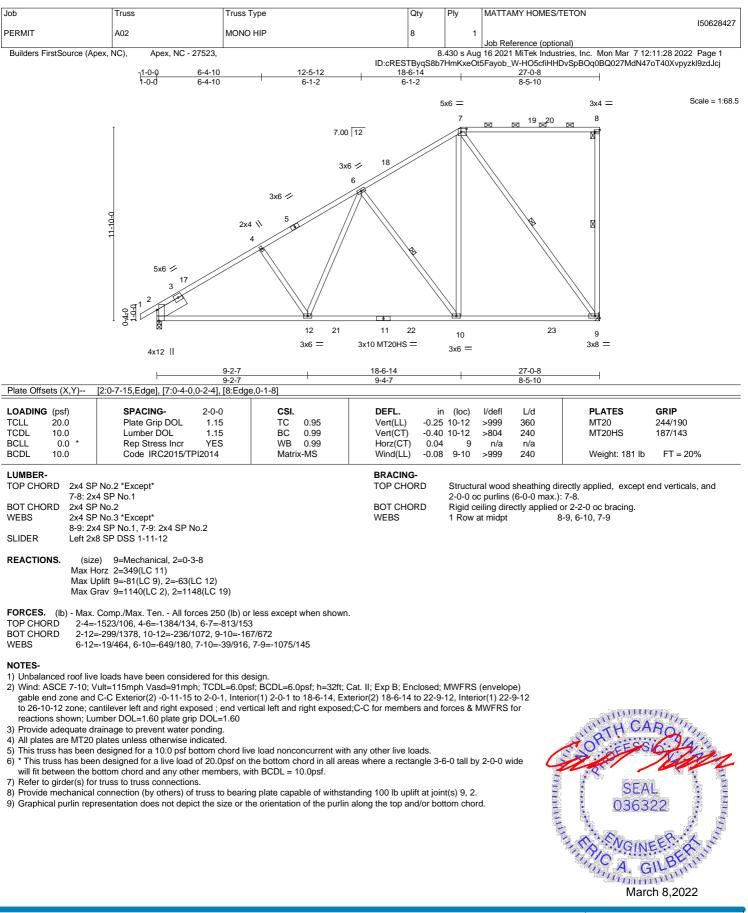
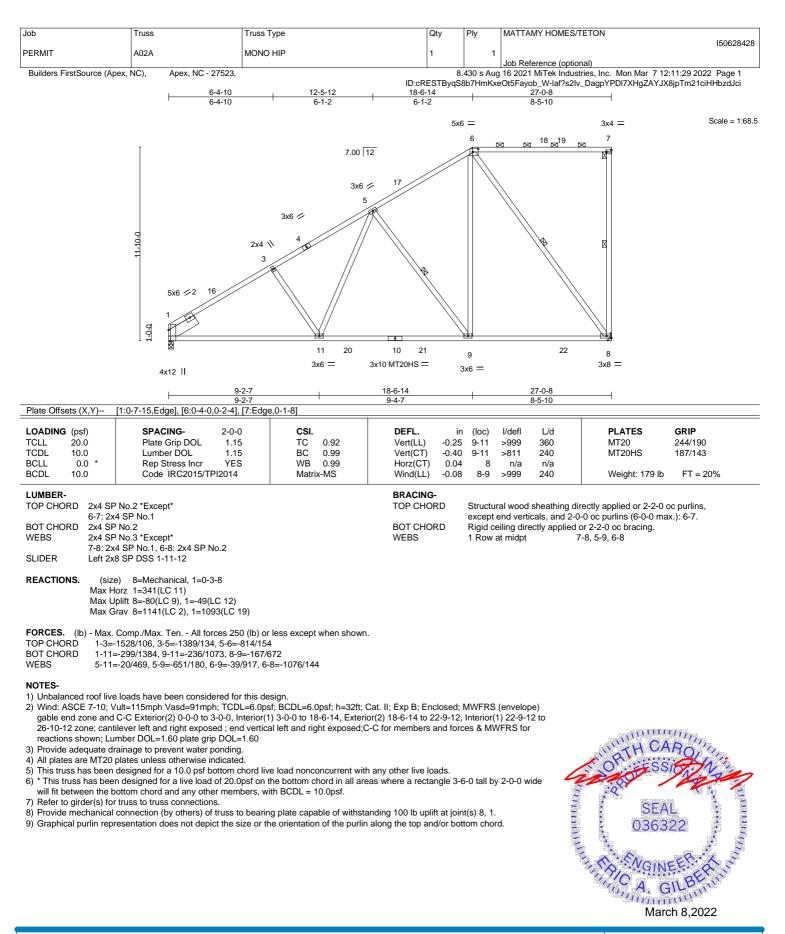


Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

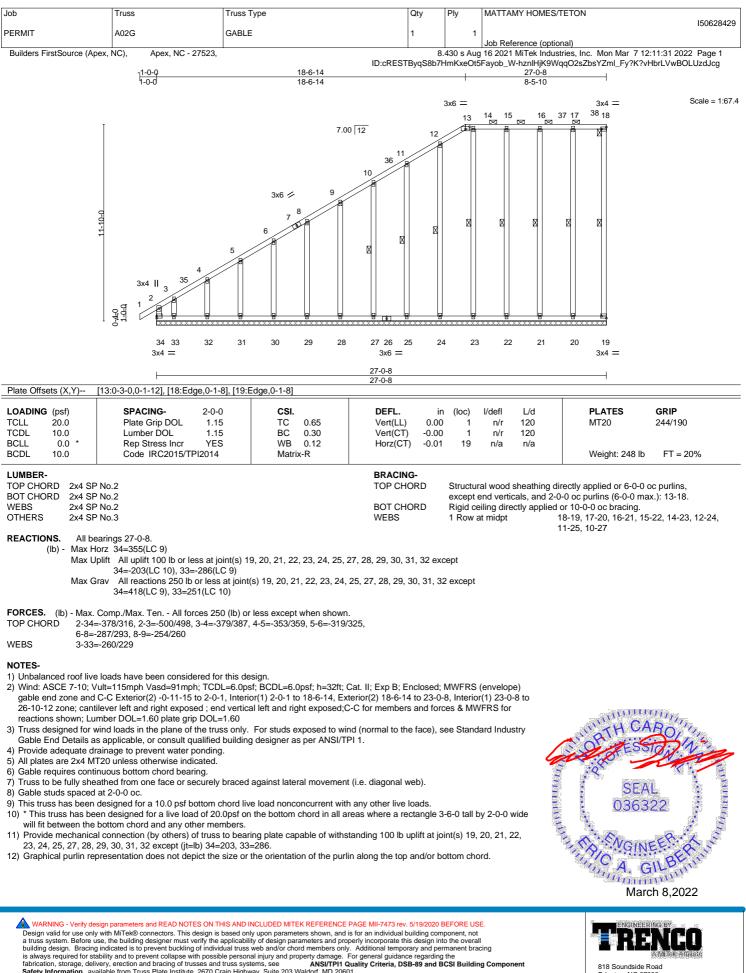
Edenton, NC 27932





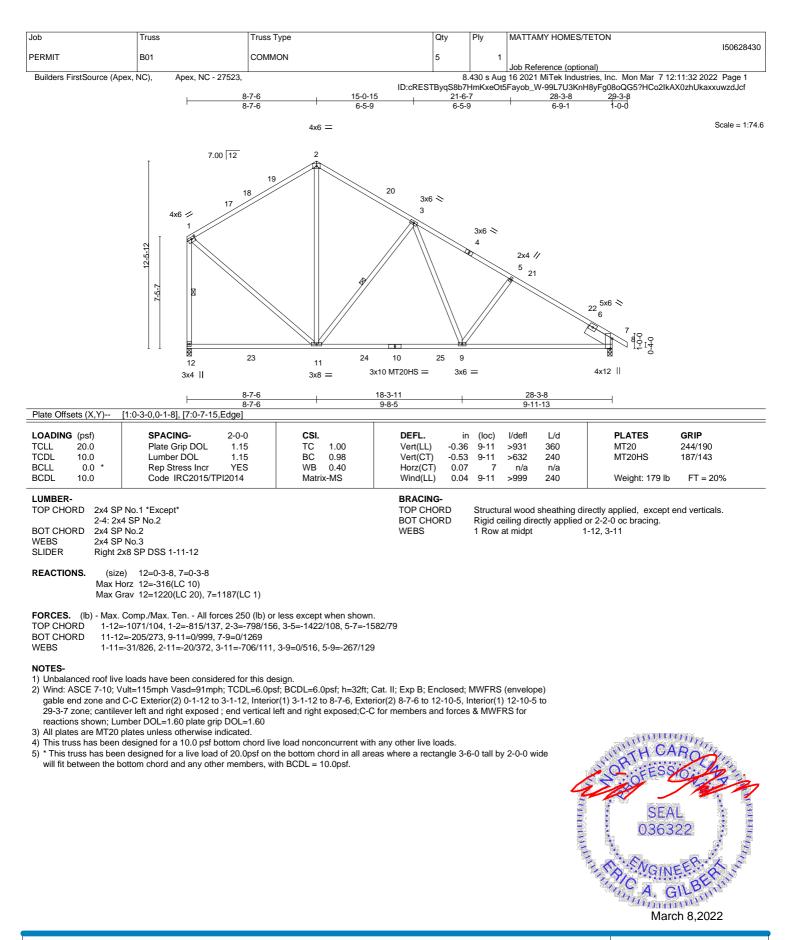




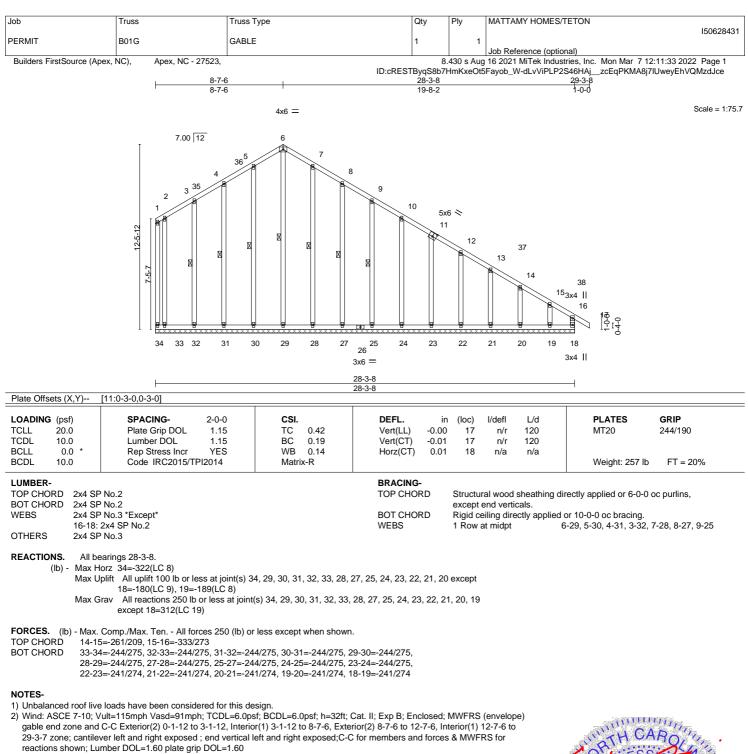


Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Edenton, NC 27932







3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

WARNING - Verify design r

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

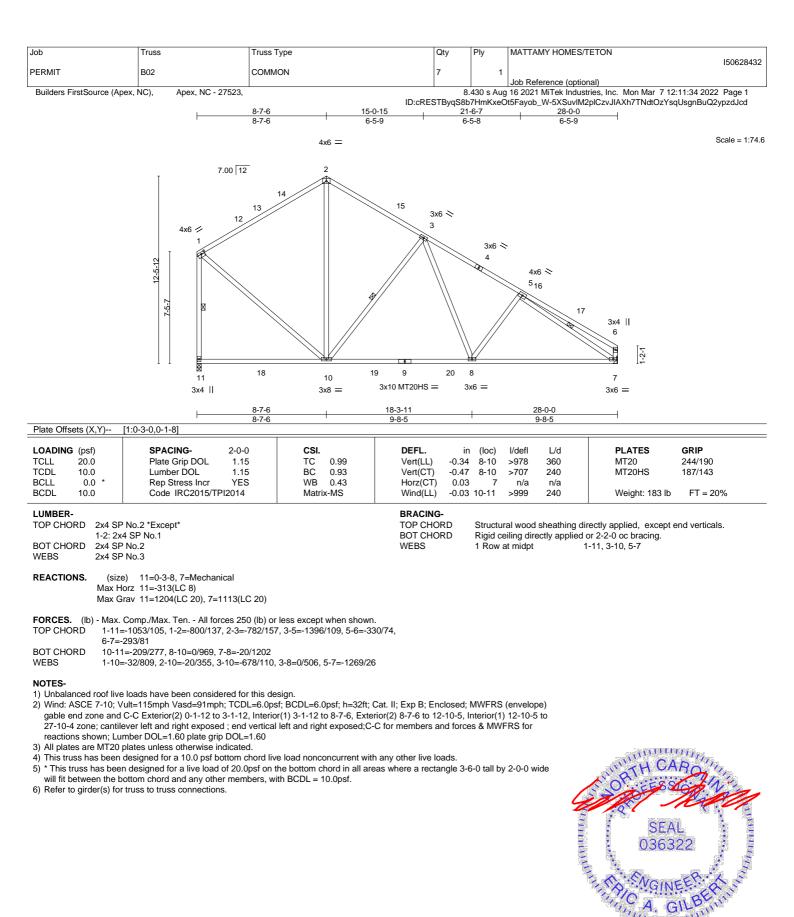
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 34, 29, 30, 31, 32, 33, 28, 27, 25, 24, 23, 22, 21, 20 except (jt=lb) 18=180, 19=189.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER KETEKENCE PAGE MIT-(#13184.019/2020 BET-ONE OSC. Design valid for use only with MITER& connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stational temporary and permanent bracing the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANSITPH Quality Criteria</u>, DSB-89 and BCSI Building Component Statis the prevent on the prevent of the 2000 Critical Hidhway. Suite 203 Valdorf MD 20601.

ters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE

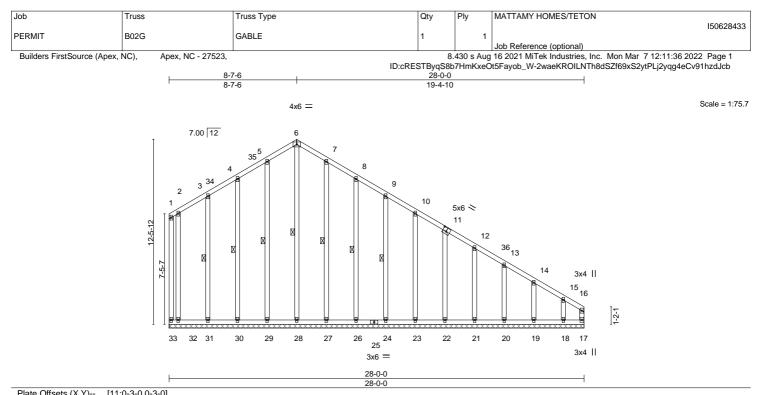
ORTH CAR Vinnenanana **UNITARY** SEAL 036322 **R**PIC A. GILBE mannan March 8.2022

Edenton, NC 27932



March 8.2022





TCLL 20.0				n (loc) l/defl L/d	PLATES GRIP
10 DI 10 0	Plate Grip DOL 1.15	TC 0.42	Vert(LL) n/a		MT20 244/190
TCDL 10.0 SCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.24	Vert(CT) n/a		
3CLL 0.0 * 3CDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.14 Matrix-R	Horz(CT) 0.01	17 n/a n/a	Weight: 255 lb FT = 20%
UMBER-			BRACING-		
OP CHORD 2x4 SP N OT CHORD 2x4 SP N			TOP CHORD	Structural wood sheathing except end verticals.	directly applied or 6-0-0 oc purlins,
VEBS 2x4 SP M	No.3		BOT CHORD	Rigid ceiling directly applie	d or 10-0-0 oc bracing.
DTHERS 2x4 SP M	No.3		WEBS	1 Row at midpt	6-28, 5-29, 4-30, 3-31, 7-27, 8-26, 9-24

17=378(LC 8), 18=303(LC 11) FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 14-15=-262/209, 15-16=-341/286
- BOT CHORD 32-33=-230/265, 31-32=-230/265, 30-31=-230/265, 29-30=-230/265, 28-29=-230/265,
  - 27-28=-230/265, 26-27=-230/265, 24-26=-230/265, 23-24=-230/265, 22-23=-230/265,
    - 21-22=-227/264, 20-21=-227/264, 19-20=-227/264, 18-19=-227/264, 17-18=-227/264

#### NOTES-

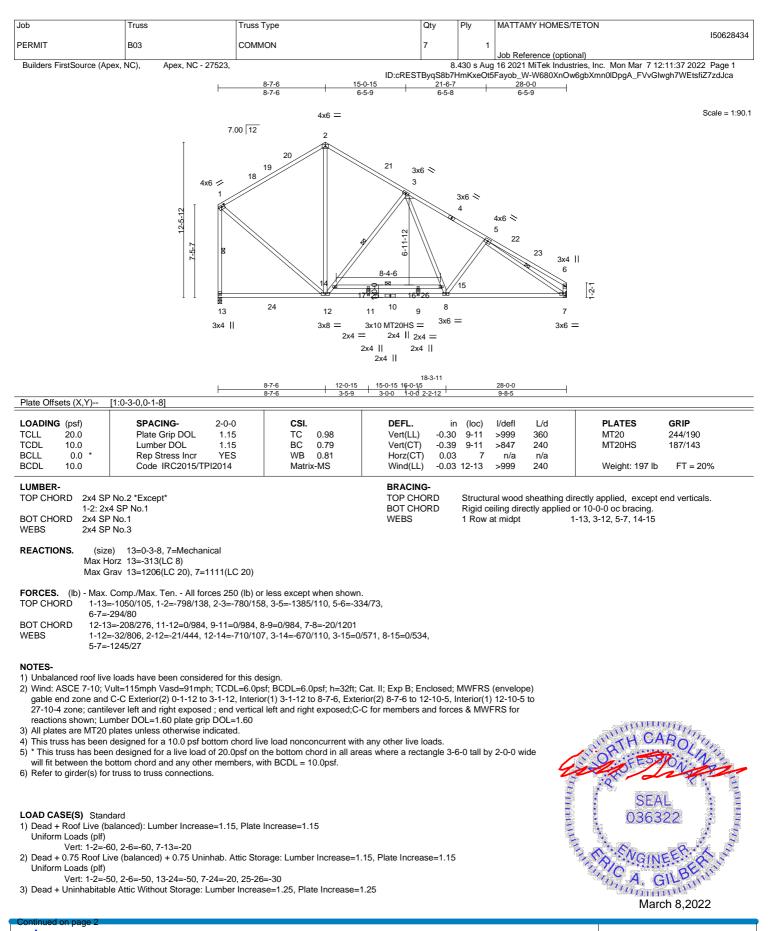
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 8-7-6, Exterior(2) 8-7-6 to 12-7-6, Interior(1) 12-7-6 to 27-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 33, 28, 29, 30, 31, 32, 27, 26, 24, 23, 22, 21, 20, 19 except (jt=lb) 17=270, 18=261.

SEAL 036322 MGINEER March 8,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses safe truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



USE.



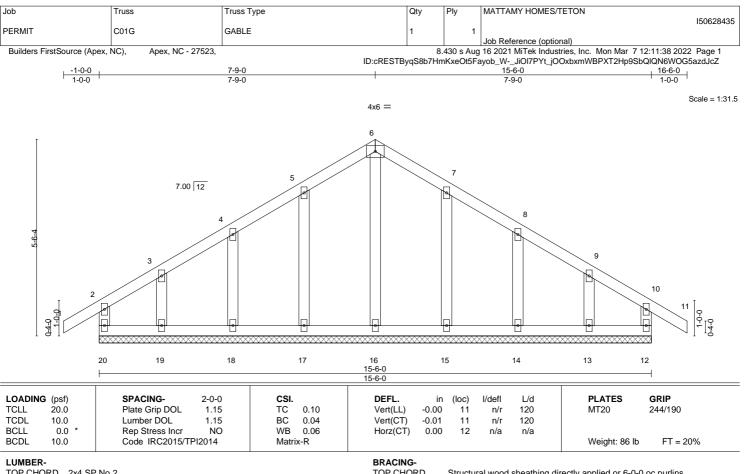
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permament bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses safe truss systems, see **ANXITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	MATTAMY HOMES/TETON	
						150628434
PERMIT	B03	COMMON	7	1	lah Deference (anti-nal)	
Builders FirstSource		C - 27523,		0 420 0 4.00	Job Reference (optional) g 16 2021 MiTek Industries, Inc. Mon Mar	7 10:11:07 0000 Dama 0
Duilders Firstoource	(Apex, NC), Apex, N	6 - 27523,			5Fayob W-W680XnOw6qbXmn0lDpqA FV	
			12.011201294000			Volligin Weldher 20000
LOAD CASE(S) S	tandard					
Uniform Loads (p	olf)					
Vert: 1-2	2=-20, 2-6=-20, 7-13=-40,	25-26=-40				
18) Dead + Uninhal	bitable Attic Storage: Lun	ber Increase=1.25, Plate Increase=1.25				
Uniform Loads	(plf)					
Vert: 1	-2=-20, 2-6=-20, 13-24=-0	60, 7-24=-20, 25-26=-40				
19) Dead + 0.75 Ro	oof Live (bal.) + 0.75 Unin	hab. Attic Storage + 0.75(0.6 MWFRS Wind	d (Neg. Int) Left): Lumb	er Increase	e=1.60, Plate Increase=1.60	
Uniform Loads						
	-2=-58, 2-6=-44, 13-24=-					
	-13=16, 1-2=8, 2-6=6, 6-					
		hab. Attic Storage + 0.75(0.6 MWFRS Wind	d (Neg. Int) Right): Lum	ber Increa	se=1.60, Plate Increase=1.60	
Uniform Loads						
	-2=-44, 2-6=-58, 13-24=-					
	-13=-6, 1-2=-6, 2-6=-8, 6					
		hab. Attic Storage + 0.75(0.6 MWFRS Wind	d (Neg. Int) 1st Parallel	): Lumber I	ncrease=1.60, Plate Increase=1.60	
Uniform Loads						
	, - , -	46, 13-24=-50, 7-24=-20, 25-26=-30				
	-13=15, 1-20=-16, 2-20=	-				
,	( /	hab. Attic Storage + 0.75(0.6 MWFRS Wind	d (Neg. Int) 2nd Paralle	i): Lumber	Increase=1.60, Plate Increase=1.60	
Uniform Loads		A 40.04 F0 7.04 00.0F 00.00				
	-2=-46, 2-22=-41, 6-22=-,  -13=-2, 1-2=-4, 2-22=9, (	34, 13-24=-50, 7-24=-20, 25-26=-30				
		+ 0.75 Uninhab. Attic Storage: Lumber Inci	roaco-1 15 Plata Inara	000-1 15		
Uniform Loads		+ 0.75 Onininab. Auto Storage. Lumber inci	rease=1.15, Flate Incre	ase=1.15		
	(pii) -2=-50, 2-6=-20, 13-24=-	50 7-24-20 25-26-30				
	,, -	+ 0.75 Uninhab. Attic Storage: Lumber Incr	rease-1.15 Plate Incre	250-1 15		
Uniform Loads		1 0.70 Ommas. Auto Storage. Europer inci	10000-1.10, 1 Idle III010	430-1.13		

Vert: 1-2=-20, 2-6=-50, 13-24=-50, 7-24=-20, 25-26=-30





 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

 OTHERS
 2x4 SP No.3

 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.** All bearings 15-6-0.

(lb) - Max Horz 20=-123(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 20, 12, 17, 18, 19, 15, 14, 13

Max Grav All reactions 250 lb or less at joint(s) 20, 12, 16, 17, 18, 19, 15, 14, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

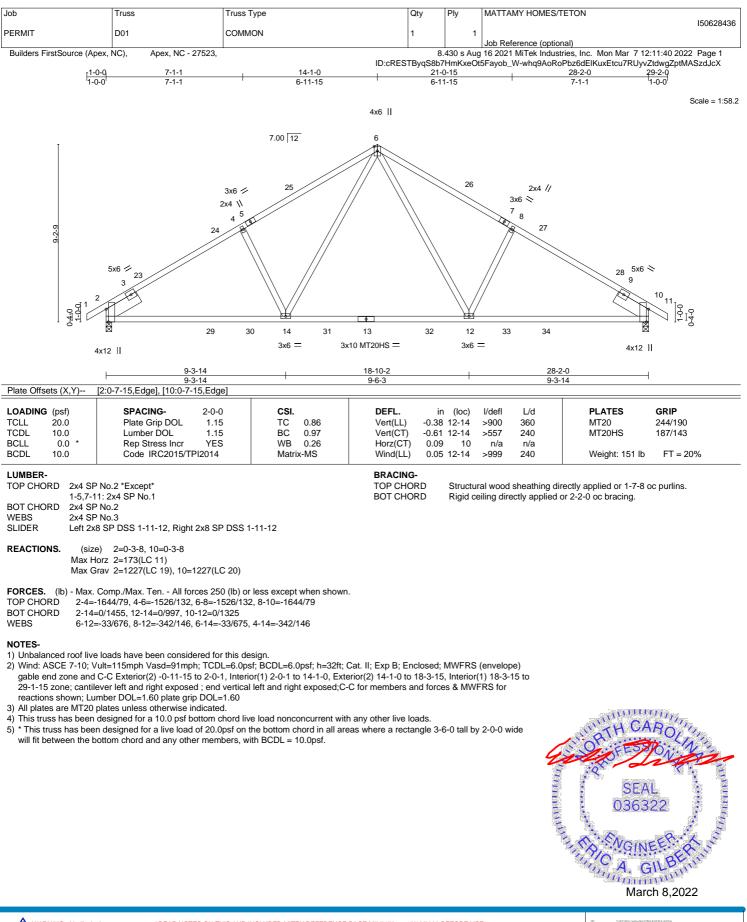
- Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-11-15 to 1-9-0, Exterior(2) 1-9-0 to 7-9-0, Corner(3) 7-9-0 to 10-9-0, Exterior(2) 10-9-0 to 16-5-15 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

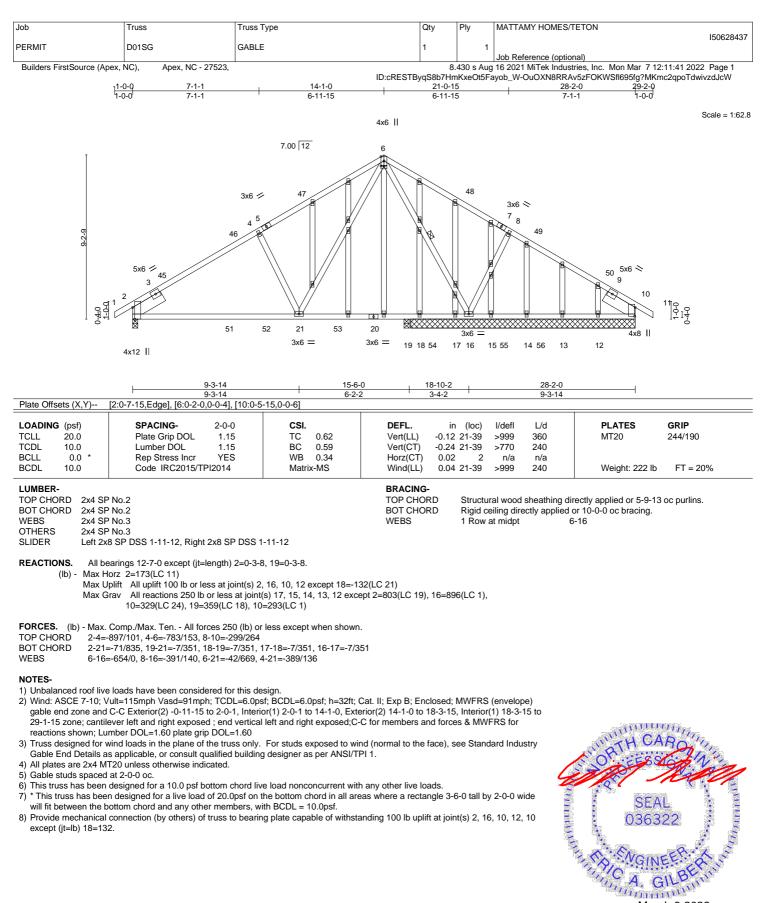
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 12, 17, 18, 19, 15, 14, 13.





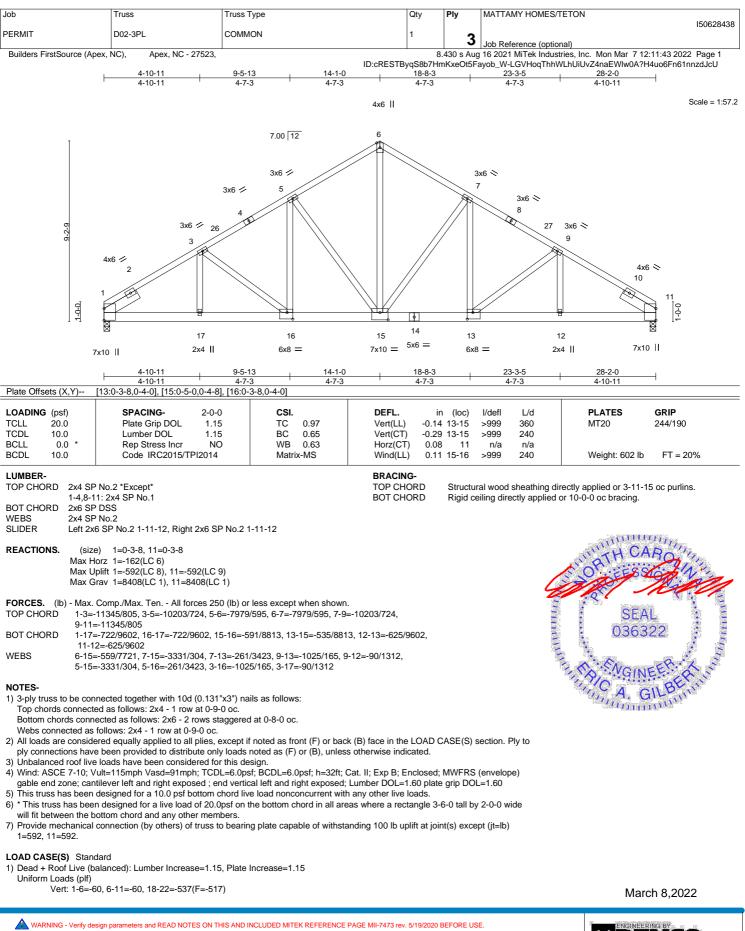






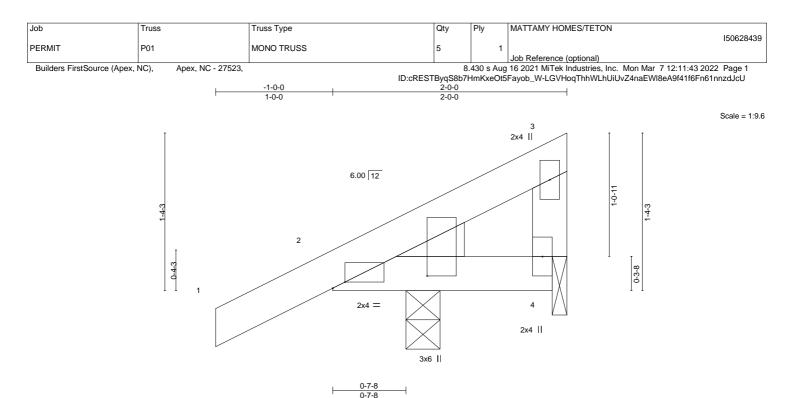
March 8,2022





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE.
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ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Edenton, NC 27932



LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	-0.00	5	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	-0.00	5	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	-0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-MP	Wind(LL)	0.00	5	>999	240	Weight: 10 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

 LUMBER 

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Left: 2x4 SP No.3

**REACTIONS.** (size) 2=0-3-8, 4=0-1-8

Max Horz 2=37(LC 11) Max Uplift 2=-33(LC 12), 4=-18(LC 1) Max Grav 2=227(LC 1), 4=18(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 1) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

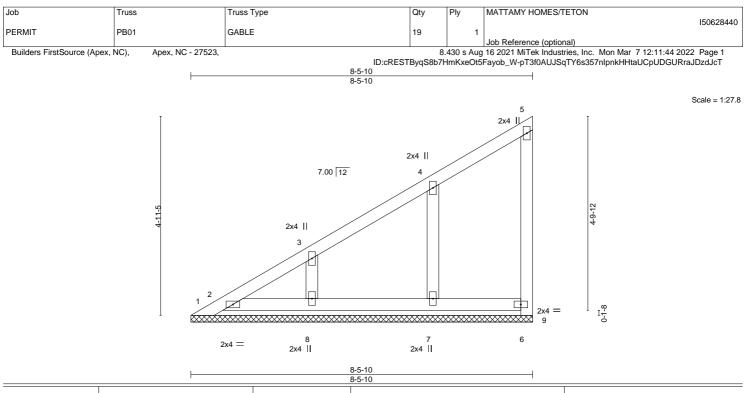
4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.







TCDL         10.0           BCLL         0.0         *           BCDL         10.0         *	Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	BC 0.09 WB 0.04 Matrix-S	Vert(CT) n/a Horz(CT) -0.00	- n/a 999 9 n/a n/a	Weight: 38 lb FT = 20%
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.19	Vert(LL) n/a	(loc) I/defl L/d - n/a 999	PLATES         GRIP           MT20         244/190

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

**REACTIONS.** All bearings 8-5-10.

(lb) - Max Horz 1=141(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 2, 6, 8, 7

Max Grav All reactions 250 lb or less at joint(s) 1, 2, 6, 8, 7

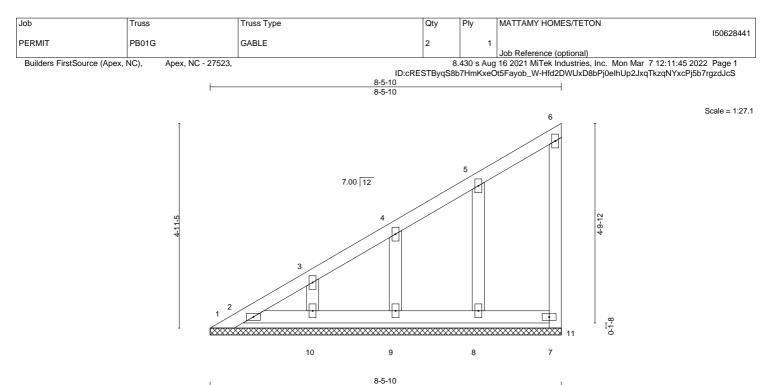
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-276/298

# NOTES-

- Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-3-8 to 3-0-0, Exterior(2) 3-0-0 to 8-3-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
   The value of the function of the functi
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 3-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 1, 9, 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 2, 6, 8, 7.
  9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building
- bee Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	TC	0.19	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.03	Horz(CT)	-0.00	11	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matrix	k-S						Weight: 41 lb	FT = 20%

 BOT CHORD
 2x4 SP No.2
 except end verticals.

 WEBS
 2x4 SP No.3
 BOT CHORD

 OTHERS
 2x4 SP No.3

**REACTIONS.** All bearings 8-5-10.

(lb) - Max Horz 1=141(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 2, 7, 8, 9, 10

Max Grav All reactions 250 lb or less at joint(s) 1, 2, 7, 8, 9, 10

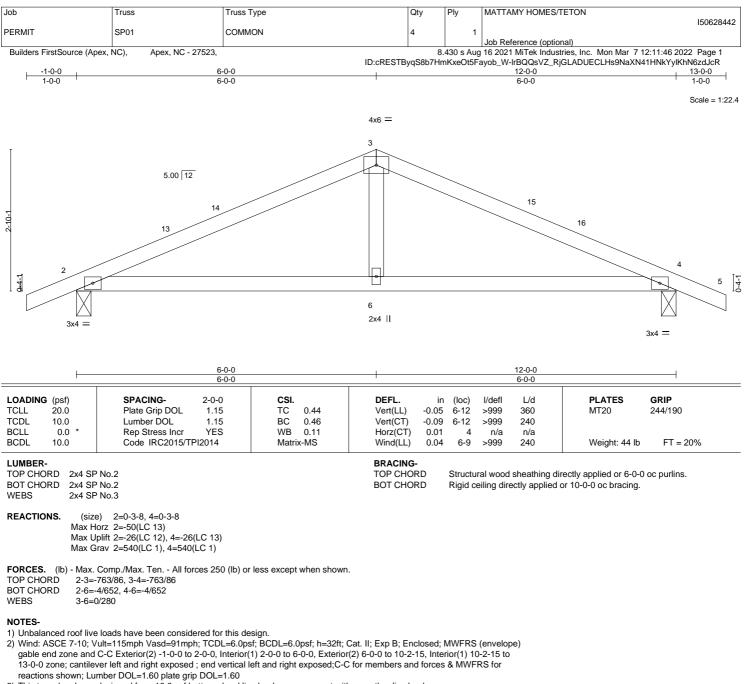
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-274/294

# NOTES-

- Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-3-8 to 3-3-8, Exterior(2) 3-3-8 to 8-3-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
   Turue devine de the interpart of the properties of the state of the s
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 1, 11, 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 2, 7, 8, 9, 10.
   See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







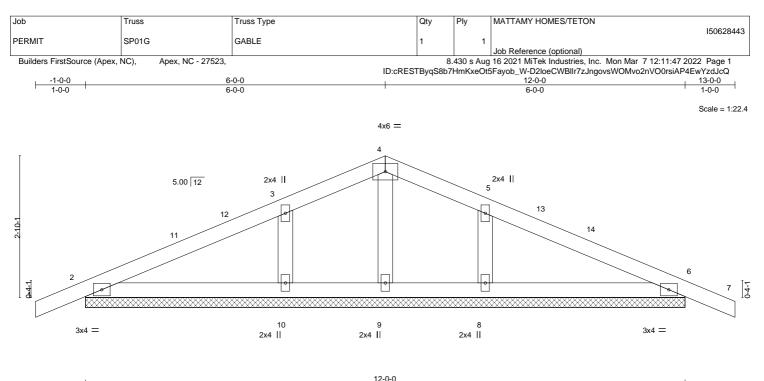
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.







12-0-0												
LOADING (ps	sf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.	.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	0.00	7	n/r	120	MT20	244/190
TCDL 10.	.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	0.01	7	n/r	120		
BCLL 0.	.0 *	Rep Stress Incr	NO	WB	0.05	Horz(CT)	0.00	6	n/a	n/a		
BCDL 10.	.0	Code IRC2015/T	PI2014	Matri	k-S						Weight: 48 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3

REACTIONS. All bearings 12-0-0.

(lb) - Max Horz 2=-39(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 10, 8

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9 except 10=321(LC 1), 8=321(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

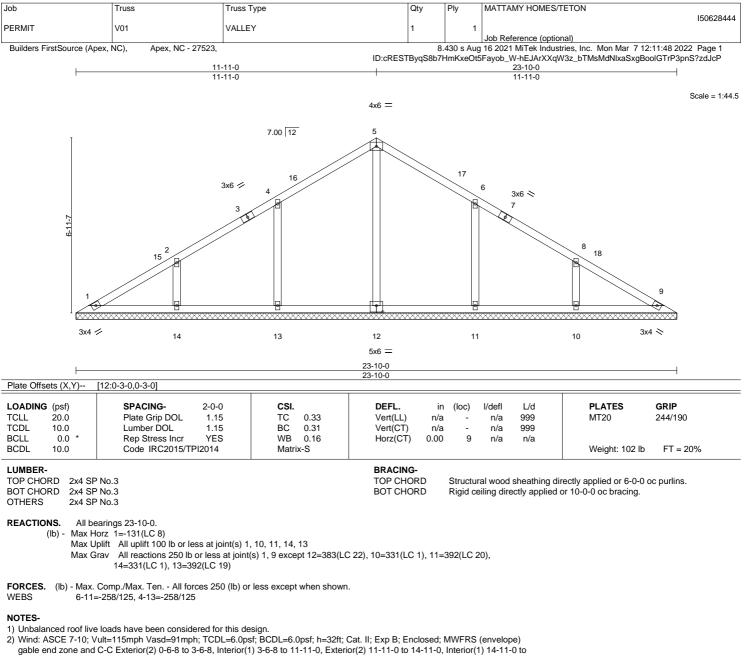
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-0-0 to 2-0-0, Exterior(2) 2-0-0 to 6-0-0, Corner(3) 6-0-0 to 9-0-0, Exterior(2) 9-0-0 to 13-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6, 10, 8.







gable end zone and C-C Exterior(2) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 11-11-0, Exterior(2) 11-11-0 to 14-11-0, Interior(1) 14-1 23-3-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

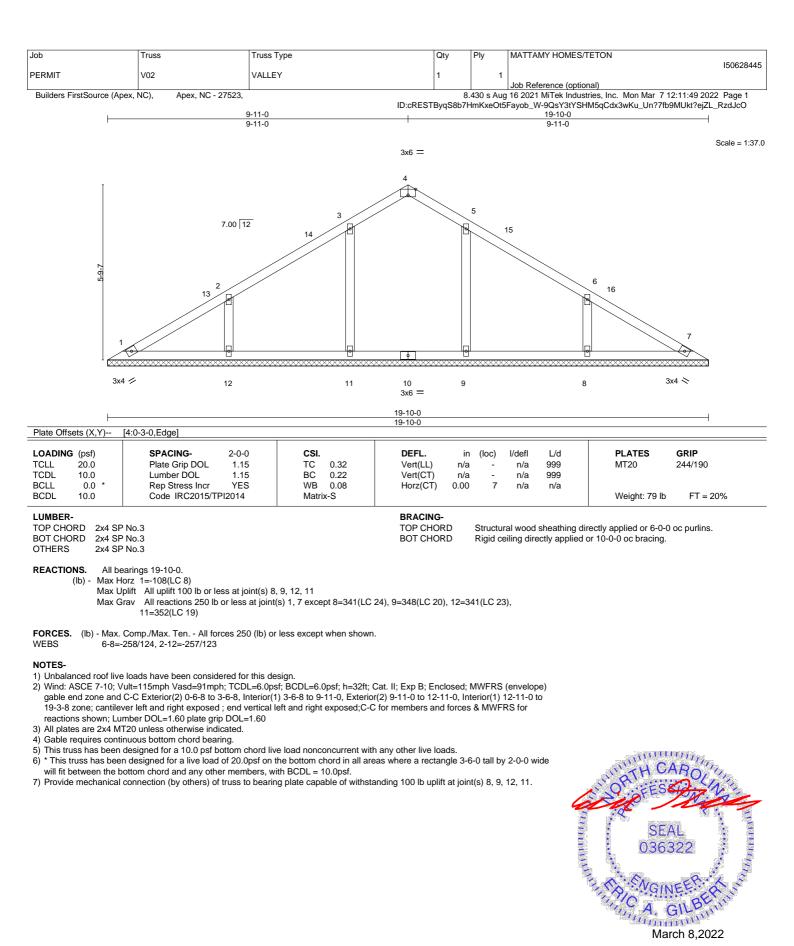
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

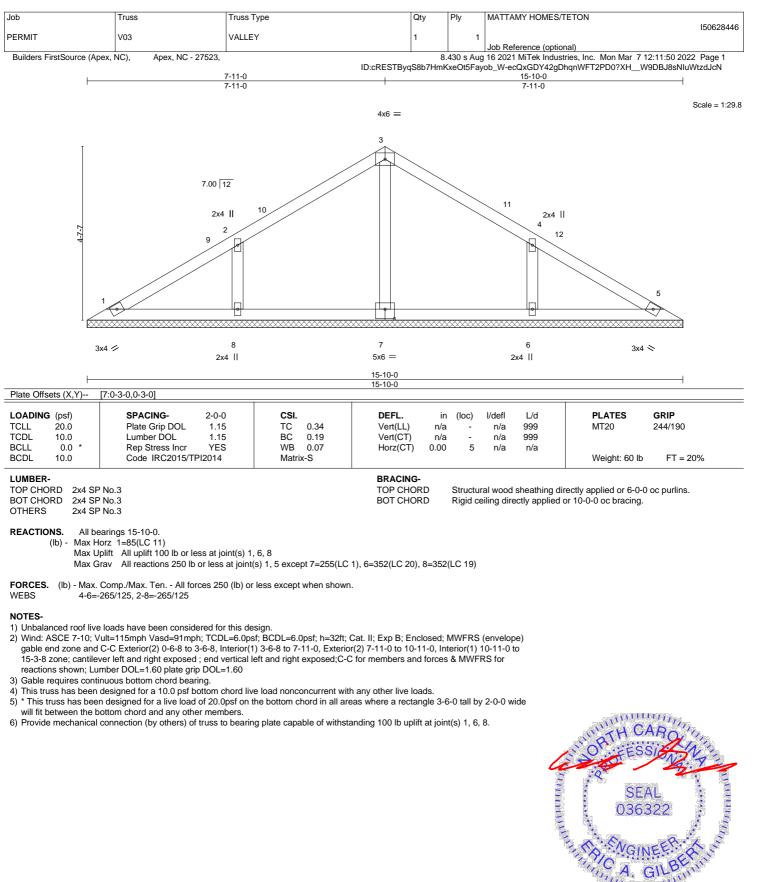
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 10, 11, 14, 13.







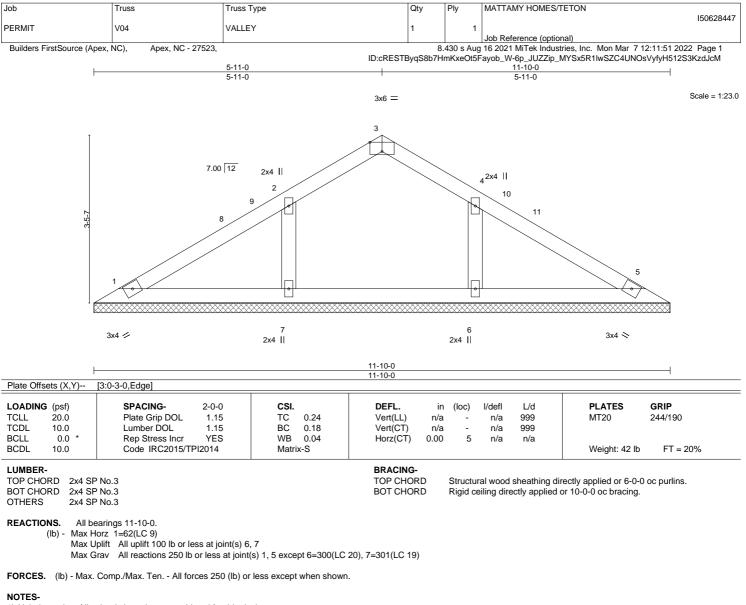




GALLAN MAN

March 8.2022





1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 5-11-0, Exterior(2) 5-11-0 to 8-11-0, Interior(1) 8-11-0 to 11-3-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

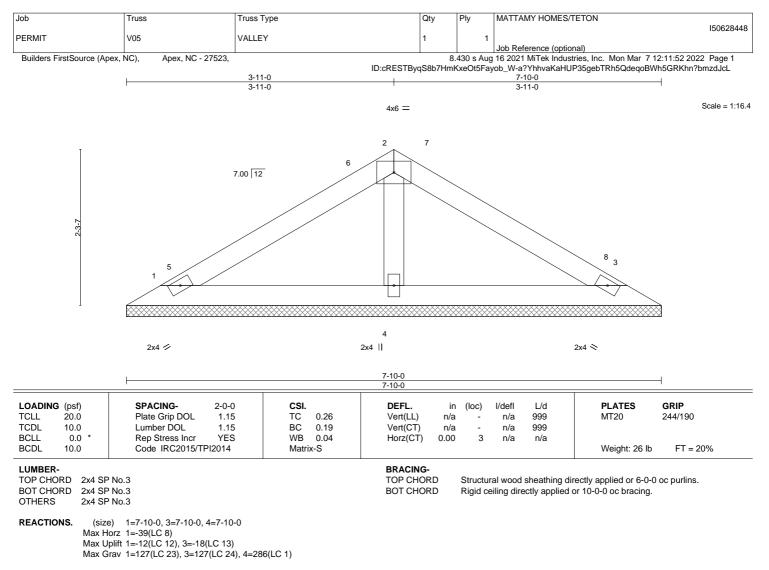
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 7.







FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

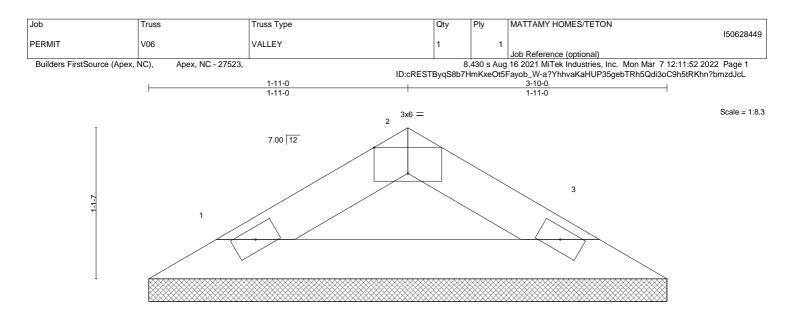
2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 3-11-0, Exterior(2) 3-11-0 to 6-11-0, Interior(1) 6-11-0 to 7-3-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







2x4 💋

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2x4 💸

			<u>3-10-0</u> 3-10-0							
Plate Offsets (X,Y) [2:0-3-0,Edge]										
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP		
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL)	n/a -	n/a	999	MT20	244/190		
TCDL 10.0	Lumber DOL 1.15	BC 0.15	Vert(CT)	n/a -	n/a	999				
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0	0.00 3	n/a	n/a				
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 11 lb	FT = 20%		

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

REACTIONS. (size) 1=3-10-0, 3=3-10-0 Max Horz 1=-16(LC 8) Max Uplift 1=-3(LC 12), 3=-3(LC 13) Max Grav 1=110(LC 1), 3=110(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

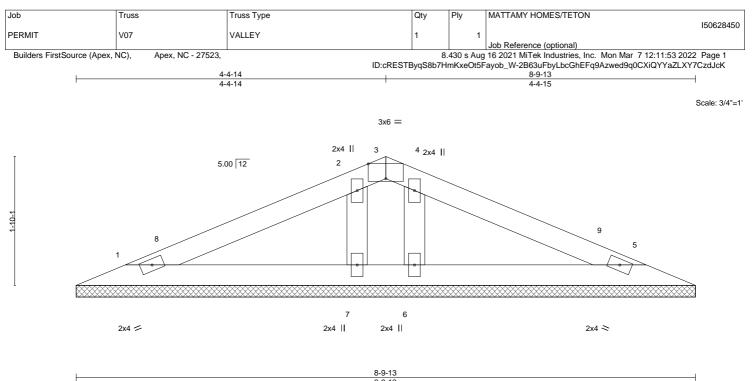


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permament bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.



OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.23	Vert(LL)	n/a -	n/a	999	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.13	Vert(CT)	n/a -	n/a	999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.04	Horz(CT)	0.00 5	n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 28 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3OTHERS2x4 SP No.3

**REACTIONS.** All bearings 8-9-13.

(lb) - Max Horz 1=-20(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 6, 7

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 6, 7

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-8-12 to 4-0-0, Interior(1) 4-0-0 to 4-4-14, Exterior(2) 4-4-14 to 7-4-14, Interior(1) 7-4-14 to 8-1-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 6, 7.





Min size shown is for crushing only.	Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.	Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.	LATERAL BRACING LOCATION	<b>4</b> X <b>4</b> The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.	PLATE SIZE	* Plate location details available in MiTek 20/20	edge of truss. This symbol indicates the required direction of slots in connector plates.	For 4 x 2 orientation, locate plates 0- <sup>1</sup> /16 <sup>th</sup> from outside		<b>Symbols</b> <b>PLATE LOCATION AND ORIENTATION</b> <b>Center plate on joint unless x, y offsets are indicated.</b> Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.
MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020	© 2012 MiTek® All Rights Reserved	Trusses are designed for wind loads in the plane of the truss unless otherwise shown. Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.		ICC-ES Reports: ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282	PRODUCT CODE APPROVALS	CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.	8 6 5 JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.	67-8 BOTTOM CHORDS	<u> </u>	Numbering System
<ul> <li>environmental, reatin or performance risks. Consult with project engineer before use.</li> <li>19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.</li> <li>20. Design assumes manufacture in accordance with ANSI/TP11 Quality Criteria.</li> <li>21. The design does not take into account any dynamic or other loads other than those expressly stated.</li> </ul>	17. Install and load vertically unless indicated otherwise.         18. Use of green or treated lumber may pose unacceptable	<ol> <li>Top chords must be sheathed or purlins provided at spacing indicated on design.</li> <li>Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.</li> <li>Connections not shown are the responsibility of others.</li> <li>Do not cut or alter truss member or plate without prior</li> </ol>	<ol> <li>Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.</li> </ol>	<ol> <li>camper is a non-subcuration consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.</li> <li>11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.</li> </ol>	<ol> <li>Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.</li> <li>Camber is a non-structural consideration and is the</li> </ol>	<ol> <li>Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.</li> </ol>	<ol> <li>Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.</li> <li>Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.</li> </ol>	<ol> <li>Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.</li> <li>Cut members to bear tightly against each other.</li> </ol>	<ol> <li>A second processing of the processing should be considered.</li> <li>Never exceed the design loading shown and never stack materials on inadequately braced trusses.</li> </ol>	General Safety Notes     Failure to Follow Could Cause Property     Damage or Personal Injury     Additional stability bracing for truss system, e.g.     diagonal or X-bracing, is always required. See BCSI.     Trust bracing must be designed by an exploser.