



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature_

David Landry

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	(à	ASED O	N TABLES	8 R502	5(t) & (t)	0))					
NU	WBER C		STUDS R NEADERN			A END OF					
END REACHON (0P 10)	REQ'D STUDS FOR (2) PLY HEADER		BND REACTION (UP TD)	REQ15 STUDS FOR (3) MY HEADER		END REACTION (UP TO)	REQUE STUDS FOR				
1700	1		2550	1		3400	1				
3400	2		5100	2		6800	2				
5100	3		7650	3		10200	3				
6800	4		10200	4		13600	4				
8500	5		12750	5		17000	5				
10200	6		15300	6							
11900	7										
13600	8										
15300	۵										

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THIS IS	BUILDER	Benjamin Stout Real Estate	COUNTY	Fayetteville / Cumberland	15300
A TRUSS	JOB NAME	Lot 2 Cypress Road	ADDRESS	Cypress Road	9
PLACEN	PLAN	The Ashville	MODEL	Roof	
ENT DIA	SEAL DATE N/A	N/A	DATE REV. 03/09/22	03/09/22	
GRAM ON	QUOTE #		DRAWN BY	DRAWN BY David Landry	
LY.	# 90 f	J0322-1265	SALESMAN	SALESMAN Marshall Naylor	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



RE: J0322-1265 Lot 2 Cypress Road **Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Benjamin Stout Real Estate Project Name: J0322-1265 Lot/Block: 2 Model: Ashville

Address: Cypress Road Subdivision: Cypress Road

City: Fayetteville State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special **Loading Conditions):**

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 21 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E16391025	A1	11/9/2021	21	E16391045	V5	11/9/2021
2	E16391026	A1GE	11/9/2021				
3	E16391027	A2	11/9/2021				
4	E16391028	A3	11/9/2021				
5	E16391029	A3GE	11/9/2021				
6	E16391030	B1	11/9/2021				
7	E16391031	B1-GR	11/9/2021				
8	E16391032	B1GE	11/9/2021				
9	E16391033	C1	11/9/2021				
10	E16391034	C1-GR	11/9/2021				
11	E16391035	C1GE	11/9/2021				
12	E16391036	D1	11/9/2021				
13	E16391037	D1GE	11/9/2021				
14	E16391038	M1	11/9/2021				
15	E16391039	M1GE	11/9/2021				
16	E16391040	PB	11/9/2021				
17	E16391041	V1	11/9/2021				
18	E16391042	V2	11/9/2021				
19	E16391043	V3	11/9/2021				
20	E16391044	V4	11/9/2021				

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

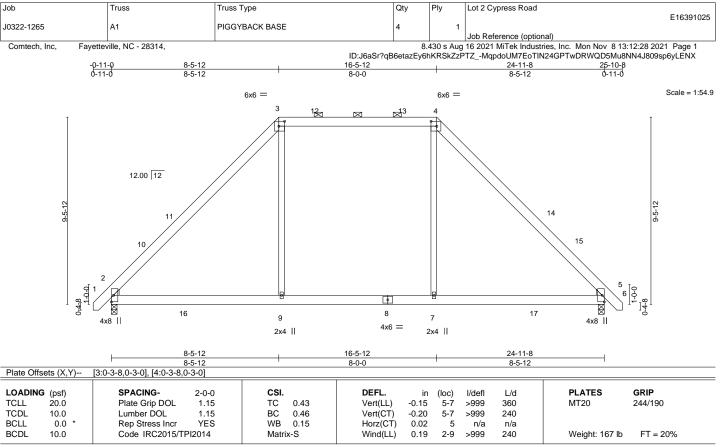
My license renewal date for the state of North Carolina is December 31, 2022

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



November 09, 2021



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

WEBS 2X4 SP

WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 5=0-3-8

Max Horz 2=223(LC 11)

Max Uplift 2=-35(LC 12), 5=-35(LC 13) Max Grav 2=1309(LC 2), 5=1309(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1465/308, 3-4=-908/350, 4-5=-1465/308 BOT CHORD 2-9=-36/930, 7-9=-33/938, 5-7=-33/928

WEBS 3-9=0/653, 4-7=0/653

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 8-5-12, Exterior(2) 8-5-12 to 14-8-7, Interior(1) 14-8-7 to 16-5-12, Exterior(2) 16-5-12 to 22-8-7, Interior(1) 22-8-7 to 25-8-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

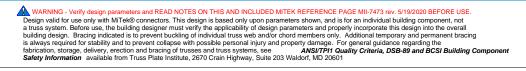


Structural wood sheathing directly applied or 6-0-0 oc purlins, except

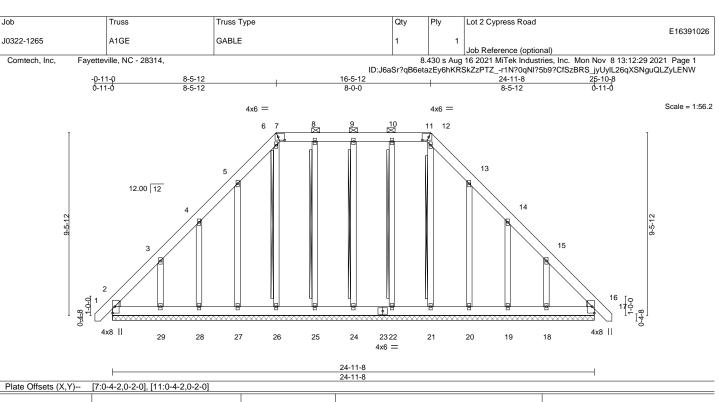
2-0-0 oc purlins (6-0-0 max.): 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing

November 9,2021







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LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl	L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.04	Vert(LL) 0.00 16 n/r	120	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) 0.00 16 n/r	120	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.13	Horz(CT) 0.01 16 n/a	n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S			Weight: 245 lb FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

OTHERS 2x4 SP No 2 WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

BRACING-TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 7-11.

Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace:

2x4 SPF No.2 - 9-24, 8-25, 6-26, 10-22,

12-21

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 24-11-8.

(lb) - Max Horz 2=280(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 24, 25, 26, 22, 16 except 2=-114(LC 8), 27=-130(LC 12), 28=-135(LC 12), 29=-221(LC 12), 20=-125(LC 13),

19=-136(LC 13), 18=-214(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 24, 25, 26, 27, 28, 22, 21, 20, 19, 18, 16 except 29=254(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-311/246

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 25, 26, 22, 16 except (jt=lb) 2=114, 27=130, 28=135, 29=221, 20=125, 19=136, 18=214.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 16.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



November 9,2021

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designs. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss experts.

Starty Information

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Job Truss Truss Type Qty Ply Lot 2 Cypress Road F16391027 J0322-1265 A2 PIGGYBACK BASE Job Reference (optional) Comtech, Inc. Fayetteville, NC - 28314, 8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:30 2021 Page 1 ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-JDxNDAONmPj0cMEfXuyhWxVZW9ZXrlvbcKezu?yLENV 8-2-4 8-2-4 8-0-0 8-5-12 Scale = 1:53.9 6x6 =5x5 = 12 \bowtie 3 12.00 12 4x4 // 4x4 // 14 15 1-3-8 6 0 8 16 9 7 3x10 4x8 || 4x6 = 2x4 || 2x4 || 16-2-4 24-8-0 Plate Offsets (X,Y)--[1:Edge,0-0-0], [3:0-3-8,0-3-0], [4:0-2-12,0-2-12] LOADING (psf) SPACING-DEFL. **PLATES** GRIP 2-0-0 CSI I/defl L/d (loc) **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.44 Vert(LL) -0.29 5-7 >999 360 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 ВС 0.53 Vert(CT) -0.34 5-7 >861 240 0.13 **BCLL** 0.0 Rep Stress Incr YES WB Horz(CT) 0.02 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Wind(LL) 0.19 5-7 >999 240 Weight: 176 lb FT = 20% **BRACING-**

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WFBS 2x4 SP No 2

WEDGE

Right: 2x4 SP No.2

SLIDER Left 2x6 SP No.1 5-11-1

REACTIONS. (size) 1=Mechanical, 5=0-3-8

Max Horz 1=-221(LC 8)

Max Uplift 1=-20(LC 12), 5=-35(LC 13) Max Grav 1=1087(LC 2), 5=1249(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 1-3=-1337/307, 3-4=-815/353, 4-5=-1306/303 TOP CHORD

BOT CHORD 1-9=-36/838, 7-9=-32/844, 5-7=-33/836

WEBS 3-9=0/518, 4-7=0/562

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 8-2-4, Exterior(2) 8-2-4 to 14-4-15, Interior(1) 14-4-15 to 16-2-4, Exterior(2) 16-2-4 to 22-4-15, Interior(1) 22-4-15 to 25-5-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 6-0-0 oc purlins, except

2-0-0 oc purlins (6-0-0 max.): 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

November 9,2021

neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Ply Lot 2 Cypress Road F16391028 J0322-1265 PIGGYBACK BASE 12 АЗ Job Reference (optional) Comtech, Inc. Fayetteville, NC - 28314, 8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:31 2021 Page 1 ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-nPVIQWP?XjrtEVpr5cTw381jxZujaknlq_NXQRyLENU 24-11-8 8-0-0 8-5-12 Scale = 1:57.7 6x6 =6x6 =2 12.00 12 13 1-0-0 T 15 16 6 4x8 || 4x8 II 3x4 || 3x4 || 3x10 =4x8 = 4x6 = 4x4 = 16-5-12 [2:0-3-8,0-3-0], [3:0-3-8,0-3-0] Plate Offsets (X,Y)--LOADING (psf) SPACING-CSI. DEFL. **PLATES** GRIP 2-0-0 I/defl L/d **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.46 Vert(LL) -0.14 1-8 >999 360 MT20 244/190 TCDL Vert(CT) 10.0 Lumber DOL 1.15 ВС 0.53 -0.19 1-8 >999 240 **BCLL** 0.0 Rep Stress Incr YES WB 0.15 Horz(CT) 0.02 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Wind(LL) 0.17 1-8 >999 240 Weight: 181 lb FT = 20% LUMBER-**BRACING-**TOP CHORD Structural wood sheathing directly applied or 5-11-9 oc purlins,

BOT CHORD

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

WFBS 2x4 SP No 2

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 4=0-3-8

Max Horz 1=-221(LC 8)

Max Uplift 1=-22(LC 12), 4=-35(LC 13) Max Grav 1=1263(LC 2), 4=1314(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-1449/307, 2-3=-925/359, 3-4=-1489/315 TOP CHORD

1-8=-35/947, 6-8=-37/955, 4-6=-32/944 BOT CHORD

2-8=0/660, 3-6=0/667 **WEBS**

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; DCDL=6.0psf; and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 8-5-12, Exterior(2) 8-5-12 to 14-8-7, Interior(1) 14-8-7 to 16-5-12, Exterior(2) 16-5-12 to 22-8-7, Interior(1) 22-8-7 to 25-8-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 4.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



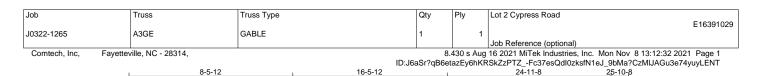
2-0-0 oc purlins (6-0-0 max.): 2-3.

Rigid ceiling directly applied or 10-0-0 oc bracing.

November 9,2021







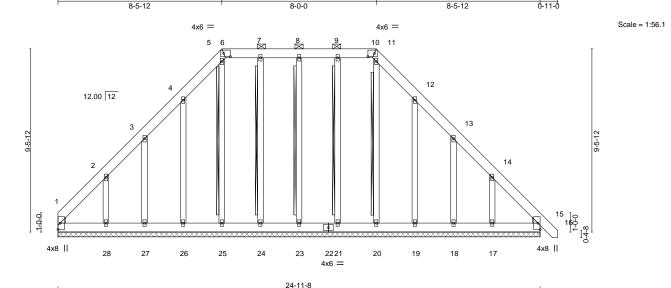


Plate Offsets (X,Y)	[6:0-4-2,0-2-0], [10:0-4-2,0-2-0]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.04	Vert(LL) 0.00 15 n/r 120	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) 0.00 15 n/r 120	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.13	Horz(CT) 0.01 15 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 242 lb FT = 20%

24-11-8

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

OTHERS 2x4 SP No.2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-

BOT CHORD WEBS Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-10.

Rigid ceiling directly applied or 10-0-0 oc bracing.

T-Brace: 2x4 SPF No.2 - 8-23, 7-24, 5-25, 9-21,

11-20

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 24-11-8.

(lb) - Max Horz 1=-277(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 23, 24, 25, 21, 15 except

26=-129(LC 12), 27=-134(LC 12), 28=-228(LC 12), 19=-125(LC 13), 18=-136(LC

13), 17=-214(LC 13), 1=-120(LC 10)

4 0 0 0 01 140 0 4 0 0 0 0

Max Grav All reactions 250 lb or less at joint(s) 23, 24, 25, 26, 27, 21, 20, 19,

18, 17, 1, 15 except 28=266(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-317/250 WEBS 2-28=-252/242

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 23, 24, 25, 21, 15 except (jt=lb) 26=129, 27=134, 28=228, 19=125, 18=136, 17=214, 1=120.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 15.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



November 9,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1265	B1	ATTIC	6	1	E16391030
30322-1203	ы	ATTIC	6	'	Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314, 8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:33 2021 Page 1

Structural wood sheathing directly applied or 5-1-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 13

		ID:J6a	Sr?qB6etazEy	6hKRSkZzPTZjoo	dWrCQF3K6bT	TpyDC1VO8Z7?ANYi2dV2IIso	JVKyLENS
5-5-8	9-1-13	10-11-8 12-9-3	16-5-8	21-11-0	22-10 _t 0		
5-5-8	3-8-5	1-9-11 1-9-11	3-8-5	5-5-8	0-11-0		

Scale = 1:71.8 6x8 =

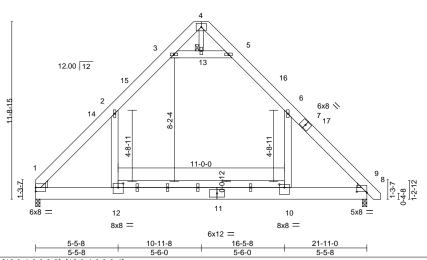


Plate Offsets (X,Y)	[8:0-8-0,0-0-13], [10:0-4-0,0-2-8], [12:0-4-0,0-3-4]

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.73	Vert(LL) -0.21 10-12 >999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.69	Vert(CT) -0.36 10-12 >720 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.14	Horz(CT) 0.01 8 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.08 10-12 >999 240	Weight: 248 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

JOINTS

LUMBER-

TOP CHORD 2x8 SP No.1

2x10 SP No.1 *Except* 10-12: 2x6 SP No.1 **BOT CHORD**

2x6 SP No.1 *Except* WEBS

4-13: 2x4 SP No.2

WEDGE

Left: 2x6 SP No.2, Right: 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 8=0-3-8

Max Horz 1=-277(LC 8)

Max Grav 1=1413(LC 21), 8=1459(LC 21)

BOT CHORD 1-12=0/1085, 10-12=0/1085, 8-10=0/1085

WEBS 6-10=0/975, 2-12=0/889, 3-13=-1621/246, 5-13=-1621/246

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-7-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x6 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-13, 5-13; Wall dead load (5.0psf) on member(s).6-10, 2-12
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 10-12
- 8) Attic room checked for L/360 deflection.



November 9,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1265	B1-GR	ATTIC	1	2	E16391031
				၂ ၁	Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:36 2021 Page 1 ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-7NIeUDT8LFUAKHhot935mClWcaZIFxRU_G5l5fyLENP

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 11

Scale = 1:77.0



5-6-0

5x8 = 5x8 = VERIFY LOADING, BY OTHERS 12.00 12 11 2x6 || 4x12 || 4x12 || 11-0-0 × 18 8 10 14 13 15 16 19 20 2x6 || 2x6 || 2x6 || 10x10 = 6x12 = 10x10 = 2x6 ||

Plate Offsets (X,Y)		[2:0-9-12,0-1-4], [6:0-9-1	:0-9-12,0-1-4], [6:0-9-12,0-1-4], [7:Edge,0-3-0], [8:0-5-0,0-2-0], [10:0-5-0,0-2-0]										
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.ó	Plate Grip DOL	1.15	TC	0.72	Vert(LL)	-0.29	8-1Ó	>905	360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	ВС	0.76	Vert(CT)	-0.39	8-10	>666	240			
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.38	Horz(CT)	0.02	7	n/a	n/a			
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S	Wind(LL)	0.01	10	>999	240	Weight: 801 lb	FT = 20%	

BRACING-

JOINTS

TOP CHORD

BOT CHORD

5-6-0

LUMBER-

TOP CHORD 2x10 SP 2400F 2.0E
BOT CHORD 2x10 SP No.1 *Except*
8-10: 2x6 SP No.1 *Except*

WEBS 2x6 SP No.1 *Except* 4-11: 2x4 SP No.2

REACTIONS. (size) 1=0-4-0, 7=0-4-0

Max Horz 1=271(LC 5)

Max Grav 1=9588(LC 14), 7=9573(LC 14)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-10113/0, 2-3=-4213/35, 3-4=-19/3638, 4-5=-20/3649, 5-6=-4203/35, 6-7=-10124/0

BOT CHORD 1-10=0/5656, 8-10=0/5720, 7-8=0/5656

WEBS 6-8=0/8207, 2-10=0/8177, 3-11=-11296/4, 5-11=-11296/4, 4-11=0/835

NOTES-

- 1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 - Top chords connected as follows: 2x10 2 rows staggered at 0-9-0 oc.
 - Bottom chords connected as follows: 2x10 5 rows staggered at 0-4-0 oc.
- Webs connected as follows: 2x6 2 rows staggered at 0-9-0 oc, 2x4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) Concentrated loads from layout are not present in Load Case(s): #3 Dead + Uninhabitable Attic Without Storage; #4 Dead + 0.6 MWFRS Wind (Pos. Internal) Left; #5 Dead + 0.6 MWFRS Wind (Pos. Internal) Right; #6 Dead + 0.6 MWFRS Wind (Neg. Internal) Left; #7 Dead + 0.6 MWFRS Wind (Neg. Internal) Right; #8 Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel; #9 Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel; #10 Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel; #11 Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel; #12 Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel; #13 Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel; #20 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) Left); #21 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) 1st Parallel

November 9,2021

(6) nThisecuse page gen designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

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818 Soundside Road

Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1265	B1-GR	ATTIC	1	_	E16391031
				3	Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:36 2021 Page 2 $ID: J6aSr? qB6etazEy6hKR\overset{\circ}{S}kZzPTZ_-7NIeUDT8LFUAKHhot935mClWcaZIFxRU_G5l5fyLENP$

NOTES-

- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-11, 5-11; Wall dead load (5.0psf) on member(s).6-8, 2-10
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-10
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1726 lb down at 1-11-12, 1726 lb down at 3-11-12, 3826 lb down at 5-2-12, 376 lb down and 34 lb up at 5-11-12, 376 lb down and 34 lb up at 7-11-12, 376 lb down and 34 lb up at 9-11-12, 376 lb down and 34 lb up at 11-11-12, 376 lb down and 34 lb up at 13-11-12, 376 lb down and 34 lb up at 15-11-12, 3826 lb down at 16-8-4, and 1726 lb down at 17-11-12, and 1726 lb down at 19-9-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 11) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-80, 3-4=-60, 4-5=-60, 5-6=-80, 6-7=-60, 1-10=-20, 8-10=-40, 7-8=-20, 3-5=-20

Drag: 6-8=-10, 2-10=-10

Concentrated Loads (lb)

Vert: 9=-62(B) 8=-1029(B) 10=-1029(B) 12=-430(B) 13=-430(B) 14=-62(B) 15=-62(B) 16=-62(B) 17=-62(B) 18=-62(B) 19=-430(B) 20=-430(B)

Job	1	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
103	22-1265	B1GE	GABLE	1	1	E16391032
303	22-1200	DIGE	GABLE	'	'	Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314, 8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:34 2021 Page 1 HBWycB1myLENR

			ID.JOASI (4D	oelaz Eyonki	NOKZZF IZD_DUJA	KIGEE 332AC	ZIIIKUUIIIIIAAIIIVZIIAND
-Q-11 ₁ 0	5-5-8	9-1-13	10-11-8 12-9-3	16-5-8	21-11-0	22-10 _T 0	
0-11-h	5-5-8	3-8-5	1-9-11 1-9-11	3-8-5	5-5-8	0-11-h	

Scale = 1:73.2 6x8 =

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 23

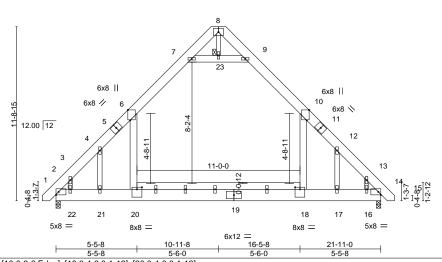


Plate Offs	sets (X,Y)	[6:0-8-6,Edge], [10:0-8-6	i,Edge], [18:0-	·4-0,0-1-12], [2	20:0-4-0,0-1	-12]						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.72	Vert(LL)	-0.17 18-20	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.63	Vert(CT)	-0.30 18-20	>855	240			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.17	Horz(CT)	0.01 14	n/a	n/a			
BCDI	10.0	Code IRC2015/T	PI2014	Matri	x-S	Wind(LL)	0.08.18-20	>999	240	Weight: 261 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

JOINTS

LUMBER-

TOP CHORD 2x8 SP No.1

2x10 SP No.1 *Except* **BOT CHORD**

18-20: 2x6 SP No.1 2x6 SP No.1 *Except* WEBS

8-23: 2x4 SP No.2

OTHERS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 14=0-3-8

Max Horz 2=349(LC 11)

Max Grav 2=1443(LC 20), 14=1443(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1821/0, 3-4=-1558/0, 4-6=-1957/23, 6-7=-1028/184, 7-8=-60/389, 8-9=-61/390, 9-10=-1028/184, 10-12=-1956/22, 12-13=-1557/0, 13-14=-1821/0 TOP CHORD

BOT CHORD 2-22=0/1098, 21-22=0/1110, 20-21=0/1095, 18-20=0/1095, 17-18=0/1095, 16-17=0/1109,

14-16=0/1092

WEBS 10-18=0/1192, 6-20=0/1192, 7-23=-1506/324, 9-23=-1506/324, 4-21=-717/135,

3-22=0/341, 12-17=-717/135, 13-16=0/341

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x6 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (10.0 psf) on member(s). 6-7, 9-10, 7-23, 9-23; Wall dead load (5.0psf) on member(s).10-18, 6-20
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 18-20 10) Attic room checked for L/360 deflection.



November 9,2021

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Starty Information

Ansity Prevent



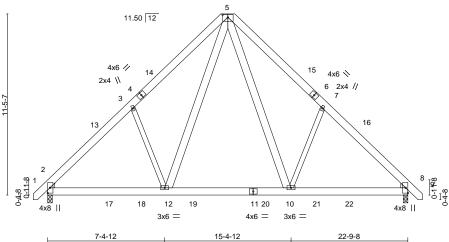
Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road		
J0322-1265	C1	COMMON	2	1	E16391033		
				·	Job Reference (optional)		
Comtech, Inc, Fayetteville, NC - 28314,			8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:37 2021 Page 1				

ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-cZs0hZTm6Zc1yQG?RsaKIPHqn_?J_PKdDwqre5yLENO 11-4-12 17-4-12 0-11-0 6-0-0 6-0-0

> Scale = 1:68.6 5x8 =

> > Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



7-4-12 8-0-0 Dieta Offesta (V.V.) [E₁O₁ A₁ O₁ O₂ O₁

Flate Olisets	(^, i)	[3.0-4-0,0-2-0]										
LOADING (p	osf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP	
TCLL 20	0.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	-0.05 10-12	>999	360	MT20	244/190	
TCDL 10	0.0	Lumber DOL	1.15	BC	0.26	Vert(CT)	-0.07 10-12	>999	240			
BCLL (0.0 *	Rep Stress Incr	YES	WB	0.27	Horz(CT)	0.01 8	n/a	n/a			
BCDL 10	0.0	Code IRC2015/TF	PI2014	Matri	x-S	Wind(LL)	0.01 2-12	>999	240	Weight: 204 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WFBS

2x6 SP No.1 *Except* 7-10,3-12: 2x4 SP No.2

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=-280(LC 10)

Max Uplift 2=-42(LC 12), 8=-42(LC 13) Max Grav 2=1057(LC 19), 8=1057(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1210/251, 3-5=-1109/415, 5-7=-1110/415, 7-8=-1210/251

BOT CHORD 2-12=-91/943, 10-12=0/619, 8-10=-42/811

WEBS 5-10=-201/658, 7-10=-406/304, 5-12=-201/657, 3-12=-406/304

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 11-4-12, Exterior(2) 11-4-12 to 15-9-9, Interior(1) 15-9-9 to 23-6-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



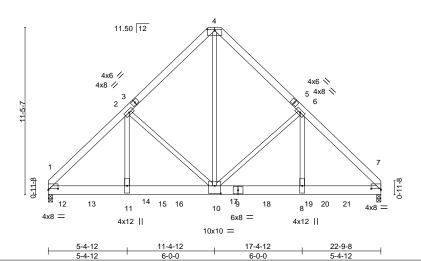
Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
					E16391034
J0322-1265	C1-GR	COMMON GIRDER	1	2	
					Job Reference (optional)
Comtech, Inc, Fayet	teville, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:40 2021 Page 1
		ID: 10-0	0-00-1	E OF KROOT	7-DT7-00V0 II-W-DU I0-0074-0-ITD0-D40VEO-I-ENI



5x12 = Scale = 1:74.3

Structural wood sheathing directly applied or 5-5-14 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



5-4-12 [1:0-8-0,0-0-15], [4:0-6-0,0-1-0], [7:0-8-0,0-0-15], [10:0-5-0,0-6-4] Plate Offsets (X,Y)--LOADING (psf) SPACING-CSI DEFL **PLATES** GRIP 2-0-0 in I/defl L/d (loc) **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) -0.10 8-10 >999 360 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 ВС 0.40 Vert(CT) -0.17 8-10 >999 240

> BRACING-TOP CHORD

BOT CHORD

BCLL 0.0 Rep Stress Incr NO WB 0.94 Horz(CT) 0.04 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Wind(LL) 0.05 8-10 >999 240 Weight: 396 lb FT = 20%

LUMBER-TOP CHORD 2x6 SP No.1

BOT CHORD 2x8 SP 2400F 2.0E WEBS 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 7=0-3-8

Max Horz 1=-270(LC 23) Max Uplift 1=-225(LC 9), 7=-237(LC 8) Max Grav 1=7977(LC 2), 7=8527(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-8698/296, 2-4=-5745/316, 4-6=-5744/316, 6-7=-8803/299 BOT CHORD 1-11=-233/5962, 10-11=-233/5971, 8-10=-139/6041, 7-8=-139/6032

WEBS 4-10=-299/7498, 6-10=-2640/260, 6-8=-42/3902, 2-10=-2545/257, 2-11=-37/3758

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-6-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) *This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=225, 7=237.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1246 lb down and 39 lb up at 0-10-4, 1243 lb down and 42 lb up at 2-10-4, 1243 lb down and 42 lb up at 4-10-4, 1243 lb down and 42 lb up at 8-10-4, 1243 lb down and 42 lb up at 10-10-4, 1243 lb down and 42 lb up at 12-10-4, 1243 lb down and 42 lb up at 12-10-4, 1243 lb down and 42 lb up at 14-10-4, 1243 lb down and 42 lb up at 12-10-4, 1243 lb down and 42 lb up at 12-10-4, 1243 lb down and 42 lb up at 12-10-4, 1243 lb down and 42 lb up at 12-10-4, 1243 lb down and 42 lb up at 120-4-4, and 1243 lb down and 35 lb up at 12-7-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15



November 9,2021

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, cerection and bracing of trusses and truss systems, see

ANSITYPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road

Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road	٦
J0322-1265	C1-GR	COMMON GIRDER	1		E16391034	
30322-1203	C1-GK	COMMON GINDER	'	2	Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:40 2021 Page 2 ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-08Y9JbWePU_bpu?a6?71w2vITB?wBcg4vu3VEQyLENL

LOAD CASE(S) Standard

Uniform Loads (plf)

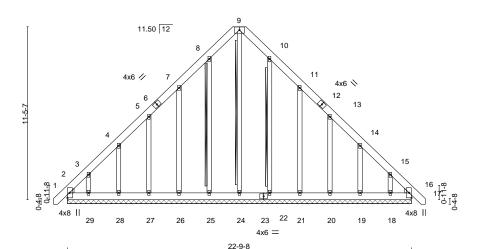
Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 9=-969(B) 7=-976(B) 12=-972(B) 13=-969(B) 14=-969(B) 15=-969(B) 16=-969(B) 17=-969(B) 18=-969(B) 19=-969(B) 20=-969(B) 21=-969(B)

Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road	
10000 4005	0405	COMMON CURRORTER CAR				E16391035
J0322-1265	C1GE	COMMON SUPPORTED GAB	1	1	Job Reference (optional)	
Comtech, Inc.	Fayetteville, NC - 28314,			130 e Aug	16 2021 MiTek Industries, Inc. Mon No	v 8 13:12:38 2021 Page 1
Connecti, inc,	1 ayottovillo, 140 - 20014,				10 2021 WILLER HIGGSHES, HIC. WOLLING	

5x8 = Scale = 1:71.8



22-9-8 LOADING (psf) SPACING-CSI. DEFL. **PLATES** GRIP 2-0-0 in (loc) I/defl L/d Plate Grip DOL Vert(LL) 244/190 **TCLL** 20.0 1.15 TC 0.05 -0.00 120 MT20 16 n/r TCDL 10.0 Lumber DOL 1.15 вс 0.03 Vert(CT) -0.00 16 n/r 120 WB **BCLL** 0.0 Rep Stress Incr YES 0.18 Horz(CT) 0.01 16 n/a n/a Weight: 227 lb BCDL Code IRC2015/TPI2014 FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No.2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-

TOP CHORD BOT CHORD WEBS Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.
T-Brace:

2x4 SPF No.2 - 9-24, 8-25, 10-22

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 22-9-8.

(lb) - Max Horz 2=-350(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 25, 22 except 2=-170(LC 10), 26=-147(LC 12), 27=-130(LC 12),

28=-137(LC 12), 29=-210(LC 12), 21=-150(LC 13), 20=-130(LC 13), 19=-136(LC 13), 18=-200(LC 13),

16=-103(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 24, 25, 26, 27, 28, 29, 22, 21, 20, 19, 18 except 2=362(LC 12), 16=317(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-506/298, 3-4=-336/228, 14-15=-285/175, 15-16=-449/302

 $24 - 25 = -221/337, \ 22 - 24 = -221/337, \ 21 - 22 = -221/337, \ 20 - 21 = -221/337, \ 19 - 20 = -220/337, \ 20 - 21 = -221/337, \ 20 - 21/337, \ 20 -$

18-19=-220/336, 16-18=-218/335

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 22 except (jt=lb) 2=170, 26=147, 27=130, 28=137, 29=210, 21=150, 20=130, 19=136, 18=200, 16=103.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



November 9,2021

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type		Qty	Ply	Lot 2 Cypress Road	E1639	11026
J0322-1265	D1	COMMON		5	1			91030
Comtech, Inc, Fayette	eville, NC - 28314,			8.	.430 s Aug	Job Reference (option 16 2021 MiTek Industrial	nal) ries, Inc. Mon Nov 8 13:12:41 2021 Page	1
-0-11-0		5-11-8	ID:J6aSr?d	qB6etazE	y6hKRSk2	zPTZUL6XXxXGAn 11-11-0	SSR2ZmgifGTFSV8bPHwHcD8Yo2nsyLEN	IK
-0-11-0 0-11-0		5-11-8				5-11-8	12-10-0 0-11-0	
							Scale:	1/2"=1'
			5x5 =					
			3					
	6.00 12			\		0		
		8			\	9		
3-8-15								
rg .	7//						10	
, 2							4 5 1	
ε _ξ 1							9-3	0.1
0.3-12							0.93	[5- [2]
	\		6					0
∠` 3x4 =	7		2x4				3x4 =	
OA.							GA.	
⊢		5-11-8 5-11-8	-			11-11-0 5-11-8		
Plate Offsets (X,Y) [2:	0-2-0,0-1-12], [4:0-2-0,0-1-12]	J-11-0				5-11-6		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	ir	n (loc)	l/defl L/d	PLATES GRIP	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.16	Vert(LL)	0.02	2-6	>999 240	MT20 244/190	
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES		Vert(CT) Horz(CT			>999 240 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	(01	, 0.01	•		Weight: 69 lb FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD BOT CHORD 2x6 SP No.1 2x6 SP No.1

2x4 SP No 2 WFBS

REACTIONS. (size) 2=0-3-0, 4=0-3-0

Max Horz 2=43(LC 11) Max Uplift 2=-106(LC 9), 4=-106(LC 8) Max Grav 2=517(LC 1), 4=517(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-626/654, 3-4=-626/654 BOT CHORD 2-6=-466/475, 4-6=-466/475

WEBS 3-6=-364/279

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-7-10 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=106, 4=106.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

November 9,2021

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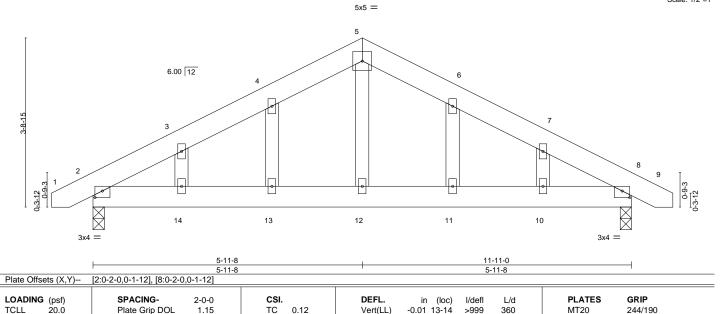
ANS/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road	
J0322-1265	D1GE	GABLE	1	1		E16391037
					Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8	13:12:42 2021 Page 1
		ll ll	D:J6aSr?qB6eta	zEy6hKR	SkZzPTZyXfvkGXvx5EJ3C8yEQAV?T?hV	?IPfksMMCYcJJyLENJ
-0-11-0		5-11-8			11-11-0	12-10-0
0-11-0		5-11-8			5-11-8	0-11-0

Scale: 1/2"=1



Vert(CT)

Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

-0.02 13-14

0.02 10-11

0.01

>999

>999

n/a

240

n/a

240

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 77 lb

FT = 20%

LUMBER-

TCDL

BCLL

BCDL

TOP CHORD BOT CHORD 2x6 SP No.1 2x6 SP No.1 WFBS

10.0

0.0

10.0

2x4 SP No.2 2x4 SP No.2 **OTHERS**

REACTIONS.

(size) 2=0-3-0. 8=0-3-0 Max Horz 2=68(LC 12)

Max Uplift 2=-137(LC 9), 8=-137(LC 8) Max Grav 2=517(LC 1), 8=517(LC 1)

Lumber DOL

Rep Stress Incr

Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD $2\text{-}3\text{--}623/674,\ 3\text{-}4\text{--}555/669,\ 4\text{-}5\text{--}530/694,\ 5\text{-}6\text{--}530/694,\ 6\text{-}7\text{--}555/669,\ 7\text{-}8\text{--}623/674}$ 2-14=-490/476, 13-14=-490/476, 12-13=-490/476, 11-12=-490/476, 10-11=-490/476, BOT CHORD

1.15

YES

8-10=-490/476 **WEBS** 5-12=-372/227

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; DCDL=6.0psf; gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

ВС

WB 0.06

Matrix-S

0.14

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=137, 8=137.



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Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1265	M1	MONOPITCH	11	1	E16391038
00022 1200		Interversion			Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:42 2021 Page 1

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

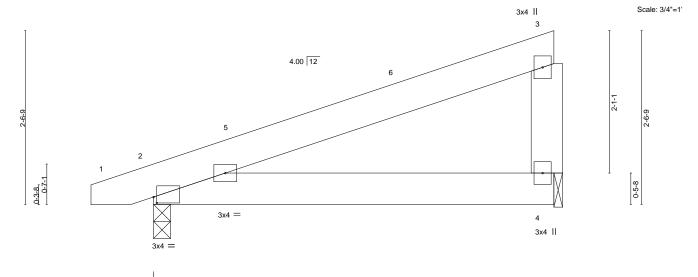


Plate Off:	sets (X,Y)	[2:0-0-9,0-1-1]											
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DE	FL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.15	TC	0.19	Ver	rt(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.12	Ver	rt(CT)	-0.03	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Hor	rz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-P	Wir	nd(LL)	0.03	2-4	>999	240	Weight: 34 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

2x6 SP No.1 WFBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=71(LC 8)

Max Uplift 2=-104(LC 8), 4=-97(LC 8) Max Grav 2=274(LC 1), 4=223(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=104.



November 9,2021





Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road	
			-			E16391039
J0322-1265	M1GE	GABLE	1	1		
					Job Reference (optional)	
Comtech, Inc, Fayettev	ville, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:43 20	21 Page 1
		ID:J6	6aSr?qB6e	etazEy6hK	RSkZzPTZQjDlycYXiPMAgLj8o7hkYgXsbP5KOBuWb	rH9rlyLENI
	-0-11-0	6-0-	-0			
	0-11-0	6-0-	-0		I	
					244 11	Scale: 3/4"=1'

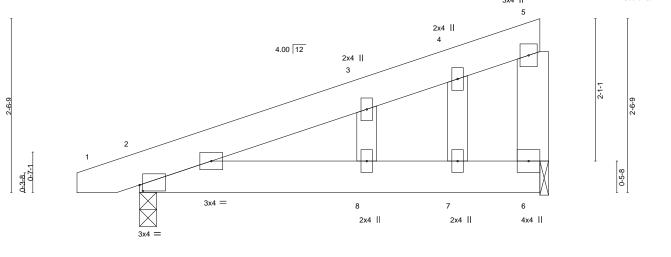


Plate Offset	ts (X,Y)	[2:0-0-9,0-1-1]										
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	0.02	2-8	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	-0.02	8	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.01	Horz(CT)	-0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2015/TP	12014	Matri	x-S						Weight: 37 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD BOT CHORD 2x6 SP No.1 2x6 SP No.1

2x6 SP No.1 WFBS

2x4 SP No.2 **OTHERS**

REACTIONS. (size) 2=0-3-0. 6=0-1-8

Max Horz 2=101(LC 8)

Max Uplift 2=-151(LC 8), 6=-142(LC 8) Max Grav 2=274(LC 1), 6=223(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable acone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 1-4-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=151, 6=142.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

November 9,2021





Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road	F.10001010
J0322-1265	РВ	Piggyback	24	1		E16391040
		331			Job Reference (optional)	
Comtech Inc	Favetteville NC - 28314	•		130 e Aug	16 2021 MiTek Industries Inc	c Mon Nov 8 13:12:44 2021 Page 1

 $ID: J6aSr? qB6etazEy6 \hbox{\r{h}} KRSkZzPTZ_-uvng9yZ9TiU1IVILLrCz5u4? doRi7evfqV1jNByLENH$ 4-0-0 4-0-0

> Scale = 1:25.9 4x4 =

> > Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

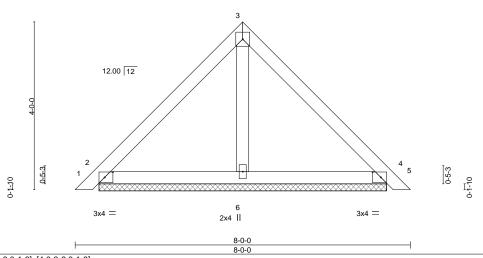


Plate Offse	ts (X,Y)	[2:0-2-6,0-1-8], [4:0-2-6,0	-1-8]									
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	0.01	5	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	0.01	5	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-P						Weight: 32 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD BOT CHORD 2x4 SP No.1 2x4 SP No.1

2x4 SP No.2 OTHERS

REACTIONS. (size) 2=6-10-6, 4=6-10-6, 6=6-10-6

Max Horz 2=-114(LC 10)

Max Uplift 2=-65(LC 13), 4=-71(LC 13)

Max Grav 2=190(LC 1), 4=190(LC 1), 6=214(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



November 9,2021



	Lot 2 Cypress Road	Ply	Qty	russ Type	Truss Type	Truss	ob
E16391041							
		1	1	ALLEY	VALLEY	V1	0322-1265
	Job Reference (optional)						
, Inc. Mon Nov 8 13:12:45 2021 Page 1	16 2021 MiTek Industries, Ir	430 s Aug	8.			eville, NC - 28314,	Comtech, Inc, Fayette
ftXvYjCd5dB6CmNs2Jp29mGwdyLENG	ZzPTZN6L2MlanE0cuwft)	Ey6hKRSk	?qB6etaz	ID:J6a			
		20-5-		10-2-8	10-2-8	_	
)	10-2-9		10-2-8	10-2-8	ı	
Scale = 1:59.				4x4 =			

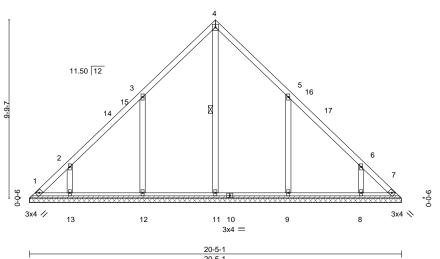


Plate Offs	ets (X,Y)	[5:0-0-0,0-0-0], [6:0-0-0,0)-0-0]										
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.16	Vert(LL)	n/a	` -	n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.19	Vert(CT)	n/a	-	n/a	999			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.21	Horz(CT)	0.00	7	n/a	n/a			
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 105 lb	FT = 20%	

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-11

REACTIONS. All bearings 20-5-1.

(lb) - Max Horz 1=226(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 12=-173(LC 12), 13=-126(LC 12), 9=-173(LC 13),

8=-126(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=440(LC 22), 12=486(LC 19), 13=285(LC 19),

9=485(LC 20), 8=285(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-12=-393/294, 2-13=-299/243, 5-9=-393/294, 6-8=-299/243

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-6 to 4-9-2, Interior(1) 4-9-2 to 10-2-8, Exterior(2) 10-2-8 to 14-7-5, Interior(1) 14-7-5 to 20-0-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 12=173, 13=126, 9=173, 8=126.



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Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1265	V2	VALLEY	1	,	E16391042
JU322-1205	VZ	VALLEY	'	'	Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:46 2021 Page 1 $ID: J6aSr? qB6etazEy6hKR\mathring{S}kZzPTZ_-rIvQaeaP?KklXpSjTFERAJ9Mfc6ebWmyHpWpS4yLENFALAMART AND STANDARD AND STA$

Scale = 1:49.9



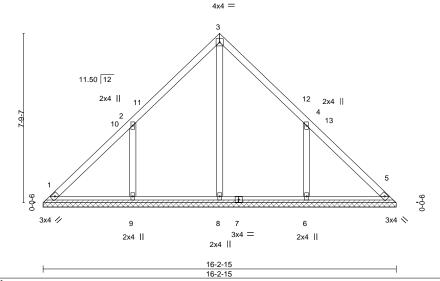


Plate Offsets (X,Y)--[4:0-0-0,0-0-0] LOADING (psf) SPACING-CSI. DEFL. **PLATES GRIP** 2-0-0 I/defI L/d 244/190 **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.17 Vert(LL) n/a n/a 999 MT20 TCDL Lumber DOL 10.0 1.15 ВС 0.19 Vert(CT) n/a n/a 999 0.13 BCLL 0.0 Rep Stress Incr YES WB Horz(CT) 0.00 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 77 lb FT = 20%

LUMBER-TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 **BRACING-**TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 16-2-15.

(lb) - Max Horz 1=-178(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-182(LC 12), 6=-182(LC 13)

All reactions 250 lb or less at joint(s) 1, 5 except 8=418(LC 22), 9=503(LC 19), 6=503(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-9=-406/302, 4-6=-406/302

NOTES-

OTHERS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-6 to 4-9-2, Interior(1) 4-9-2 to 8-1-8, Exterior(2) 8-1-8 to 12-6-4, Interior(1) 12-6-4 to 15-10-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=182, 6=182,



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Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1265	V3	VALLEY	1	1	E16391043
30322-1203	V 3	VALLE	'		Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:47 2021 Page 1 $ID: J6aSr? qB6etazEy6hKRS\check{k}ZzPTZ_-JUTon_b1mdsc9z1w1zlgiWiXz0TMK_y6WTFN_WyLENE$

6-0-7 12-0-14 6-0-7

> Scale = 1:36.2 4x4 =

> > Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

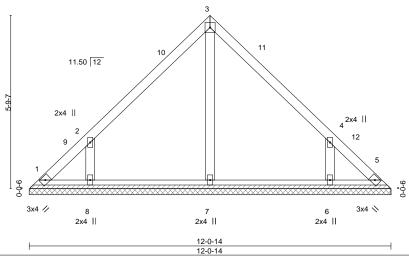


Plate Offsets (X,) [4:0-0-0,0-0-0]			
LOADING (psf)	SPACING- 2-0-0	CSI. DEFL.	in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.13 Vert(LL) n/a - n/a 999	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09 Vert(C1	Γ) n/a - n/a 999	
BCLL 0.0	* Rep Stress Incr YES	WB 0.07 Horz(C	r) 0.00 5 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 53 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD

2x4 SP No.2 OTHERS

REACTIONS. All bearings 12-0-14.

(lb) - Max Horz 1=-130(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-150(LC 12), 6=-150(LC 13)

All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=334(LC 19), 6=334(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-344/282, 4-6=-344/282

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-6 to 4-9-2, Interior(1) 4-9-2 to 6-0-7, Exterior(2) 6-0-7 to 10-5-4, Interior(1) 10-5-4 to 11-8-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=150, 6=150,



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Job	Truss	Truss	Туре		Qty	Ply	Lot 2 Cypress Road		
J0322-1265	V4	VALLE	v		1	1			E16391044
30322-1203	V-4	VALLE	. 1			'	Job Reference (option	onal)	
Comtech, Inc,	Fayetteville, NC - 28314,	<u> </u>					16 2021 MiTek Indus	stries, Inc. Mon Nov 8 1	
				ID:J6aSr	'qB6etazl			_Tn7c6agHvFkFhXQpV	V3SsFI7?wWyyLEND
		 	3-11-6 3-11-6			7-10-1 3-11-6	2		
			0110			0110	•		
				4x4 =					Scale = 1:25.3
	Ŧ			2					
				// \ `					
		1	1.50 12	/ `					
	3-9-6				\				
	ૡ૾ૺ					//	_		
		/	/ /				3		
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							——————————————————————————————————————		
	9-6-0				/////			9-0-0	
	Ó	·····		~~~~~	·····	*****	·····	Ó	
				4					
		3x4 //		2x4			3x4 📏		
		-		7-10-12					
		'	1	7-10-12					
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	I/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DC		TC 0.21	Vert(LL)	n/a		n/a 999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.10	Vert(CT)	n/a		n/a 999		
BCLL 0.0 *			WB 0.03	Horz(CT)	0.00	3	n/a n/a		
BCDL 10.0	Code IRC201	5/TPI2014	Matrix-P					Weight: 31 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 **OTHERS**

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=7-10-12, 3=7-10-12, 4=7-10-12

Max Horz 1=-82(LC 8)

Max Uplift 1=-30(LC 13), 3=-31(LC 13)

Max Grav 1=173(LC 1), 3=173(LC 1), 4=228(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



November 9,2021





Job Truss Truss Type Qty Ply Lot 2 Cypress Road F16391045 J0322-1265 V5 VALLEY Job Reference (optional) Comtech, Inc. Fayetteville, NC - 28314, 8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:12:48 2021 Page 1 $ID: J6aSr? qB6etazEy6hKRSkZzPTZ_-nh1B?KcfXx_Tn7c6agHvFkFkIQqk3SEFI7?wWyyLEND\\$ 1-10-5 1-10-5 Scale = 1:11.7 4x4 = 2 11.50 12 3 9-0-0 9-0-0 3x4 // 2x4 || 3x4 📏 3-8-11 LOADING (psf) SPACING-2-0-0 CSI. DEFL. **PLATES GRIP** in (loc) I/defl L/d 20.0 Plate Grip DOL TC Vert(LL) 244/190 TCLL 1.15 0.03 n/a 999 MT20 n/a ВС **TCDL** 10.0 Lumber DOL 1.15 0.02 Vert(CT) n/a n/a 999 WB **BCLL** 0.0 Rep Stress Incr YES 0.01 Horz(CT) 0.00 3 n/a n/a BCDL Code IRC2015/TPI2014 Weight: 13 lb FT = 20%

LUMBER-

2x4 SP No.1 2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.2 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 3-8-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=3-8-11, 3=3-8-11, 4=3-8-11

Max Horz 1=34(LC 9)

Max Uplift 1=-12(LC 13), 3=-13(LC 13)

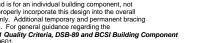
Max Grav 1=72(LC 1), 3=72(LC 1), 4=95(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



November 9,2021



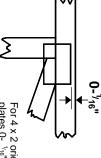


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Apply plates to both sides of truss Dimensions are in ft-in-sixteenths.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss

ω

O

S

required direction of slots in This symbol indicates the

connector plates

* Plate location details available in MiTek 20/20 software or upon request

PLATE SIZE



to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



output. Use T or I bracing Indicated by symbol shown and/or if indicated. by text in the bracing section of the

BEARING



number where bearings occur.

Min size shown is for crushing only Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint

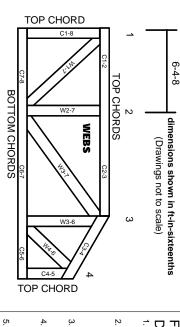
Industry Standards:

ANSI/TPI1: National Design Specification for Metal

DSB-89:

Installing & Bracing of Metal Plate Connected Wood Trusses. Plate Connected Wood Truss Construction. Guide to Good Practice for Handling, Building Component Safety Information Design Standard for Bracing

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

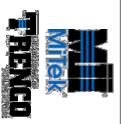
ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1

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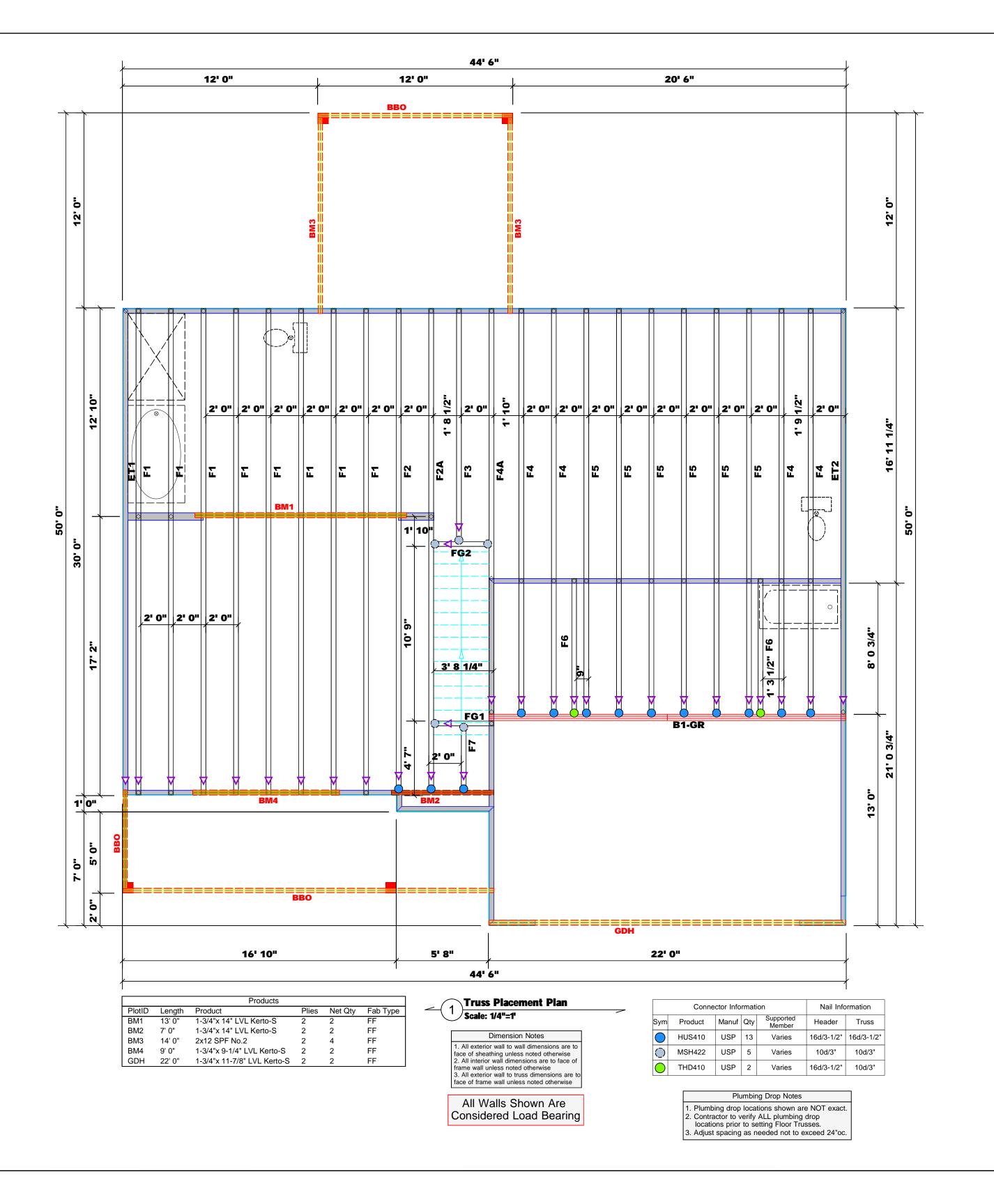


MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection. esponsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.





Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature_

David Landry

LO	AD (CHAR	RT FO	R J	ACK.	STUD	s
	(8	ASED C	N TABLES	ROOZ	5(t) & (t)	2))	
NUA	WBER C					A END OF	
END REACTION (UP 10)	REQ'O STUDS FOR (2) PLY HEADER		BND REACTION (OF TU)	REQ16 STUDS FOR (3) MY HEADER		END REACTION (UP TO)	REQ15 STUDS FOR
1700	1		2550	1		3400	1
3400	2		5100	2		6800	2
5100	3		7650	3		10200	3
6800	4		10200	4		13600	4
8500	5		12750	5		17000	5
10200	6		15300	6			
11900	7						
13600	8						
15300	9						
	1700 3400 5100 6800 10200 11900 13600	1700 1 3400 2 5100 3 6800 4 8500 5 10200 6 11900 7 13600 8	(SASED OF NUMBER OF JACK) 2	(8ASED ON TABLES) NUMBER OF TACK STUDS R HEADESON 2	SASED ON TABLES ROOF NUMBER OF LACK STUDS REQUIRED NUMBE	Compared on tables redering and number of lack stude redeviations Course of tables Course of tables	No. St. No. No.

THIS IS	BUILDER	Benjamin Stout Real Estate	COUNTY	Fayetteville / Cumberland	.0000
A TRUSS	JOB NAME	Lot 2 Cypress Road	ADDRESS	Cypress Road	7
PLACEN	PLAN	The Ashville	MODEL	Floor	
ENT DIA	SEAL DATE N/A	N/A	DATE REV.	03/09/22	
GRAM ON	QUOTE #		DRAWN BY	DRAWN BY David Landry	
LY.	JOB #	J0322-1266	SALESMAN	SALESMAN Marshall Naylor	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Client: Benjamin Stout Real Estate

Project: Address:

Cypress Road Fayetteville, NC 28304

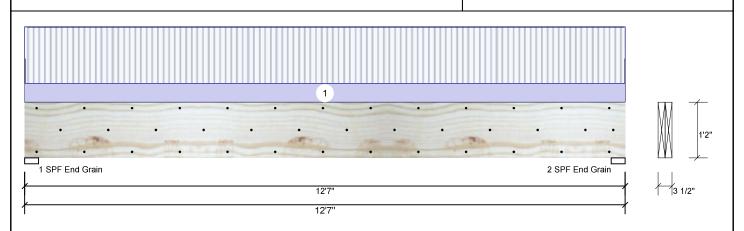
3/9/2022 Date:

Input by: David Landry Job Name: Lot 2 Cypress Road Page 1 of 12

J0322-1266 Project #: Level: Level

Reactions UNPATTERNED lb (Uplift)

Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED **BM1**



Туре:	Girder	Application:	Floor	Br	g	Live	Dead	Snow	٧	Vind	Const
Plies:	2	Design Method:	ASD	-	1	4568	1591	0		0	0
Moisture Con	dition: Dry	Building Code:	IBC/IRC 2015	2	2	4568	1591	0		0	0
Deflection LL	: 480	Load Sharing:	No								
Deflection TL	: 360	Deck:	Not Checked								
Importance:	Normal										
Temperature:	Temp <= 100°F										
'	•			В	earings	5					
				T I	Bearing	Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.
					1 - SPF	3.500"	58% 1	1591 / 4568	6159	L	D+L
					End						
Analysis Re	esults	(Grain								
Analysis	Actual Location	Allowed Capac	ity Comb.	Case I	2 - SPF End	3.500"	58% 1	1591 / 4568	6159	L	D+L
NA4	17000 ft lb 612 1/011	00000 # Ib 0 000 /	670() D.I	. ! '	LIIU						

Grain

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	17989 ft-lb	6'3 1/2"	26999 ft-lb	0.666 (67%)	D+L	L
Unbraced	17989 ft-lb	6'3 1/2"	18055 ft-lb	0.996 (100%)	D+L	L
Shear	4792 lb	11'2 1/4"	10453 lb	0.458 (46%)	D+L	L
LL Defl inch	0.252 (L/578)	6'3 1/2"	0.303 (L/480)	0.830 (83%)	L	L
TL Defl inch	0.340 (L/428)	6'3 1/2"	0.404 (L/360)	0.840 (84%)	D+L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 5
- 6

4 Top loads must be supported equally by all plies.	
5 Top must be laterally braced at a maximum of 5'4 1/2" o.c.	
6 Bottom braced at bearings.	
7 Lateral slenderness ratio based on single ply width	

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	242 PLF	726 PLF	0 PLF	0 PLF	0 PLF	F1
	Self Weight				11 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

(800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info





isDesign

Client: Benjamin Stout Real Estate

Project: Address:

Cypress Road Fayetteville, NC 28304

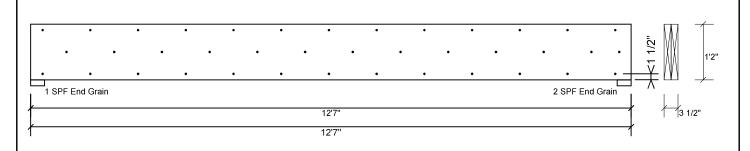
3/9/2022 Date:

Input by: David Landry Job Name: Lot 2 Cypress Road J0322-1266 Project #:

Page 2 of 12

Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED BM1

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity 0.0 % 0.0 PLF Load Yield Limit per Foot 245.6 PLF Yield Limit per Fastener 81.9 lb. IV Yield Mode Edge Distance 1 1/2" Min. End Distance 3" Load Combination Duration Factor 1.00

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023



Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633







Client: Benjamin Stout Real Estate

Project: Address:

Cypress Road Fayetteville, NC 28304

3/9/2022 Date:

189

1 - SPF 3.500"

2 - SPF 3.500"

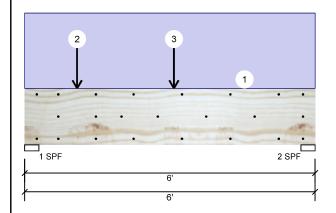
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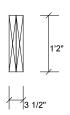
Input by: David Landry Job Name: Lot 2 Cypress Road

J0322-1266 Project #:

1.750" X 14.000" **Kerto-S LVL** 2-Ply - PASSED

Level: Level





0

D+L

D+I

Page 3 of 12

Member Ir	nformation
Type:	Girder
Plies:	2

Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal Temp <= 100°F Application: Floor Design Method: ASD **Building Code:** IBC/IRC 2015

Load Sharing: Deck:

Not Checked

Reactio	ns UNPAT	TERNED IL	(Uplift)		
Brg	Live	Dead	Snow	Wind	Const
1	678	709	0	0	0

546

27%

14%

Bearings			
Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.

709 / 678

546 / 189

1387 L

734 I

Analysis Results

Temperature:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1305 ft-lb	2'8 7/16"	26999 ft-lb	0.048 (5%)	D+L	L
Unbraced	1305 ft-lb	2'8 7/16"	17623 ft-lb	0.074 (7%)	D+L	L
Shear	1162 lb	1'4 3/4"	10453 lb	0.111 (11%)	D+L	L
LL Defl inch	0.003 (L/21799)	2'7 3/8"	0.139 (L/480)	0.020 (2%)	L	L
TL Defl inch	0.008 (L/8727)	2'10 1/16"	0.185 (L/360)	0.040 (4%)	D+L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above
2	Point	1-1-0		Far Face	238 lb	714 lb	0 lb	0 lb	0 lb	F2A
3	Point	3-1-0		Far Face	51 lb	153 lb	0 lb	0 lb	0 lb	F7
	Self Weight				11 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals Handling & Installation

- Handling & Installation

 1. IVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Metsä Wood

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA





isDesign

Client: Benjamin Stout Real Estate

Project: Address:

Cypress Road Fayetteville, NC 28304

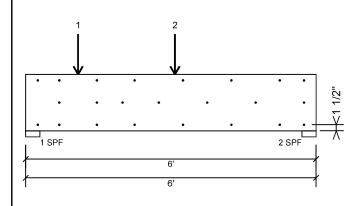
3/9/2022 Date: Input by: David Landry

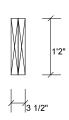
Job Name: Lot 2 Cypress Road J0322-1266 Project #:

Level: Level

Kerto-S LVL 1.750" X 14.000" BM₂

2-Ply - PASSED





Page 4 of 12

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

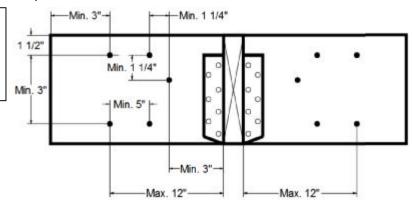
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 1-1-0 with a minimum of (6) – 10d Box nails (.128x3") in the lpattern shown

pattern snown.		
Capacity	96.9 %	
Load	476.0lb.	
Total Yield Limit	491.0 lb.	
Cg	0.9998	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Load Combination	D+L	
Duration Factor	1.00	

Min/Max fastener distances for Concentrated Side Loads



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

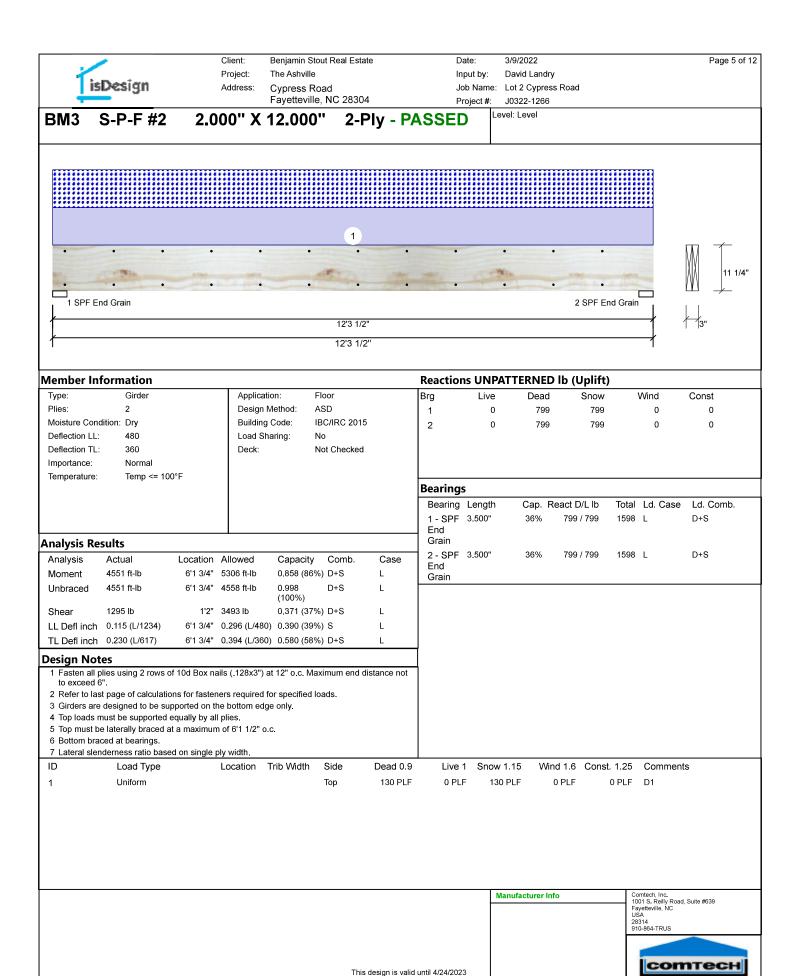
This design is valid until 4/24/2023

Manufacturer Info 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633









Client: Benjamin Stout Real Estate

Project: The Ashville Address: Cypress Ro

Cypress Road Fayetteville, NC 28304 Date: 3/9/2022

Input by: David Landry

Job Name: Lot 2 Cypress Road

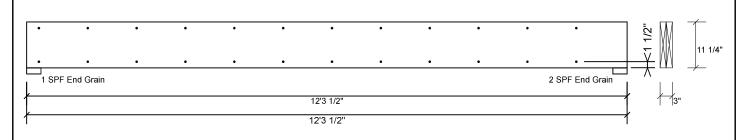
Project #: J0322-1266

Page 6 of 12

BM3 S-P-F #2 2.000" X 12.000" 2

2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

0.0 % Capacity 0.0 PLF Load Yield Limit per Foot 157.4 PLF Yield Limit per Fastener 78.7 lb. Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination Duration Factor 1.00

Manufacturer Info

Comtech, Inc.
1001 S. Reilly Road, Suite #639
Fayetteville, NC
USA
28314
910-864-TRUS

This design is valid until 4/24/2023





Client: Benjamin Stout Real Estate

Project: Address:

Cypress Road Fayetteville, NC 28304

3/9/2022 Date: Input by:

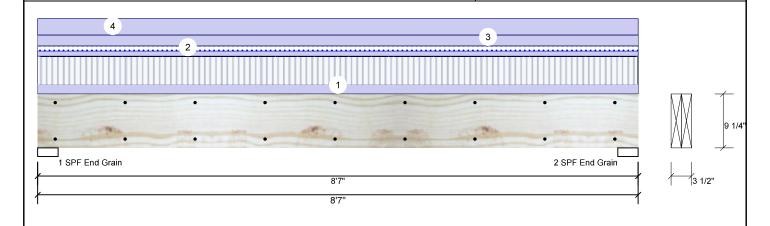
David Landry Job Name: Lot 2 Cypress Road J0322-1266 Project #:

Page 7 of 12

2-Ply - PASSED **Kerto-S LVL** 1.750" X 9.250" BM4

Level: Level

Peactions LINDATTERNED In (Linlift)



weinber if	niormation						Reaction	IS UNPAI	ICKINE	יוווקט) מו ע:)		
Type:	Girder		Applicat	ion: F	loor		Brg	Live	Dea	d Snow		Wind	Const
Plies:	2		Design I	Method: A	ASD		1	1330	200	5 240		0	0
Moisture Cor	ndition: Dry		Building	Code: I	BC/IRC 201	5	2	1330	200	5 240		0	0
Deflection LI	L: 480		Load Sh	naring: 1	10								
Deflection Tl	L: 360		Deck:	1	Not Checked	l							
Importance:	Normal												
Temperature	: Temp <=	100°F											
							Bearing	S					
							Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	3.500"	31%	2005 / 1330	3335	L	D+L
							End						
Analysis R	esults						Grain						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2-SPF	3.500"	31%	2005 / 1330	3335	L	D+L
Moment	6413 ft-lb	4'3 1/2"	12542 ft-lb	0.511 (51%	5) D+L	L	End Grain						

L

L

L

Design Notes

Unbraced

Shear

1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".

4'3 1/2" 8468 ft-lb

1' 6907 lb

4'3 9/16" 0.203 (L/480) 0.370 (37%) L

4'3 9/16" 0.271 (L/360) 0.690 (69%) D+L

0.757 (76%) D+L

0.370 (37%) D+L

- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.

6413 ft-lb

2558 lb

LL Defl inch 0.075 (L/1301)

TL Defl inch 0.188 (L/519)

5 Top braced at bearings.

Member Information

- 6 Bottom braced at bearings.
- al slenderness ratio based on single ply width

i Lateral sienderness ratio based on single ply width.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	104 PLF	310 PLF	0 PLF	0 PLF	0 PLF	F1	
2	Uniform			Тор	56 PLF	0 PLF	56 PLF	0 PLF	0 PLF	M1	
3	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above	
4	Uniform			Тор	180 PLF	0 PLF	0 PLF	0 PLF	0 PLF	C1GE	
	Self Weight				7 PI F						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals Handling & Installation

- Handling & Installation

 1. IVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info





isDesign

Client: Benjamin Stout Real Estate

Project: Address:

Cypress Road Fayetteville, NC 28304

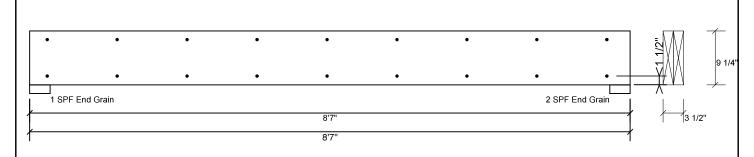
3/9/2022 Date:

Input by: David Landry Job Name: Lot 2 Cypress Road J0322-1266

Page 8 of 12

Project #: **Kerto-S LVL** 2-Ply - PASSED 1.750" X 9.250" BM4

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity 0.0 % 0.0 PLF Load Yield Limit per Foot 163.7 PLF Yield Limit per Fastener 81.9 lb. Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination Duration Factor 1.00

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 4/24/2023

Manufacturer Info 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633







Client: Benjamin Stout Real Estate

Project: Address:

Cypress Road Fayetteville, NC 28304

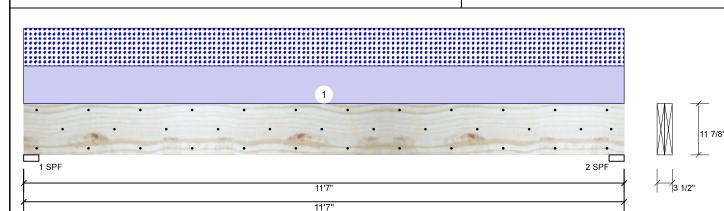
3/9/2022 Date:

Input by: David Landry Job Name: Lot 2 Cypress Road J0322-1266 Project #:

Level: Level

Page 9 of 12

Kerto-S LVL 2-Ply - PASSED 1.750" X 11.875" BM₅



Member Information				Reactio	Reactions UNPATTERNED lb (Uplift)						
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const		
Plies:	2	Design Method:	ASD	1	0	1559	1506	0	0		
Moisture Conditio	n: Dry	Building Code:	IBC/IRC 2015	2	0	1559	1506	0	0		
Deflection LL:	480	Load Sharing:	No								
Deflection TL:	360	Deck:	Not Checked								
Importance:	Normal										
Temperature:	Temp <= 100°F										
				Bearing	js						
				Bearing	Length	Cap. Read	ct D/L lb	Total Ld. Case	Ld. Comb.		
				1 - SPF	3.500"	59% 155	9 / 1506	3065 L	D+S		
				2 - SPF	3.500"	59% 155	9 / 1506	3065 L	D+S		

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8188 ft-lb	5'9 1/2"	22897 ft-lb	0.358 (36%)	D+S	L
Unbraced	8188 ft-lb	5'9 1/2"	8589 ft-lb	0.953 (95%)	D+S	L
Shear	2935 lb	1'2 5/8"	10197 lb	0.288 (29%)	D+S	L
LL Defl inch	0.103 (L/1298)	5'9 1/2"	0.278 (L/480)	0.370 (37%)	S	L
TL Defl inch	0.209 (L/637)	5'9 1/2"	0.371 (L/360)	0.560 (56%)	D+S	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	260 PLF	0 PLF	260 PLF	0 PLF	0 PLF	A2
	Self Weight				9 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Manufacturer Info Metsä Wood

301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633





isDesign

BM₅

Client: Benjamin Stout Real Estate

Project: Address:

Cypress Road Fayetteville, NC 28304

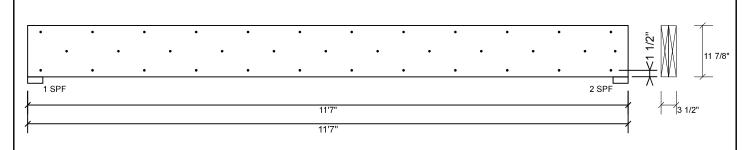
3/9/2022 Date:

Project #:

Input by: David Landry Job Name: Lot 2 Cypress Road J0322-1266

Page 10 of 12

Kerto-S LVL 2-Ply - PASSED Level: Level 1.750" X 11.875"



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity 92.1 % 260.0 PLF Load Yield Limit per Foot 282.4 PLF Yield Limit per Fastener 94.1 lb. IV Yield Mode Edge Distance 1 1/2" 3" Min. End Distance D+S Load Combination Duration Factor 1.15

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

(800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 4/24/2023



Member Information

Client: Benjamin Stout Real Estate

Project: Address:

Cypress Road Fayetteville, NC 28304

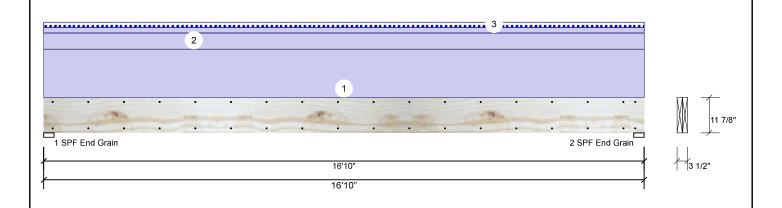
3/9/2022 Date:

Input by: David Landry Job Name: Lot 2 Cypress Road Page 11 of 12

Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED **GDH**

J0322-1266 Project #: Level: Level

Reactions UNPATTERNED lb (Uplift)



										(· p · ,			
Type:	Girder		Applicat	ion: Fl	oor		Brg	Live	Dead	l Snow	,	Wind	Const
Plies:	2		Design I	Method: A	SD		1	0	2266	168		0	0
Moisture Cond	lition: Dry		Building	Code: IB	C/IRC 2015		2	0	2266	168		0	0
Deflection LL:	480		Load Sh	aring: N	0								
Deflection TL:	360		Deck:	N	ot Checked								
Importance:	Normal												
Temperature:	Temp <= 1	00°F											
,							Bearing:	s					
							Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF End	3.500"	23%	2266 / 168	2434	L	D+S
Analysis Re	sults						Grain						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2 - SPF End	3.500"	23%	2266 / 168	2434	L	D+S
Moment	9024 ft-lb	8'5"	17919 ft-lb	0.504 (50%)) D	Uniform	Grain						
Unbraced	9694 ft-lb	8'5"	9704 ft-lb	0.999 (100%)	D+S	L							
Shear	1938 lb	15'7 3/8"	7980 lb	0.243 (24%)) D	Uniform							

Design Notes

1 Fasten all plies using 2 rows of 10d Box nails (128x3") at 12" o.c. Maximum end distance not

8'5 1/16" 0.409 (L/480) 0.090 (9%) S

8'5 1/16" 0.546 (L/360) 0.930 (93%) D+S

- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 9'6 3/4" o.c.
- 6 Bottom braced at bearings.

LL Defl inch 0.035 (L/5617)

TL Defl inch 0.506 (L/388)

7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	180 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE	
2	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above	
3	Tie-In	0-0-0 to 16-10-0	1-0-0	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	Roof Load	
	Self Weight				9 PLF						

L

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





isDesign

Client: Benjamin Stout Real Estate

Project: Address:

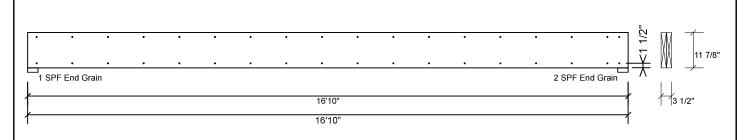
Cypress Road Fayetteville, NC 28304

3/9/2022 Date:

Input by: David Landry Job Name: Lot 2 Cypress Road Page 12 of 12

Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED **GDH**

J0322-1266 Project #: Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity 0.0 % 0.0 PLF Load Yield Limit per Foot 163.7 PLF Yield Limit per Fastener 81.9 lb. Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination Duration Factor 1.00

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Handling & Installation

1. IVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 4/24/2023

Manufacturer Info 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







RE: J0322-1266 Lot 2 Cypress Road **Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Benjamin Stout Real Estate Project Name: J0322-1266 Lot/Block: 2 Model: Ashville

Address: Cypress Road Subdivision: Cypress Road

City: Fayetteville State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: N/A Wind Speed: N/A mph Roof Load: N/A psf Floor Load: 55.0 psf

This package includes 13 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E16391095	ET1	11/9/2021
2	E16391096	ET2	11/9/2021
3	E16391097	F1	11/9/2021
4	E16391098	F2	11/9/2021
5	E16391099	F2A	11/9/2021
6	E16391100	F3	11/9/2021
7	E16391101	F4	11/9/2021
8	E16391102	F4A	11/9/2021
9	E16391103	F5	11/9/2021
10	E16391104	F6	11/9/2021
11	E16391105	F7	11/9/2021
12	E16391106	FG1	11/9/2021
13	E16391107	FG2	11/9/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



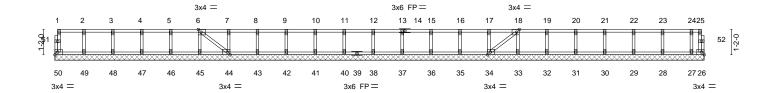
ſ	Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
	10000 4000		0.815			E16391095
ľ	J0322-1266	EI1	GABLE	1	1	Job Reference (optional)

0-<u>1</u>-8

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:25 2021 Page 1 $ID: J6aSr?qB6etazEy6hKRS \check{k}ZzPTZ_-ComAmz3tpJfDjpxDFyRbFXH3ZKSZEB?hFOTOJJyLEMe$

0-<u>1</u>-8

Scale = 1:50.0



+ 8-0-0 + 9-4-0 + 10-8-0 + 12-0-0 + 13-4-0 + 14-8-0 + 16-0-0 + 17-4-0 + 18-8-0 + 20-0-0 + 21-4-0 + 22-8-0 + 24-0-0 + 25-4-0 + 26-8-0 + 28-0-0 + 29-4-0 + 14-

_ Flate OII	Seis (A, I)	[0.0-1-0,Euge], [10.0-1-0	,⊑ugej, [34.0-1	-o,⊏ugej, [4	4.0-1-0,⊑uge	1						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	-0.00	26	n/a	n/a		
BCDL	5.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 128 lb	FT = 20%F, 11%E

BRACING-LUMBER-TOP CHORD TOP CHORD 2x4 SP No.1(flat) Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.1(flat) except end verticals. 2x4 SP No.3(flat) BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except: WFBS 2x4 SP No.3(flat) 10-0-0 oc bracing: 49-50,48-49,47-48,46-47,45-46,44-45.

REACTIONS. All bearings 29-11-0.

Max Uplift All uplift 100 lb or less at joint(s) 26 (lb) -

Max Grav All reactions 250 lb or less at joint(s) 50, 49, 48, 47, 46, 45, 44, 43, 42, 41, 40, 38, 37, 36, 35,

34, 33, 32, 31, 30, 29, 28, 27

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

OTHERS

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





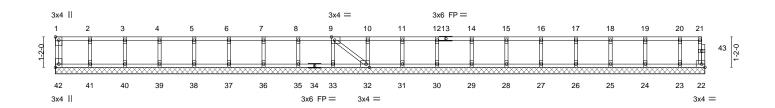


Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1266	ETO	GABLE	1	1	E16391096
30322-1266	E12	GABLE	'	'	Job Reference (ontional)

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:26 2021 Page 1 ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-g?KYzJ3Vacn4KyWPpfyqnlpEHkoozeErU2CxrlyLEMd

0-<u>1</u>-8

Scale = 1:41.7



	1-4-0 2-8-0) 4-0-0 5-4-0 6	5-8-0 8-0-0	9-4-0 10-8-0	12-0-0 13-4	14-8-0) 16-0-0	0 17-4-0	18-8-0 2	20-0-0 21-	4-0 22-8-0	24-0-0 24-11-8
	1-4-0 1-4-0)	I-4-0 1-4-0	1-4-0 1-4-0	1-4-0 1-4	-0 ' 1-4-0	1-4-0	1-4-0	1-4-0	1-4-0 ' 1-4	-0 ' 1-4-0	1-4-0 '0-11-8'
Plate Off	sets (X,Y)	[1:Edge,0-1-8], [9:0-1-8,	Edge], [32:0-1-8	3,Edge], [42:Edge,0	-1-8]							
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in ((loc) I/def	l L/d		PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC 0.06	١ ,	/ert(LL)	n/a `	n/a	a 999		MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC 0.01	'	/ert(CT)	n/a	- n/a	a 999			
BCLL	0.0	Rep Stress Incr	YES	WB 0.03		Horz(CT)	0.00	22 n/a	a n/a			
BCDL	5.0	Code IRC2015/T	TPI2014	Matrix-S						'	Weight: 106 II	b FT = 20%F, 11%E

 LUMBER

 TOP CHORD
 2x4 SP No.1(flat)
 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

 BOT CHORD
 2x4 SP No.3(flat)
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

 OTHERS
 2x4 SP No.3(flat)
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 24-11-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 42, 22, 41, 40, 39, 38, 37, 36, 35, 33, 32, 31, 30, 29, 28, 27, 26, 25, 24, 23

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.







Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
10000 1000		_			E16391097
J0322-1266	F1	Floor	8	1	Job Reference (optional)

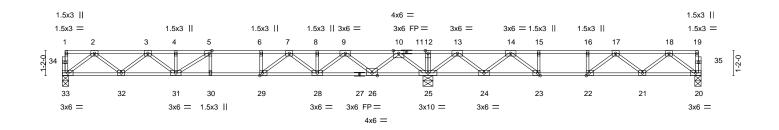
8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:28 2021 Page 1 $ID: J6aSr? qB6etazEy6hKRSkZzPTZ_-cNRIO? 5I6E1naGfow4_ltAvMLYG4RO68xMh2weyLEMb$

0-1-8

HI 1-3-0 1-6-0 2-3-0

2-2-0

0-1-8 Scale = 1:50.8



_			17-1-8					29-11-0					
		17-1-8					12-9-8						
Plate Offse	ets (X,Y)	[5:0-1-8,Edge], [22:0-1-8	,Edge], [23:0-1	-8,Edge], [29	9:0-1-8,Edge	e]							
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	40.ó	Plate Grip DOL	1.00	TC	0.86	Vert(LL)	-0.20	` 3Ó	>999	480	MT20	244/190	
TCDL	10.0	Lumber DOL	1.00	BC	0.85	Vert(CT)	-0.28	30	>735	360			
BCLL	0.0	Rep Stress Incr	YES	WB	0.58	Horz(CT)	0.05	20	n/a	n/a			
BCDL	5.0	Code IRC2015/TF	PI2014	Matri	k-S						Weight: 149 lb	FT = 20%F, 11%E	

LUMBER-

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat)

BOT CHORD WFBS

2x4 SP No.3(flat)

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 33=0-3-8, 25=0-5-8, 20=0-3-8

Max Grav 33=826(LC 3), 25=1934(LC 1), 20=608(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-3=-1695/0, 3-4=-2732/0, 4-5=-2732/0, 5-6=-2963/0, 6-7=-2963/0, 7-8=-2067/0, $8-9 = -2067/0, \ 9-10 = -539/303, \ 10-12 = 0/2152, \ 12-13 = 0/2152, \ 13-14 = -551/972,$

14-15=-1575/269, 15-16=-1575/269, 16-17=-1575/269, 17-18=-1162/0

32-33=0/1030, 31-32=0/2330, 30-31=0/2963, 29-30=0/2963, 28-29=0/2561,

 $26 - 28 = -37/1417, \ 25 - 26 = -833/0, \ 24 - 25 = -1263/0, \ 23 - 24 = -662/1135, \ 22 - 23 = -269/1575,$

21-22=-24/1515, 20-21=0/747

WEBS 2-33=-1290/0, 2-32=0/866, 3-32=-826/0, 3-31=0/514, 10-25=-1655/0, 10-26=0/1228,

9-26=-1188/0, 9-28=0/878, 7-28=-685/0, 7-29=0/807, 6-29=-359/0, 5-31=-475/153, 13-25=-1321/0, 13-24=0/882, 14-24=-926/0, 14-23=0/942, 18-20=-935/0, 18-21=0/540,

17-21=-460/99, 17-22=-345/76, 15-23=-417/0

NOTES-

BOT CHORD

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. 5) CAUTION, Do not erect truss backwards.







Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
10000 4000	F0	Flore			E16391098
J0322-1266	F2	Floor	1	1	Job Reference (optional)

1-3-0

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:29 2021 Page 1 $ID: J6aSr? qB6etazEy6hKRSkZzPTZ_-4a?hcL6NtX9eBQE_UnWXPNRXuxcHAsbHA?RbS4yLEMa$

29-7-8

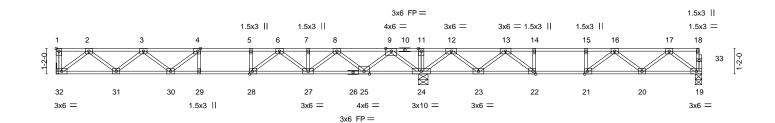
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

2-3-0

Scale = 1:49.8



0- ¹ -8 12-10-8 '
1-8,Edge]
DEFL. in (loc) I/defl L/d PLATES GRIP
Vert(LL) -0.18 28-29 >999 480 MT20 244/190
Vert(CT) -0.25 28-29 >810 360
Horz(CT) 0.05 19 n/a n/a
Weight: 146 lb FT = 20%F, 11%E

16-9-0

BOT CHORD

BRACING-LUMBER-TOP CHORD TOP CHORD 2x4 SP No.1(flat)

16-7-8

2-3-0

BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WFBS

REACTIONS.

(size) 32=Mechanical, 19=0-3-8, 24=0-5-4 Max Grav 32=814(LC 3), 19=615(LC 4), 24=1907(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1658/0, 3-4=-2582/0, 4-5=-2848/0, 5-6=-2848/0, 6-7=-2034/0, 7-8=-2034/0,

 $8-9 = -563/296, \ 9-11 = 0/2082, \ 11-12 = 0/2082, \ 12-13 = -578/905, \ 13-14 = -1611/221,$

14-15=-1611/221, 15-16=-1611/221, 16-17=-1179/0

BOT CHORD 31-32=0/1002, 30-31=0/2282, 29-30=0/2848, 28-29=0/2848, 27-28=0/2500, $25 - 27 = -36/1414,\ 24 - 25 = -810/0,\ 23 - 24 = -1188/0,\ 22 - 23 = -602/1165,\ 21 - 22 = -221/1611,$

20-21=0/1540, 19-20=0/756

WEBS 2-32=-1257/0, 2-31=0/854, 3-31=-813/0, 3-30=0/392, 4-30=-431/27, 9-24=-1619/0,

9-25=0/1194, 8-25=-1155/0, 8-27=0/843, 6-27=-652/0, 6-28=0/750, 5-28=-323/0, 17-19=-946/0, 17-20=0/550, 16-20=-470/84, 16-21=-317/90, 12-24=-1316/0,

12-23=0/876, 13-23=-920/0, 13-22=0/936, 14-22=-417/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION. Do not erect truss backwards.







Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
					E16391099
J0322-1266	F2A	Floor	1	1	Lab Datamana (anti-mal)
				I	Job Reference (optional)

1-3-0

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:31 2021 Page 1

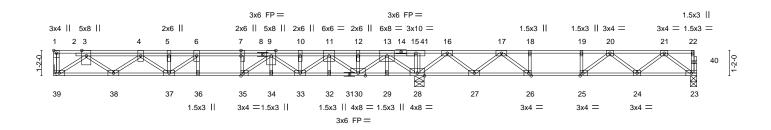
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

 $ID: J6aSr? qB6etazEy6hKRSkZzPTZ_-0y7R017eP9PMRkONcCY? UoWwalOYekCadJwiXzyLEMY\\$ 1-2-8 1-1-10 1-2-8 1-2-8 1-2-8 1-2-8 1-2-8 1-2-8 2-3-0

Scale = 1:50.0



			10-7-0		י-ערסו	U		29-7-0		
			16-7-8		0-1-8	3		12-10-8		1
Plate Offs	sets (X,Y)	[1:Edge,0-1-8], [7:0-3-0,Ed	lge], [25:0-1-	8,Edge], [26:0-1-8,Edg	e], [35:0-1-8,Edge]					
LOADING TCLL TCDL BCLL	40.0 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.00 1.00 NO	CSI. TC 0.66 BC 0.48 WB 0.69	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.15 35 -0.20 35-36	l/defl L/d >999 480 >993 360	0	PLATES MT20	GRIP 244/190
BCDL	0.0 5.0	Rep Stress Incr Code IRC2015/TPI:		WB 0.69 Matrix-S	Holz(C1)	0.04 23	n/a n/a	4	Weight: 173 lb	FT = 20%F, 11%E

BOT CHORD

LUMBER-**BRACING-**TOP CHORD 2x4 SP 2400F 2.0E(flat) TOP CHORD

BOT CHORD 2x4 SP 2400F 2.0E(flat) 2x4 SP No 3(flat) WFBS

REACTIONS. (size) 39=Mechanical, 28=0-5-4, 23=0-3-8

Max Grav 39=951(LC 3), 28=2436(LC 1), 23=579(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-2099/0, 4-5=-3320/0, 5-6=-3320/0, 6-7=-3521/0, 7-9=-3521/0, 9-10=-2750/0,

10-11=-2750/0, 11-12=-1201/0, 12-13=-1201/0, 13-15=0/2484, 15-16=0/2500,

 $16\text{-}17\text{=-}224/1085,\ 17\text{-}18\text{=-}1394/330,\ 18\text{-}19\text{=-}1394/330,\ 19\text{-}20\text{=-}1394/330,\ 20\text{-}21\text{=-}1095/0$ BOT CHORD

 $38 - 39 = 0/1255,\ 37 - 38 = 0/2907,\ 36 - 37 = 0/3521,\ 35 - 36 = 0/3521,\ 34 - 35 = 0/3180,\ 33 - 34 = 0/3180,$

32-33=0/2041, 30-32=0/2041, 29-30=-501/77, 28-29=-501/77, 27-28=-1403/0, $26\hbox{-}27\hbox{-}-753/865,\ 25\hbox{-}26\hbox{-}-330/1394,\ 24\hbox{-}25\hbox{-}-59/1406,\ 23\hbox{-}24\hbox{-}0/710$

3-39=-1541/0, 3-38=0/1072, 4-38=-1027/0, 4-37=0/515, 5-37=-259/12, 13-28=-2425/0,

13-30=0/1449, 11-30=-1115/0, 11-33=0/936, 9-33=-593/0, 9-35=0/799, 7-35=-397/0, 16-28=-1479/0, 6-37=-372/209, 16-27=0/905, 17-27=-970/0, 17-26=0/989, 18-26=-436/0,

21-23=-888/0, 21-24=-6/501, 20-24=-405/117, 20-25=-370/0

NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x6 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION. Do not erect truss backwards.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 222 lb down at 4-1-4, and 576 lb down at 15-1-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

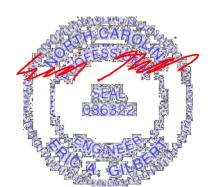
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 23-39=-10, 1-22=-100

Concentrated Loads (lb)

Vert: 4=-142(F) 13=-496(F)



November 9,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



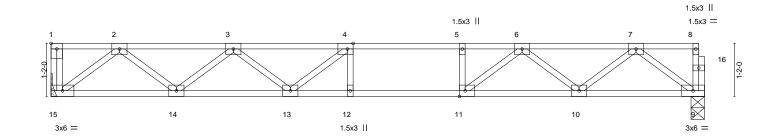
	Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
	J0322-1266	F3	Floor	1	1	E16391100
	30322-1200		11001	'	'	Job Reference (optional)
Comtach Inc. Founttoville NC 39314						16 2021 MiTak Industrias Inc. Man Nov. 9 12:12:22 2021 Page 1

Fayetteville, NC - 28314,

1-3-0

ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-U8hpEN8G9SXD2tzZ9w3E1036M9egNGIjszfF3PyLEMX 0118

Scale: 1/2"=1'



14-4-0 [1:Edge,0-1-8], [4:0-1-8,Edge], [11:0-1-8,Edge] Plate Offsets (X,Y)--LOADING (psf) SPACING-CSI. DEFL. **PLATES GRIP** 2-0-0 (loc) I/defl L/d 1.00 0.60 244/190 **TCLL** 40.0 Plate Grip DOL TC Vert(LL) -0.19 12-13 >906 480 MT20 TCDL -0.25 12-13 10.0 Lumber DOL 1.00 ВС 0.87 Vert(CT) >687 360 BCLL 0.0 Rep Stress Incr YES WB 0.38 Horz(CT) 0.04 n/a n/a BCDL Code IRC2015/TPI2014 Matrix-S Weight: 71 lb FT = 20%F, 11%E

LUMBER-

BRACING-TOP CHORD

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

2x4 SP No.3(flat) WFBS

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 15=Mechanical, 9=0-3-8 Max Grav 15=775(LC 1), 9=768(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1559/0, 3-4=-2384/0, 4-5=-2550/0, 5-6=-2550/0, 6-7=-1538/0

BOT CHORD $14 - 15 = 0/947,\ 13 - 14 = 0/2143,\ 12 - 13 = 0/2550,\ 11 - 12 = 0/2550,\ 10 - 11 = 0/2118,\ 9 - 10 = 0/954$

WEBS 2-15=-1188/0, 2-14=0/797, 3-14=-761/0, 3-13=0/398, 7-9=-1194/0, 7-10=0/760,

 $6-10=-755/0,\ 6-11=0/740,\ 5-11=-317/0,\ 4-13=-437/18$

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.



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Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1266	F4	Floor	4	1	E16391101
30322-1200	F 4	FIOOI	4	'	Job Reference (optional)

2-5-4

1-3-0

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:33 2021 Page 1 $ID: J6aSr? qB6etazEy6hKRSkZzPTZ_-zLFBRj9uwmf4g1YljdaTaDcD9Z_A6g4t5dPpbryLEMW\\$

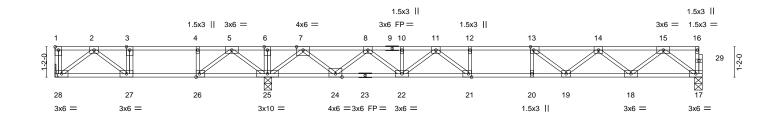
2-3-4

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

Scale = 1:41.8



	8-2-4	8-2-8	24-11-8	
	8-2-4	0-0-4	16-9-0	'
Plate Offsets (X,Y)	[1:Edge,0-1-8], [13:0-1-8,Edge	e], [21:0-1-8,Edge], [26:0-1-	3,Edge]	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	Plate Grip DOL 1. Lumber DOL 1.	0-0 CSI. 0.00 TC 0.88 0.00 BC 0.80 NO WB 0.5- 14 Matrix-S	Vert(CT) -0.28 21-22 >719 360	PLATES GRIP MT20 244/190 Weight: 125 lb FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat)

2x4 SP No.3(flat) WFBS

REACTIONS. (size) 28=Mechanical, 25=0-3-8, 17=0-3-8

Max Grav 28=1746(LC 3), 25=1571(LC 1), 17=851(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD $1-28-1403/0,\ 2-3-629/297,\ 3-4-629/297,\ 4-5-629/297,\ 5-6-0/1219,\ 6-7-0/1219,\$

7-8=-1123/0, 8-10=-2493/0, 10-11=-2493/0, 11-12=-3157/0, 12-13=-3157/0,

13-14=-2793/0, 14-15=-1763/0

BOT CHORD 27-28=-55/424, 26-27=-297/629, 25-26=-730/183, 24-25=-35/277, 22-24=0/1926,

15-18=0/917, 14-18=-871/0, 14-19=0/501, 7-25=-1524/0, 7-24=0/1128, 8-24=-1076/0,

8-22=0/753, 11-22=-559/0, 11-21=0/624, 12-21=-277/0, 13-19=-602/0

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Concentrated Loads (lb)

Vert: 17-28=-10, 1-16=-100 Vert: 1=-1350



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ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1266	F4A	Floor Girder	1	1	E16391102
30322-1200	F4A	Floor Girder	'	'	Job Reference (optional)

34

3x4 |

33

32

31

30

4x6 =

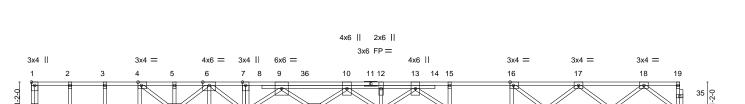
1-2-0 1-2-8 1-2-8 1-2-8 1-3-0

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:34 2021 Page 1 $ID: J6aSr? qB6etazEy6hKRSkZzPTZ_-RXpaf2AWh4nxIB7yHL5i6R8QtyJ?r5R0JH8M7IyLEMV\\$

2-3-0

Scale = 1:41.5

3x6 =



25

3x6 =

24

3x4 =

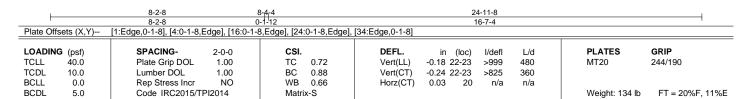
23

22

3x4 =

21

3x4 =



LUMBER-**BRACING-**TOP CHORD 2x4 SP No.1(flat) TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.1(flat) except end verticals. 2x4 SP No.3(flat) Rigid ceiling directly applied or 10-0-0 oc bracing, Except: BOT CHORD WFBS 6-0-0 oc bracing: 29-30,28-29,27-28.

27

26

4x6 = 3x6 FP =

REACTIONS. All bearings 8-4-4 except (jt=length) 20=0-3-8.

Max Uplift All uplift 100 lb or less at joint(s) except 29=-516(LC 4), 30=-346(LC 4), 31=-239(LC 4) Max Grav All reactions 250 lb or less at joint(s) 34, 30, 31, 32, 33 except 28=2681(LC 1), 28=2681(LC 1), 20=773(LC 4)

 \bowtie

3x10 =

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 4-5=0/372, 5-6=0/372, 6-7=0/2805, 7-9=0/2806, 9-10=-255/233, 10-12=-1747/0,

12-13=-1747/0, 13-15=-2589/0, 15-16=-2589/0, 16-17=-2406/0, 17-18=-1570/0

BOT CHORD 29-30=-1225/0, 28-29=-1225/0, 27-28=-1194/0, 25-27=0/1076, 24-25=0/2211,

23-24=0/2589, 22-23=0/2589, 21-22=0/2158, 20-21=0/952 **WEBS** 6-28=-1983/0. 6-29=0/505. 6-30=0/1101. 4-30=-472/0. 4-31=0/251. 9-28=-2137/0.

9-27=0/1383, 10-27=-1347/0, 10-25=0/837, 18-20=-1191/0, 18-21=0/804, 17-21=-766/0,

17-22=0/375, 16-22=-401/0, 13-25=-579/0, 13-24=0/628, 15-24=-253/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 1.5x3 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 516 lb uplift at joint 29, 346 lb uplift at joint 30 and 239 lb uplift at joint 31.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION. Do not erect truss backwards.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 407 lb down at 10-5-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 20-34=-10, 1-19=-100 Concentrated Loads (lb)

Vert: 36=-327(B)



November 9,2021

neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1266	T.F.	Floor	C	_	E16391103
JU322-1200	r5	Floor	О	'	Job Reference (optional)

2-5-4

1-3-0

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:36 2021 Page 1 ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-NwwK4kBmDh1fXVHKOl8ABsEm8m05J1pJnbdTCAyLEMT

2-3-4 0-1/8

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

Scale = 1:41.8

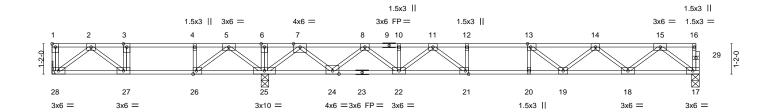


Plate Offsets (X.Y	8-2-4 0-	2-8 0-4	24-11-8 16-9-0	
Flate Offsets (A, I	[1.Euge,0-1-6], [13.0-1-6,Euge], [21.0-	-8,Eugej, [20.0-1-8,Eugej		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.74	Vert(LL) -0.21 21-22 >964 480	MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.78	Vert(CT) -0.28 21-22 >719 360	
BCLL 0.0	Rep Stress Incr YES	WB 0.54	Horz(CT) 0.05 17 n/a n/a	
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S		Weight: 125 lb FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 28=Mechanical, 25=0-3-8, 17=0-3-8

Max Uplift 28=-14(LC 4)

Max Grav 28=396(LC 3), 25=1571(LC 1), 17=851(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-629/297, 3-4=-629/297, 4-5=-629/297, 5-6=0/1219, 6-7=0/1219, 7-8=-1123/0,

 $8-10 = -2493/0,\ 10-11 = -2493/0,\ 11-12 = -3157/0,\ 12-13 = -3157/0,\ 13-14 = -2793/0,$

14-15=-1763/0

BOT CHORD 27-28=-56/423, 26-27=-297/629, 25-26=-730/184, 24-25=-34/277, 22-24=0/1927, 21-22=0/2904, 20-21=0/3157, 19-20=0/3157, 18-19=0/2432, 17-18=0/1059

2-28=-531/70, 2-27=-302/259, 5-25=-823/0, 5-26=0/878, 4-26=-429/0, 15-17=-1326/0,

15-18=0/917, 14-18=-871/0, 14-19=0/501, 7-25=-1524/0, 7-24=0/1128, 8-24=-1076/0,

 $8-22 = 0/753,\ 11-22 = -559/0,\ 11-21 = 0/624,\ 12-21 = -277/0,\ 13-19 = -603/0$

NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 28.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.



November 9,2021

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



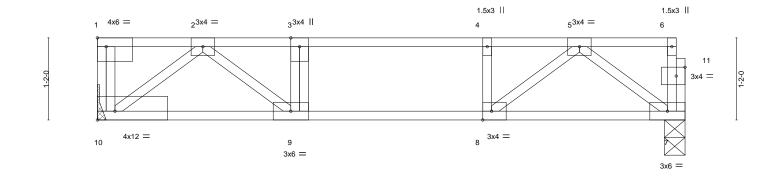
818 Soundside Road

Job	Truss	Truss Type	Qty	Ply	Lot 2 Cypress Road
J0322-1266	F6	Floor	2	1	E16391104
					Job Reference (optional)
Comtech, Inc,		8	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:36 2021 Page 1	

1-3-0

 $ID: J6aSr? qB6etazEy6hKRSkZzPTZ_-NwwK4kBmDh1fXVHKOl8ABsErcm8_J6jJnbdTCAyLEMT$

Scale = 1:15.4



[1:Edge,0-1-8], [8:0-1-8,Edge], [10:Edge,0-1-8], [11:0-1-8,0-1-8] Plate Offsets (X,Y)--LOADING (psf) SPACING-CSI. DEFL. **PLATES GRIP** 2-0-0 I/defl L/d (loc) TCLL 40.0 Plate Grip DOL 1.00 TC 0.39 Vert(LL) -0.04 9-10 >999 480 MT20 244/190 TCDL Vert(CT) 10.0 Lumber DOL 1.00 ВС 0.27 -0.05 9-10 >999 360 BCLL 0.0 Rep Stress Incr NO WB 0.22 Horz(CT) 0.01 n/a n/a BCDL Code IRC2015/TPI2014 Matrix-S Weight: 43 lb FT = 20%F, 11%E

LUMBER-

TOP CHORD BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat)

2x4 SP No.3(flat) WFBS

BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS. (size) 10=Mechanical, 7=0-3-8

Max Grav 10=3846(LC 1), 7=440(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-10=-3459/0, 2-3=-821/0, 3-4=-821/0, 4-5=-821/0

BOT CHORD 9-10=0/493, 8-9=0/821, 7-8=0/489

WEBS 2-10=-619/0, 2-9=0/460, 5-7=-609/0, 5-8=0/469

NOTES-

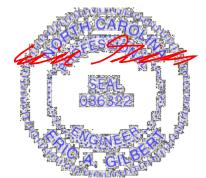
- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 7-10=-10, 1-6=-100 Concentrated Loads (lb)

Vert: 1=-3400



November 9,2021

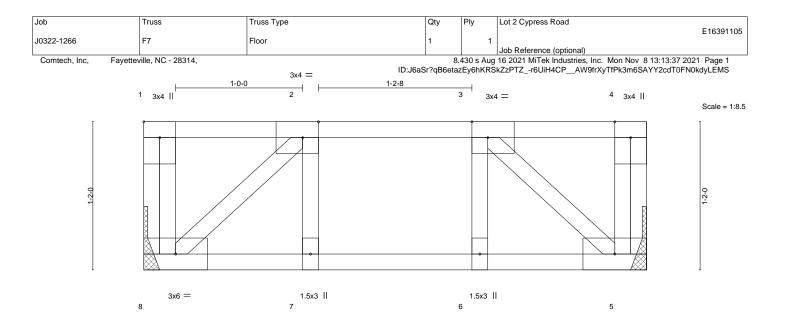
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3x6 =

Plate Offsets (X	Plate Offsets (X,Y) [1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,Edge]										
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC	0.06	Vert(LL)	-0.00	` ź	>999	480	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	ВС	0.06	Vert(CT)	-0.00	7	>999	360		
BCLL 0.0	Rep Stress Inc	r YES	WB	0.05	Horz(CT)	0.00	5	n/a	n/a		
BCDL 5.0	Code IRC2015	5/TPI2014	Matr	x-S						Weight: 24 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat) BOT CHORD

2x4 SP No.3(flat) WFBS

REACTIONS. (size) 8=Mechanical, 5=Mechanical **BRACING-**TOP CHORD

Structural wood sheathing directly applied or 3-11-8 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

Max Grav 8=204(LC 1), 5=204(LC 1)

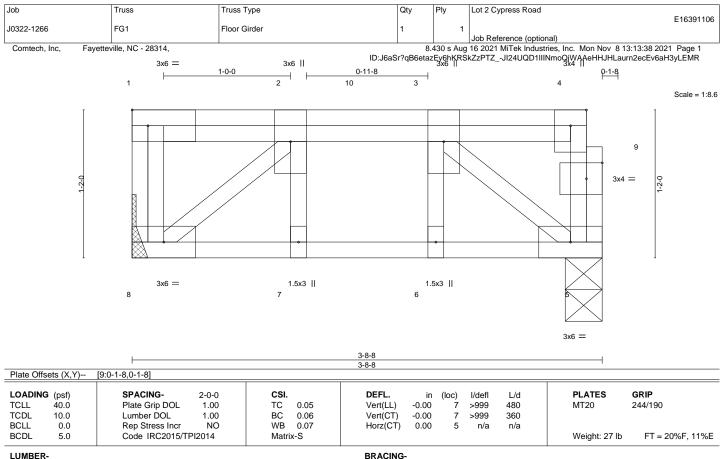
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.









TOP CHORD

BOT CHORD

LUMBER-TOP CHORD

2x4 SP No.1(flat) 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.3(flat) WFBS

REACTIONS. (size) 8=Mechanical, 5=0-3-8

Max Grav 8=242(LC 1), 5=236(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-8=-294/0, 3-5=-291/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 122 lb down at 1-10-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb) Vert: 10=-104(F)



Structural wood sheathing directly applied or 3-8-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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Job Truss Truss Type Qty Ply Lot 2 Cypress Road F16391107 J0322-1266 FG2 Floor Girder Job Reference (optional) Comtech, Inc. Fayetteville, NC - 28314, 8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 13:13:38 2021 Page 1 ID:J6aSr?qB6etazEy6hKRSkZzPTZ_-JI24UQD1IIINmoQiWAAeHHJFkarVn1EcEv6aH3yLEMR 3x6 = 3x6 || 1-0-0 0-8-0 Scale = 1:8.6

3x6 =

5

3-5-0 LOADING (psf) SPACING-2-0-0 CSI. DEFL. **PLATES** GRIP in (loc) I/defl L/d Plate Grip DOL TC Vert(LL) 244/190 **TCLL** 40.0 1.00 0.16 -0.01 >999 480 MT20 **TCDL** 10.0 Lumber DOL 1.00 вС 0.21 Vert(CT) -0.01 >999 360 WB **BCLL** 0.0 Rep Stress Incr NO 0.16 Horz(CT) 0.00 5 n/a n/a BCDL Code IRC2015/TPI2014 Weight: 26 lb FT = 20%F, 11%E

1.5x3 II

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) BRACING-

1.5x3 II

TOP CHORD Structural wood sheathing directly applied or 3-5-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 8=Mechanical, 5=Mechanical

Max Grav 8=596(LC 1), 5=427(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3x6 =

TOP CHORD 2-3=-528/0

BOT CHORD 7-8=0/528, 6-7=0/528, 5-6=0/528 WEBS 2-8=-684/0, 3-5=-684/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 705 lb down at 1-6-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 5-8=-10, 1-4=-100

Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb) Vert: 2=-675(B)



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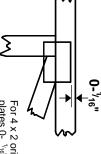
818 Soundside Road

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Apply plates to both sides of truss Dimensions are in ft-in-sixteenths.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss

ω

O

S

required direction of slots in connector plates This symbol indicates the

* Plate location details available in MiTek 20/20 software or upon request

PLATE SIZE



to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



output. Use T or I bracing Indicated by symbol shown and/or if indicated. by text in the bracing section of the

BEARING



number where bearings occur.

Min size shown is for crushing only Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint

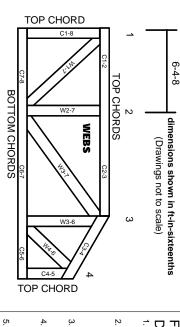
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.

DSB-89:

Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling, Building Component Safety Information Design Standard for Bracing

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection. esponsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.