

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 238_2338_C KB Home 238.2338.C

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I44252907 thru I44252936

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



January 7,2021

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



818 Soundside Road Edenton, NC 27932

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TRENCO A MiTek Affiliate 818 Soundside Road

Edenton, NC 27932



 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This
connection is for uplift only and does not consider lateral forces.



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A MiTek Aft 818 Soundside Road Edenton, NC 27932



			<u>5-2-0</u> 5-2-0						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.31 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.04 -0.07 -0.00	(loc) 2-4 2-4 3	l/defl >999 >821 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 17 lb	GRIP 197/144 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 TOP CHORD BOT CHORD

BRACING-

Structural wood sheathing directly applied or 5-2-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=69(LC 12) Max Uplift 3=-60(LC 16), 2=-48(LC 12)

Max Grav 3=142(LC 2), 2=266(LC 2), 4=99(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 5-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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818 Soundside Road Edenton, NC 27932



		20-8-0)						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.04 WB 0.12 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 0.00 0.00	(loc) 14 14 14	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 130 lb	GRIP 197/144 FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 OTHERS
 2x4 SP No.3

 SLIDER
 Left 2x4 SP No.3 - t 1-6-9, Right 2x4 SP No.3 - t 1-6-9

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 20-8-0.

(lb) - Max Horz 2=157(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 22, 23, 24, 25, 19, 18, 17, 16

Max Grav All reactions 250 lb or less at joint(s) 2, 20, 22, 23, 24, 25, 19, 18, 17, 16, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-4-0, Exterior(2) 2-4-0 to 10-4-0, Corner(3) 10-4-0 to 13-4-0, Exterior(2) 13-4-0 to 21-6-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

All plates are 1.5x4 MT20 unless otherwise indicated.

7) Gable requires continuous bottom chord bearing.

8) Gable studs spaced at 2-0-0 oc.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

11) n/a

12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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Plate Offsets (X,Y) [2:0-1-4,	0-1-12], [8:0-1-12,0-2-6], [9:0-5-4,0-1-8]	, [10:0-3-8,0-5-4], [11:0-6·	0,0-6-4], [13:0-3-8,	,0-6-0]				
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.86 BC 0.51 WB 0.73 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.12 10-11 -0.23 10-11 0.05 8	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 338 lb	GRIP 197/144 FT = 20%
BCDL 10.0								
LUMBER- TOP CHORD 2x4 SP No.2 or BOT CHORD 2x8 SP DSS WEBS 2x4 SP No.3 *E 5-11: 2x6 SP No. 3 *E WEDGE Left: 2x4 SP No.3 , Right: 2x6 SP	2x4 SPF No.2 xcept* 5.2 P No.2	BI TO BO	RACING- DP CHORD SI DT CHORD R	tructural wood igid ceiling dire	sheathing c ectly appliec	lirectly applied I or 10-0-0 oc	d or 2-8-12 oc purlin bracing.	S.
REACTIONS. (size) 8=0- Max Horz 2=19 Max Uplift 8=-6 Max Grav 8=9	5-8, 2=(0-3-8 + TBE4 Simpson Strong- 55(LC 33) 07(LC 11), 2=-595(LC 10) 107(LC 2), 2=6363(LC 2)	Гіе) (req. 0-5-0)						
FORCES. (lb) - Max. Comp./N TOP CHORD 2-3=-10072/9 7-8=-11495/79 BOT CHORD 2-14=-806/793	lax. Ten All forces 250 (lb) or less exc 54, 3-4=-9329/815, 4-5=-7505/621, 5-6= 30 30, 13-14=-806/7980, 11-13=-645/7725,	ept when shown. -7505/620, 6-7=-9568/70 10-11=-498/7925, 9-10=	4, -583/9208,					
8-9=-583/920 WEBS 5-11=-623/80 4-11=-2713/4	3 43, 6-11=-3071/278, 6-10=-216/3513, 7- 48, 4-13=-417/3085, 3-13=-346/208, 3-1	10=-1675/172, 7-9=-114/ 4=-187/900	2366,			7	TH CARO	114
NOTES- 1) 2-ply truss to be connected to Top chords connected as foll Bottom chords connected as Webs connected as follows:	bgether with 10d (0.131"x3") nails as foll ows: 2x4 - 1 row at 0-9-0 oc. follows: 2x8 - 2 rows staggered at 0-4-0 2x6 - 2 rows staggered at 0-9-0 oc. 2x4	ows: l oc. - 1 row at 0-9-0 oc.				Be	SEAL	rier
 All loads are considered equiply connections have been p Unbalanced roof live loads hit Wind: ASCE 7-10: Vult=1200 	ally applied to all plies, except if noted as rovided to distribute only loads noted as ave been considered for this design.	(F) or (B), unless otherwi	e in the LOAD CAS se indicated. (0 B: Enclosed: MV	E(S) section. F	Ply to		044925	
 gable end zone; Lumber DOI 5) TCLL: ASCE 7-10; Pr=20.0 p roof snow: Lumber DOL=1.1 6) This truss has been designed 	 a.1.60 plate grip DOL=1.60 sf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; I for greater of min roof live load of 12.0 	tte DOL=1.15); Pg=15.0 p Partially Exp.; Ct=1.10 psf or 1.00 times flat roof	esf (ground snow); F load of 11.6 psf on	Pf=11.6 psf (fla n overhangs	t		MGINEER	E
non-concurrent with other live	e loads.						- munit	004
 I his truss has been designed Continued on page 2 	t for a 10.0 pst bottom chord live load no	onconcurrent with any oth	er live loads.				January 7,2	021
WARNING - Verify design para Design valid for use only with Mil a truss system. Before use, the b building design. Bracing indicate is always required for stability an fabrication, storage, delivery, ere Safety Information available fro	meters and READ NOTES ON THIS AND INCLUDE fet@ connectors. This design is based only upon p uilding designer must verify the applicability of desi d is to prevent buckling of individual truss web and t to prevent collapse with possible personal injury tion and bracing of trusses and truss systems, see m Truss Plate Institute, 2670 Crain Highway, Suite	D MITEK REFERENCE PAGE M arameters shown, and is for an i gn parameters and properly inco for chord members only. Additio and property damage. For gene e ANS/TP11 Quality C 203 Waldorf, MD 20601	I-7473 rev. 5/19/2020 BE ndividual building compo proprate this design into nal temporary and perm ral guidance regarding th riteria, DSB-89 and BC:	FFORE USE. onent, not the overall aanent bracing he SI Building Comp	onent	8	ENGINEERING BY A MITER A MITER 18 Soundside Road identon, NC 27932	Affiliate

Job	Truss	Truss Type	Qty	Ply	KB Home 238.2338.C	
						I44252915
238_2338_C	CG	COMMON GIRDER	1	2		
				_	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	430 s Nov	30 2020 MiTek Industries, Inc. Wed Jan 6 16:13:24 2021	Page 2
		ID:W	/17blGclad	2WNY Th	Txie4vHe28-Gkr5uHINxZTaY1tOHnaY9ezvidtpFEP7ruTx	Gwzv0zv

NOTES-

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) TDE4 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

10) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8. This connection is for uplift only and does not consider lateral forces.

11) Use Simpson Strong-Tie HHUS28-2 (22-10d Girder, 4-10d Truss) or equivalent at 5-6-0 from the left end to connect truss(es) to back face of bottom chord.

12) Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent spaced at 2-3-0 oc max. starting at 7-5-4 from the left end to 20-7-4 to connect truss(es) to back face of bottom chord.

13) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-5=-43, 5-8=-43, 2-8=-20

Concentrated Loads (lb)

Vert: 8=-1121(B) 15=-2034(B) 16=-1303(B) 17=-1382(B) 18=-1424(B) 19=-1452(B) 20=-1111(B) 21=-1111(B) 22=-1111(B)

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	5-4-4	10-2-0	18-2-0	20	6-2-0	30-11-12	36-4-0	_
Plate Offsets (5-4-4 (X,Y) [2:Edge.(4-9-12 0-3-2], [5:0-4-0.0-4-8], [8:Edae,	8-0-0 0-3-21		-0-0	4-9-12	5-4-4	·
	of)	<u> </u>						
TCLL (roof) Snow (Pf/Pg) TCDL BCLL	20.0 16.5/15.0 10.0 0.0 *	SPACING- 2-0- Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr YE Code IBC2015/JPI2014	0 CSI. 5 TC 0.50 5 BC 0.89 S WB 0.42 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT	in (loc) -0.17 13) -0.39 13-15) 0.14 8	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 203 lb	GRIP 197/144 FT = 20%
BCDL	10.0		Width -0				Weight. 203 lb	11 = 2078
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE Left: 2x4 SP N	2x4 SP No.2 or 4-5,5-6: 2x6 SP 2x4 SP No.2 or 2x4 SP No.3 Io.3 , Right: 2x4 S	2x4 SPF No.2 *Except* No.2 2x4 SPF No.2 P No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood 2-0-0 oc purlins Rigid ceiling dir 1 Row at midpt	l sheathing directly app (4-7-13 max.): 4-6. ectly applied or 10-0-0 5-15, 5-11	olied or 3-1-9 oc purlins oc bracing. I	s, except
REACTIONS.	(size) 2=0- Max Horz 2=-7 Max Uplift 2=-3 Max Grav 2=15	3-8, 8=0-3-8 7(LC 17) 9(LC 16), 8=-39(LC 17) 503(LC 2), 8=1503(LC 2)						
FORCES. (II TOP CHORD BOT CHORD WEBS	b) - Max. Comp./M 2-3=-2648/210 7-8=-2648/210 2-16=-133/225 8-10=-131/22 3-15=-383/121 7-11=-383/122	lax. Ten All forces 250 (lb) or), 3-4=-2326/220, 4-5=-2025/22) 54, 15-16=-133/2254, 13-15=-11 54 1, 4-15=0/659, 5-15=-877/128, 5 2	less except when shown. 6, 5-6=-2025/226, 6-7=-232 07/2687, 11-13=-107/2687, 5-13=0/345, 5-11=-877/127,	6/220, 10-11=-131/2254, 6-11=0/659,				
NOTES- 1) Unbalanced 2) Wind: ASC gable end z 26-2-0, Ext exposed;C- 3) TCLL: ASC roof snow: I governs. R 4) Unbalanced 5) This truss f non-concur 6) Provide add 7) This truss f 8) * This truss s will fit betwe 9) One H2.5A connection 10) Graphical	d roof live loads ha E 7-10; Vult=120r zone and C-C Exte erior(2) 26-2-0 to 3 C for members ar E 7-10; Pr=20.0 p Lumber DOL=1.15 Rain surcharge app d snow loads have has been designed that been designed has been designed has been designed has been designed is has been designed is has been designed is has been designed is for uplift norg- is for uplift norg-	ave been considered for this de nph Vasd=95mph; TCDL=6.0ps rrior(2) -0-10-8 to 2-1-8, Interior 30-4-15, Interior(1) 30-4-15 to 3 do forces & MWFRS for reaction sf (roof live load: Lumber DOL= 5 Plate DOL=1.15); Category II; olied to all exposed surfaces wite been considered for this desig f for greater of min roof live load beads. p prevent water ponding. I for a 10.0 psf bottom chord live d for a live load of 20.0psf on t ord and any other members. Tie connectors recommended the nd does not consider lateral forc ion does not depict the size of 1	sign. f; BCDL=6.0psf; h=30ft; Cat (1) 2-1-8 to 10-2-0, Exterior 7-2-8 zone; cantilever left al is shown; Lumber DOL=1.6 1.15 Plate DOL=1.15); Pg= Exp B; Partially Exp.; Ct=1. h slopes less than 0.500/12 n. l of 12.0 psf or 1.00 times flat a load nonconcurrent with a he bottom chord in all areas o connect truss to bearing w ses.	. II; Exp B; Enclosed; (2) 10-2-0 to 14-4-15, dright exposed; end 0 plate grip DOL=1.6(15.0 psf (ground snov 10, Lu=50-0-0; Min. fl in accordance with IE at roof load of 11.6 ps my other live loads. where a rectangle 3- alls due to UPLIFT at along the top and/or b	MWFRS (envelop Interior(1) 14-4-1 d vertical left and 0 w); Pf=16.5 psf (fl at roof snow load 3C 1608.3.4. f on overhangs 6-0 tall by 2-0-0 w jt(s) 2 and 8. This ottom chord.	pe) 5 to right at vide	SEAL 044925	the community

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January 7,2021



	6-8-4		12-10-0	1	18-2-0	23-6-0			29-7-12	1	36-4-0	
	6-8-4	. '	6-1-12		5-4-0	5-4-0			6-1-12		6-8-4	
Plate Offsets (X,Y) [2:Edge,	0-3-2], [10:Edg	ge,0-3-2]									
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	sf) 20.0 16.5/15.0 10.0	SPACI Plate G Lumber Rep St	NG- 2-0- Grip DOL 1.1 r DOL 1.1 tress Incr YE	-0 C 15 Ti 15 B	SI. C 0.71 C 0.78 /B 0.56	DEFL. Vert(LL) Vert(CT) Horz(CT	in -0.14 -0.30 0 13	(loc) 15 15-16 10	l/defl >999 >999	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCLL	0.0 *	Code I	RC2015/TPI2014	4 M	atrix-S		,				Weight: 199 lb	FT = 20%
BCDL	10.0										Ŭ	
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE Left: 2x4 SP N	2x4 SP No.2 or 2x4 SP No.2 or 2x4 SP No.3 lo.3 , Right: 2x4 S	2x4 SPF No.2 2x4 SPF No.2 P No.3				BRACING- TOP CHORD BOT CHORD WEBS	Structura 2-0-0 oc Rigid ce 1 Row a	al wood purlins iling dire it midpt	sheathin (4-0-6 m ectly appl	g directly app ax.): 5-7. ied or 10-0-0 6-16, 6-14	olied or 2-7-0 oc purlins) oc bracing. 4	, except
REACTIONS.	(size) 2=0- Max Horz 2=-9 Max Uplift 2=-6 Max Grav 2=15	·3-8, 10=0-3-8 97(LC 21) 61(LC 16), 10= 503(LC 2), 10=	-61(LC 17) =1503(LC 2)									
FORCES. (IIL TOP CHORD BOT CHORD WEBS	b) - Max. Comp./N 2-3=-2634/19 9-10=-2634/19 2-18=-112/22 10-12=-109/22 3-18=0/277, 3 7-14=0/611, 9	4ax. Ten All f 6, 3-5=-2125/2 96 40, 16-18=-112 240 -16=-607/145, -14=-607/145,	forces 250 (lb) or :18, 5-6=-1814/23 2/2240, 15-16=-4 ; 5-16=0/611, 6-1 ; 9-12=0/277	r less except wher 30, 6-7=-1814/230 5/2023, 14-15=-4 6=-455/83, 6-15=1	n shown.), 7-9=-212: 5/2023, 12- 0/266, 6-14	5/218, 14=-109/2240, =-455/82,						
NOTES- 1) Unbalancec 2) Wind: ASCC gable end z 23-6-0, Exte exposed;C- 3) TCLL: ASC roof snow: I governs. R 4) Unbalancec 5) This truss h non-concurr 6) Provide add 7) This truss h 8) * This truss will fit betwe 9) One H2.5A connection 10) Graphical	d roof live loads ha E 7-10; Vult=120n cone and C-C Exte erior(2) 23-6-0 to 3 C for members ar E 7-10; Pr=20.0 p Lumber DOL=1.11 cain surcharge app d snow loads have has been designed has been designed has been designed has been designed has been designed has been designed in the bottom ch Simpson Strong- is for uplift only ar purlin represental	ave been cons nph Vasd=95m erior(2) -0-10-8 27-8-15, Interio nd forces & MV bisf (roof live los 5 Plate DOL=1 blied to all expre- e been conside d for greater of e loads. b prevent wate d for a 10.0 pg d for a live los ord and any of Tie connectors nd does not co tion does not d	idered for this de ph; TCDL=6.0ps 3 to 2-1-8, Interior or(1) 27-8-15 to 3 WFRS for reaction ad: Lumber DOL= .15); Category II; osed surfaces will ared for this desig imin roof live load r ponding. f bottom chord liv ad of 20.0psf on t ther members, will recommended t unsider lateral for depict the size or	esign. sf; BCDL=6.0psf; 1 r(1) 2-1-8 to 12-10 37-2-8 zone; cantil ns shown; Lumbe =1.15 Plate DOL= ; Exp B; Partially E th slopes less that gn. d of 12.0 psf or 1.1 re load nonconcur the bottom chord if ith BCDL = 10.0ps to connect truss to ces. the orientation of	n=30ft; Cat 0-0, Exterio ever left ar r DOL=1.6(1.15); Pg=' Exp.; Ct=1 n 0.500/12 00 times fla rent with ar n all areas sf. bearing was the purlin a	. II; Exp B; Enclosed; r(2) 12-10-0 to 17-0-1 dright exposed; end 0 plate grip DOL=1.6(15.0 psf (ground snov 10, Lu=50-0-0; Min. fl in accordance with IE at roof load of 11.6 ps ⁻ hy other live loads. where a rectangle 3-1 alls due to UPLIFT at long the top and/or b	MWFRS (5, Interior) vertical (a)) Pf=16.5 at roof sno C 1608.3 ⁵ on overh 5-0 tall by jt(s) 2 and	(envelop (1) 17-(eft and i 5 psf (fla ow load .4. aangs 2-0-0 w d 10. Th rd.	oe))-15 to right at vide	and annument State	SEAL 044925 MGINEER January 7,2	Notes

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	1	8-0-0	7-6-0		5-	-4-0	7-6-0		1	8-0-0	·
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES 1/2014	CSI. TC BC WB Matrix-	0.54 0.55 0.31 ·S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc -0.10 14 -0.21 14-16 0.09 9	/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 249 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SP No.2 *E 5-6: 2x4 SP No. 2x6 SP No.2 2x4 SP No.3	xcept* 2 or 2x4 SPF No.2			E T B V	BRACING- OP CHORD SOT CHORD VEBS	Structural woo 2-0-0 oc purlir Rigid ceiling c 1 Row at midp	od sheathin is (4-0-9 m irectly appl ot	g directly app ax.): 5-6. ied or 10-0-0 3-14, 5-13	lied or 4-2-2 oc purlins, oc bracing. ;, 8-13	, except
REACTIONS.	(size) 2=0- Max Horz 2=-1 Max Uplift 2=-7 Max Grav 2=14	3-8, 9=0-3-8 15(LC 21) /8(LC 16), 9=-78(LC 17) 490(LC 2), 9=1490(LC 2)									

20-10-0

28-4-0

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2750/186, 3-5=-2023/212, 5-6=-1672/229, 6-8=-2024/212, 8-9=-2749/186

BOT CHORD 2-16=-126/2398, 14-16=-126/2398, 13-14=-0/1671, 11-13=-93/2397, 9-11=-93/2397

WEBS 3-16=0/336, 3-14=-819/180, 5-14=-11/531, 6-13=0/523, 8-13=-816/181, 8-11=0/334

NOTES-

1) Unbalanced roof live loads have been considered for this design.

8-0-0

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-7-14 to 2-4-2, Interior(1) 2-4-2 to 15-6-0, Exterior(2) 15-6-0 to 19-8-15, Interior(1) 19-8-15 to 20-10-0, Exterior(2) 20-10-0 to 25-0-15, Interior(1) 25-0-15 to 36-11-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

15-6-0

TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



36-4-0

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	L	8-9-8	11-6-10 I	17-2-) .	19-2-0	2	4-9-6	1	27-6-7	1	36-1-0	
		8-9-8	2-9-2	5-7-6	, <u> </u>	2-0-0	£	5-7-6	1	2-9-0	1	8-6-9	
Plate Offsets (X,Y) [6:0-4-0,0	0-1-13], [9:0-1-4,Edge], [1	4:0-3-6,0-2-0]									
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matri	0.53 0.69 0.37 x-S		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.15 -0.33 0.08	(loc) 2-15 2-15 9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 247 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SP No.2 *E: 5-6: 2x4 SP No. 2x6 SP No.2 2x4 SP No.3	xcept* 2 or 2x4 SPF No.2				BRACIN TOP CH BOT CH WEBS	IG- ORD ORD	Structura except 2-0-0 oc Rigid cei 1 Row a	al wood purlins iling dir t midpt	sheathin (4-1-14 r ectly appl	g directly ap nax.): 5-6. lied or 10-0-0 5-13, 6-1	plied or 3-10-11 oc purl) oc bracing. 2	ins,

REACTIONS. (size) 2–0-3-8, 9=Mechanical Max Horz 2=130(LC 20) Max Uplift 2=-88(LC 16), 9=-74(LC 17) Max Grav 2=1504(LC 40), 9=1472(LC 40)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2741/194, 3-5=-2489/218, 5-6=-1638/217, 6-8=-2537/233, 8-9=-2756/196

BOT CHORD 2-15=-182/2398, 13-15=0/1636, 12-13=0/1637, 10-12=0/1637, 9-10=-94/2412

WEBS 3-15=-571/255, 5-15=-119/819, 5-13=-122/283, 6-10=-134/886, 8-10=-596/258

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-7-14 to 2-4-2, Interior(1) 2-4-2 to 17-2-0, Exterior(2) 17-2-0 to 23-4-15, Interior(1) 23-4-15 to 36-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 9) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9.
 One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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H	8- 8-	0-4 0-4	15-6-0 7-5-12		+ <u>20</u> 5	-10-0		28-3-12 7-5-12			36-1-0 7-9-4	
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 16 TCDL BCLL BCDL	20.0 6.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DC Lumber DOL Rep Stress Ir Code IRC20	2-0-0 DL 1.15 1.15 nor YES 15/TPI2014	CSI. TC BC WB Matrix	0.45 0.82 0.30 <-S	DEFL. Vert(LL) Vert(CT) Horz(CT	in -0.12 -0.26) 0.13	(loc) 12-13 13-15 9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 221 lb	GRIP 197/144 FT = 20%

LUMBER-		BRACING-			
TOP CHORD	2x6 SP No.2	TOP CHORD	Structural wood sheat	thing directly applied or 3-11-8 oc purlins,	
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2		except		
WEBS	2x4 SP No.3		2-0-0 oc purlins (5-7-1	12 max.): 5-6.	
		BOT CHORD	Rigid ceiling directly a	applied or 10-0-0 oc bracing.	
		WEBS	1 Row at midpt	3-13, 5-12, 8-12	
REACTIONS.	(size) 2=0-3-8, 9=Mechanical				
	Max Horz 2=120(LC 20)				
	Max Uplift 2=-81(I C 16) 9=-63(I C 17)				

Max Uplift 2=-81(LC 16), 9=-63(LC 17) Max Grav 2=1497(LC 2), 9=1444(LC 39)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2694/184, 3-5=-1996/213, 5-6=-1652/229, 6-8=-1993/215, 8-9=-2669/182

BOT CHORD 2-15=-126/2339, 13-15=-126/2339, 12-13=-14/1654, 10-12=-92/2307, 9-10=-92/2307

WEBS 3-15=0/335, 3-13=-775/174, 5-13=-11/518, 6-12=0/494, 8-12=-742/175, 8-10=0/332

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 15-6-0, Exterior(2) 15-6-0 to 19-8-15, Interior(1) 19-8-15 to 20-10-0, Exterior(2) 20-10-0 to 25-0-15, Interior(1) 25-0-15 to 36-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9.
 One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	0-0-4	1	+	2 10 0		10	-	£ 4 C			0 1 10		0.5.4	\neg
	0-8-4	+ 0 0 01 [44:0	0.0.0.0.1.1		5	0-4-U		5-4-0			0-1-12	-	0-0-4	
Plate Utisets (∧, i) [2:⊨dge,	0-3-2], [11:0	0-0,0-2-3], [13.0-3-4,0-3-0]									-	
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	f) 20.0 16.5/15.0 10.0	SPAC Plate Lumb Rep S	CING- Grip DOL per DOL Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.90 0.78 0.56		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.14 -0.29 0.13	(loc) 14 13-14 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BOLL	0.0	Code	IRC2015/T	PI2014	Matr	ix-S							Weight: 201 lb	FT = 20%
BCDL	10.0												····g···· _•· ···	
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE Left: 2x4 SP N SLIDER	2x4 SP No.2 or 2x4 SP No.2 or 2x4 SP No.3 o.3 Right 2x4 SP No	2x4 SPF No 2x4 SPF No 0.3 -t 3-6-7	.2 .2			E	BRACI TOP CI BOT CI WEBS	NG- HORD HORD	Structur 2-0-0 or Rigid ce 1 Row a	ral wood c purlins eiling dire at midpt	l sheathin (4-0-6 m ectly appl	g directly ap ax.): 5-7. ied or 10-0-(6-15, 6-1	plied, except) oc bracing. 3	
REACTIONS.	(size) 2=0- Max Horz 2=10 Max Uplift 2=-6 Max Grav 2=15	-3-8, 11=Mec 00(LC 16) 62(LC 16), 11 500(LC 2), 1	hanical =-44(LC 17) 1=1437(LC 2) 2)										
FORCES. (Ib TOP CHORD) - Max. Comp./N 2-3=-2628/199	/lax. Ten A 9, 3-5=-2118	ll forces 250 /221, 5-6=-1	(lb) or less exc 808/233, 6-7=-7	ept when sl 1786/231, 7	nown. '-9=-2126/21	8,							
BOT CHORD	2-17=-118/223 11-12=-110/2	95 35, 15-17=-1 170	18/2235, 14	-15=-53/2013, 1	3-14=-53/2	013, 12-13=	-110/2	170,						
WEBS	3-17=0/277, 3 7-13=-2/590, 9	-15=-608/14 9-13=-520/14	5, 5-15=0/60 43	07, 6-15=-452/8	2, 6-14=0/2	73, 6-13=-47	76/81,							
NOTES- 1) Unbalanced 2) Wind: ASCE gable end z: 23-6-0, Exté Lumber DO 3) TCLL: ASCI roof snow: L governs. R: 4) Unbalanced 5) This truss h: non-concurr 6) Provide ade 7) This truss h: 8) * This truss h: 8) * This truss h: 9) Refer to gird 10) Provide me 11) One H2.5/ connection 12) Graphical	roof live loads ha 7-10; Vult=120n one and C-C Exten- rior(2) 23-6-0 to 3 L=1.60 plate grip 7-10; Pr=20.0 p umber DOL=1.12 ain surcharge app snow loads have as been designed ent with other live quate drainage to as been designed has been designed the for truss to ochanical connecd A simpson Strong is for uplift only a purlin representat	ave been cor mph Vasd=95 erior(2) -0-10 27-8-15, Inte DOL=1.60 ssf (roof live I 5 Plate DOL= blied to all ex been consis d for greater e loads. b prevent wa d for a live I ord and any truss connect truss connect and does not tion does not	nsidered for imph; TCDL +8 to 2-1-8, rior(1) 27-8- oad: Lumbe =1.15); Cate posed surfa dered for this of min roof li ter ponding. isf bottom ch oad of 20.0p other memb ztions. rs) of truss t tors recomm t consider la t depict the s	this design. =6.0psf; BCDL= Interior(1) 2-1-8 15 to 36-1-0 zoo r DOL=1.15 Pla gory II; Exp B; F ces with slopes s design. ve load of 12.0 hord live load no sf on the bottor ers, with BCDL o bearing plate ended to conne teral forces. size or the orien	=6.0psf; h=3 to 12-10-0 he;C-C for r te DOL=1.1 Partially Exp less than 0 psf or 1.00 nconcurrer n chord in a = 10.0psf. capable of ct truss to h	30ft; Cat. II; I , Exterior(2) members an 5); Pg=15.0 5.; Ct=1.10, I .500/12 in a times flat roo at with any of all areas whe withstanding bearing walls	Exp B; 12-10-i d force psf (gi Lu=50-i ccorda of load ther live ere a re g 100 lb s due to	Enclosed; 0 to 17-0-1 s & MWFR ound snow 0-0; Min. fla nce with IB of 11.6 psf 0 loads. ctangle 3-6 uplift at joi 0 UPLIFT a p and/or bo	MWFRS 5, Interio S for rea (); Pf=16 at roof sr C 1608.3 on overl S-0 tall by int(s) 11. t jt(s) 2.	(envelop r(1) 17-0 ctions sl 5 psf (fli ow load 8.4. nangs y 2-0-0 w This prd.	pe))-15 to hown; at vide	Communities States	SEAL 044925 MGINEE January 7,2	021
A														

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven touckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





L	5-4-4 10-2-0 18-2-0 5-4-4 4-9-12 8-0-0			<u>26-2-0</u> 8-0-0				36-1-0				
Plate Offsets (X,Y)	<u>5-4-4</u> [5:0-4-0,0-4-8]	<u>4-9-12</u> , [8:0-0-0,0-1-0]		8-0-0		8	0-0-0				9-11-0	
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 16.5/1 TCDL	20.0 5.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC (BC (WB ().73).67).41	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.14 -0.29 1	(loc) 11 1-13 8	l/defl >999 >999	L/d 240 180 p/a	PLATES MT20	GRIP 197/144
BCLL	0.0 *	Code IRC2015/TF	912014	Matrix-S	S.41		0.03	0	n/a	n/a	Weight: 224 lb	FT = 20%
LUMBER- TOP CHORD 2x4 S 4-5,5 BOT CHORD 2x6 S WEBS 2x4 S	SP No.2 or 2x4 S -6: 2x6 SP No.2 SP No.2 SP No.3	PF No.2 *Except*			B T B W	I RACING- OP CHORD OT CHORD /EBS	Structural except 2-0-0 oc p Rigid ceili 1 Row at	l wood ourlins ing dire midpt	sheathing (4-7-10 n ctly appli	g directly app nax.): 4-6. ed or 10-0-0 5-13, 5-9	l plied or 3-2-12 oc purlir) oc bracing.	
REACTIONS. (s Max Max Max	ize) 8=Mechar Horz 2=81(LC 2 Uplift 8=-21(LC Grav 8=1434(L0	nical, 2=0-3-8 20) 17), 2=-39(LC 16) C 2), 2=1497(LC 2)										
FORCES. (Ib) - Ma. TOP CHORD 2-3 7-8 BOT CHORD 2-1 WEBS 2-1	x. Comp./Max. T =-2670/216, 3-4= =-2564/251 4=-146/2285, 13 3=-389/122, 4-11	en All forces 250 =-2353/223, 4-5=-20 -14=-146/2285, 11- 3=0/672, 5-13=-866	(lb) or less exce)50/229, 5-6=-2 13=-123/2705, (133, 5-11=0/3)	ept when show 043/216, 6-7= 9-11=-123/27(vn. =-2349/206 05, 8-9=-1	5, 74/2196 /667						
 7-9 NOTES- 1) Unbalanced roof li 2) Wind: ASCE 7-10; gable end zone ar 26-2-0, Exterior(2) Lumber DOL=1.6C 3) TCLL: ASCE 7-10 roof snow: Lumbe governs. Rain sur 4) Unbalanced snow 5) This truss has bee non-concurrent wi 6) Provide adequate 7) This truss has bee will fit between the will fit between the 9) Refer to girder(s) f 10) Provide mechani 11) One H2.5A Simp connection is for 12) Graphical purling 	=-290/171 ve loads have be Vult=120mph V ad C-C Exterior(2 26-2-0 to 30-4-1) plate grip DOL= ; Pr=20.0 psf (roor r DOL=1.15 Plate charge applied to loads have beer en designed for g en designed for a bettom chord a en designed for a sen designed for a sen designed for a sen designed for a cal connection (to son Strong-Tie c uplift only and de representation do	een considered for ti asd=95mph; TCDL=) -0-10-8 to 2-1-8, li 5, Interior(1) 30-4-1 (1.60) of live load: Lumber of DDL=1.15); Categ of all exposed surfact of considered for this reater of min roof live s. ent water ponding. 10.0 psf bottom chi a live load of 20.0pp and any other member connections. by others) of truss to connectors recomme bes not consider late bes not depict the si	his design. 6.0psf; BCDL= hterior(1) 2-1-8 5 to 36-0-4 zor DOL=1.15 Plat ory II; Exp B; P es with slopes design. e load of 12.0 p ord live load no af on the bottom ers. bearing plate (ended to conne eral forces. ze or the orient	6.0psf; h=30ft to 10-2-0, Ext le;C-C for mer e DOL=1.15); artially Exp.; (less than 0.50 psf or 1.00 tim nconcurrent w n chord in all a capable of witt ct truss to bea ation of the pu	t; Cat. II; E erior(2) 10 mbers and Pg=15.0 Ct=1.10, L 00/12 in ac ies flat roo vith any oth reas when hstanding ring walls urlin along	ixp B; Enclosed; N)-2-0 to 14-4-15, I I forces & MWFRS u=50-0-0; Min. fla cordance with IBC f load of 11.6 psf ner live loads. re a rectangle 3-6 100 lb uplift at join due to UPLIFT at the top and/or bo	/WFRS (enterior(1) S for react t; Pf=16.5 t roof snov C 1608.3.4 on overha -0 tall by 2 -0 tall by 2 -0 tall by 2 -0 tall by 2 -0 tall by 2	envelop 14-4-1 ions sh psf (fla w load I. ngs 2-0-0 w sis	e) 5 to own; t	Contraction Series	SEAL 044925	Alex Manual

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January 7,2021





Job	Truss	Truss Type	Qty	Ply	KB Home 238.2338.C	
						144252923
238_2338_C	HG1	HIP GIRDER	1	2		
				-	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	430 s Nov	30 2020 MiTek Industries, Inc. Wed Jan 6 16:13:44 2021 I	Page 2

ID:W17bIGclgq2WNY_TbTxie4yHe28-ga3f56YwEi?_x6PES_BEzsoNMhfixMF3R?J_Izy0zb

NOTES-

13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-43, 4-9=-53, 9-12=-43, 2-11=-20

Concentrated Loads (lb) Vert: 4=-63(B) 9=-63(B) 20=-33(B) 14=-33(B) 23=-78(B) 24=-76(B) 25=-58(B) 27=-58(B) 28=-58(B) 29=-58(B) 30=-58(B) 31=-58(B) 32=-58(B) 33=-58(B) 34=-58(B) 35=-58(B) 37=-76(B) 38=-78(B) 40=-69(B) 41=-38(B) 42=-33(B) 43=-33(B) 44=-33(B) 45=-33(B) 45=-33(B) 45=-33(B) 48=-33(B) 49=-33(B) 50=-33(B) 51=-33(B) 52=-38(B) 53=-69(B)

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eonent 818 Soundside Road Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	KB Home 238.2338.C	
						144252924
238_2338_C	HG2	HIP GIRDER	1	2		
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	430 s Nov	30 2020 MiTek Industries, Inc. Wed Jan 6 16:13:52 2021 F	Page 2
		ID:W17	blGclgg2V	VNY TbTx	kie4yHe28-R7YhmsexLA?svK0mwgK6IY7lhvL3pz8EHFFQGI	Izy0zT

NOTES-

13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-43, 4-9=-53, 9-11=-43, 2-11=-20

Concentrated Loads (lb)

Vert: 4=-63(F) 6=-58(F) 19=-33(F) 8=-58(F) 15=-33(F) 22=-78(F) 23=-76(F) 24=-58(F) 26=-58(F) 27=-58(F) 28=-58(F) 29=-58(F) 30=-58(F) 31=-58(F) 33=-58(F) 34=-67(F) 35=-80(F) 36=-69(F) 37=-38(F) 38=-33(F) 40=-33(F) 41=-33(F) 42=-33(F) 43=-33(F) 44=-33(F) 45=-33(F) 46=-33(F) 46=-33(F) 48=-33(F) 49=-33(F) 50=-39(F)

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LOADING (pst TCLL (roof) Snow (Pf/Pg) TCDL BCLL	f) 20.0 11.6/15.0 10.0 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.54 BC 0.36 WB 0.00	DEFL. in Vert(LL) -0.05 Vert(CT) -0.10 Horz(CT) 0.01	(loc) / 2-5 > 2-5 > 4	l/defl L/d >999 240 >667 180 n/a n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P				Weight: 24 lb	FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 SLIDER
 Left 2x4 SP No.3 -t 3-3-2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-4-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical Max Horz 2=135(LC 14) Max Uplift 4=-102(LC 14)

Max Grav 4=165(LC 26), 2=269(LC 2), 5=106(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=102.



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		3-10-8	1-6-0	
Plate Offsets (X,Y) [4:0-2-13]	,0-2-15]			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 PCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.46 BC 0.28 WB 0.00	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.03 2-6 >999 240 MT20 197/14 Vert(CT) -0.07 2-6 >898 180 MT20 197/14 Horz(CT) 0.07 5 n/a n/a 14	4
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Weight: 24 lb FT =	= 20%
LUMBER- TOP CHORD 2x4 SP No 2 or 2	2x4 SPF No.2 *Except*	BF	RACING- OP CHORD Structural wood sheathing directly applied or 5-4-8 oc purlins, excer	ot

 4-5: 2x6 SP No.2
 BOT CHORD
 2x4 SP No.2
 BOT CHORD
 2x4 SP No.2
 BOT CHORD
 2x4 SP No.2
 BOT CHORD
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

 SLIDER
 Left 2x4 SP No.3 -t 2-4-5
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=Mechanical, 2=0-3-8, 6=Mechanical Max Horz 2=102(LC 14) Max Uplift 5=-47(LC 14), 2=-10(LC 14)

Max Grav 5=146(LC 2), 2=269(LC 2), 6=95(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-10-8, Exterior(2) 3-10-8 to 5-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This
 connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	2-4	4-8	5-	4-8	1		
	2-4	4-8	3-	0-0	1		
Plate Offsets (X,Y) [2:Edge,0)-3-2], [3:0-3-12,0-2-0]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNO	CSI. TC 0.46 BC 0.23 WB 0.00	DEFL. Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) 0.0	n (loc) l/d 1 2-5 >9 3 2-5 >9 8 4 r	efl L/d 99 240 99 180 n/a n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P				Weight: 24 lb	FT = 20%
LUMBER-		BF	RACING-				
TOP CHORD 2x4 SP No.2 or 2 BOT CHORD 2x6 SP No.2	2x4 SPF No.2	тс	DP CHORD Structor 2-0-0	ural wood she oc purlins: 3-4	athing directly ap	plied or 5-4-8 oc purlir	is, except

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

WEDGE

Left: 2x4 SP No.3

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=71(LC 10) Max Uplift 4=-59(LC 7), 2=-29(LC 10) Max Grav 4=145(LC 2), 2=321(LC 2), 5=131(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs
- non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This
 connection is for uplift only and does not consider lateral forces.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-3=-43, 3-4=-53, 2-5=-20

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	KB Home 238.2338.C	
						144252927
238_2338_C	J3	JACK-OPEN GIRDER	3	1		
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	430 s Nov	30 2020 MiTek Industries, Inc. Wed Jan 6 16:13:55 2021	Page 2

ID:W17bIGclgq2WNY_TbTxie4yHe28-siDpOtgqe5NRmolLbotpvBIIA6WT0QShzDT4tdzy0zQ

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 6=-44(F) 7=-11(F) 8=-16(F) 9=-13(F)

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Plate Offsets (X,Y) [2:Edge,	0-1-0], [2:0-0-15,0-5-4]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.16 BC 0.13 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc -0.01 2- -0.01 2- -0.00	c) l/defl 4 >999 4 >999 3 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 14 lb	FT = 20%
LUMBER-		BR	ACING-					

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEDGE

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Left: 2x4 SP No.3

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=70(LC 16) Max Uplift 3=-51(LC 16), 2=-15(LC 16) Max Grav 3=89(LC 2), 2=202(LC 2), 4=66(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

Left: 2x4 SP No.3

WEDGE

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=50(LC 16) Max Uplift 4=-25(LC 13), 2=-22(LC 16) Max Grav 4=87(LC 35), 2=235(LC 36), 5=61(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 11) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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> 818 Soundside Road Edenton, NC 27932

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Edenton, NC 27932

January 7,2021



	9-4-4 12-0-0	18-2-0	24-4-0	26-11-12	36-1-0				
Plate Offsets (X,Y) [4:0-4-0,I	Edge], [6:0-4-0,Edge], [8:0-4-0,Edge],	[11:0-4-0,0-4-8], [14:0-4-0,0	0-4-8]	2-1-12	<u> </u>				
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.64 BC 0.76 WB 0.75 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.20 2-14 >999 -0.38 2-14 >999 0.07 10 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 248 lb	GRIP 244/190 FT = 20%		
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3		BI TC BC	RACING- DP CHORD S DT CHORD F	Structural wood sheath Rigid ceiling directly ap	ing directly applie plied or 10-0-0 of	ed or 3-6-8 oc purlins c bracing.			
REACTIONS. (size) 2=0- Max Horz 2=13 Max Uplift 2=-9 Max Grav 2=14	REACTIONS. (size) 2=0-3-8, 10=Mechanical Max Horz 2=138(LC 20) Max Uplift 2=-94(LC 16), 10=-79(LC 17) Max Grav 2=1484(LC 2), 10=1434(LC 2)								
FORCES. (lb) Max. Comp./M TOP CHORD 2-3=-2508/157 BOT CHORD 2-14=-176/215 WEBS 7-11=-74/822,	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2508/157, 3-5=-2272/176, 7-9=-2257/183, 9-10=-2487/160 BOT CHORD 2-14=-176/2158, 11-14=-11/1625, 10-11=-48/2133 WEBS 7-11=-74/822, 9-11=-538/266, 5-14=-75/846, 3-14=-562/264, 5-7=-1465/155								
 NOTES- 1) Unbalanced roof live loads ha 2) Wind: ASCE 7-10; Vult=120m gable end zone and C-C Exter 36-0-4 zone;C-C for members 3) TCLL: ASCE 7-10; Pr=20.0 p roof snow: Lumber DOL=1.15 4) Unbalanced snow loads have 5) This truss has been designed non-concurrent with other live 6) This truss has been designed ron-concurrent with other live 6) This truss has been designed will fit between the bottom che 8) Refer to girder(s) for truss to 19) Provide mechanical connection 10) One H2.5A Simpson Strong connection is for uplift only a 11) ATTIC SPACE SHOWN IS I 	ave been considered for this design. http://www.ave.org/ave.o	L=6.0psf; h=30ft; Cat. II; Ex- 2 to 18-2-0, Exterior(2) 18 shown; Lumber DOL=1.60 f late DOL=1.15); Pg=15.0 p ; Partially Exp.; Ct=1.10 0 psf or 1.00 times flat roof nonconcurrent with any oth om chord in all areas where DL = 10.0psf. capable of withstanding 10 nect truss to bearing walls of	xp B; Enclosed; M -2-0 to 20-11-3, In olate grip DOL=1.6 osf (ground snow); f load of 11.6 psf of er live loads. e a rectangle 3-6-0 00 lb uplift at joint(due to UPLIFT at j	WFRS (envelope) terior(1) 20-11-3 to 50 Pf=11.6 psf (flat on overhangs 0 tall by 2-0-0 wide s) 10. ti(s) 2. This	Jan Martin	SEAL 044925	A Secondaria		



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January 7,2021



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 5-11-14, Exterior(2) 5-11-14 to 8-11-14, Interior(1) 8-11-14

to 11-5-15 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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REACTIONS. (size) 1=8-11-12, 3=8-11-12, 4=8-11-12 Max Horz 1=-56(LC 10) Max Uplift 1=-26(LC 14), 3=-33(LC 15) Max Grav 1=172(LC 2), 3=172(LC 2), 4=298(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-5-14, Exterior(2) 4-5-14 to 7-5-14, Interior(1) 7-5-14 to 8-5-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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TOP CHORD 2x4 SP No.3

BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-11-12 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=5-11-0, 3=5-11-0, 4=5-11-0 Max Horz 1=35(LC 11) Max Uplift 1=-16(LC 14), 3=-21(LC 15) Max Grav 1=107(LC 2), 3=107(LC 2), 4=186(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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