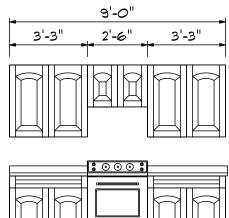
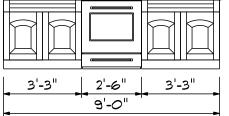
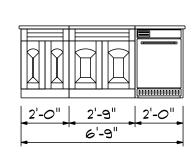
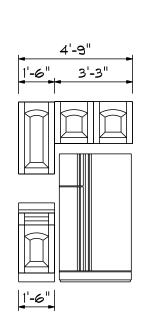


# Kitchen Cabinets

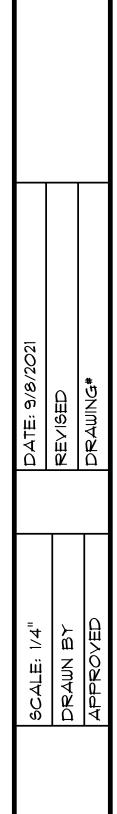




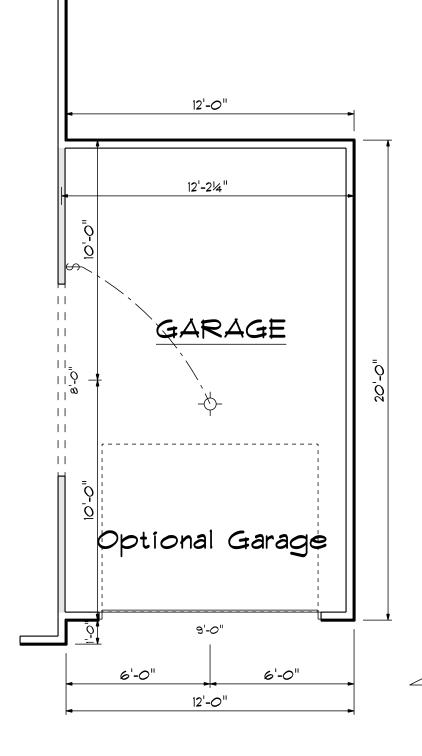




FIRST FLOOR OPENING	SCHEDULE		
PRODUCT CODE	SIZE	HINGE	COUNT
36X80 COLONIAL A 1	3'-0"	R	1
32X80 FRENCH A 1	2'-8"	L	1
108X84 - 1 PANEL - GARAGE DOOR(OPT)	9'-0"	U	1
192X84 - 8 PANEL - GARAGE DOOR	18'-0"	U	1
2-0 Door Unit	2'-0"	R	1
2-4 Door Unit	2'-4"	R	1
2-4 Door Unit	2'-4"	L	2
2-6 Door Unit	2'-6"	R	1
2-6 Door Unit	2'-6"	L	1
2-8 Door Unit	2'-8"	R	2
28x52 single	2'-8" x 5'-2"	N	3
28x52 twin	5'-4" x 5'-2"	NN	3
12X12 GLASS BLOCK	4'-0" x 4'-0"	N	1



Williams

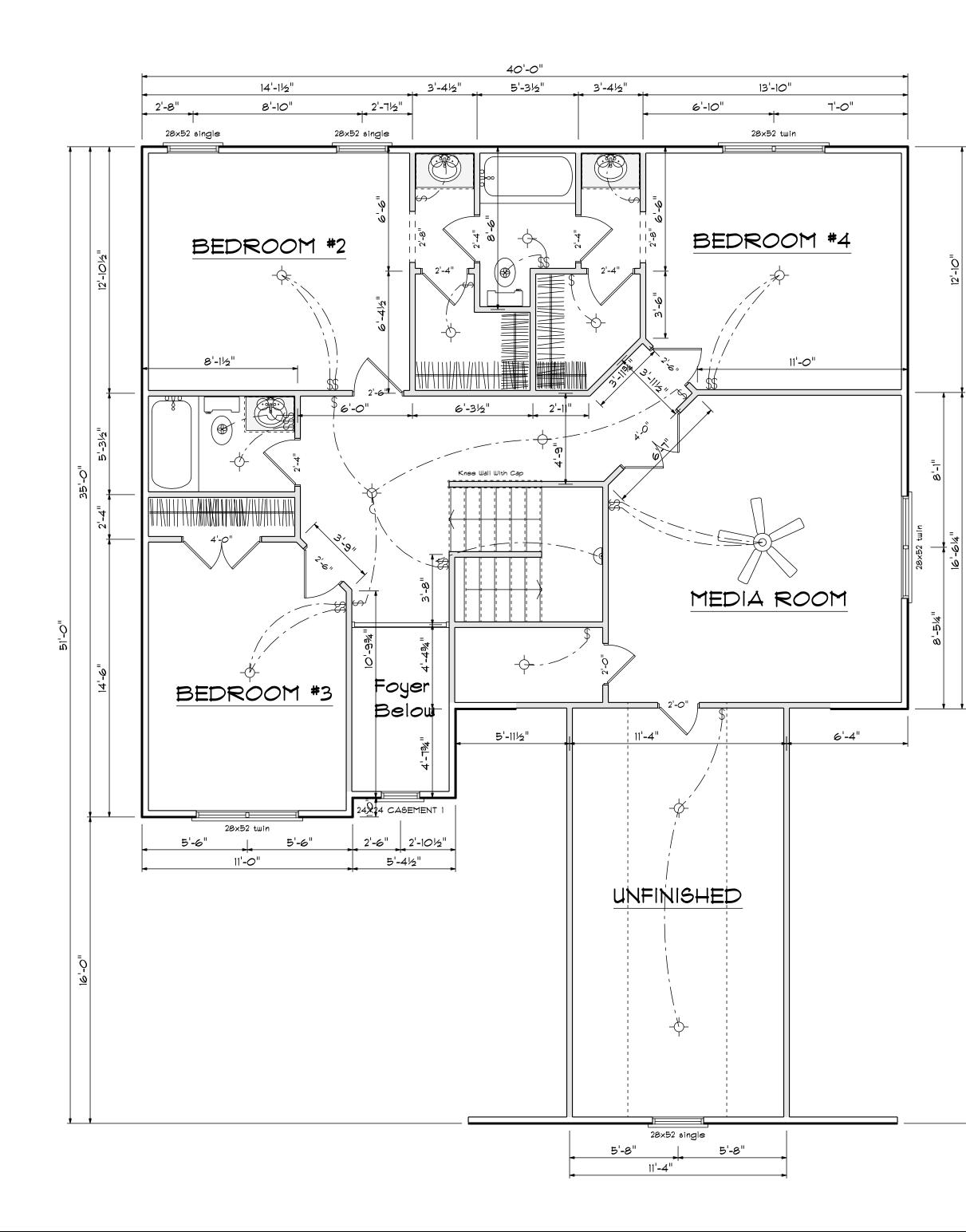




First Floor	1233
Second Floor	1156
	=======
Total Heated	2389
Garage	559
Porch	100
Storage	253
Opt. Garage	240



Scale: 1/4"= 1'-0"



SECOND FLOOR (	OPENING SCH	EDULE	
PRODUCT CODE	SIZE	HINGE	COUNT
2-0 Door Unit	2'-0"	R	2
2-4 Door Unit	2'-4"	R	2
2-4 Door Unit	2'-4"	L	3
2-6 Door Unit	2'-6"	R	2
2-6 Door Unit	2'-6"	L	1
4-0 Doublehung Door Unit	4'-0"	LR	2
24X24 CASEMENT 1	2'-0" x 2'-0"	N	1
28x52 single	2'-8" x 5'-2"	N	3
28x52 twin	5'-4" x 5'-2"	NN	3

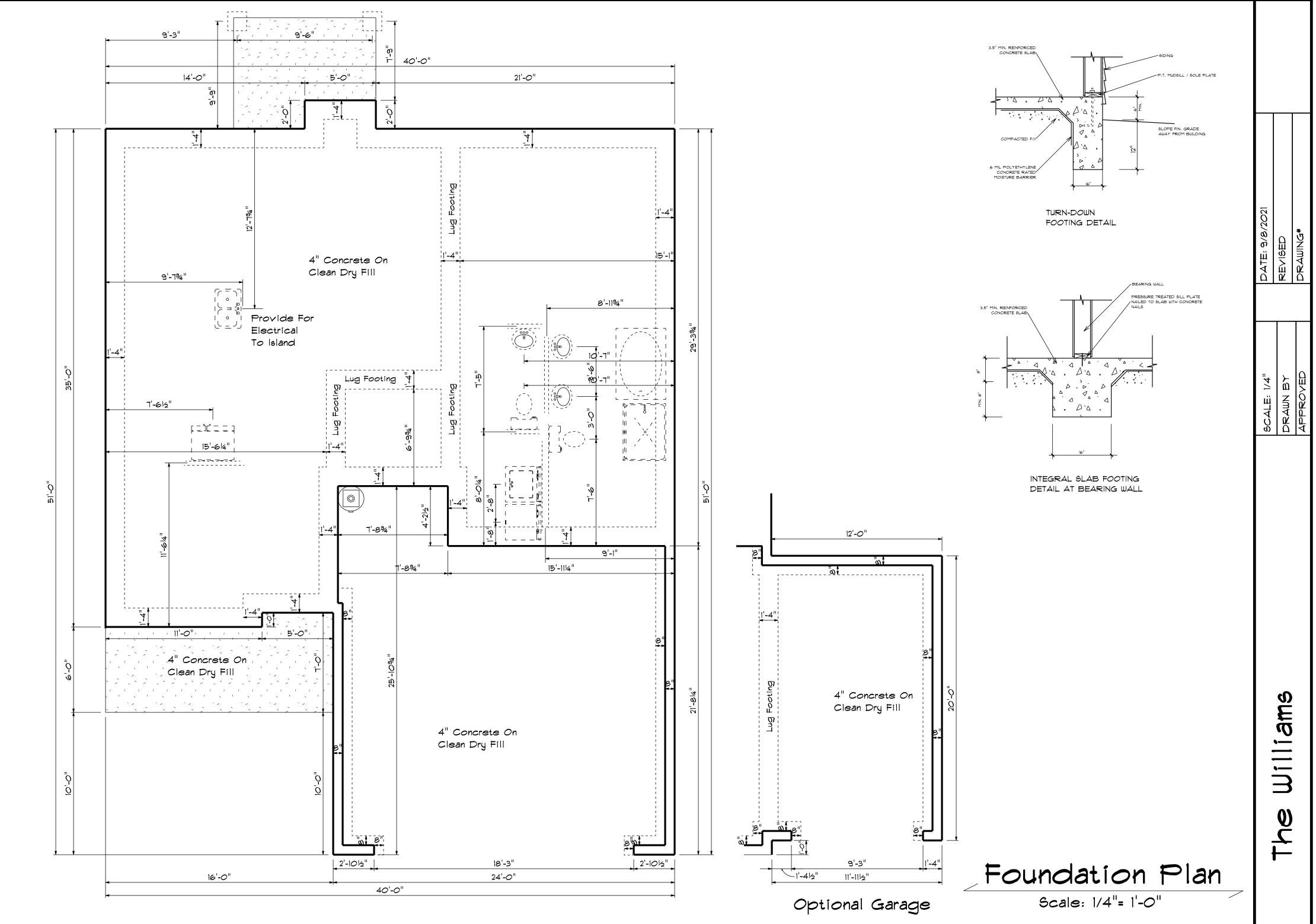
29'-4¼'

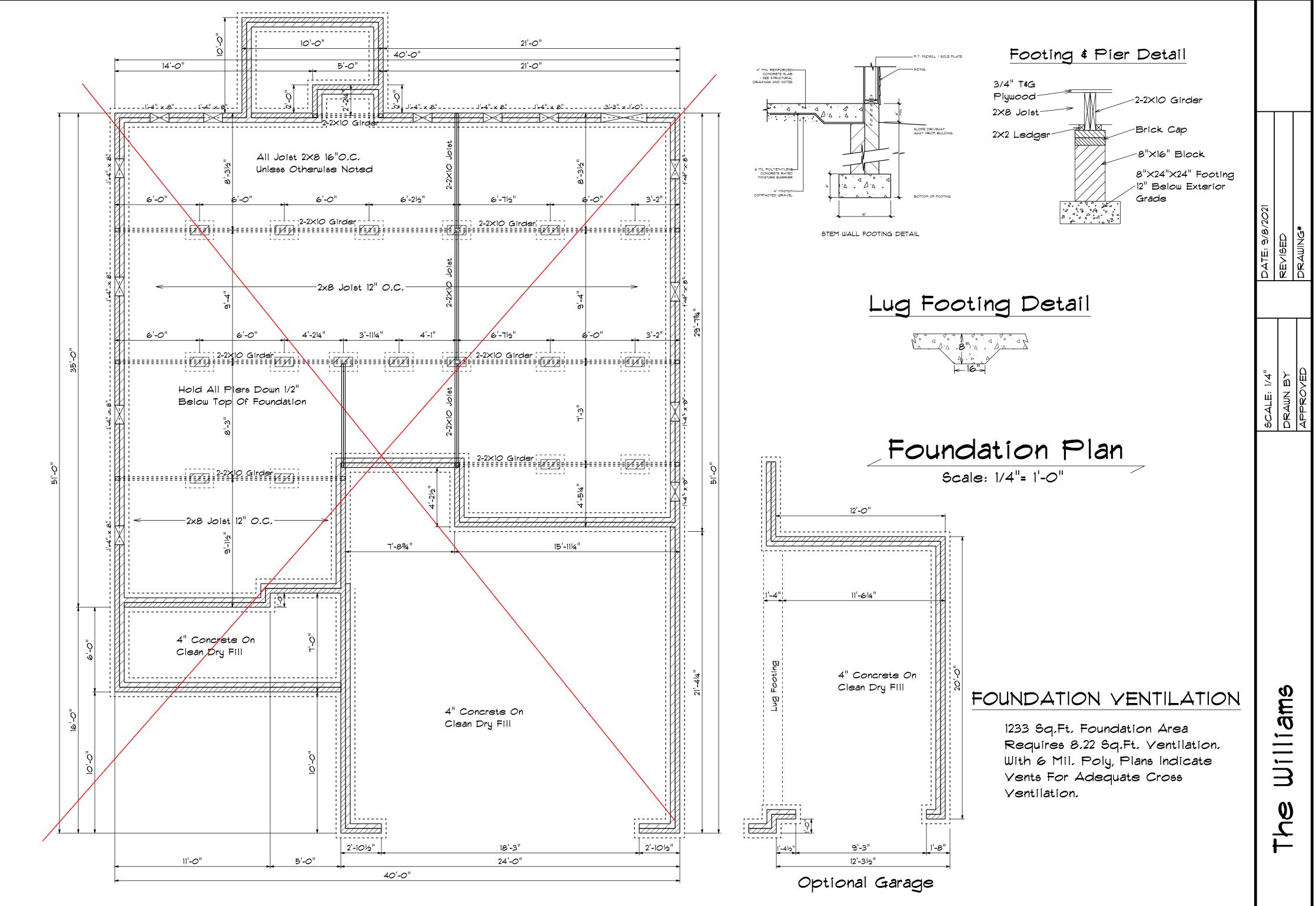
<u>5</u> 0

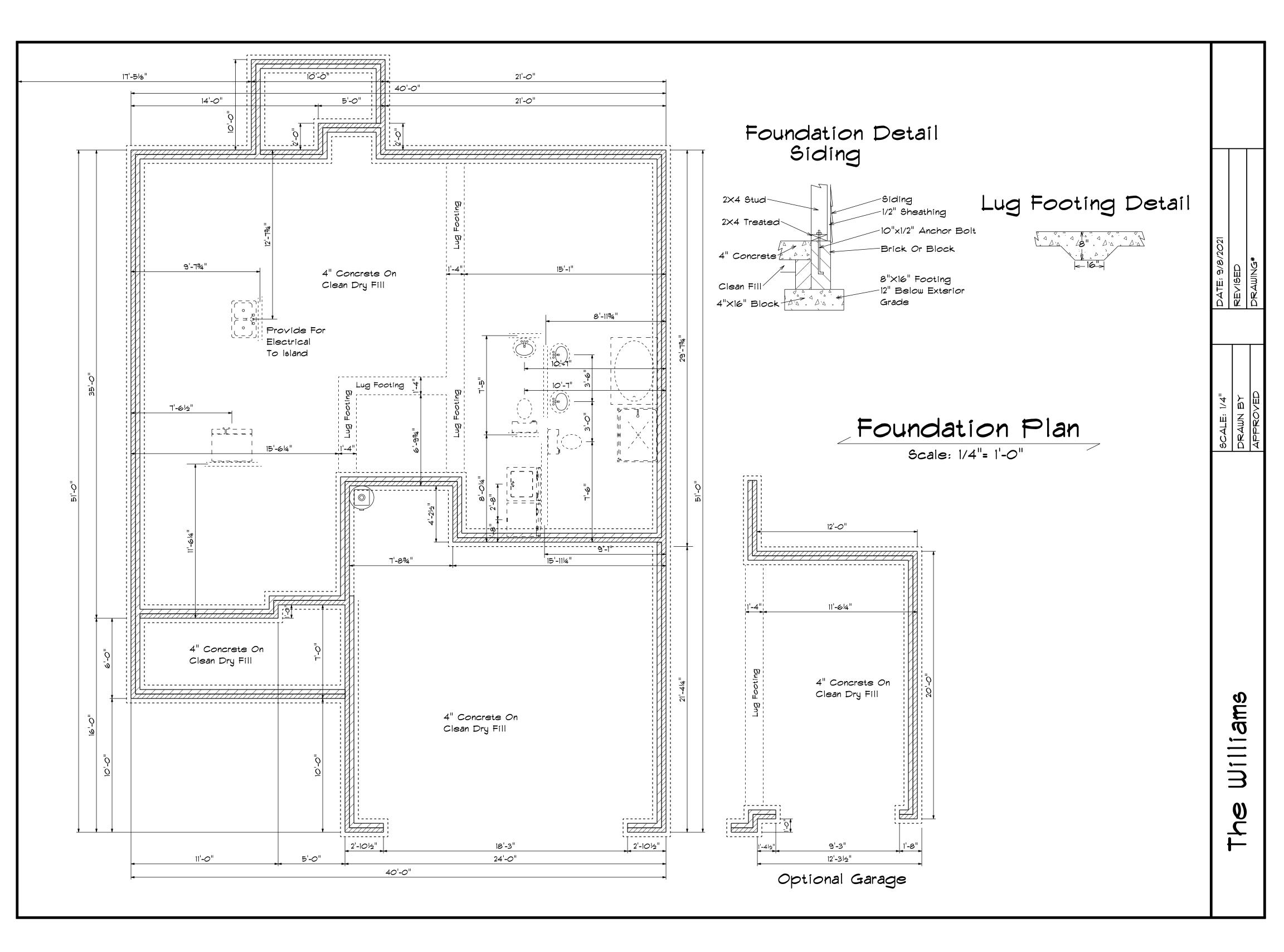
21'-7%

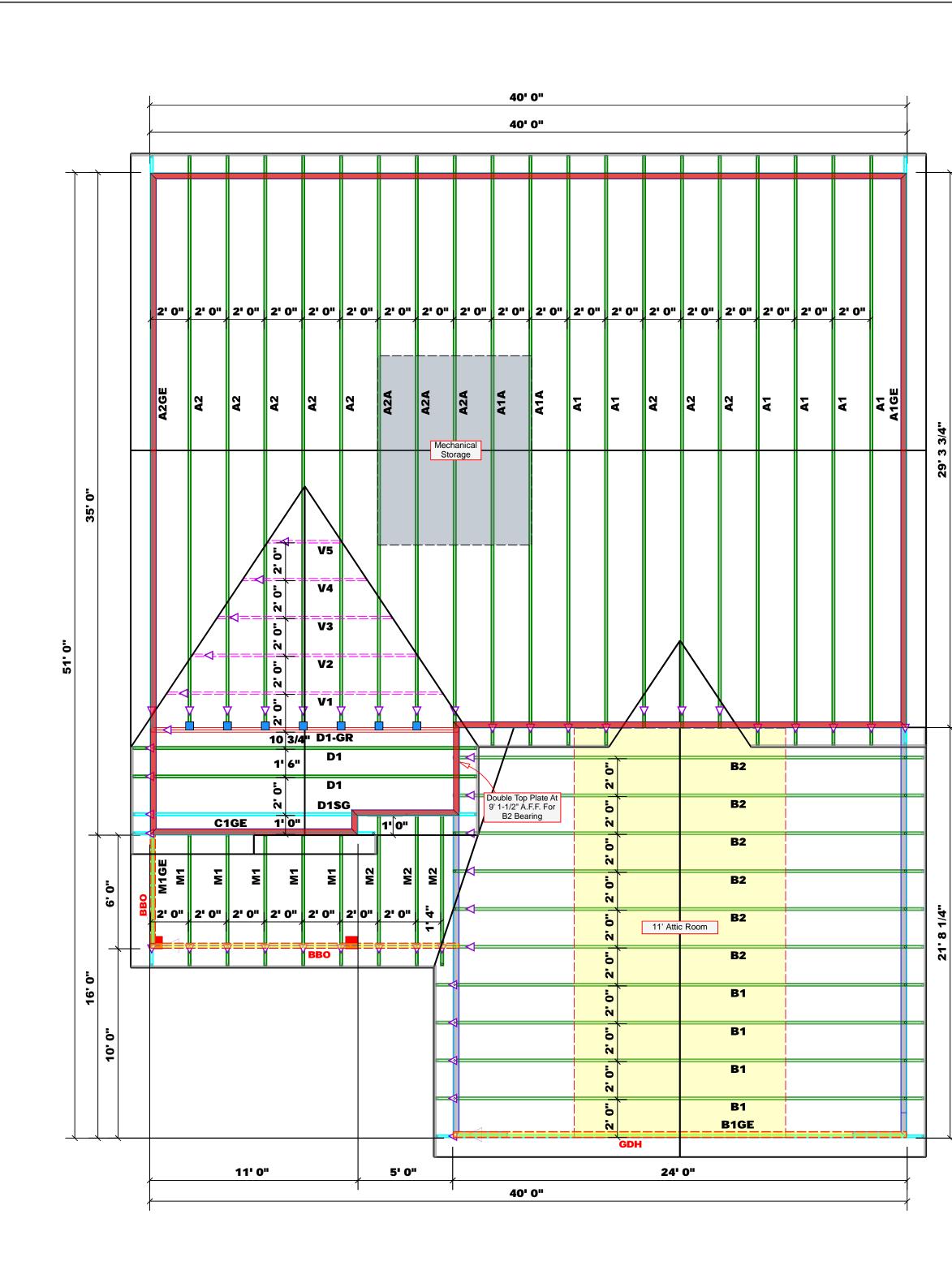
SCALE: 1/4"	DATE: 9/8/2021	
DRAWN BY	REVISED	
APPROVED	DRAWING#	

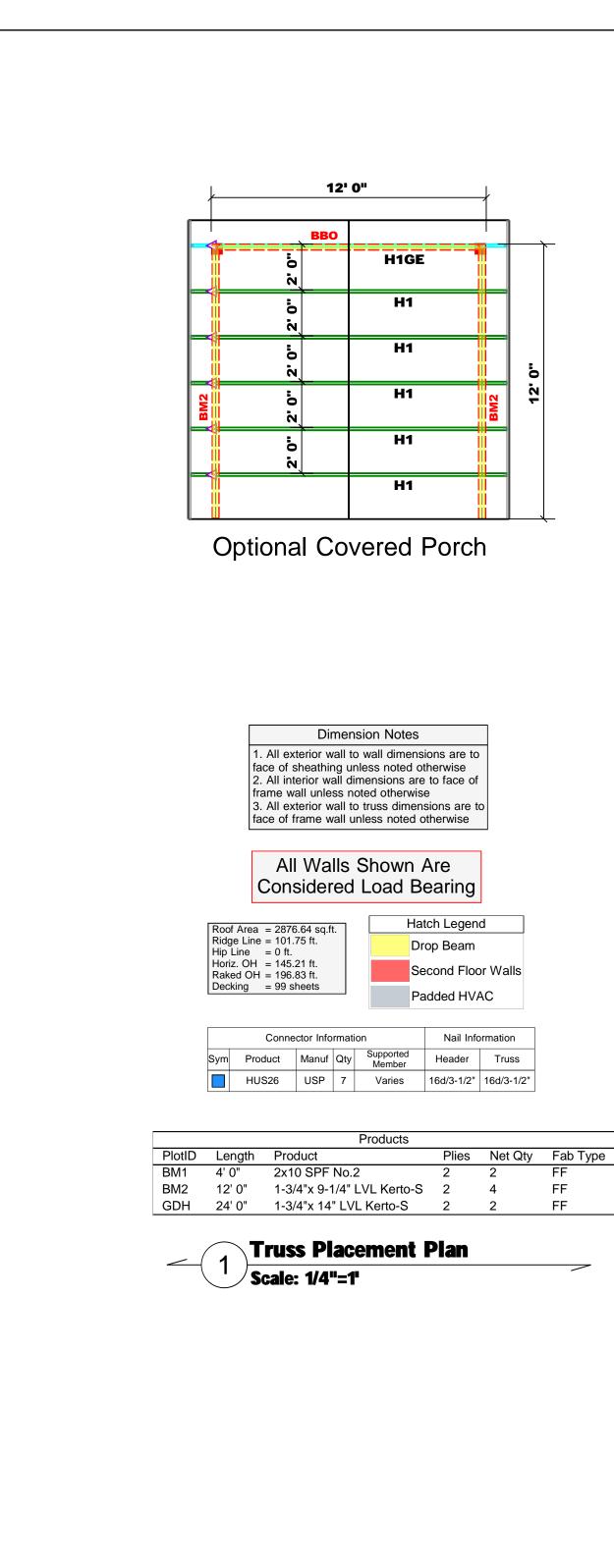
Second Floor Plan











0

51.

These to comport design See ind identifie designe perman for the	<b>Benjamin Stout Real Estate</b>	COUNTY	Fayetteville / Cumberland	deeme requirt attach Code I founda requirt but no profes suppo those registe design exceed	Т
russes ar nents to b at the spe lividual de ed on the er is respe ent braci	E Lot 4 Cypress Road	ADDRESS	Cypress Road	d to com ements. T ed Tables requiremention size ad to sup t greater sional sh rt system specified red design the supp d 15000#. Signature	RO RUS Reilly F Fayet Phon
e designe e incorpo ecification esign she placemen onsible fo ng of the ructure. 1	The Williams / 2GRF, CP	MODEL	Roof	ply with t he contra ( deriver and num port react for any r in the att for any r in the att port syste Davi ART F( b on TABL S(K STUD)	OF & SES load li teville
<b>EAL DATE</b> <b>NUMERATION</b>	E N/A	DATE REV.	03/10/22	2 0 1 0 2 0 3 0 4 0 5	<b>Te</b> & FL & B ndustr 2, N.C. 0) 864-4
vidual bu o the buil uilding de ch truss g. The bu ary and floor syst n of the to		DRAWN BY	DRAWN BY David Landry	iptive Co I refer to prescript the minir od studs iter than 3 istered d lesign the esign the esign the at excee bles. A all be reta reactions	<b>OOI</b> EAN 28309 -8787
ilding ding esigner. design ilding tem and russ	J0322-1267	SALESMAN	SALESMAN Marshall Naylor	de the the tive num       30000# esign ods       inned to that       JDS       001 State       001 State       001 1       000 3       000 4	<b>∕IS</b> rk

and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.cor

 $\geq$ 



RE: J0322-1267 Lot 4 Cypress Road Trenco 818 Soundside Rd Edenton, NC 27932

Truss Name

V3 V4

V5

Date 9/14/2021

9/14/2021

9/14/2021

Site Information: Customer: Benjamin Stout Real Estate Project Name: J0322-1267 Lot/Block: 4 Model: Williams Address: Cypress Road City: Fayetteville

Subdivision: Cypress Road State: NC

## General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10

Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 23 individual, dated Truss Design Drawings and 0 Additional Drawings.

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19	Seal# E16166663 E16166664 E16166665 E16166666 E16166667 E16166669 E16166669 E16166670 E16166672 E16166673 E16166673 E16166675 E16166676 E16166678 E16166679 E16166680 E16166681 E16166681 E16166682	Truss Name A1 A1A A1GE A2 A2A A2GE B1 B1GE B2 C1GE D1 D1-GR D1SG H1 H1GE M1 M1GE M2 V1 V2	Date 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021 9/14/2021	No. 21 22 23	Seal# E16166683 E16166684 E16166685
---	--	---	---	-----------------------	--

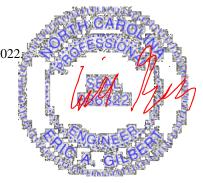
The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

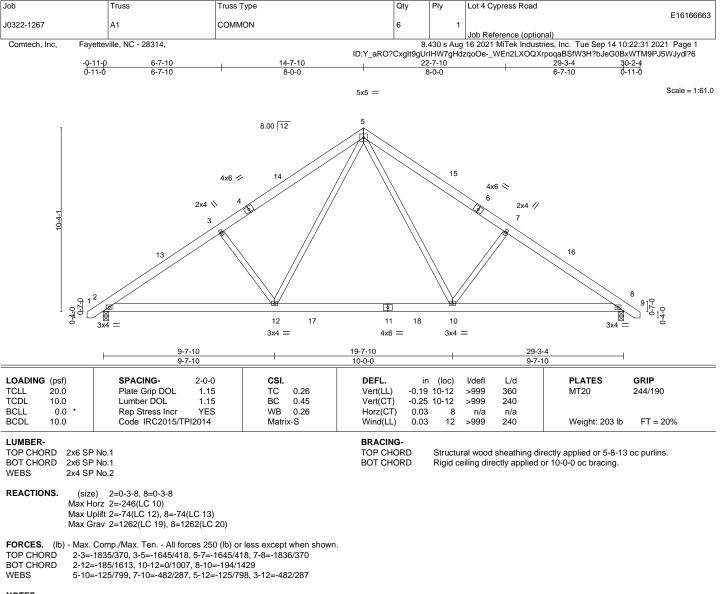
My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

September 14, 2021



#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 14-7-10, Exterior(2) 14-7-10 to 19-0-7, Interior(1) 19-0-7 to 30-0-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

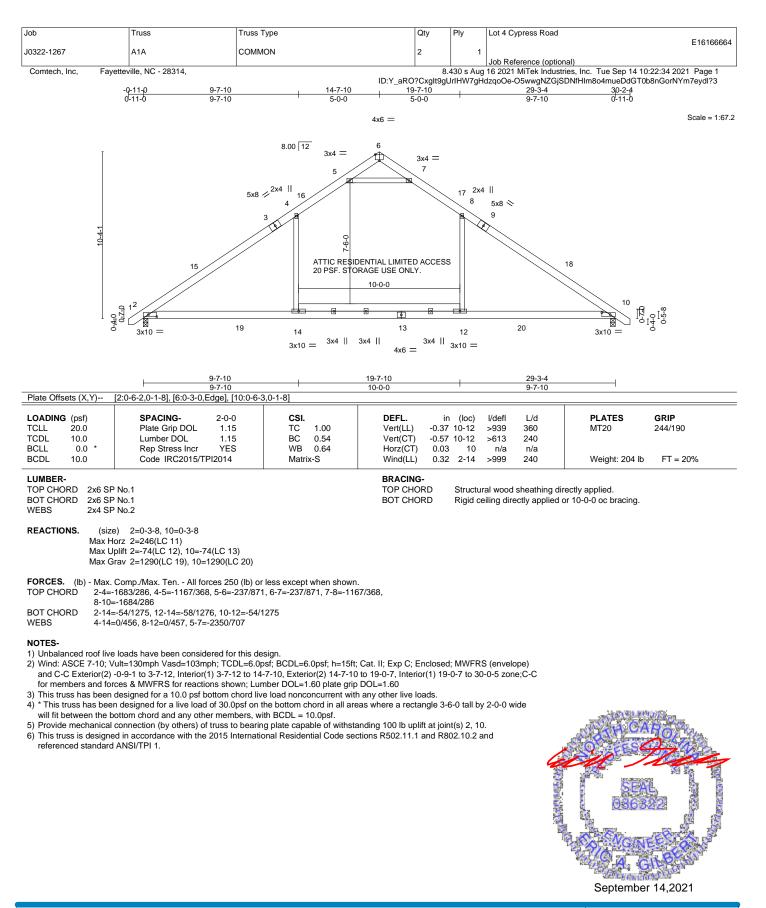
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



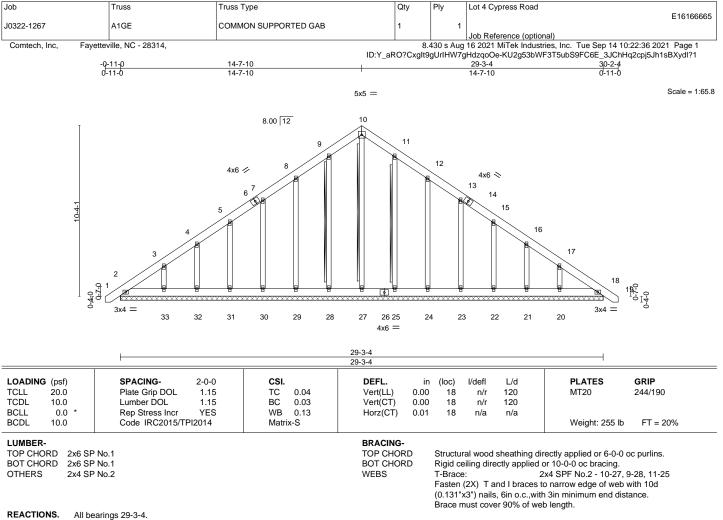
meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE 🛕 WARNING - Verify design pa Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





(lb) -Max Horz 2=307(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 28, 29, 30, 31, 32, 25, 23, 22, 21, 18 except 33=-120(LC 12), 24=-101(LC 13), 20=-118(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 27, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-300/229, 9-10=-233/262, 10-11=-233/262 TOP CHORD

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

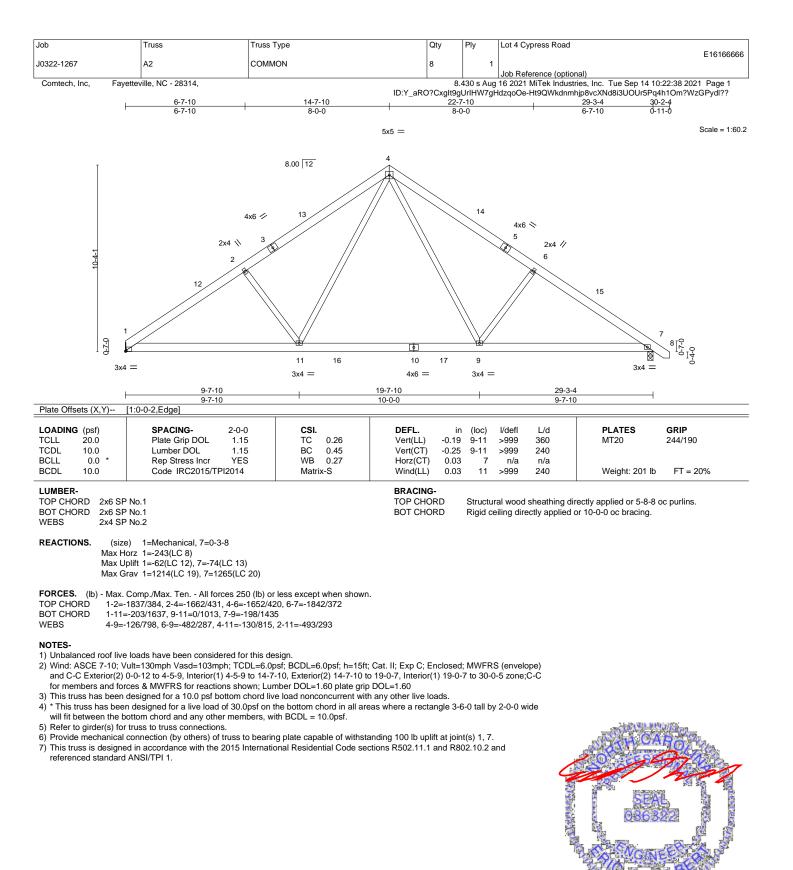
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 28, 29, 30, 31, 32, 25, 23, 22, 21, 18 except (jt=lb) 33=120, 24=101, 20=118.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



September 14,2021

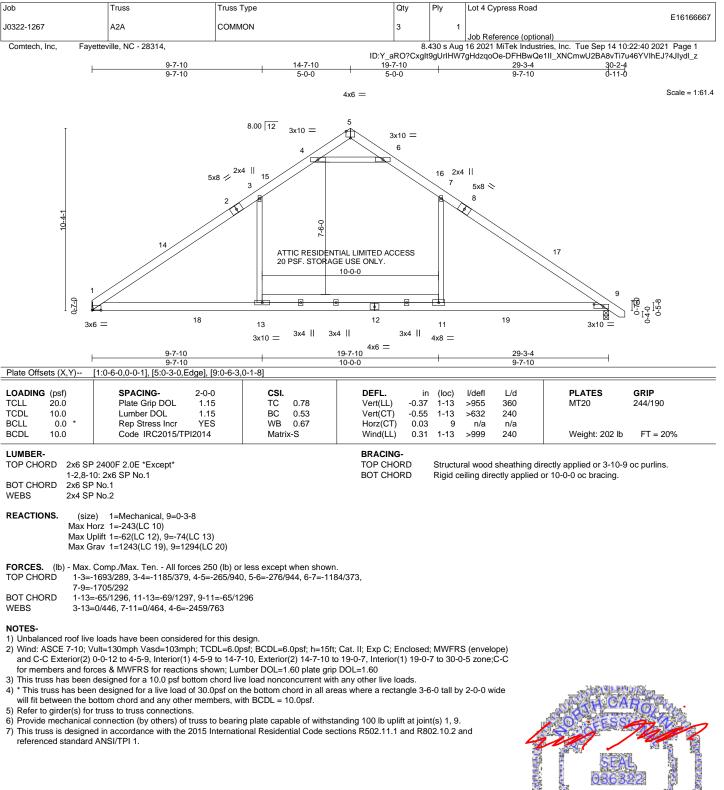






WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

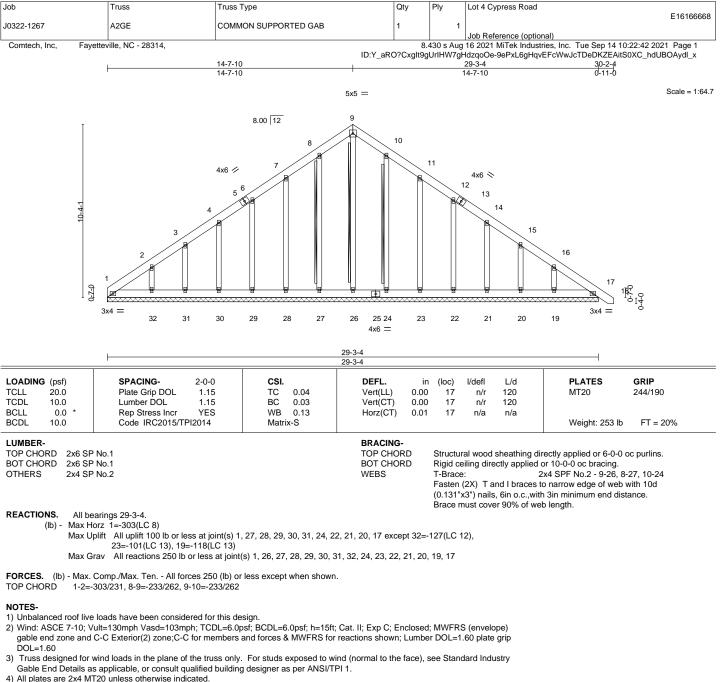






meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE 🛕 WARNING - Verify design pa Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 27, 28, 29, 30, 31, 24, 22, 21, 20, 17 except (jt=lb) 32=127, 23=101, 19=118.

10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

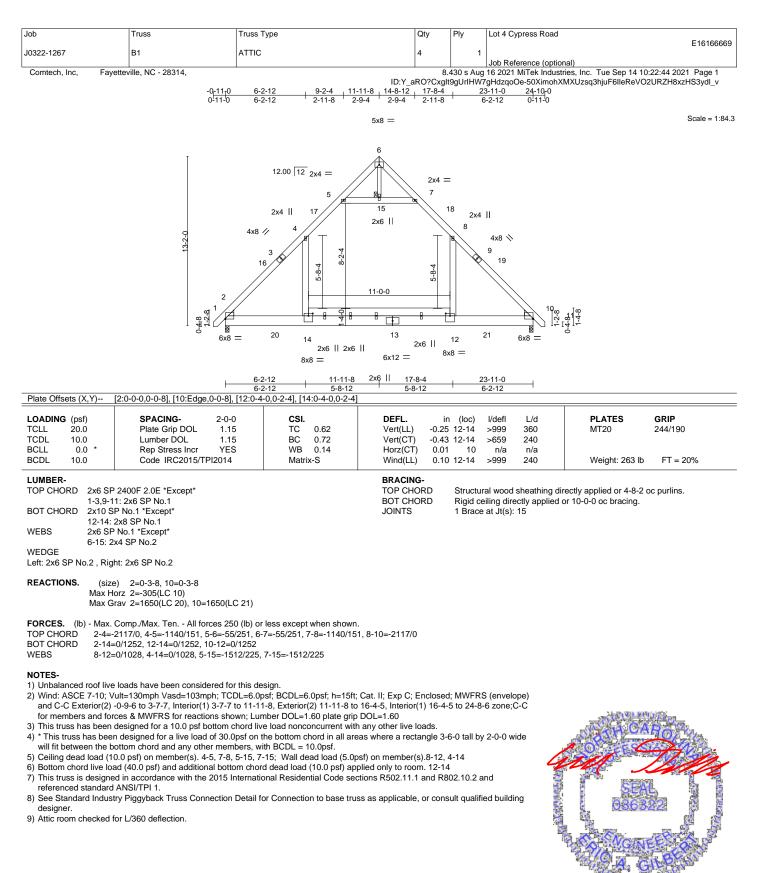
11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



September 14,2021

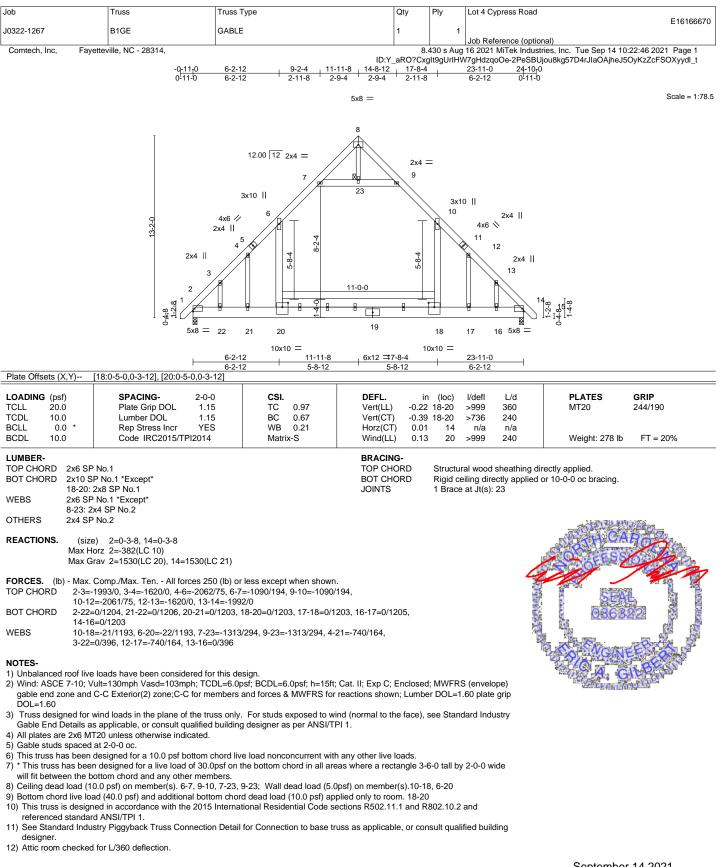


🗥 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



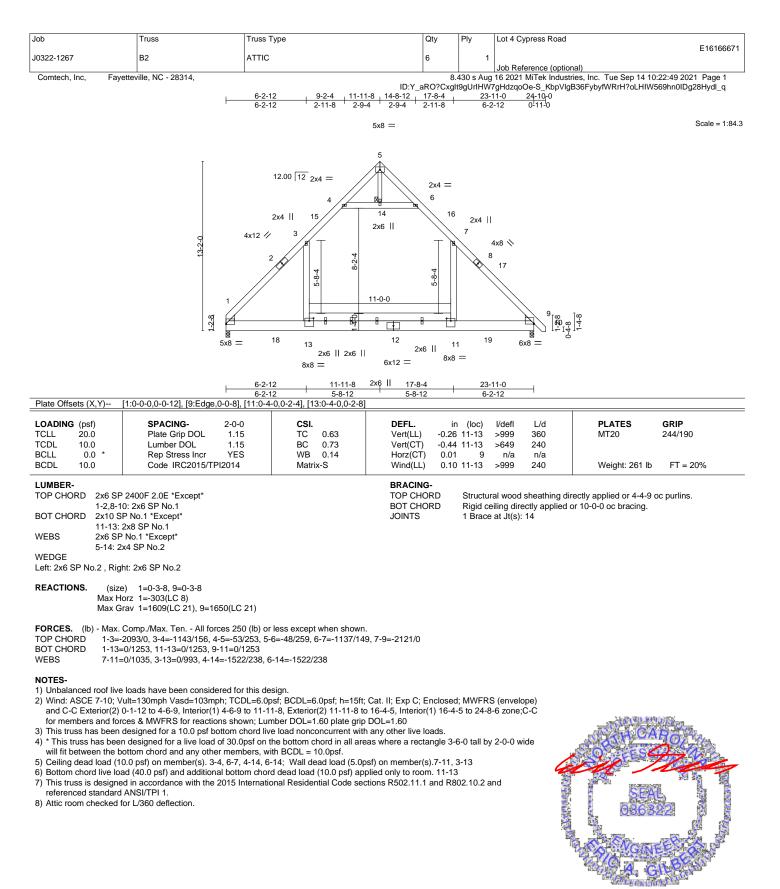
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





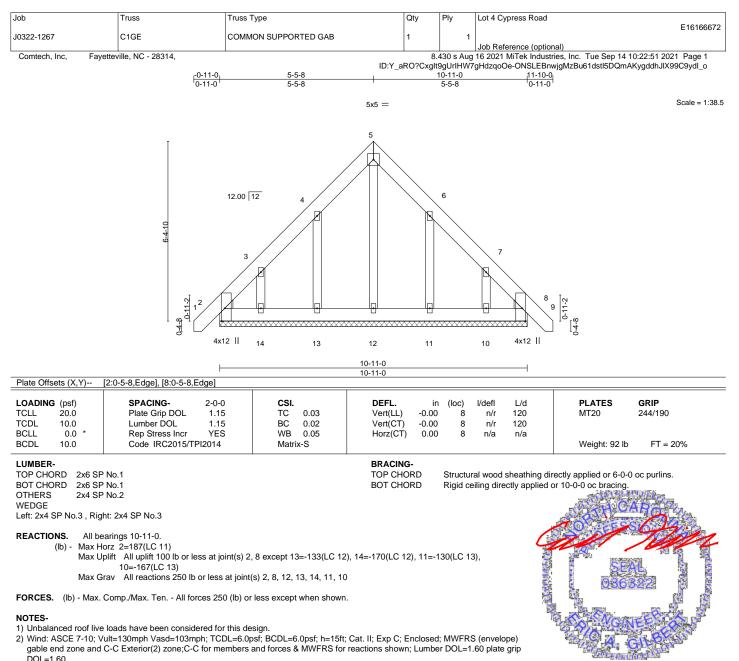
🛕 WARNING - Verify design p eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 September 14,2021





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

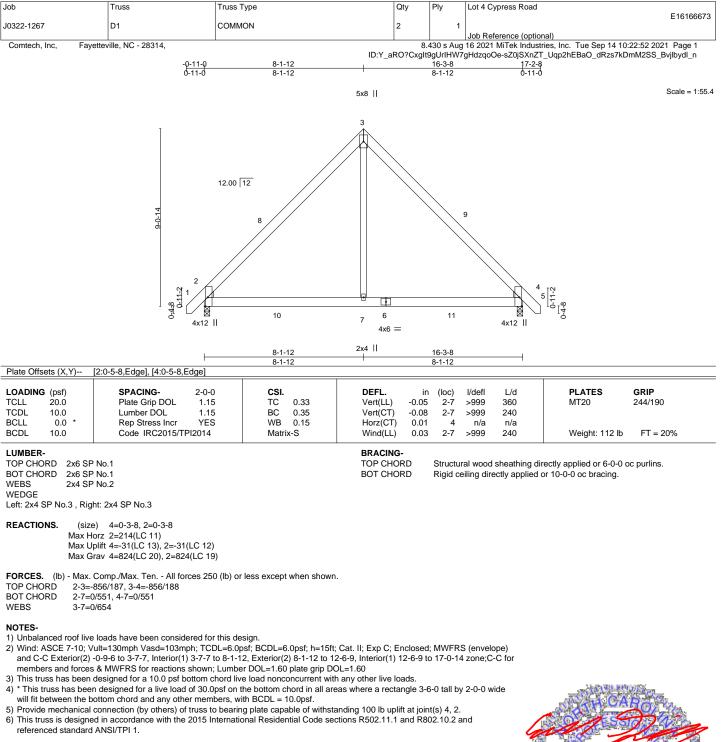




- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8 except (jt=lb) 13=133, 14=170, 11=130, 10=167.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

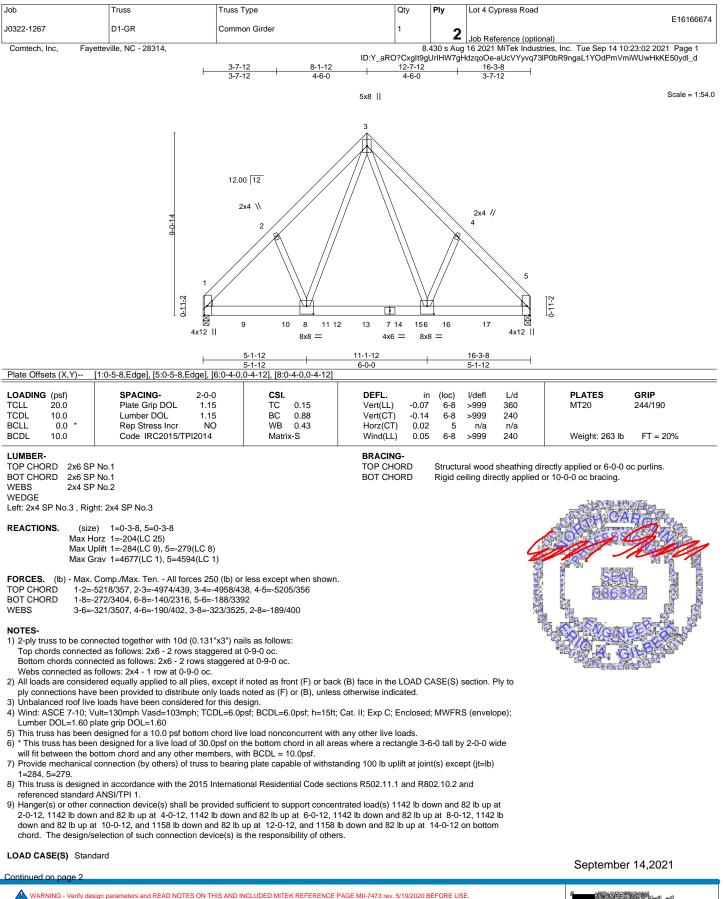






WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITER KEERENCE PAGE MIT-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITeR® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings of individual truss systems, see ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 4 Cypress Road
J0322-1267	D1-GR	Common Girder	1		E16166674
JU322-1207	DI-GR	Common Gilder	1	2	Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,				16 2021 MiTek Industries, Inc. Tue Sep 14 10:23:03 2021 Page 2

ID:Y\_aRO?CxgIt9gUrlHW7gHdzqoOe-2gAtllwSuMtGek0LKN5aalwo99r?Rzk4WO3odTydl\_c

### LOAD CASE(S) Standard

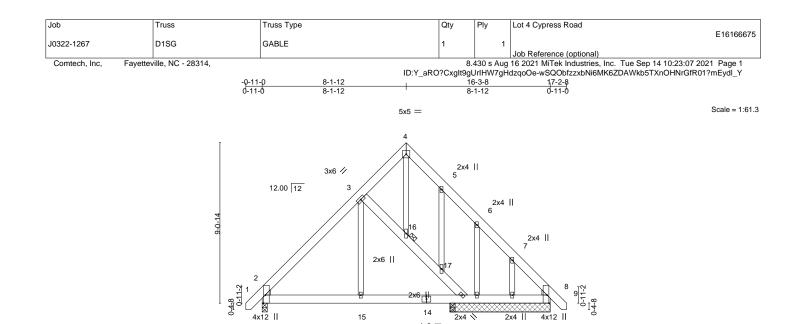
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20

Concentrated Loads (lb) Vert: 9=-1142(B) 10=-1142(B) 11=-1142(B) 13=-1142(B) 15=-1142(B) 16=-1142(B) 17=-1142(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design properly incorporet his design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





4x6 =

2x4 ||

13 12 11

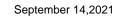
2x4 ||

10

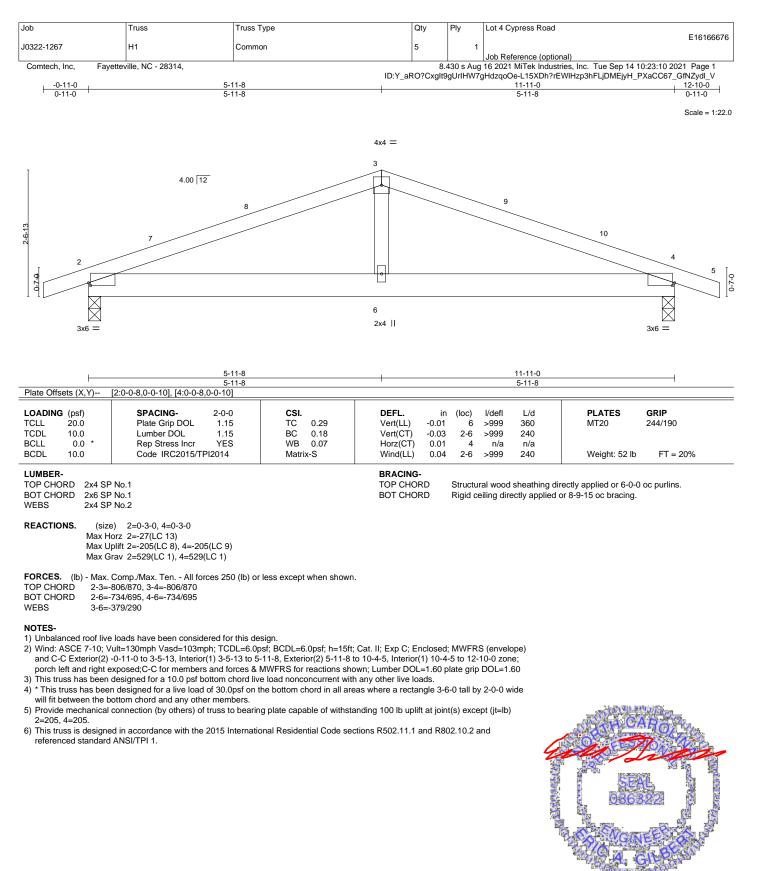
		<b> </b>	5-6-15 5-6-15		<u>10-11-0</u> 5-4-1		<u>16-3</u> 5-4				
Plate Offsets (X,Y)	[2:0-5-8,Edge], [8:0-5-8,Edge]	lge]								-	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI	2-0-0 1.15 1.15 YES 2014	BC (	).13 ).09 ).08	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.01 -0.01 0.00		l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 150 lb	<b>GRIP</b> 244/190 FT = 20%
					. ,				2.10		
	No.1 No.1 *Except* x4 SP No.2				BRACING TOP CHOP BOT CHOP JOINTS	RD RD	Rigid c		ectly applied	directly applied or 6-0-0 d d or 10-0-0 oc bracing.	oc purlins.
eft: 2x4 SP No.3 , Rig	ht: 2x4 SP No.3									39400	100
(Ib) - Max H Max U	arings 5-8-0 except (jt=ler orz 2=-267(LC 10) plift All uplift 100 lb or les 13), 10=-194(LC 13) rav All reactions 250 lb c 8=272(LC 22), 11=25	s at joint(s) 2 or less at joint(	except 12=-28 (s) 12, 10 exce	( ),	,						2 A
TOP CHORD 2-3=- BOT CHORD 2-15=	Comp./Max. Ten All forc 511/53, 7-8=-369/247 98/380, 13-15=-98/380, 1 207/309								and and a		
WEBS 3-16=	-438/274, 16-17=-383/246	6, 12-17=-467	/285							A G	TBE SS
2) Wind: ASCE 7-10; V	loads have been consider ult=130mph Vasd=103mpl C-C Exterior(2) zone;C-C	h; TCDL=6.0p	osf; BCDL=6.0							- Alberta	
	wind loads in the plane of t s applicable, or consult qua at 2-0-0 oc					face), s	ee Stan	ndard Ind	ustry		
	designed for a 10.0 psf bo	ttom chord liv	e load noncon	current wi	th any other live loa	ads.					

- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 12=281, 11=163, 10=194.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouckling of individual truss systems, see **AVSUFPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

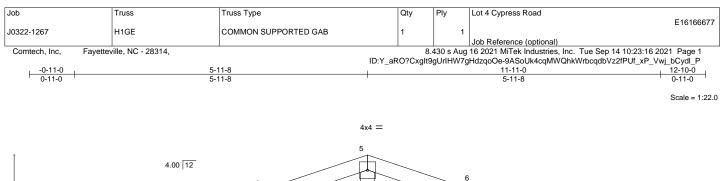


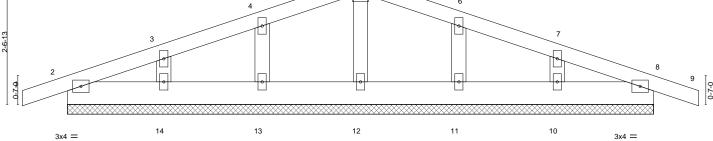




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







		11-11-0 11-11-0										
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.04 BC 0.01	Vert(CT)	in -0.00 -0.00	(loc) 8 8	l/defl n/r n/r	L/d 120 120	PLATES MT20	<b>GRIP</b> 244/190			
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.02 Matrix-S	Horz(CT)	0.00	8	n/a	n/a	Weight: 57 lb	FT = 20%			

.....

#### LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. All bearings 11-11-0.

(lb) - Max Horz 2=-46(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 14, 11, 10.

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



September 14,2021

ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE 🛕 WARNING - Verify design pa ARXING - Verify design parameters and READ NOTES ON THIS AND INCLUDED WITER REFERENCE FAGE mit-14/3 fev. 31 BLC2D BEFORE DEC. Design valid for use only with MiTek® connectors. This design is based only upon parameters and with a parameters and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

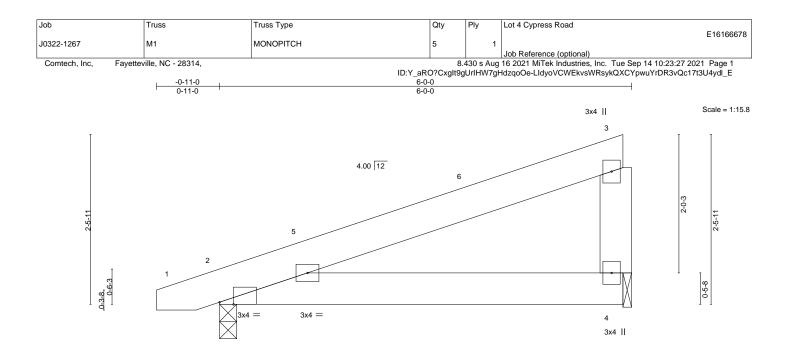


Plate Offsets	(X,Y)	[2:0-2-7,Edge]										
LOADING (p	,	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
	0.0 0.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC BC	0.19 0.12	Vert(LL) Vert(CT)	-0.01 -0.03	2-4 2-4	>999 >999	360 240	MT20	244/190
	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2 1	n/a	n/a		
BCDL 1	0.0	Code IRC2015/T	PI2014	Matrix	k-P	Wind(LL)	0.03	2-4	>999	240	Weight: 34 lb	FT = 20%
LUMBER-						BRACING-					-	
TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1						TOP CHOP		Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals				

2x6 SP No.1

except end vertica BOT CHORD Rigid ceiling direct

except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=72(LC 8)

Max Uplift 2=-105(LC 8), 4=-96(LC 8) Max Grav 2=274(LC 1), 4=223(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

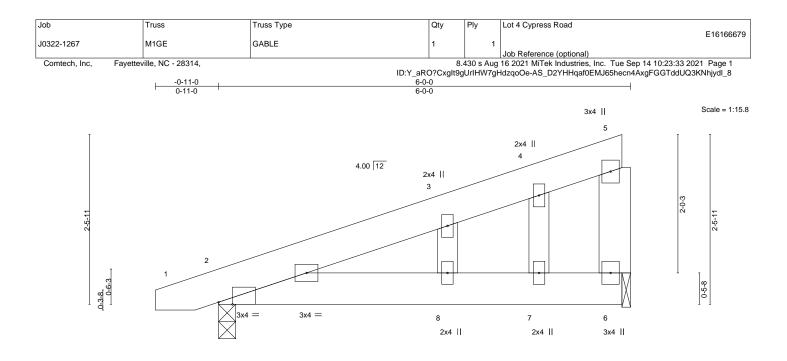
WEBS

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=105.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





OADING (	psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 2	20.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	-0.01	8	>999	360	MT20	244/190
CDL 1	10.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	-0.02	8	>999	240		
CLL	0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	6	n/a	n/a		
SCDL 1	10.0	Code IRC2015/TI	PI2014	Matri	x-S	Wind(LL)	0.01	8	>999	240	Weight: 37 lb	FT = 20%
UMBER-						BRACING-		_				
OP CHOR						TOP CHOP	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.					
WEBS 2x6 SP No.1				BOT CHOR	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.							
THERS	2x4 SP	No.2										

Max Horz 2=102(LC 8) Max Uplift 2=-89(LC 8), 6=-79(LC 12) Max Grav 2=274(LC 1), 6=223(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 1-4-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



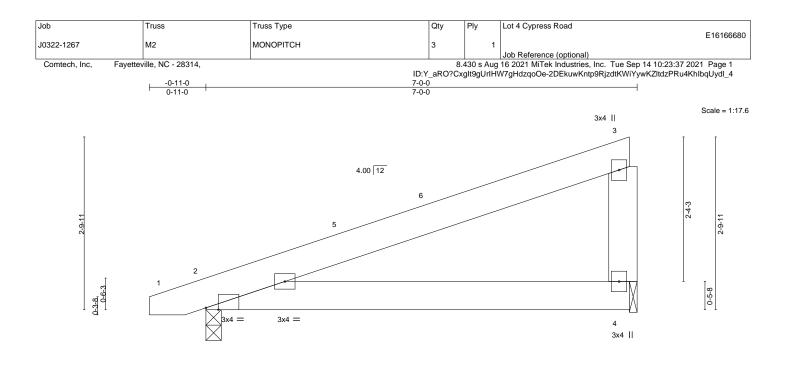


Plate Offsets (X,Y) [2:0-2	2-7,Edge]							1	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.27	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.18	Vert(CT)	-0.05	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00		n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.06	2-4	>999	240	Weight: 40 lb	FT = 20%
LUMBER-			BRACING-						
TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1			TOP CHOR			ral wood end verti	•	rectly applied or 6-0-0	oc purlins,

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=83(LC 8) Max Uplift 2=-119(LC 8), 4=-114(LC 8) Max Grav 2=314(LC 1), 4=263(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 6-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

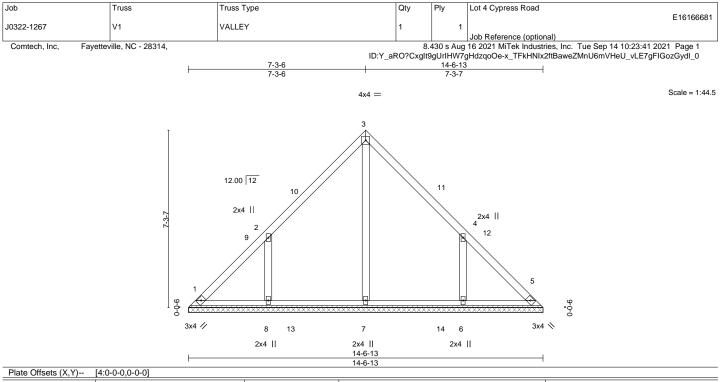
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=119, 4=114.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Standard Road Edenton, NC 27932



LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.15 BC 0.18 WB 0.11 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in ( n/a n/a 0.00	(loc) l/de - n/ - n/ 5 n/	'a 999 'a 999	PLATES GRII MT20 244/ Weight: 69 lb F	
LUMBER- TOP CHORD 2x4 SP	BRACING- TOP CHORE	D Si	tructural we	ood sheathing d	irectly applied or 6-0-0 oc pu	rlins.		

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 14-6-13.

- (lb) Max Horz 1=-166(LC 8)
  - Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-175(LC 12), 6=-175(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=407(LC 22), 8=427(LC 19), 6=427(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. 2-8=-380/298, 4-6=-380/298 WEBS

#### NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 7-3-6, Exterior(2) 7-3-6 to 11-8-3, Interior(1) 11-8-3 to 14-2-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=175. 6=175.

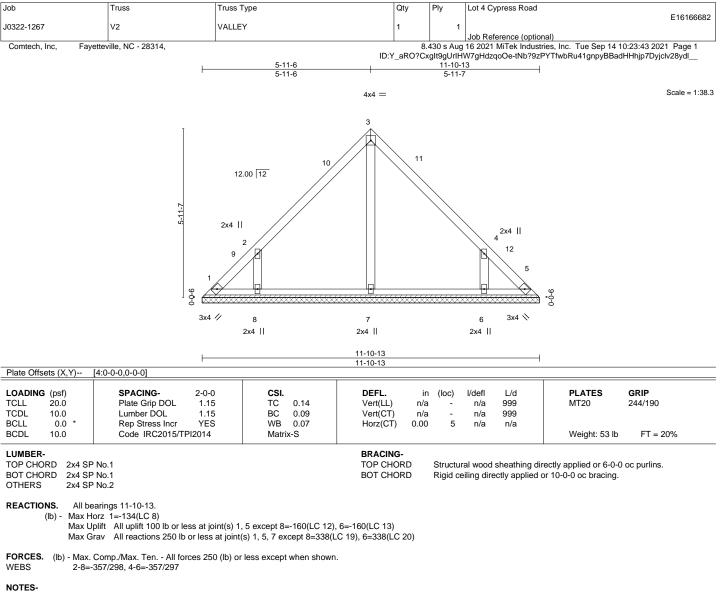
7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-11-6, Exterior(2) 5-11-6 to 10-4-3, Interior(1) 10-4-3 to 11-6-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

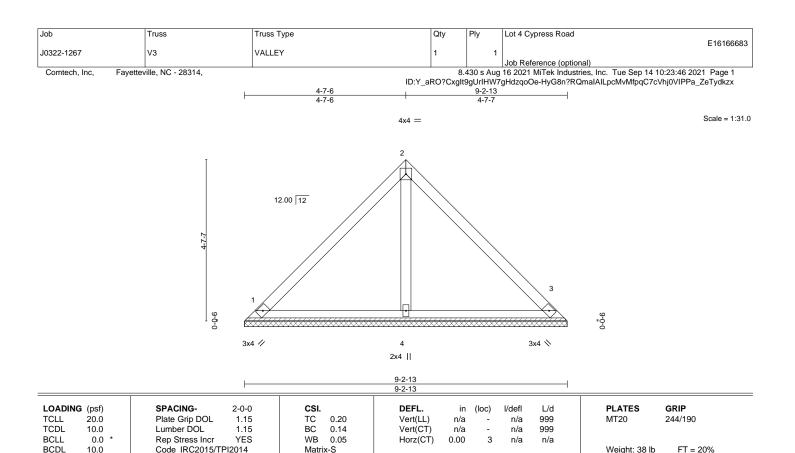
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=160, 6=160.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	1846		•
LU	ם ועו כ	эсг	ς-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=9-2-13, 3=9-2-13, 4=9-2-13 Max Horz 1=102(LC 9) Max Uplift 1=-25(LC 13), 3=-25(LC 13) Max Grav 1=193(LC 1), 3=193(LC 1), 4=296(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

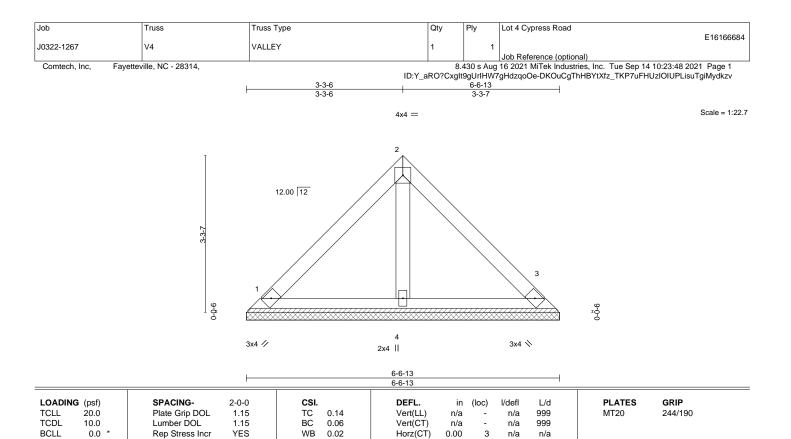
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
  7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LUMBER-	L	JM	BE	R-
---------	---	----	----	----

BCDL

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

10.0

4 SP No.1 4 SP No.1 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 26 lb

FT = 20%

REACTIONS. (size) 1=6-6-13, 3=6-6-13, 4=6-6-13 Max Horz 1=-70(LC 8) Max Uplift 1=-25(LC 13), 3=-25(LC 13) Max Grav 1=143(LC 1), 3=143(LC 1), 4=183(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2015/TPI2014

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-P

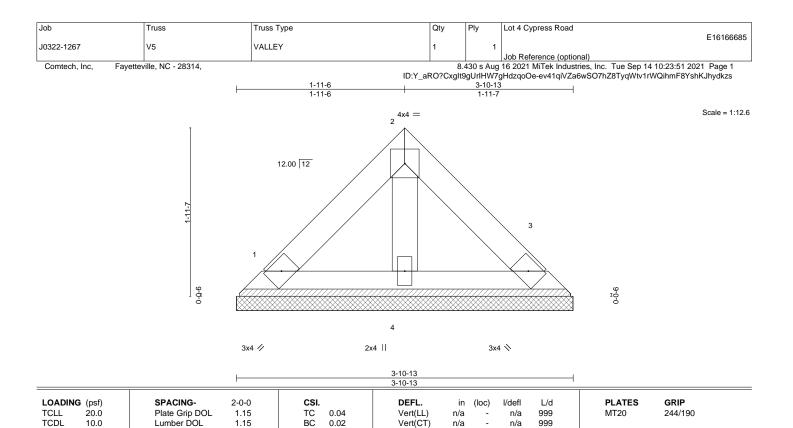
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
  7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LU	MBE	R-
----	-----	----

BCLL

BCDL

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

0.0

10.0

BRACING-TOP CHORD BOT CHORD

Horz(CT)

0.00

3

n/a

n/a

Structural wood sheathing directly applied or 3-10-13 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 14 lb

FT = 20%

REACTIONS. (size) 1=3-10-13, 3=3-10-13, 4=3-10-13 Max Horz 1=-38(LC 10) Max Uplift 1=-14(LC 13), 3=-14(LC 13) Max Grav 1=78(LC 1), 3=78(LC 1), 4=100(LC 1)

Rep Stress Incr

Code IRC2015/TPI2014

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB 0.01

Matrix-P

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

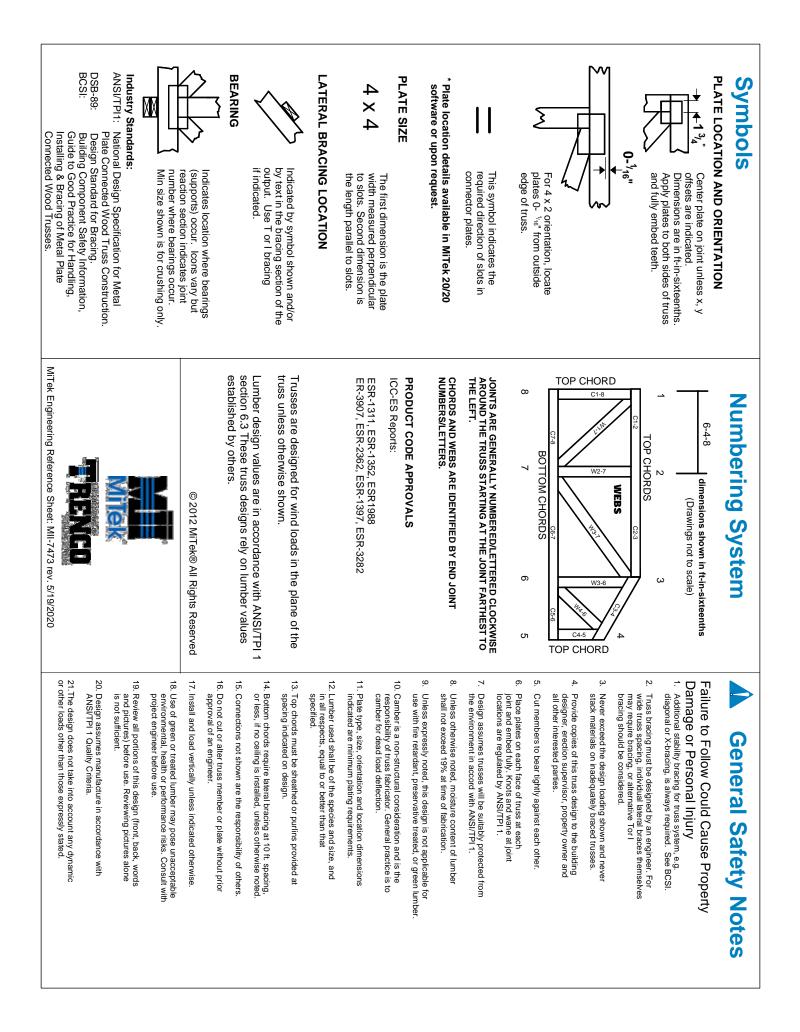
YES

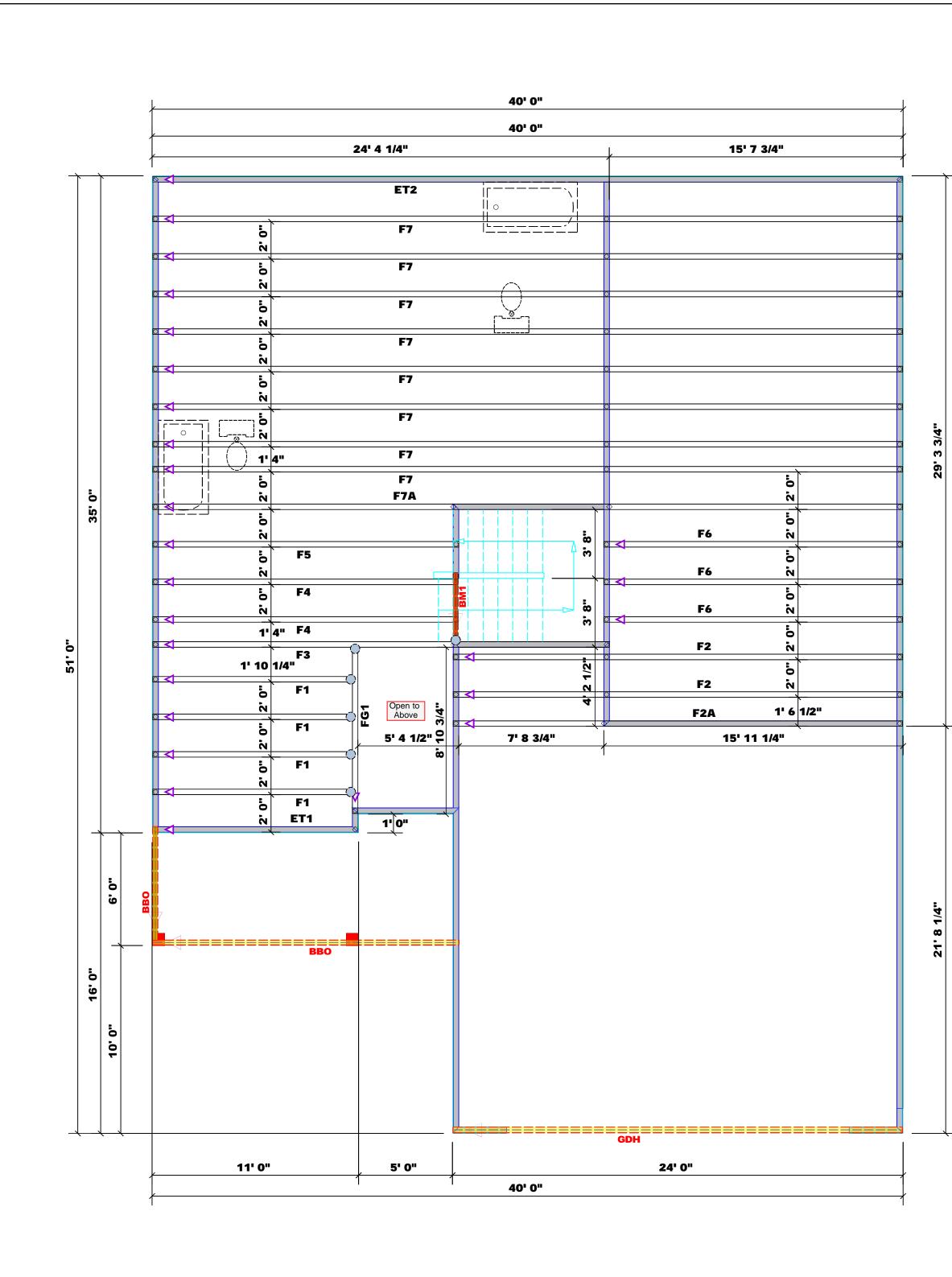
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



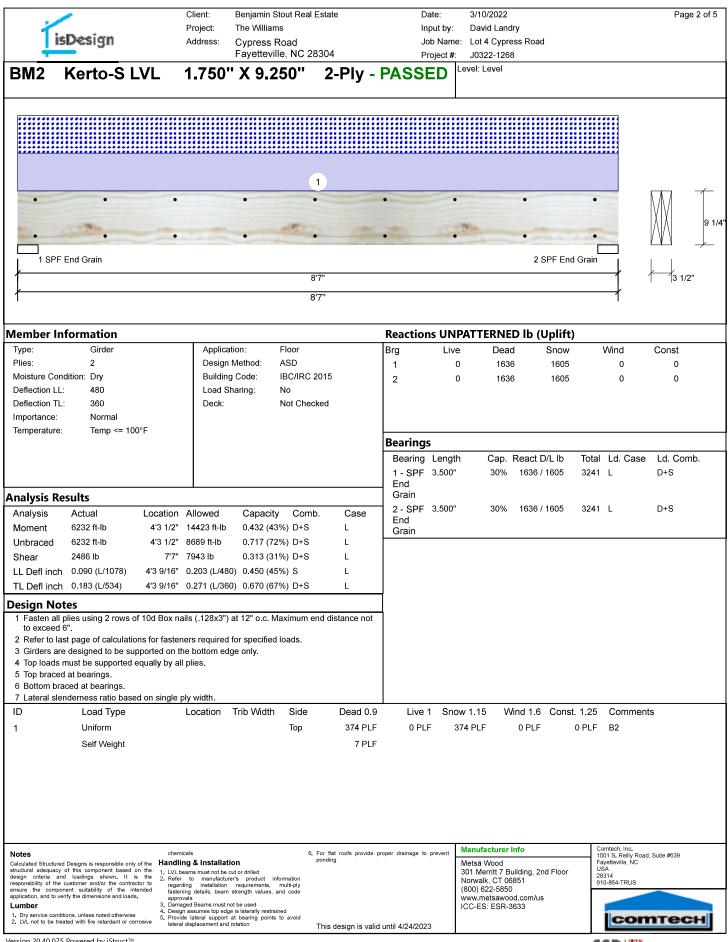




Products           PlotID         Length         Product         Plies         Net Qty         Fab Type           BM1         4' 0"         2x10 SPF No.2         2         2         FF           BM2         12' 0"         1-3/4"x 9-1/4" LVL Kerto-S         2         4         FF           GDH         24' 0"         1-3/4"x 14" LVL Kerto-S         2         2         FF           Truss Placement Plan           Scale: 1/4"=1'	Real Estate		2GLF, CP			
Connector InformationSymProductManufQtySupported MemberHeaderTrussImage: MSH422USP6Varies10d/3"10d/3"	COUNTY	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALESMAN
<ol> <li>All exterior wall to wall dimensions are to face of sheathing unless noted otherwise</li> <li>All interior wall dimensions are to face of frame wall unless noted otherwise</li> <li>All exterior wall to truss dimensions are to face of frame wall unless noted otherwise</li> </ol> All Walls Shown Are Considered Load Bearing           Plumbing Drop Notes           1. Plumbing drop locations shown are NOT exact.           2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.           3. Adjust spacing as needed not to exceed 24"oc.	Fayetteville / Cumberland	Cypress Road	Floor	03/10/22	David Landry	Marshall Naylor
Dimension Notes		WEEK OF 2 B) 4 9001 5 0,028 1 2 3 4 5 6 7 8	Ack 3105: HE405 255 5100 765 1020 1275	S REQUIRE SAGDORE SAGDORE SAGDORES SAGD	00117438 012 34 688 102 136	(UP TO) REQ15 STILES FOR (4) FLV HEADER
	Code i founda requirin but no profes suppo those registe design exceed	requirementation size ed to sup to greater sional sh rt system specified ared desin the sup d 15000#. Signature	ents) to de and num opport reac than 150 hall be ret n for any l in the at gon profes port syste	determine nber of w titions gre 000#. A re tained to reaction f tached T ssional sh em for all id La	ndry	mum 3000# lesign e sds ained to s that
	F	Reilly F Fayet Phor Fax	Road I tteville he: (910)	<b>5 &amp; E</b> Indust e, N.C. 10) 864- ) 864-	.001 BEAN : 28309 4-8787 4444 uual to 300 rriptive Co ill refer to	MS rk

51' 0"

			Client:	Beniamin Sto	out Real Estat	e	D	ate:	3/10/2022				Pa	ge 1 of
- 7			Project:	The Williams				put by:	David Landı	У				.g- · · ·
	isDesign	,	Address:	Cypress R	oad e, NC 28304	1			Lot 4 Cypre					
	S-P-F #1	2.00						roject #:	J0322-1268 evel: Level					
BM1	<b>Э-Р-г</b> #1	2.00	JU X	10.000	Z-P	'iy - P/	ASSE	ן נ						
					-			I						
		1												,
													M	1
													IXIX	9 1
1	C. The												MM	5
														<u> </u>
1 SF				2 SPF	l									
1	:	3'8"			1								1 13"	
1	;	3'8"			1									
	Information Girder		Appliq	ation	loor					D lb (Uplift	-	Alizad	Canat	
Type: Plies:	2		Applica Design		ASD		Brg 1	Live 592	Dead 198			Wind 0	Const 0	
	ondition: Dry		Buildin	g Code: I	BC/IRC 2015		2	592	198			0	0	
Deflection				0	No									
Deflection Importance			Deck:	r	Not Checked									
Temperatu		=												
							Bearing							
							Bearing	-		React D/L lb		Ld. Case	Ld. Co	mb.
							1 - SPF 2 - SPF		18% 18%	198 / 592 198 / 592	790 790		D+L D+L	
nalysis I							-	0.000	1070	1007002	100	L	DIL	
Analysis		Location		Capacity	Comb.	Case								
Moment Unbraced	555 ft-lb 555 ft-lb		3431 ft-lb 3338 ft-lb	0.162 (16% 0.166 (17%		L								
Shear	359 lb		2498 lb	0.144 (14%		L								
LL Defl ind		1'10"	0.080 (L/48	30) 0.030 (3%	) L	L								
TL Defl in	(L/13850) ch 0.004	1'10"	0 160 (1 /24	10) 0.020 (2%)	) D+l	1								
TE Dell'III	(L/10380)			10) 0.020 (270)		-	]							
Design N				las este			1							
	are designed to be support plies must be fastened to				5.									
	ls must be supported equ	ually by all p	plies.											
	ced at bearings. braced at bearings.													
	slenderness ratio based o													
ID 1	Load Type Uniform	I	Location	Trib Width	Side	Dead 0.9 108 PLF	Live 323 PL	1 Snov	v 1.15 VVI 0 PLF	nd 1.6 Cons 0 PLF	st. 1.25 0 PLF		tS	
1	Uniform				Тор	100 PLF	323 PL	Г	UPLF	UPLF	UPLF	Г4		
								H	Manufacturer l	nfo	10	mtech, Inc. 01 S. Reilly Roac yetteville, NC	, Suite #639	
											US 28	A 314		
											91	0-864-TRUS		
											1			
					This	design is valid	until 4/24/202	3				con	пте	СН



Version 20.40.075 Powered by iStruct™

CSD 🚟

isDesign	Project: Th Address: Cy	njamin Stout Real Estate e Williams /press Road ayetteville, NC 28304	Date: Input by: Job Nam Project #	3/10/2022 David Landry e: Lot 4 Cypress Road : J0322-1268	Page 3 of 5
BM2 Kerto-S LVL			ly - PASSED	Level: Level	
• •	•	•	•	•	•
	•	• •	•	•	•
1 SPF End Grain				2 SPF End G	Grain
		8'7"			3 1/2"
		8'7"			Ι
Yield Limit per Foot 163	% PLF 3.7 PLF 9 lb. /2"	<u>(8x3") at 12" o.c.</u> . Max	imum end distance n	ot to exceed 6"	
design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber	IVL beams must not be cut or Refer to manufacturer's regarding installation req fastening details, beam stren approvals     Damaged Beams must not be Design assumes top edge is la Provide lateral support at be	ponding drilled product information uirements, multi-ply gth values, and code used teraily restrained aring points to avoid	s provide proper drainage to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
Version 20.40.075 Powered by iStruct™	lateral displacement and rotation	This desig	n is valid until 4/24/2023		

CSD 🗱

is	Design	F	Project: Address:	The Williams Cypress Re			In; Jo	ate: put by: b Name: oject #:	3/10/2022 David Lar Lot 4 Cyp J0322-12	idry ress Road				Page 4 o
GDH I	Kerto-S LV	L 1.	750" X	( 14.00	0" 2-	Ply - P	ASSEI	<b>)</b>	evel: Level					
		2			1	• •							·	M T
1 SPF End	Grain				18'10" 18'10"	at .		-	Cin.	2	SPF E	nd Grai		1'2"
lember Inf			A.p P 4		-1		Reaction				-		A.61	
Type: Plies: Moisture Cond Deflection LL: Deflection TL: Importance:	480 360 Normal	_	Application Design M Building Load Sha Deck:	lethod: / Code: I aring: I	Floor ASD BC/IRC 2015 No Not Checked		Brg 1 2	Live 0 0	Dea 24 24	10	Snow 0 0		Wind 0 0	Const 0 0
Temperature:	Temp <= 100°	=					Bearings	:						
							Bearing 1 - SPF End	Length	Cap. 23%	React D/ 241	'L lb 0 / 0		Ld. Case Uniform	Ld. Comb. D
nalysis Re	sults						Grain							_
Analysis Moment Unbraced Shear LL Defl inch	Actual 10800 ft-lb 10800 ft-lb 2052 lb 0.000 (L/999)	9'5" 1 1'4 3/4" 9	Allowed 14299 ft-Ib 0826 ft-Ib 1408 Ib 199.000 (L/0)	Capacity 0.444 (44% 0.998 (100%) 0.218 (22% 0.000 (0%)	D 6) D	Case Uniform Uniform Uniform	2 - SPF End Grain	3.500"	23%		0/0	2410	Uniform	D
TL Defl inch	0.435 (L/506)	9'5 1/16" (	0.612 (L/360)	0.710 (71%	%) D	Uniform	_							
to exceed 6 2 Refer to las 3 Girders are 4 Top loads m 5 Top must be 6 Bottom brac	lies using 3 rows of 1	s for fastene orted on the ually by all p i maximum o	rs required fo bottom edge lies. of 9'10 1/8" o	or specified lee only.		istance not								
ID 1	Load Type Uniform	L	ocation T	Trib Width	Side Top	Dead 0.9 200 PLF	Live 1 0 PLF	1 Snow	1.15 \ 0 PLF	Wind 1.6 0 PLF		1.25 ) PLF	Commen B1GE	ts
2	Uniform Self Weight				Тор	45 PLF 11 PLF	0 PLF	= (	0 PLF	0 PLF	C	) PLF	Wall Above	9
tructural adequacy of esign criteria and esponsibility of the compone pplication, and to veri .umber . Dry service condition	Designs is responsible only of f this component based on loadings shown, It is ustomer and/or the contractor nt suitability of the intenry the dimensions and loads. ons, unless noted otherwise d with fire retardant or corros	the 1. LVL bea to regardin fastening 3. Damage 4. Design a 5. Provide	<b>&amp; Installatio</b> ms must not be cut to manufacturer' g installation g details, beam st	or drilled s product info requirements, n rength values, an be used s laterally restraine bearing points to	pondin mation nulti-ply d code od o avoid	at roofs provide p g design is valid		N 3 N (8 W IC	lanufacture letsä Wood 01 Merritt 7 lorwalk, CT ( 300) 622-585 ww.metsaw CC-ES: ESR	Building, 2nd 06851 50 00d.com/us	Floor	100 Fay US 283	314 0-864-TRUS	1, Suite #639

CSD 🚟

	-		Client:	Benjamin Stout Re	al Estate	D	ate:	3/10/2022		Page 5 of 5
1			Project:	The Williams			iput by:	David Landry		5
	isDesign		Address:	Cypress Road		Jo	ob Name	: Lot 4 Cypress Road		
-				Fayetteville, NC			roject #:	J0322-1268		
GDH	Kerto-S	LVL	1.750''	X 14.000"	2-Ply	- PASSE	Dľ	_evel: Level		
<u>.</u>	• •		• •	• •	• •	• •	•	· · · ·	1/2"	$\Pi \uparrow$
.	• •	•	• •				•		· [	1'2"
<u> -</u>	• •		• •	•••	• •	• •	•		<u></u>	
1 SPF	End Grain							2 SPF Er	Id Grain 7	
╆───					18'10"					3 1/2"
/					18'10"					
					10 10				I	
Multi-Ply	-									
	plies using 3			(.128x3") at 12"	o.c Maxim	um end dista	ince no	ot to exceed 6"		
Capacity Load		0.0	% PLF							
Yield Limit pe	er Foot		5.6 PLF							
Yield Limit pe	er Fastener		9 lb.							
Yield Mode Edge Distand	ce	IV 1 1/	/2"							
Min. End Dist	tance	3"	-							
Load Combin		1.0	0							
Duration Fac	201	1.0	0							
Notes			chemicals		6. For flat roofs pro-	vide proper drainage to	prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road	l, Suite #639
structural adequa	tured Designs is responsibl acy of this component ba	ased on the	Handling & Installa 1. LVL beams must not be	cut or drilled	ponding			Metsä Wood 301 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA	
responsibility of	and loadings shown, the customer and/or the o mponent suitability of the	It is the contractor to	<ol> <li>Refer to manufacture regarding installation</li> </ol>	urer's product information requirements, multi-ply				Norwalk, CT 06851 (800) 622-5850	28314 910-864-TRUS	
application, and to	to verify the dimensions and	l loads.	approvals 3. Damaged Beams must	n strength values, and code not be used				www.metsawood.com/us ICC-ES: ESR-3633		
1. Dry service co	onditions, unless noted othe e treated with fire retardant	erwise	<ol> <li>Design assumes top ed</li> <li>Provide lateral support</li> </ol>	lge is laterally restrained t at bearing points to avoid				100-LO. LON-3033	Con	птесн
			lateral displacement an	unotation	This design is	valid until 4/24/202	3			
version 20.40.0	075 Powered by iStru	ict								

CSD 🗱



RE: J0322-1268 Lot 4 Cypress Road **Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information:Customer: Benjamin Stout Real Estate<br/>Lot/Block: 4Project Name: J0322-1268<br/>Model: WilliamsAddress: Cypress Road<br/>City: FayettevilleSubdivision: Cypress Road<br/>State: NC

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 12 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
INO.			
1	150533833	ET1	3/2/2022
2	150533834	ET2	3/2/2022
3	150533835	F1	3/2/2022
4	150533836	F2	3/2/2022
5	150533837	F2A	3/2/2022
6	150533838	F3	3/2/2022
7	150533839	F4	3/2/2022
8	150533840	F5	3/2/2022
9	150533841	F6	3/2/2022
10	150533842	F7	3/2/2022
11	150533843	F7A	3/2/2022
12	150533844	FG1	3/2/2022

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

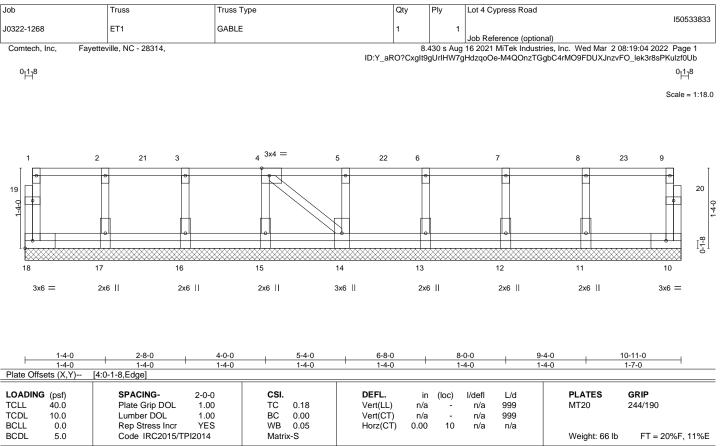
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



March 02, 2022



LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1(flat)	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.1(flat)		except end verticals.
WEBS	2x4 SP No.3(flat)	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3(flat)		

**REACTIONS.** All bearings 10-11-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 18, 10, 17, 16, 15, 14, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

## 3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

#### LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

## Uniform Loads (plf)

## Vert: 10-18=-10, 1-9=-100

Concentrated Loads (lb)

Vert: 4=-92 7=-92 21=-92 22=-92 23=-95



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses and truss systems, see **ANSUTPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job		Truss				Truss	Туре					Qty	Ply		Lot 4	Cypres	s Road	l						
J0322-1268		ET2				GABLI	=					1		1									l:	50533834
30322-1200		L12										l'			Job F	Referen	ce (opti	onal)						
Comtech, Inc,	Fayettev	/ille, NC	C - 283	314,						IDi	( aBO	2014	8.430 s / 9gUrIHW7	Aug	16 202	21 MiTe	k Indu	stries,						
										ID:1	г_ако	Cxgn	gonnw/	gнa	2q006	9-11180	ЛОМА	J1040	I VIVIE	w?0C.	левд	Apeloc	•	
0-1-8																							0	-1-8 H
																							Sca	ale = 1:66.9
					3x4 =			3x6 FP=					3x6 FP	=			3x4	=						
1 2	3 4	5	6	7	8 9	10	11	12 13 1415	16	17	18	19	20 2122	2	23 2	24 2	5 26	27	28	29	30	31	32 3	33
	9 9	1	1	1		1	f		1	8	9	9	<u>a</u> = a	F		9 9	Å	1	1	f	-	1	1	68 4
													┋╼╝								_ <b>_</b>			4
00 05			~~~								40	40												2
66 65	64 63	62	61	60	59 58	57	56	55 54 5352	51	50	49	48	47 46	454	4 4	43 4	- ··	40	39	38	37	36		34
3x4 =					3x4	=		3x6 FP =					3x6 FP	=		3x	4 =						3>	4 =

1-4-0 2-8-0 4-0-0 5-4-0 6-8-0 8-00 9-4-0 10-8-0 12-0-0 13-4-0 14-8-0 16-0-0 17-4-0 18-8-0 20-0-0 21-4-0 22-8-0 24-0-0 25-4-0 26-8-0 28-0-0 29-4-0 30-8-0 32-4-0 33-4-0 34-8-0 36-0-0 37-4-0 38-8-0 1-4

DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

in

n/a

n/a

-0.00

(loc)

42

I/defl

n/a

n/a

n/a

except end verticals.

L/d

999

999

n/a

Rigid ceiling directly applied or 6-0-0 oc bracing.

1) All plates are 1.5x3 MT20 unless otherwise indicated.

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

51, 50, 49, 48, 47, 45, 44, 43, 42, 41, 40, 39, 38, 37, 36, 35

[8:0-1-8,Edge], [26:0-1-8,Edge], [42:0-1-8,Edge], [58:0-1-8,Edge]

2-0-0

1.00

1.00

YES

CSI.

тс

вС

WB 0.03

Matrix-S

0.06

0.01

5) Gable studs spaced at 1-4-0 oc.

Plate Offsets (X,Y)--

40.Ó

10.0

0.0

5.0

TOP CHORD 2x4 SP No.1(flat)

2x4 SP No.1(flat)

2x4 SP No.3(flat)

2x4 SP No.3(flat)

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

WFBS

OTHERS

NOTES-

LUMBER-

BOT CHORD

REACTIONS.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

All bearings 39-11-0.
 (lb) - Max Grav All reactions 250 lb or less at joint(s) 66, 34, 65, 64, 63, 62, 61, 60, 59, 58, 57, 56, 55, 53, 52,

Strongbacks to be attached to walls at their outer ends or restrained by other means.

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



March 2,2022

39-11-0

FT = 20%F, 11%E

GRIP

244/190

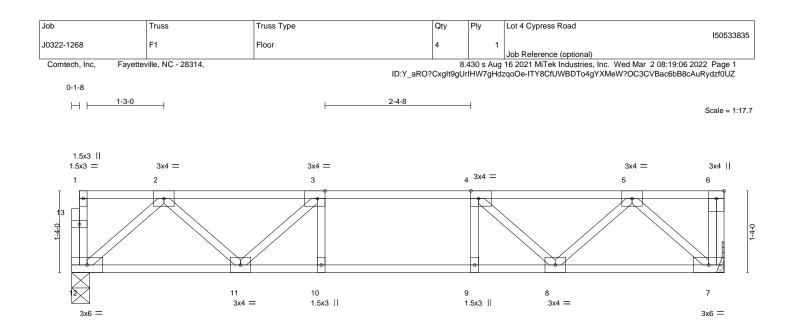
PLATES

Weight: 176 lb

MT20

Structural wood sheathing directly applied or 6-0-0 oc purlins,





			<u>10-7-8</u> 10-7-8			
Plate Offsets (X,Y)	[3:0-1-8,Edge], [4:0-1-8,Edge]	1			1	
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.32 BC 0.43 WB 0.20 Matrix-S	DEFL.         ir           Vert(LL)         -0.07           Vert(CT)         -0.08           Horz(CT)         0.01	10 >999 480 10 >999 360	PLATES MT20 Weight: 56 lb	<b>GRIP</b> 244/190 FT = 20%F. 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF	P No.1 (flat) P No.1 (flat) P No.3 (flat)	Matiko	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0	
REACTIONS. (size	e) 12=0-3-8, 7=Mechanical					

KEACHONS. (SIZE) 12=0-3-8, 7=Mechanical Max Grav 12=564(LC 1), 7=571(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-899/0, 3-4=-1197/0, 4-5=-900/0

BOT CHORD 11-12=0/591, 10-11=0/1197, 9-10=0/1197, 8-9=0/1197, 7-8=0/592

WEBS 2-12=-785/0, 2-11=0/428, 3-11=-448/0, 5-7=-788/0, 5-8=0/428, 4-8=-447/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

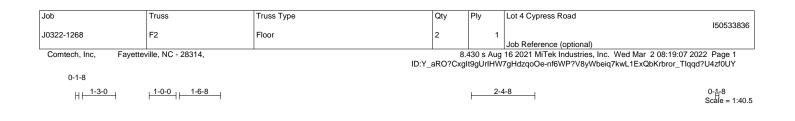
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

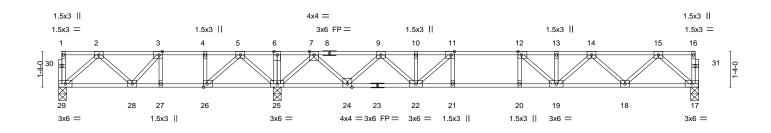
Strongbacks to be attached to walls at their outer ends or restrained by other means. 5) CAUTION, Do not erect truss backwards.



March 2,2022







	<u>8-2-0</u> 8-2-0				-11-0				
Plate Offsets (X,Y		-8,Edge], [26:0-1-8,Edge]		1:	5-9-0				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.54 BC 0.75 WB 0.46 Matrix-S	Vert(CT) -	in (loc) 0.16 19-20 0.21 19-20 0.03 17	l/defl >999 >899 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 126 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E	
LUMBER- TOP CHORD 2x BOT CHORD 2x WEBS 2x REACTIONS.	BRACING- TOP CHORD BOT CHORD	except	end verti	cals.	rectly applied or 6-0-0 c or 6-0-0 oc bracing.	oc purlins,			
	lax Uplift 29=-14(LC 4) lax Grav 29=402(LC 3), 25=1500(LC 1), 17=	794(LC 7)							
TOP CHORD									
BOT CHORD	14-15=-1534/0 28-29=-17/416, 27-28=-306/584, 26-27=-306/584, 25-26=-618/240, 22-24=0/1525, 21-22=0/2354, 20-21=0/2354, 19-20=0/2354, 18-19=0/1909, 17-18=0/853 2-29=-552/22, 5-25=-767/0, 5-26=0/719, 7-25=-1316/0, 7-24=0/960, 9-24=-908/0, 9-22=0/578, 11-22=-722/0, 15-17=-1133/0, 15-18=0/754, 4-26=-329/0, 3-28=-59/277, 14-18=-716/0, 14-19=0/419, 12-19=-415/60								

## NOTES-

1) Unbalanced floor live loads have been considered for this design.

- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 29.

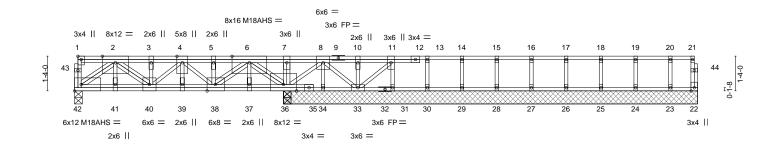
5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means. 6) CAUTION, Do not erect truss backwards.





[	Job	Truss	Truss Type	Qty	Ply	Lot 4 Cypress Road
	10000 1000					150533837
	J0322-1268	F2A	Floor	1	1	Job Reference (optional)
l	Output the last Franklin	W- NO 00014				
	Comtech, Inc, Fayettev	ille, NC - 28314,				16 2021 MiTek Industries, Inc. Wed Mar 2 08:19:10 2022 Page 1
			ID:Y_al	RO?Cxglt9	9gUrlHW7g	gHdzqoOe-BEnf21Y1FRzDZHsJbTbxY2DtBoy52FLkWosf5Ozf0UV
	0-1-8					
	H <b>⊢</b> <del>1-3-0</del>	<u>  1-0-14</u>   <u>1-0-14</u>	<u> </u>	-  -1-2-8	-   <u>1-2-8</u>	<del>  1-2-8</del>   <del>  1-2-8</del>   <del>  1-2-8</del>   <del>  1-2-8</del>   <del>  1-2-8</del>   <b>0-1</b> <sub>1</sub> -8 Scale = 1:41.6



	8-0-4		23-11-0				
Plate Offsets (X,Y)	[1:Edge,0-1-8], [38:0-3-12,Edge]			15-10-12	<u>.</u>		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.41 BC 0.33 WB 0.83 Matrix-S	DEFL. i Vert(LL) -0.0: Vert(CT) -0.0: Horz(CT) 0.0:	5 39 >99	99 480	PLATES MT20 M18AHS Weight: 164 lb	<b>GRIP</b> 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2x4 SP WEBS 2x4 SP	2400F 2.0E(flat) 2400F 2.0E(flat) No.3(flat) earings 15-10-12 except (it=length) 42=1	1-2-8	BRACING- TOP CHORD BOT CHORD	except end Rigid ceiling	verticals.	rectly applied or 6-0-0 o or 10-0-0 oc bracing, 4.	•
	rav All reactions 250 lb or less at joint 1), 34=425(LC 1), 33=1379(LC 1), 27=1241(LC 1), 26=1241(LC 1), 25	(s) except 42=3211(LC 1) 31=675(LC 1), 30=1190(	LC 1), 29=1266(LC 1), 2			CAN CA	Rozer
TOP CHORD 1-42= 6-7=0 BOT CHORD 41-42 34-31 WEBS 7-36= 5-38= 11-31	Comp./Max. Ten All forces 250 (lb) o -520/0, 21-22=-263/0, 2-3=-5037/0, 3-/ 0/2850, 7-8=0/2769, 8-10=0/699, 10-11: 2=0/3546, 40-41=0/3546, 39-40=0/5194 6=-1244/0, 33-34=-1244/0 1528/0, 2-42=-4405/0, 2-40=0/1932, 3 -1170/0, 8-36=-1902/0, 8-34=-470/0, 8 I=-665/0, 13-30=-1176/0, 14-29=-1253/ 3=-1228/0, 18-25=-1219/0, 19-24=-1257	l=-5099/0, 4-5=-4078/0, 5 =0/699 , 38-39=0/5194, 37-38=0/ -40=-1098/0, 6-36=-5292 -33=0/719, 10-33=-1209/( 0, 15-28=-1220/0, 16-27=	-6=-4011/0, 1312, 36-37=0/1312, /0, 6-38=0/3499, ), 11-33=-940/0, -1228/0,				
<ol> <li>All plates are MT20  </li> <li>All plates are 1.5x3  </li> <li>The Fabrication Tole joint 16 = 3%, joint 2</li> <li>Plates checked for a</li> <li>Load case(s) 1, 2, 3, 35, 36, 37, 38 has/ha truss.</li> <li>Recommend 2x6 str</li> </ol>	e loads have been considered for this d plates unless otherwise indicated. MT20 unless otherwise indicated. erance at joint 30 = 7%, joint 13 = 7%, jo 6 = 3%, joint 17 = 3%, joint 25 = 7%, jo plus or minus 1 degree rotation about , 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, ave been modified. Building designer m ongbacks, on edge, spaced at 10-0-0 of ttached to walls at their outer ends or re rect truss backwards.	bint 29 = 3%, joint 14 = 3% int 18 = 7%, joint 24 = 3% ts center. 16, 17, 18, 19, 20, 21, 22, ust review loads to verify the cand fastened to each tr	, joint 19 = 3% 23, 24, 25, 26, 27, 28, 2 that they are correct for	29, 30, 31, 32, the intended u	33, 34,		
Uniform Loads (plf)	dard Except: ialanced): Lumber Increase=1.00, Plate =-10, 1-21=-920	Increase=1.00					

2) Dead: Lumber Increase=1.00, Plate Increase=1.00

on pac

March 2,2022



Job	Truss	Truss Type	Qty	Ply	Lot 4 Cypress Road	
J0322-1268	F2A	Floor	1	1		150533837
Comtach Inc. Equation	ville, NC - 28314,			120 0 4110	Job Reference (optional) 16 2021 MiTek Industries, Inc. Wed Mar 2 08	2:10:10 2022 Bags 2
Comtech, Inc, Fayette	ville, NC - 26314,				gHdzqoOe-BEnf21Y1FRzDZHsJbTbxY2DtBoy	
LOAD CASE(S) Standard Uniform Loads (plf)	1					
Vert: 22-42=-10 3) 1st chase Dead + Floor Uniform Loads (plf)		ncrease=1.00, Plate Increase=1.00				
Vert: 22-42=-10		Increase=1.00, Plate Increase=1.00				
Uniform Loads (plf)	· · · ·	increase=1.00, Flate increase=1.00				
Vert: 22-42=-1( 5) 3rd chase Dead + Floor Uniform Loads (plf)		ncrease=1.00, Plate Increase=1.00				
Vert: 22-42=-10		ncrease=1.00, Plate Increase=1.00				
Uniform Loads (plf) Vert: 22-42=-10	( ,	1016a36-1.00, 1 late increase-1.00				
Uniform Loads (plf)	( ,	ncrease=1.00, Plate Increase=1.00				
Vert: 22-42=-10 8) 6th chase Dead + Floor Uniform Loads (plf)		ncrease=1.00, Plate Increase=1.00				
Vert: 22-42=-10 9) 7th chase Dead + Floor Uniform Loads (plf)		ncrease=1.00, Plate Increase=1.00				
Vert: 22-42=-10 10) 8th chase Dead + Floo		Increase=1.00, Plate Increase=1.00				
Uniform Loads (plf) Vert: 22-42=- 11) 9th chase Dead + Flor		Increase=1.00, Plate Increase=1.00				
Uniform Loads (plf) Vert: 22-42=-1	. ,					
12) 10th chase Dead + Flo Uniform Loads (plf)	oor Live (unbalanced): Lumbe	er Increase=1.00, Plate Increase=1.00				
		er Increase=1.00, Plate Increase=1.00				
Uniform Loads (plf) Vert: 22-42=-						
14) 12th chase Dead + Fic Uniform Loads (plf) Vert: 22-42=-		er Increase=1.00, Plate Increase=1.00				
15) 13th chase Dead + Flo Uniform Loads (plf)	oor Live (unbalanced): Lumbe	er Increase=1.00, Plate Increase=1.00				
		er Increase=1.00, Plate Increase=1.00				
Uniform Loads (plf) Vert: 22-42=-1		er Increase=1.00, Plate Increase=1.00				
Uniform Loads (plf) Vert: 22-42=-	,	er increase=1.00, Plate increase=1.00				
<ol> <li>18) 16th chase Dead + Flo Uniform Loads (plf)</li> </ol>	oor Live (unbalanced): Lumbe	er Increase=1.00, Plate Increase=1.00				
Vert: 22-42=- 19) 17th chase Dead + Flo Uniform Loads (plf)		er Increase=1.00, Plate Increase=1.00				
Vert: 22-42=-		er Increase=1.00, Plate Increase=1.00				
Uniform Loads (plf) Vert: 22-42=-						
21) 19th chase Dead: Lum Uniform Loads (plf) Vert: 22-42=-	nber Increase=1.00, Plate Inc	rease=1.00				
	ber Increase=1.00, Plate Inc	rease=1.00				
	10, 1-21=-570 Iber Increase=1.00, Plate Inc	rease=1.00				
Uniform Loads (plf) Vert: 22-42=- 24) 22nd chase Dead: Lur	10, 1-21=-570 nber Increase=1.00, Plate Inc	crease=1.00				
Uniform Loads (plf) Vert: 22-42=-	10, 1-21=-570					
25) 23rd chase Dead: Lun Uniform Loads (plf) Vert: 22-42=-	hber Increase=1.00, Plate Inc	rease=1.00				
	ber Increase=1.00, Plate Inc	rease=1.00				
Vert: 22-42=-	10, 1-21=-570 ber Increase=1 00, Plate Inc	rease=1.00				

27) 25th chase Dead: Lumber Increase=1.00, Plate Increase=1.00

nued on page 3



Lot 4 Cypress Road
150533837
1
Job Reference (optional)
Aug 16 2021 MiTek Industries, Inc. Wed Mar 2 08:19:10 2022 Page 3

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Mar 2 08:19:	10 2022 Page 3
ID:Y_aRO?Cxglt9gUrlHW7gHdzqoOe-BEnf21Y1FRzDZHsJbTbxY2DtBoy52FL	kWosf5Ozf0UV

	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
28)	26th chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
,	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
29)	27th chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
30)	28th chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
,	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
31)	29th chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
,	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
32)	30th chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
,	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
33)	31st chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
34)	32nd chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
35)	33rd chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
36)	34th chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
37)	35th chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	
38)	36th chase Dead: Lumber Increase=1.00, Plate Increase=1.00	
	Uniform Loads (plf)	
	Vert: 22-42=-10, 1-21=-570	

LOAD CASE(S) Standard

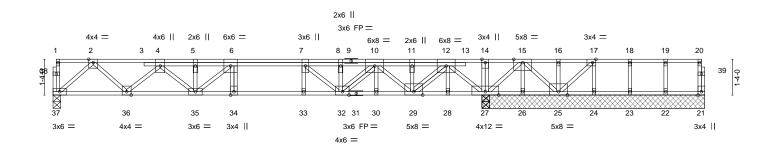


Job	Truss	Truss Type	Qty	Plv	Lot 4 Cypress Road	
				,	15053383	
J0322-1268	F3	Floor Girder	1	1		
					Job Reference (optional)	
Comtech Inc Equation Inc. 2021 MiTek Industrias Inc. Wed Mar 2 09:10:12 2022 Page 1						

Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Mar 2 08:19:12 2022 Page 1 ID:Y\_aRO?Cxglt9gUrIHW7gHdzqoOe-7dvPSiZHn3Dxob?hjudPeTJ8DcXnW8o1\_6LmAHzf0UT

## 0-1-8

H <u>1-3-0</u> <u>1-3-0</u> <u>1-3-0</u> <u>1-3-0</u> <u>2-5-0</u> <u>1-3-0</u> <u>1-2-8</u> 1-2-8 1-2-8 0-1-8 Scale = 1:40.4



2-9-0 2-9-0 Plate Offsets (X,Y)	[17:0-1-8,Edge]	<u>16-3-8</u> 13-6-8				-		24-3-12 8-0-4	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCCL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.69 BC 0.83 WB 0.90 Matrix-S	Vert(CT) -	in 0.15 0.21 0.04	(loc) 33 33 27	l/defl >999 >910 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 154 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
9-20: 2 BOT CHORD 2x4 SF WEBS 2x4 SF	<ul> <li>No.1(flat) *Except*</li> <li>2x4 SP 2400F 2.0E(flat)</li> <li>No.1(flat)</li> <li>No.3(flat)</li> <li>earings 8-3-12 except (it=length) 37=0-3</li> </ul>		BRACING- TOP CHORD BOT CHORD		except Rigid c	end vert eiling dir	icals. ectly applied	rectly applied or 6-0-0 o or 10-0-0 oc bracing, 1 8,26-27,25-26.	•
Max G FORCES. (lb) - Max. TOP CHORD 2-4=- 10-1: BOT CHORD 36-3: 29-3 WEBS 14-2: 12-2: 17-2!	Jplift All uplift 100 lb or less at joint(s) e frav All reactions 250 lb or less at joint Comp./Max. Ten All forces 250 (lb) or -1763/0, 4-5=-3023/0, 5-6=-3023/0, 6-7= 1=0/392, 11-12=0/399, 12-14=0/42425, 17 -0/1020, 35-36=0/2501, 34-35=0/3544 30=0/1723, 28-29=-2236/0, 27-28=-2238 7=-407/0, 2-37=-1355/0, 2-36=0/1025, 4 7=-2792/0, 15-27=-3125/0, 12-29=0/265 5=-891/0, 17-24=0/459, 10-29=-2567/0, =-174/304	(s) 21, 23, 22 except 37=5 less except when shown. -3544/0, 7-8=-3460/0, 8-1 4-15=0/4423, 15-16=0/65/ 33-34=0/3544, 32-33=0// /0, 26-27=-2058/0, 25-26 -36=-1009/0, 4-35=0/693, 9, 15-26=0/713, 15-25=0/	053(LC 1), 27=4269) 00=-3398/0, 8, 16-17=0/658 3544, 30-32=0/1724 2058/0 6-35=-992/0, 1890,	(LC 1)		269(LC 1	)		
<ul><li>2) All plates are 1.5x3</li><li>3) Plates checked for a</li></ul>	re loads have been considered for this do MT20 unless otherwise indicated. a plus or minus 1 degree rotation about i connection (by others) of truss to bearin	ts center.	anding 744 lb uplift a	t joint	26, 502	2 lb uplift	at	STA CA	Rona and

joint 25 and 448 lb uplift at joint 24 Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1330 lb down at 10-9-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

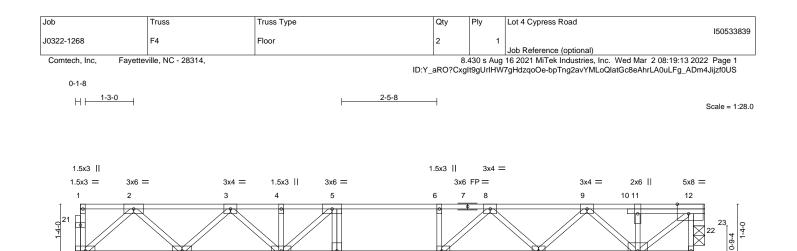
Vert: 21-37=-10, 1-20=-100 Concentrated Loads (Ib)

Vert: 8=-1250(F)



March 2,2022





16

3x4 =

15

3x4 =

14

4x6 =

13

1.5x3 ||

17

3x4 ||

18

3x6 =

H			<u>16-3-0</u> 16-3-0			
Plate Offsets (X,Y)	[12:0-3-8,Edge], [16:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	<b>CSI.</b> TC 0.55 BC 0.74 WB 0.58	Vert(LL) -0.18	n (loc) l/defl L/ 3 15-16 >999 480 3 15-16 >817 360 3 23 n/a n/	0 MT20 0	<b>GRIP</b> 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 89 lb	FT = 20%F, 11%E
BOT CHORD 2x4 SF WEBS 2x4 SF OTHERS 4x4 SF REACTIONS. (siz	<ul> <li>No.1 (flat)</li> <li>No.3 (flat)</li> <li>No.3 (flat)</li> <li>No.2 (flat)</li> <li>No.2 (flat)</li> <li>e) 20=0-3-8, 23=0-3-8</li> <li>aray 20=868(LC 1), 23=861(LC 1)</li> </ul>		BRACING- TOP CHORD BOT CHORD	except end verticals.	thing directly applied or 6-0-0 applied or 10-0-0 oc bracing.	oc purlins,
TOP CHORD 2-3=	Comp./Max. Ten All forces 250 (lb) or -1557/0, 3-4=-2522/0, 4-5=-2522/0, 5-6= - 1002/0, 11 12 - 1002/0					

 9-11=-1003/0, 11-12=-1003/0

 BOT CHORD
 19-20=0/937, 18-19=0/2146, 17-18=0/2827, 16-17=0/2827, 15-16=0/2578, 14-15=0/1700

 WEBS
 12-14=0/1225, 2-20=-1245/0, 2-19=0/862, 3-19=-820/0, 3-18=0/510, 5-18=-687/0, 9-14=-947/0, 9-15=0/629, 8-15=-594/0, 8-16=-307/0, 12-23=-886/0

## NOTES-

2

3x6 =

1) Unbalanced floor live loads have been considered for this design.

19

3x6 =

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Bearing at joint(s) 23 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

capacity of bearing surface.

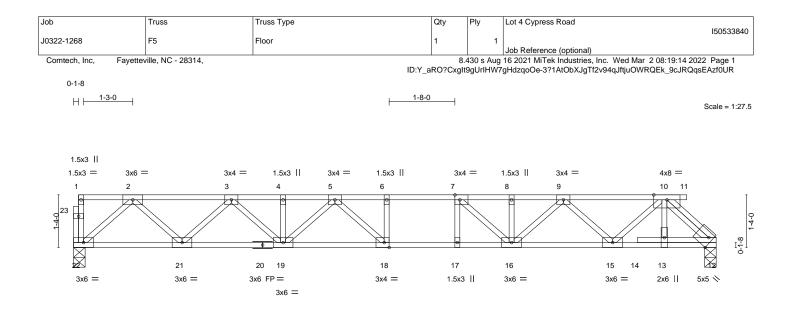
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



March 2,2022





H			16-3-8					
Plate Offsets (X,Y)	[7:0-1-8,Edge], [12:Edge,0-3-0], [18:0-1	-8,Edge]	16-3-8					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.52 BC 0.73 WB 0.43 Matrix-S	Vert(LL) -0.1 Vert(CT) -0.2	in (loc) 17 18-19 24 18-19 04 12	l/defl >999 >823 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 92 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 S WEBS 2x4 S REACTIONS. (st	5P No.1(flat) 5P No.1(flat) 5P No.3(flat) 2e) 22=0-3-8, 12=0-3-8 Grav 22=879(LC 1), 12=818(LC 1)		BRACING- TOP CHORD BOT CHORD	excep	t end ver	icals.	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
TOP CHORD         2-3           8-9           BOT CHORD         21-           13-           WEBS         10-	<ol> <li>Comp./Max. Ten All forces 250 (lb) o</li> <li>-1578/0, 3-4=-2578/0, 4-5=-2578/0, 5-6=</li> <li>-2538/0, 9-10=-1528/0</li> <li>22=0/948, 19-21=0/2182, 18-19=0/2836, 15=0/852, 12-13=0/850</li> <li>12=-1167/0, 2-22=-1260/0, 2-21=0/876, 3</li> <li>-879/0, 9-16=0/514, 7-16=-693/0, 5-19</li> </ol>	2903/0, 6-7=-2903/0, 7-8 17-18=0/2903, 16-17=0/29 -21=-841/0, 3-19=0/538, 1	=-2538/0, 903, 15-16=0/2161,					

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

4) CAUTION, Do not erect truss backwards.





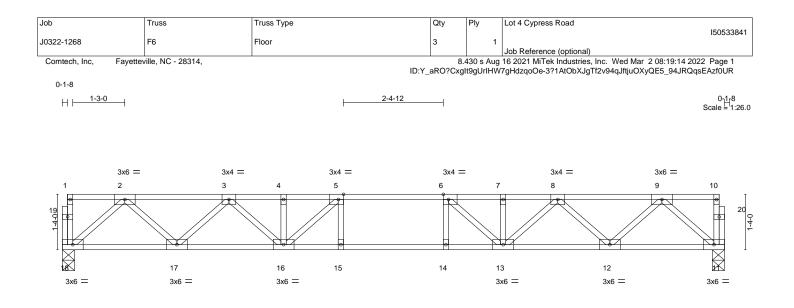


Plate Offsets (X,Y	) [5:0-1-8,Edge], [6:0-1-8,Edge]		15-10-12 15-10-12					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.48 BC 0.71 WB 0.40 Matrix-S	Vert(LL) -0.16	15-16 = 15-16 =	l/defl >999 >914 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 84 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%I
BOT CHORD 22 WEBS 22 REACTIONS.	4 SP No.1(flat) 4 SP No.1(flat) 4 SP No.3(flat) (size) 18=0-3-8, 11=0-3-8 lax Grav 18=854(LC 1), 11=854(LC 1)		BRACING- TOP CHORD BOT CHORD	except er	nd vertic	als.	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
TOP CHORD	Max. Comp./Max. Ten All forces 250 (lb) o 2-3=-1527/0, 3-4=-2463/0, 4-5=-2463/0, 5-6: 3-9=-1527/0 17-18=0/921, 16-17=0/2102, 15-16=0/2743,	=-2743/0, 6-7=-2463/0, 7-8	3=-2463/0,					
WEBS	11-12=0/921 2-18=-1224/0, 2-17=0/842, 3-17=-800/0, 3-1 9-12=0/842, 8-12=-800/0, 8-13=0/490, 6-13=	6=0/490, 5-16=-648/0, 9-1	, ,					

# NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

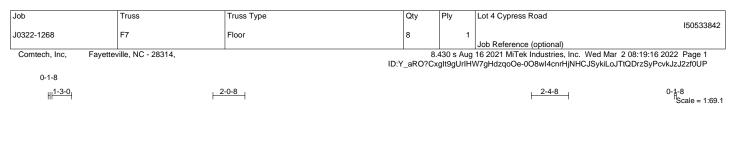
3) Plates checked for a plus or minus 1 degree rotation about its center.

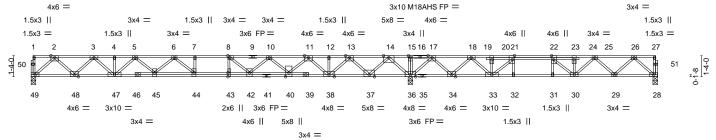
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.









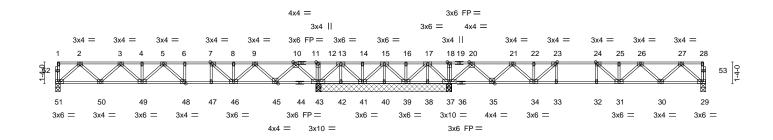
L	24-2-0				39-11-0		
Plate Offsets (X		3-0.Edge]. [43:0-3-0.0-0-0]	, 		15-9-0		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-         2-0-0           Plate Grip DOL         1.00           Lumber DOL         1.00           Rep Stress Incr         YES	CSI. TC 0.49 BC 1.00 WB 0.86 Matrix-S	DEFL. in Vert(LL) -0.40	44 >716 4 44-45 >533 3	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 230 lb	<b>GRIP</b> 244/190 186/179 FT = 20%F, 11%
BOT CHORD	2x4 SP 2400F 2.0E(flat) 2x4 SP No.1(flat) 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood she except end verticals Rigid ceiling directly	s.	tly applied or 6-0-0 c	oc purlins,
CORCES. (Ib)	(size) 49=0-3-8, 36=0-3-8, 28=0-3-8 Max Uplift 28=-42(LC 3) Max Grav 49=1137(LC 3), 36=2725(LC 1), 24 - Max. Comp./Max. Ten All forces 250 (lb) o 2-3=-2144/0, 3-4=-3653/0, 4-5=-3653/0, 5-6	r less except when shown =-4728/0, 6-7=-5086/0, 7-8	8=-5086/0,				
3OT CHORD	8-10=-4578/0, 10-11=-3422/0, 11-12=-1647, 14-15=0/4215, 15-17=0/4215, 17-18=0/2526 21-22=-1896/1079, 22-23=-1895/481, 23-25 48-49=0/1244, 47-48=0/2997, 45-47=0/4306 40-42=0/4136, 38-40=0/2650, 37-38=-583/5 33-34=-2091/703, 32-33=-1079/1896, 31-32 29-30=-312/1632, 28-29=-66/759	5, 18-20=-1225/1772, 20-2 =-1895/481, 25-26=-1209/ 3, 44-45=0/5035, 43-44=0/ 598, 36-37=-2553/0, 34-36	1=-1225/1772, 176 5086, 42-43=0/5086, =-3007/0,				
WEBS	2-49=-1654/0, 2-48=0/1251, 3-48=-1187/0, 3 6-45=-429/0, 6-44=-393/427, 14-36=-2212/0 13-38=0/1478, 11-38=-1405/0, 11-40=0/108 8-42=-1107/0, 8-43=-118/490, 17-36=-1684/ 18-33=0/891, 20-33=0/412, 21-33=-1533/0, 25-29=-588/190, 25-30=-230/358, 23-30=-43	, 14-37=0/1807, 13-37=-1 6, 10-40=-1007/0, 10-42=( 0, 17-34=0/1315, 18-34=- 26-28=-1008/89, 26-29=-1	766/0, D/720, 1275/0,			_	
2) All plates are 3) All plates are 4) Plates check 5) Provide mecl 6) Required 2x6	floor live loads have been considered for this d MT20 plates unless otherwise indicated. 3x6 MT20 unless otherwise indicated. ed for a plus or minus 1 degree rotation about hanical connection (by others) of truss to beari 5 strongbacks, on edge, spaced at 10-0-0 cc a o walls at their outer ends or restrained by oth	its center. ng plate capable of withsta nd fastened to each truss			to	DB68	

7) CAUTION, Do not erect truss backwards.

March 2,2022



Job		Truss	Truss Type		Qty	Ply	Lot 4 Cypress Road	
								150533843
J0322-1268		F7A	Floor		1	1		
							Job Reference (optional)	
Comtech, Inc,	Fayettev	ille, NC - 28314,			8.4	430 s Aug	16 2021 MiTek Industries, Inc. Wed Mar 2 08:19:19 2022	2 Page 1
				ID:Y_aF			gHdzqoOe-Qzq3w5fg8C5x8g21dsF2Qy5M6RwAfPj3biYdv	
0-1-8								
1-3-0		1-6-0	1-2-8 1-2-8	1.2.8 1.2.8 1.2	-81-1-12		2-4-4	0-1-8
<mark>1-3-0</mark>		1-6-0	1-2-0 1-2-0	1-2-8 1-2-8 1-2	-0  -1-12		2-4-4	Scale = 1:66.6



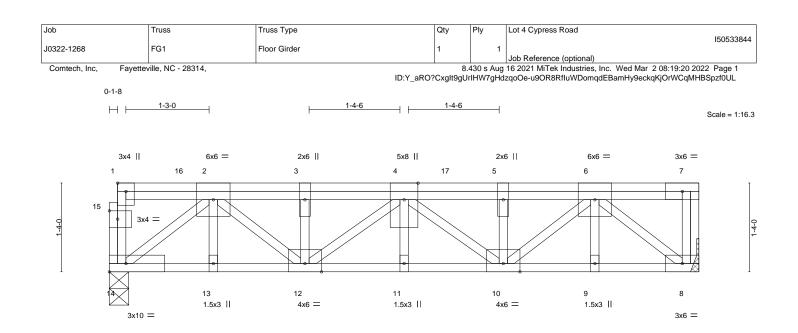
<b> </b>	<u>16-0-0</u> 16-0-0		24-3-12 8-3-12	+		39-11-0 15-7-4	
Plate Offsets (X,Y)-		-8,Edge], [48:0-1-8,Edge]	8-3-12			15-7-4	
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	<b>CSI.</b> TC 0.58 BC 0.78 WB 0.50	Vert(LL) -0.1	in (loc) l/defl 6 31-32 >999 1 31-32 >892 5 29 n/a	L/d 480 360 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S				Weight: 218 lb	FT = 20%F, 11%E
BOT CHORD 2x4	9 SP No.1(flat) 9 SP No.1(flat) 9 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	except end vert	icals.	rectly applied or 6-0-0 o	oc purlins,
(lb) - Ma	Il bearings 8-3-12 except (jt=length) 51=0-3 ix Uplift All uplift 100 lb or less at joint(s) 4 38=-252(LC 4) ix Grav All reactions 250 lb or less at joint 29=742(LC 4), 37=1700(LC 7), 37=	0 except 42=-225(LC 9), 4 (s) 40, 39, 38 except 51=7			),		
TOP CHORD 2- 8- 11	lax. Comp./Max. Ten All forces 250 (lb) o -3=-1283/0, 3-4=-2003/0, 4-5=-2003/0, 5-6= -9=-1357/0, 10-12=0/1699, 12-13=0/1699, 1 6-17=0/361, 17-18=0/1493, 18-20=0/1493, 2-23=-1485/0, 23-24=-2022/0, 24-25=-2007	-1950/0, 6-7=-1950/0, 7-8 3-14=0/389, 14-15=0/389 20-21=-430/0, 21-22=-148	3=-1357/0, 9, 15-16=0/361, 35/0,				
BOT CHORD 50 43 3	0-51=0/793, 49-50=0/1753, 48-49=0/2110, 3-45=-633/0, 42-43=-829/0, 41-42=-829/0, 37-38=-746/0, 35-37=-591/0, 34-35=0/1030, 1-32=0/2022, 30-31=0/1744, 29-30=0/794	47-48=0/1950, 46-47=0/1 40-41=-287/0, 39-40=-287	950, 45-46=0/857, 7/0, 38-39=-746/0,				
WEBS 2- 13 2 2	-51=-1054/0, 2-50=0/681, 3-50=-654/0, 3-4 -51=-1054/0, 2-50=0/681, 3-50=-654/0, 3-4 -43=-1169/0, 13-41=0/625, 17-39=0/548, 7 7-29=-1054/0, 27-30=0/677, 26-30=-645/0, 0-37=-1364/0, 20-35=0/996, 10-45=0/1057, 1-35=-953/0, 21-34=0/619, 23-34=-772/0	7-38=-15/251, 17-37=-10 26-31=0/357, 24-31=-267	07/0, 7/117,			A CA	P A
<ul><li>2) All plates are 1.5</li><li>3) Plates checked f</li><li>4) Provide mechanical</li></ul>	r live loads have been considered for this d ix3 MT20 unless otherwise indicated. or a plus or minus 1 degree rotation about i ical connection (by others) of truss to bearin , 39=218, 38=252.	ts center.	anding 100 lb uplift at jo	int(s) 40 except (jt	=lb)		

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

March 2,2022





			8-10-4 8-10-4			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [15:0-1-8,0-1-8]	1				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrNOCode IRC2015/TPI2014	<b>CSI.</b> TC 0.27 BC 0.57 WB 0.53 Matrix-P	DEFL. ir Vert(LL) -0.04 Vert(CT) -0.06 Horz(CT) 0.02	11 >999 480 11 >999 360	PLATES MT20 Weight: 66 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER-       TOP CHORD     2x4 SP No.1(flat)       BOT CHORD     2x4 SP No.1(flat)       WEBS     2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	<i>y</i> 11	) oc purlins,

Max Grav 14=1475(LC 1), 8=1350(LC 1)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-2485/0, 3-4=-2485/0, 4-5=-2414/0, 5-6=-2414/0

BOT CHORD 13-14=0/1625, 12-13=0/1625, 11-12=0/2734, 10-11=0/2734, 9-10=0/1566, 8-9=0/1566 WEBS 2-14=-2067/0, 2-12=0/1119, 3-12=-500/0, 6-8=-2003/0, 6-10=0/1104, 5-10=-436/0, 4-10=-404/0, 4-12=-315/0

NOTES-

- 1) Plates checked for a plus or minus 1 degree rotation about its center.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.
- Strongbacks to be attached to walls at their outer ends or restrained by other means.

4) CAUTION, Do not erect truss backwards.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 473 lb down at 1-1-12, 471 lb down at 3-1-12, and 471 lb down at 3-1-12, and 471 lb down at 7-1-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 8-14=-10, 1-7=-100 Concentrated Loads (Ib)

Vert: 3=-471(B) 6=-471(B) 16=-473(B) 17=-471(B)



March 2,2022



