"	0			
Min. End Distance 6"				
Load Combination				
Duration Factor 1.00				
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise		<ol> <li>For flat roofs provide proper drainage to prevent ponding</li> <li>This design is valid until 2/26/2023</li> </ol>	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 08851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	Comtech, Inc. 1001 S. Relily Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS

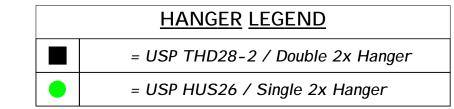
i	sDesign	Pr	lient: We roject: ddress:	eaver Develo	opment		In	put by:	5/10/2021 Curtis Quick The Lauren				Page 7 of 1(
	Kerto-S L	VL 1	.750" >	<b>x 9.25</b>	0" 2	2-Ply -		roject #: ED	vel: Level				
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Deflection LL	: 360		Load Shari	ng: No	1		-	Ũ	1000			ů.	Ũ
eflection TL			Deck:	No	t Checked								
mportance:	Normal												
emperature	: Temp <= 100°	F					Bearing	5					
							Bearing		Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	-		1659 / 1640	3299		D+S
							End						
nalysis Re							Grain 2 - SPF	3 500"	31%	1659 / 1640	3299		D+S
Analysis		Location Al			Comb.	Case	End	0.000	0170		0200	-	5.0
<i>l</i> loment Jnbraced	3675 ft-lb 3675 ft-lb			0.255 (25%) 0.311 (31%)		L	Grain						
Shear	2062 lb	4'4" 79		0.260 (26%)		L							
	0.023 (L/2497)		162 (L/360) (			L							
	0.047 (L/1241)		244 (L/240) (			L							
esign No	tes						7						
-	plies using 2 rows of	SDW22338 at	t 24" o.c. Maxi	imum end di	stance not	to exceed	-						
12". 2 Refer to la	st page of calculation	s for fastanar	required for	specified loa	de								
	asteners applied from		•	•		published.							
	e designed to be supp		•	only.									
	must be supported ec d at bearings.	lually by all pli	165.										
	aced at bearings.												
D Lateral sie	nderness ratio based Load Type			b Width	Side	Dead 0.9	) Live	1 Snow	1 15 Wi	nd 1.6 Const	1 25	Commen	te
	Uniform				Тор	615 PLI			PLF		0 PLF		13
	Self Weight				F	7 PLI							
	een rreigin												
otes		chemicals			6. For	flat roofs provide	proper drainage to	prevent Ma	anufacturer lı	nfo	Cor 100	ntech, Inc. I1 S. Reilly Road	Suito #630
	d Designs is responsible only of of this component based on			والترا	pond			Me	etsä Wood	ilding 2nd Eleor	Fay	etteville, NC	, Suite #035
uctural adequacy	id loadings shown. It is	the 2 Pofor to	s must not be cut or manufacturer's		ition				1 Merritt 7 Bu prwalk, CT 068	ilding, 2nd Floor 351	283	14 -864-TRUS	
ructural adequacy sign criteria ar	customer and/or the contracto	r to regarding	installation rea	uirements, mult	i-ply						910	-004-11(00	
ructural adequacy esign criteria ar sponsibility of the isure the compo oplication, and to ve	customer and/or the contracto onent suitability of the inten erify the dimensions and loads.	of to regarding ided fastening approvals	installation req details, beam stren	uirements, mult igth values, and c	i-ply code			(80 wv	00) 622-5850 vw.metsawood	d.com/us	910	1004-11(00	
ructural adequacy sign criteria ar sponsibility of the soure the compo- oplication, and to ve umber . Dry service cond	customer and/or the contracto ment suitability of the inter	r to regarding ided fastening approvals 3. Damaged 4. Design as	installation req details, beam stren	uirements, mult ogth values, and o used aterally restrained	ode			(80 wv	00) 622-5850	d.com/us	910	Con	

	Client: Weaver Developm		5/10/2021	Page 8 of 10
isDesign	Project: Address:	Input by	/: Curtis Quick me: The Lauren H Beams	
	Address.	Project		
BM2 Kerto-S LVL	1.750" X 9.250"		Level: Level	
		<b>,</b>		
			·	
				,
•	•	•		$\overline{M}$ 1
•	•	•¥		
1 SPF End Grain		2 SPF End Grain		
	5'4"			3 1/2"
	5'4"			0 1/2
I	54	Ι		
MA IN DI ANNI ST				
Multi-Ply Analysis				
Fasten all plies using 2 rows of S Capacity 0.0 9		um end distance not to excee	ed 12"	
Load 0.0 F	PLF			
Yield Limit per Foot 255. Yield Limit per Fastener 255.	0 PLF			
Yield Mode Look	kup			
Edge Distance 1 1/2 Min. End Distance 6"	2"			
Load Combination				
Duration Factor 1.00				
			Manufacturer Info	Comtech, Inc.
	chemicals Iandling & Installation	<ol><li>For flat roofs provide proper drainage to prevent ponding</li></ol>	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC
design criteria and loadings shown. It is the 2 responsibility of the customer and/or the contractor to	. LVL beams must not be cut or drilled . Refer to manufacturer's product information regarding installation requirements, multi-ply		301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the component suitability of the intended application, and to verify the dimensions and loads.	fastening details, beam strength values, and code approvals		(800) 622-5850 www.metsawood.com/us	
1. Dry service conditions, unless noted otherwise 4	<ul> <li>Damaged Beams must not be used</li> <li>Design assumes top edge is laterally restrained</li> <li>Provide lateral support at bearing points to avoid</li> </ul>		ICC-ES: ESR-3633	соттесн
<ol> <li>LVL not to be treated with fire retargant or corrosive</li> </ol>	lateral displacement and rotation	This design is valid until 2/26/2023		Соптеся
Version 20 20 0// Powered by iStruct <sup>TM</sup>				

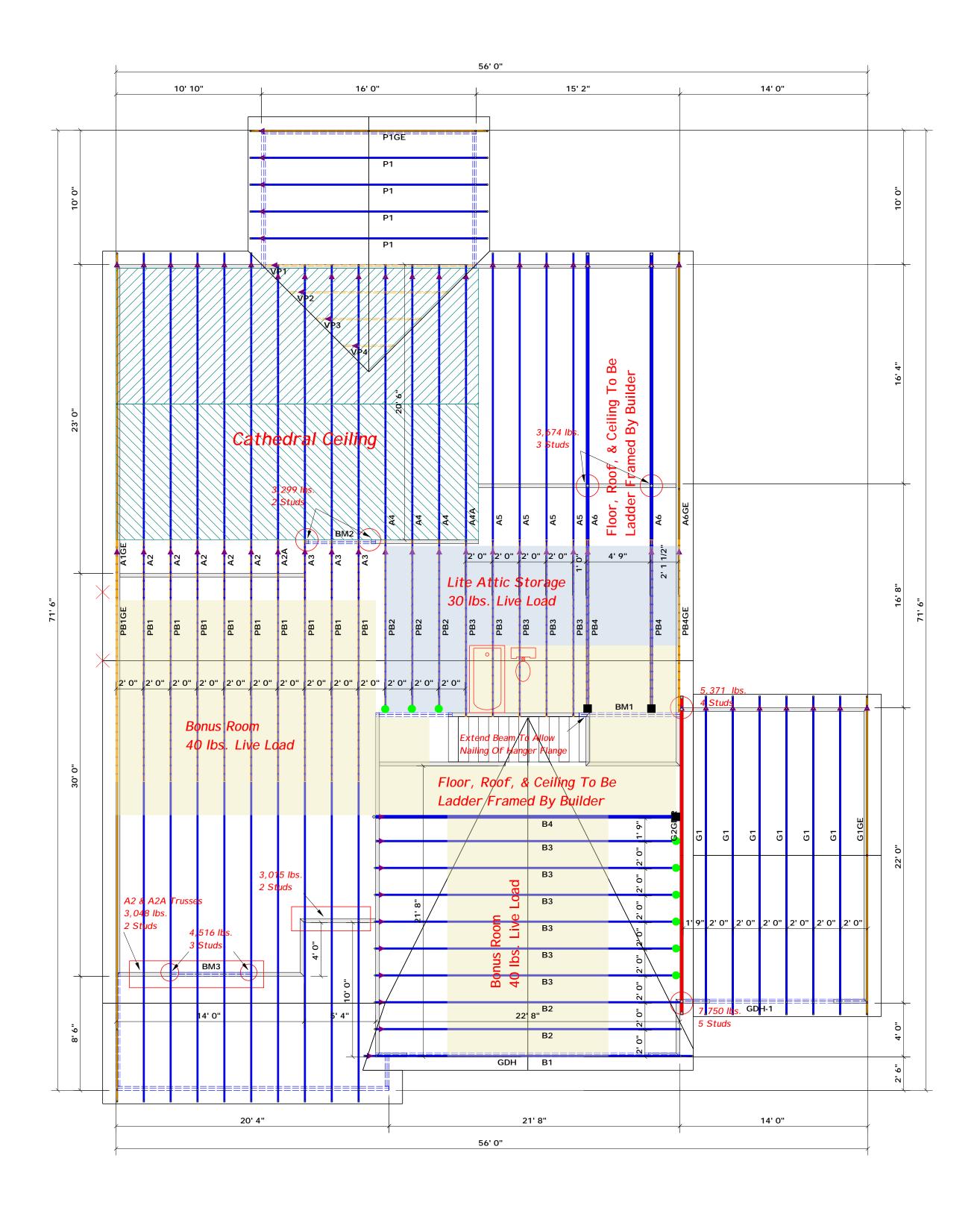


		Client:	Weaver Developmer	nt	Date:	5/10/2021	Page 10 of 10
		Project:			Input by:		5
isDesig	an	Address:				e: The Lauren H Beams	
					Project #		
DM2 Kart		4 750				Level: Level	
BM3 Kerte	o-S LVL	1.750	" X 9.250"	2-Piy	- PASSED	201011 20101	
•		•		٠	•	=	$\Lambda \Lambda \Lambda$
						1/2	V V
						<b></b> 11/2"	9 1/4
•		٠		•	• -		
1 SPF End Grain	n				2 SPF End Grain		
1			6'3"				3 1/2"
<u>/</u>						/	
			6'3"			I	
Multi-Ply Analysis							
			o	1 11		1.40%	
Fasten all plies usin		N22338 at	24" o.c Maximu	m end dista	nce not to excee	d 12"	
Capacity	0.0 %						
Load	0.0 PLF						
Yield Limit per Foot Yield Limit per Fastener	255.0 Pl 255.0 lb						
Yield Mode	Lookup						
Edge Distance	1 1/2"						
Min. End Distance	6"						
Load Combination							
Duration Factor	1.00						
						Manufactur-1-6-	Comtach Inc
Notes		micals		. For flat roofs provide ponding	e proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is res structural adequacy of this compo	onent based on the 1. IVI	lling & Installat beams must not be		,8		Metsä Wood 301 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA
design criteria and loadings s responsibility of the customer and/	shown. It is the 2. Ref /or the contractor to reg	er to manufactu	rer's product information requirements, multi-ply			Norwalk, CT 06851	28314 910-864-TRUS
ensure the component suitability application, and to verify the dimensi	y of the intended fast	ening details, beam provals	strength values, and code			(800) 622-5850 www.metsawood.com/us	
Lumber	3. Dar	maged Beams must r	not be used ge is laterally restrained			ICC-ES: ESR-3633	
<ol> <li>Dry service conditions, unless no</li> <li>LVL not to be treated with fire re</li> </ol>	oted otherwise 5. Pro		at bearing points to avoid	<b>T</b> 1.1.1.1.1.1	11 June 11 0/00/00000		соттесн
	late	anopiarioniterit difu		i nis design is va	lid until 2/26/2023		





							Beam Legend				
					PlotID	Length	Product	Plies	Net Qty	Fab Type	
		All Truss Reactions are L			BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
		than 3,000 lbs. Unless Noted 0	Otherwise.		BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
▲ = Denotes Left End	▲ = Denotes Left End of Truss		enotes Reaction Greater than 3,000 lbs.		BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
(Reference Engineered T	•	() Denotes Reaction Greater t	nan 3,000 lbs.	Truss Placement Plan	GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF	
Do Not Erect Trusses	s Backwards			SCALE: 3/16" = 1'	GDH	23' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF	
LOAD CHART FOR JACK STUDS MANFE ON TABLES (\$5025(1) 1-0) STANLES OF STATES SECTION (AC USE OF	BUILDER	Weaver Development	CITY/CO.	Angier / Harnett		These t the build	S A TRUSS PLACEMENT DIAGRAM ONLY. russes are designed as individual building components to be incorpor ling design at the specification of the building designer. See individual or each truss design identified on the placement drawing. The building	design			
	JOB NAME	Lot 8 Mitchell Manor	ADDRESS	is responsible for temporary and permanent bracing of the roof and floor system at the overall structure. The design of the truss support structure including headers, I walls, and columns is the responsibility of the building designer. For general guida regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery p		em and for ers, beams, juidance	CO	тесн			
CND RE CND RE	PLAN	Lauren H / Elev. A / 3 Car / BR	MODEL	Roof		or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables			<b>ROOF &amp; FLOOR</b>		
1700         1         2550         1         3400         !           3400         2         5100         2         6600         2           5100         3         7650         3         10200         3	SEAL DATE	2/24/20	DATE REV.	01/05/22		foundat than 30 be retai	d from the prescriptive Code requirements) to determine the m ion size and number of wood studs required to support reaction 00# but not greater than 15000#. A registered design profession ned to design the support system for any reaction that exceeds	ns greater al shall those		SES & BEAMS ad Industrial Park	
6800 4 10200 4 13600 4 8500 5 12750 5 17000 5 10200 6 15300 6	QUOTE #	Quote #	DRAWN BY	Curtis Quick		specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Curtis Quick		be 00#.	Fayette	eville, N.C. 28309 :: (910) 864-8787	
11900 7 13600 8 15300 9	JOB #	J1221-7127	SALES REP.	Lenny Norris		Sigr	Curtis Quick			(910) 864-4444	





							Beam Legend			
					PlotID	Length	Product	Plies	Net Qty	Fab Type
		All Truss Reactions are			BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
		than 3,000 lbs. Unless Noted	d Otherwise.		BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
= Denotes Left End	▲ = Denotes Left End of Truss		r than 2 000 lba	ban 3,000 lbs		6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
(Reference Engineered T	0.			Truss Placement Plan	GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
Do Not Erect Trusses	Backwards			SCALE: 3/16" = 1'	GDH	23' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF
LOAD CHART FOR JACK STUDS MANFOON 1481-F5 (\$5025(1) 4.00) MANFOON 2481-F5 (\$5025(1) 4.00)	BUILDER	Weaver Development	CITY/CO.	Angier / Harnett		These the bui	IS A TRUSS PLACEMENT DIAGRAM ONLY. trusses are designed as individual building components to be incorpor Iding design at the specification of the building designer. See individua for each truss design identified on the placement drawing. The building	ated into design		
HEROCOLOGICA ACTION STATE ACTION STATE AC	JOB NAME	Lot 8 Mitchell Manor	ADDRESS	Mitchell Manor Drive		is resp	nonsible for temporary and permanent bracing of the roof and floor syste erall structure. The design of the truss support structure including head and columns is the responsibility of the building designer. For general ( ing bracing, consult BCSI-B1 and BCSI-B3 provided with the truss deliven ne @ sbcindustry.com	em and for	CO	тесн
IND RIV OF CONTRACT OF CONTRA	PLAN	Lauren H / Elev. A / 3 Car / BR	MODEL	Roof		Bearin	g reactions less than or equal to 3000# are deemed to comply w iptive Code requirements. The contractor shall refer to the attacl	ith the ned Tables		F & FLOOR
1700         1         2550         1         3400         1           3400         2         5100         2         6600         2           5100         3         7650         3         10200         3	SEAL DATE	2/24/20	DATE REV.	01/05/22		founda than 30 be reta	ed from the prescriptive Code requirements ) to determine the m ation size and number of wood studs required to support reactio 000# but not greater than 15000#. A registered design professior ained to design the support system for any reaction that exceed	ns greater al shall those	wanter a	SES & BEAMS
6800 4 10200 4 13600 4 8500 5 12750 5 17000 5 10200 6 15300 6	QUOTE #	Quote #	DRAWN BY	Curtis Quick	uick specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Signature Signature		be 00#.	Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787		
11900 7 13600 8 15300 9	JOB #	J1221-7127	SALES REP.	Lenny Norris				Fax: (910) 864-444		